

**City of Poughkeepsie
Planning Board
Meeting Agenda**

Common Council Chambers
Tuesday, November 21, 2023
6:00 PM

I. CALL TO ORDER

II. APPROVAL OF MEETING MINUTES

1. Approval of October 17, 2023 Meeting Minutes

III. PUBLIC HEARINGS FOR SITE PLANS/SPECIAL PERMITS/SUBDIVISIONS

1. 16 PERSHING AVENUE - SPECIAL PERMIT

Adjourned to December

2. LOT#151783 SOUTH HAMILTON AVENUE: SITE PLAN

An application for a three-story building consisting of 29 one-bedroom units and 29 two-bedroom units. Owner/Applicant: O'Neill Group Hamilton LLC.; Consultant: Arico Associates; Grid # 6161-30-151782; Zoning District: Medium High-Density Residential (R-4); File #2023-041

IV. SITE PLANS/SPECIAL PERMITS/SUBDIVISIONS

1. SOUTH AVENUE LOT #937583 - SITE PLAN AMENDMENT

Application for Site Plan review regarding amendments to roof top open space, balcony and front entry design. Owner/Applicant: South Avenue Real Estate Associates LLC, Michael Abuladez, CEO, Argo Development; Consultant: Christopher Lapine, P.E., LEED AP, LaBella Associates; Grid #6061-52-937583; Zoning District: R-D Research and Development District; File #2022-055

2. 149 NORTH HAMILTON - SITE PLAN

An application for the creation of a parking area for the existing multi-unit building. Owner: RC Home Hamilton Street; Applicant: Roy Campbell; Consultant: Paul Tirums, P.E.; Grid #6062-63-275350; Zoning District: Central Medium Density Residential (R-3A); File #2023-051

3. 4 SOUTH CLINTON STREET: SITE PLAN

Site Plan application for the proposed use of an existing accessory building (carriage house) at 4 S. Clinton Street for commercial use as artist studios; Owner/Applicant: 4 S. Clinton, LLC; Consultant: Natalie Quinn, KARC Consulting; Grid #6162-78-243002; Zoned: Central Commercial (C-2) District; File #2023-053

4. 50 DELAFIELD STREET - SITE PLAN

An application for the addition of three (3) HVAC units to the south rear of an existing building. Owner: Astor Services; Applicant/Consultant: Jay Diesing RA AIA, Mauri Architects PC.; Grid #6062-59-857380; Zoning District: Medium High-Density Residential (R-4); File #2023-055

PROJECT INFORMATION

New construction: ☒ (yes) ☐ (no)
Change of use: ☐ (yes) ☒ (no)
Expansion/addition: ☐ (yes) ☒ (no)
Alteration: ☐ (yes) ☒ (no)
Cost of Construction: \$ 6,000,000

LOT AND BULK INFORMATION:

Lot area: 3.378 (acres) 147,145.68 (square feet)
Building Footprint: 22,800 (proposed square feet for new construction)
 (existing square feet for change of use/expansion)
 (additional proposed square feet for expansion)
 (total building footprint, square feet)
Lot Coverage: 15.4 (percentage, building coverage only)
Gross Floor Area: 68,400 (proposed square feet for new construction)
 (existing square feet for change of use/expansion)
 (additional proposed square feet for expansion)
 (total gross floor area, square feet)
Building Height: 3 (stories) 40 (feet)
Floor Area Ratio: .46 (ratio of gross floor area divided by lot area)

VIII. PROJECT PARKING INFORMATION

A) Number of Off-Street Parking Spaces required: 102
B) Number of Off-Street Parking Spaces provided: 109
C) Waiver Requested: (Yes) X (No)
Is municipal parking available within 600 feet? (Yes) X (No)

If yes, name all such municipal facilities: _____

(If a waiver is requested based on proximity to municipal facilities, a written request for such waiver, addressed to the City Planning Staff and the Chairman of the Planning Board, must be submitted with the application.)

If a private parking facility is to be utilized, is such facility within 600 feet?

_____ (Yes) ☒ (No)

If yes, provide the address of the facility, name of the property owner, number of spaces in the lot, number of spaces available for lease, and length of lease. Attach copy of lease. Additional information may be required by the Planning Board):

IX. RESIDENTIAL PROPOSALS

A) Type of Development: LUXURY MULTIPLE FAMILY RENTAL
(Townhouse, condominium, multiple residence rental, etc.)

B) Unit Breakdown:

Number of efficiency or "studio" units: _____

Number of one (1) bedroom units: 29

Number of two (2) bedroom unit's 29

Number with three or more bedrooms: _____

Total number of units: 58

C) Is funding public? ☐ Yes ☒ No

If yes, describe funding source: _____

X. OFFICE PROPOSALS (☐ Medical/Dental ☐ Professional ☐ Business)

A) Number of Employees: _____

B) Number of doctors/dentists/medical practitioners: _____

C) Days/Hours of Operation: _____

XI. SERVICE BUSINESS PROPOSALS

- A) Specify Business: _____
(e.g., laundromat, drycleaner, beauty parlor, travel agency, banks)
- B) Number of Employees: _____
- C) Number of Washing Machines (for laundromat): _____
- D) Days/Hours of Operation: _____

XII. MERCANTILE PROPOSALS (Retail _____ Wholesale _____)

- A) Type of Mercantile (specify): _____
- B) Number of Employees: _____
- C) Days/Hours of Operation: _____

XIII. STANDARD RESTAURANT/FAST FOOD RESTAURANT/COFFEE SHOP/DONUT SHOP/NIGHTCLUB/DISCOTHEQUE/BAR PROPOSALS

- A) Specify Use: _____
- B) Number of seats (excluding bar stools and outdoor seating): _____
- C) Meals served (i.e., breakfast, lunch, dinner): _____
- D) Type of Menu: _____
- E) Days/Hours of Operation: _____
- F) Entertainment (live and/or recorded): _____
- G) Will the restaurant contain a bar/lounge?: _____ Yes _____ No
- H) If yes, how many seats at the bar? _____
- I) Is a drive-through window proposed? _____
- J) Is a walk-up window proposed? _____
- K) Is an outdoor seating area proposed? _____
- L) If yes, how many seats are proposed? _____
- M) If yes, is outdoor cooking proposed? _____

XIV. INDUSTRIAL/COMMERCIAL PROPOSALS (Automobile repair, taxi, motor vehicle sales, warehouse, manufacturing, animal hospital, research facilities, etc.)

- A) Specify Use: _____
- B) Number of shifts (if any): _____
- C) Number of employees: _____
(If shifts are proposed, list number of employees per shift)
- E) Number of work bays (if automobile repair): _____
- F) Days/Hours of operation: _____

XV. EDUCATIONAL/INSTITUTIONAL/RECREATIONAL PROPOSALS (e.g., Museums, Theatres, Conference Centers, Hotels, Motels, Hospitals, Nursing Homes, Assisted Living Facilities, Nursery Schools, Private Schools, Day Care Centers, Places of Worship, Marinas, Membership Clubs, etc.)

A) Specify: _____

B) Number of shifts (if any): _____

C) Number of employees: _____
(If shifts are proposed, list number of employees per shift)

D) Number of beds (hospitals, nursing homes, etc): _____

E) Number of seats in largest assembly space: _____

F) Number of classrooms (schools): _____

G) Number of rental units (hotels, motels, etc.) _____

H) Number of boat slips/courts (e.g., tennis, handball, etc.), alleys: _____

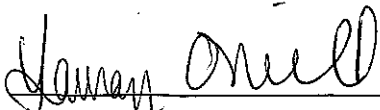
I) Days/Hours of Operation: _____

XVI. PROPOSALS NOT MENTIONED ABOVE (Please be specific):

XVII. APPLICANT CERTIFICATION

I, O'NEILL GROUP HAMILTON LLC, certify that the this application has addressed the objectives outlined in the Zoning Ordinance with consideration given to the public health, safety and welfare; the comfort and convenience of the public in general or the residents or users of the proposed development and of the immediate neighborhood.

The applicant certifies that all information contained herein is accurate and complete as of the date of this application.



Signature of Applicant

4/21/2023

Date



~~October 5, 2023~~

Revised October 9, 2023

City of Poughkeepsie Planning Board
62 Civic Center Plaza, 2nd Floor.
Poughkeepsie, NY 12601
Attention: Lori Garcia

Re: S. Hamilton Street (151783) proposed construction of a 58-unit multifamily building .

Dear Ms. Garcia:

Enclosed please find a thumb drive containing the following:

1. Site Plan Set prepared by Arico Associates Engineering revised October 3, 2023. In response to Board comments, a Boundary and Topographic Survey prepared by Vincent Ausfeld, PLS dated September 28, 2023, which highlights existing conditions is added to the plan set. A Landscape Plan prepared by The Michael Boice Collaboration dated September 27, 2023, is included. Two gazebo gathering spaces and four benches have been added to the plan in response to comments from the Board. Landscape and Lighting plans are separate plan sheets at the request of the board. A Lighting and Photometric Plan is added to the plan set in response to comments. In addition, the Grading and Drainage plan is now a separate sheet as is the Water and Sewer Utility plan. An Erosion Control Plan is added to the plan set. Several sheets of construction details and notes are added to the plan set in response to Board comments.
2. Stormwater Pollution Prevention Plan (SWPPP) prepared by Arico. Associates Engineering dated October 3, 2023.

3. Floor Plans & Elevations by Mija Architecture revised October 4, 2023. The entrance lobby is shifted to the center of the two building wings. The stairwells are relocated to the front of the building and the access doors that were previously located in the rear of the building now open to the front. Line of site perspectives are added to the drawing set in response to comments.
4. Color Renderings prepared by Mija Architecture and dated October 4, Stone is proposed in place of brick that was shown on previous rendering. In response to comments balcony partitions and railings are revised and proposed landscaping is depicted in the renderings.
5. Subsurface Investigation Report prepared by Kevin L. Patton; P.E. dated July 6, 2023.
6. Traffic Impact Study prepared by Creighton Manning Engineering LLP Dated July 6, 2023. The study concludes that the Build condition of the study intersections will operate at the same levels of service consistent with the No-Build conditions.
7. Poughkeepsie Natural Resource Inventory Conservation Analysis Prepared by Cleary Consulting dated October 5, 2023. Based on the Analysis, the Action is consistent with the policy objectives of the Natural Resources Inventory for the City of Poughkeepsie

We look forward to our appearance before the Board on November 21st.

Sincerely,

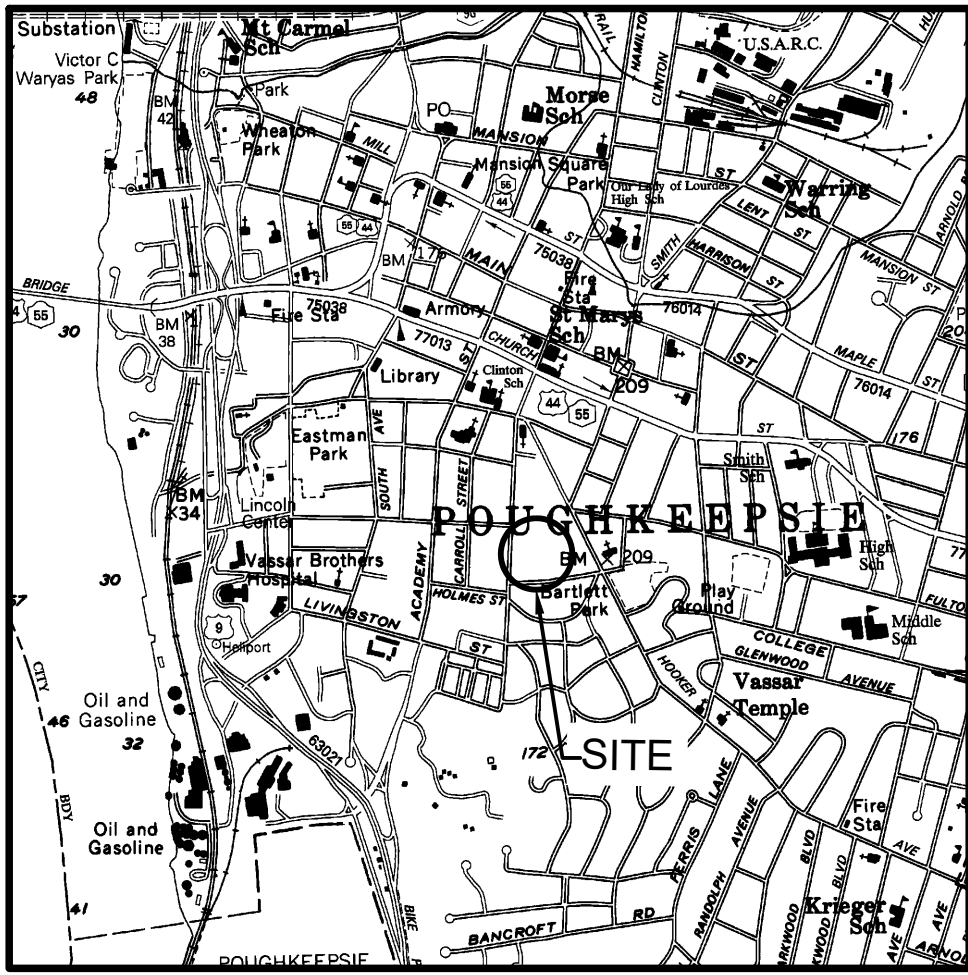


George Brandt
gbrandt@onekeyllc.com
(201) 488-4455 ext. 122

SOUTH HAMILTON

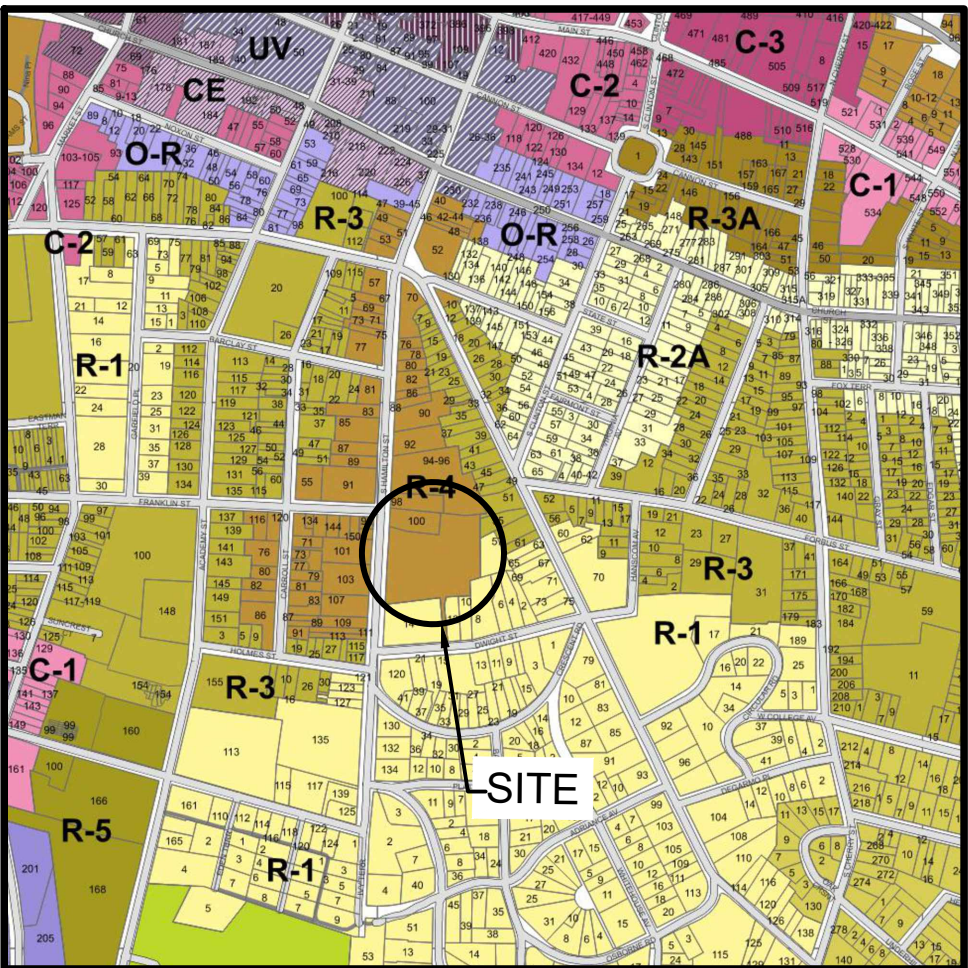
CITY OF POUGHKEEPSIE
COUNTY OF DUTCHESS STATE OF NEW YORK

OCTOBER 3, 2023



SITE LOCATION MAP

SCALE: 1"=2000'



ZONING MAP

SCALE: 1"=2000'

MAP REFERENCE: BOUNDARY AND TOPOGRAPHIC INFORMATION GENERATED FROM
AUSFELD & WALDRUFF LAND SURVEYORS LLP DATED JANUARY 5, 2010 ENTITLED "P
BOUNDARY & TOPOGRAPHIC SURVEY OF THE LANDS OF STEVEN REIFLER.

CITY OF POUGHKEEPSIE ZONING DISTRICT INFORMATION

DESCRIPTION	REQUIREMENT	PROPOSED
R4 - MEDIUM-HIGH DENSITY RESIDENTIAL		
MINIMUM LOT AREA	145,000 SF (= 3 BEDROOM UNITS)	147,145.7 SF (58 UNITS)
MINIMUM LOT WIDTH	50 FT	346.3 FT
MINIMUM LOT FRONTAGE	30 FT	346.3 FT
FRONT SETBACK	35 FT	39.02 FT
SIDE SETBACK	20 FT (0.5 MAX HEIGHT OF 40 FT)	20.0 FT (LEFT SIDE) 65.04 FT (RIGHT SIDE)
REAR SETBACK	20 FT (0.5 MAX HEIGHT OF 40 FT)	215.44 FT
USABLE OPEN SPACE	40%	52.24%
MAX. BUILDING HEIGHT	40 FT	40.0 FT
LOT COVERAGE	40%	15.4%

* - SEE SITE AREA CALCULATIONS TABLE

SITE STATISTICS

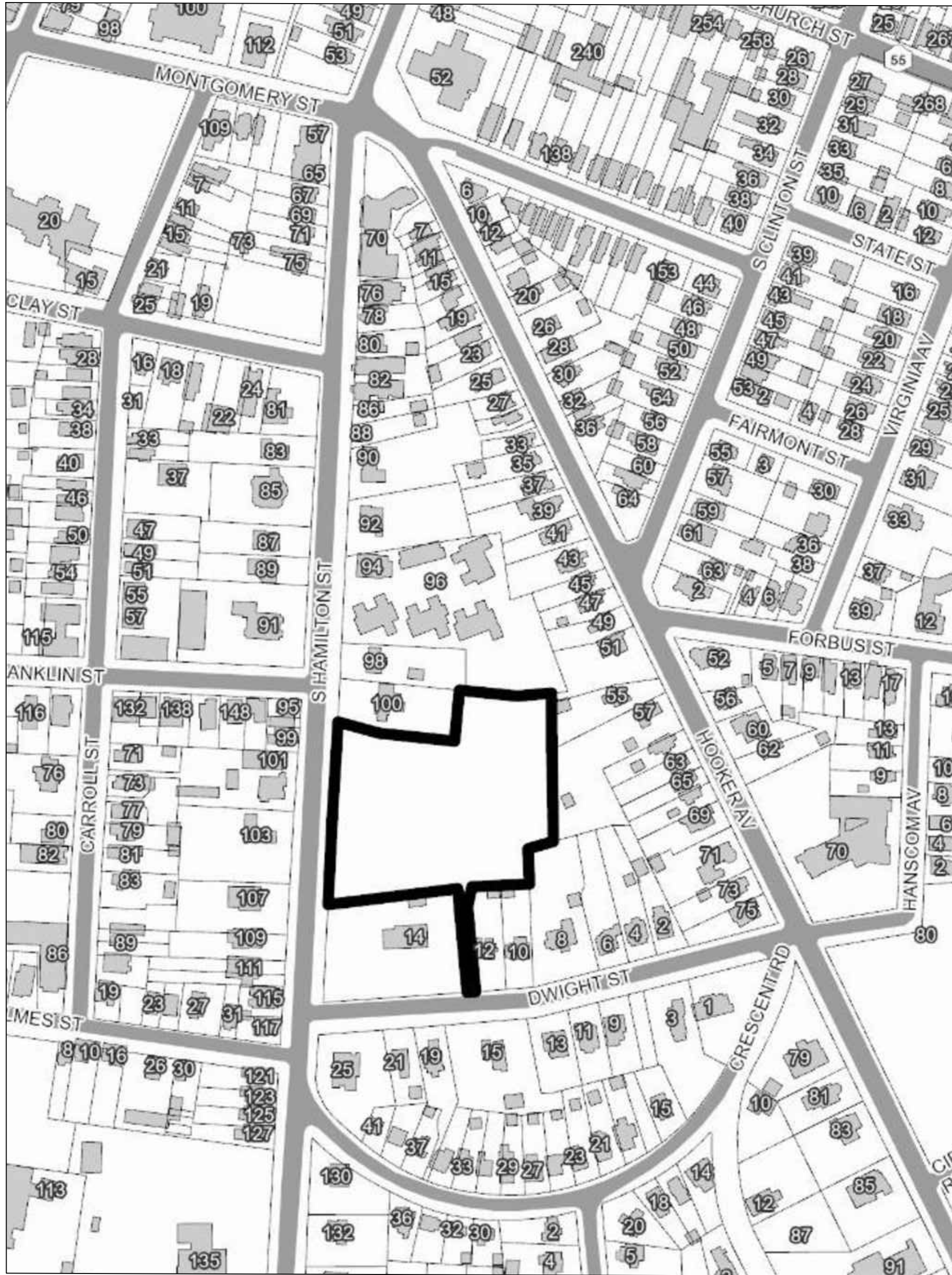
TOTAL PROJECT AREA 147,145.74 SF 3.3782 AC	EXISTING			PROPOSED		
	SQUARE FEET	ACRES	COVERAGE	SQUARE FEET	ACRES	COVERAGE
BUILDING AREA	0.00	0.00	0.00%	24,850 SF	0.564	16.5%
PAVEMENT AREA	2,000.02	0.0504	1.5%	46,835.02	1.074	31.7%
GREEN/OPEN AREA	145,145.72	3.3282	98.5%	75,660.72	1.752	51.8%

PROJECT INFORMATION

ADDRESS	SOUTH HAMILTON STREET
TAX MAP No.	6161-30-151783
ZONING DISTRICT	R-4 MEDIUM HIGH DENSITY RESIDENTIAL
SITE COORDINATES	N 41°41'42.82" W 73°55'28.39"
TOTAL PROJECT AREA	3.3782 ACRES
EXISTING USE	RESIDENTIAL / VACANT
PROPOSED USE	MULTI-FAMILY
PARCEL FRONTAGE	346.3 LF (S. HAMILTON STREET) 17.0 LF (DWIGHT STREET)
WATER SUPPLY	CITY OF POUGHKEEPSIE
SANITARY SERVICE	CITY OF POUGHKEEPSIE
FIRE PROTECTION	CITY OF POUGHKEEPSIE
SCHOOL DISTRICT	CITY OF POUGHKEEPSIE
PROPSD DENSITY	58 UNITS

SITE AREA CALCULATIONS

PARKING REQUIREMENTS	
1.5 SPACES PER 1-BEDROOM UNIT @ 29 UNITS	= 43.5 SPACES
2.0 SPACES PER 2-BEDROOM UNIT @ 29 UNITS	= 58.0 SPACES
TOTAL PARKING REQUIRED	= 102 SPACES
TOTAL PARKING PROVIDED	= 102 SPACES
MINIMUM LOT AREA	
2,200 SF PER 1-BEDROOM UNIT @ 29 UNITS	= 81,200 SF
2,800 SF PER 2-BEDROOM UNIT @ 29 UNITS	= 81,200 SF
TOTAL LOT AREA REQUIRED	= 145,000 SF
TOTAL LOT AREA PROVIDED	= 147,145.7 SF
USABLE OPEN SPACE	
300 SF PER 1-BEDROOM UNIT @ 29 UNITS	= 8,700 SF
400 SF PER 2-BEDROOM UNIT @ 29 UNITS	= 11,600 SF
TOTAL OPEN SPACE REQUIRED	= 20,300 SF
TOTAL OPEN SPACE PROVIDED	= 32,850 SF



OVERALL SITE LOCATION

SCALE: 1"= 200'

INDEX TO DRAWINGS

SHEET No.	DWG. NO.	DESCRIPTION
1		BOUNDARY & TOPOGRAPHIC SURVEY
2	S1	SITE LAYOUT PLAN
3	S2	WATER AND SEWER UTILITY PLAN
4	S3	GRADING AND DRAINAGE PLAN
5	S4	EROSION CONTROL PLAN
6	D1	STANDARD DETAILS
7	D2	STANDARD CITY DETAILS
8	D3	STANDARD DETAILS
9	D4	STANDARD DETAILS
10	L1	LANDSCAPING PLAN (BY OTHERS)
11	L1A	LANDSCAPING PLAN (BY OTHERS)
12	L2	LANDSCAPING PLAN (BY OTHERS)
13	TBD	LIGHTING PLAN (TBD)

GENERAL NOTES:

1. THE APPLICANT SHALL COMPLY WITH ALL APPLICABLE FEDERAL, STATE, AND LOCAL RULES AND REGULATIONS, INCLUDING BUT NOT LIMITED TO THE SEGR, FRESHWATER WETLAND PERMIT REGULATIONS, THE TOWN GRADING LAW AND THE TOWN FLOOD PLAIN MANAGEMENT.
2. THE APPLICANT SHALL BEAR THE SOLE RESPONSIBILITY FOR ENSURING THAT ALL IMPROVEMENTS ARE COMPLETED AND MAINTAINED IN ACCORDANCE WITH APPROVED PLANS, SPECIFICATIONS AND STANDARDS.
3. NO CERTIFICATE OF OCCUPANCY SHALL BE ISSUED UNTIL ALL REQUIRED IMPROVEMENTS ARE SATISFACTORILY COMPLETED, AND THE PLANNING BOARD HAS PROVIDED WRITTEN AUTHORIZATION TO THE BUILDING DEPARTMENT.
4. THE APPLICANT SHALL BE RESPONSIBLE FOR KEEPING EXISTING PUBLIC HIGHWAYS AND ADJACENT LANDS FREE OF DEBRIS, SOIL AND OTHER MATTER WHICH MAY ACCUMULATE DUE TO CONSTRUCTION RELATED TO THE SITE.
5. ALL PLANT MATERIALS INSTALLED PURSUANT TO THIS SITE DEVELOPMENT PLAN SHALL CONFORM TO THE AMERICAN STANDARD NURSERY STOCK (ANSI Z60.1-1986) OF THE AMERICAN ASSOCIATION OF NURSERYMEN OR EQUIVALENT RECOGNIZED STANDARD, AND SHALL BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH ACCEPTED INDUSTRY PRACTICE.
6. NO PORTION OF THIS SITE SHALL BE USED FOR STORAGE OR DISPLAY OF ANY PRODUCT OR MATERIAL, OR FOR PARKING OF ANY VEHICLES, OR FOR THE CONDUCT OF ANY OTHER BUSINESS OPERATIONS, UNLESS SPECIFICALLY DESIGNATED FOR SUCH USE ON THIS SITE DEVELOPMENT PLAN.
7. ALL REQUIRED EROSION & SEDIMENT CONTROL MEASURES SHALL BE INSTALLED AND THE BUILDING DEPARTMENT SHALL BE NOTIFIED PRIOR TO ANY SOIL DISTURBANCE OR ISSUANCE OF ANY GRADING PERMIT.
8. THE CONTRACTOR SHALL COORDINATE ALL WORK WITH ANY AND ALL OTHER CONTRACTORS PERFORMING WORK ON-SITE.
9. THE CONTRACTOR AND/OR HIS REPRESENTATIVE(S) ARE ENTIRELY RESPONSIBLE FOR EXECUTING THE WORK AND MAINTAINING THE WORK SITE IN ACCORDANCE WITH ALL APPLICABLE SAFETY CODES AND STANDARDS.
10. THE CONTRACTOR SHALL CLEAN-UP AT THE END OF EACH WORK DAY AND LEAVE THE WORK SITE CLEAN, WITH ALL ITEMS ATTENDED TO; NO FASTENERS, TOOLS AND/OR ANY OTHER ITEM MAY NOT BE LEFT UNATTENDED.
11. THE CONTRACTOR IS RESPONSIBLE FOR THE REMOVAL AND DISPOSAL OF ALL CONSTRUCTION MATERIALS AND DEBRIS FROM THE WORK SITE ON A DAILY BASIS.
12. ON-SITE VERIFICATION OF ALL DIMENSIONS AND CONDITIONS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
13. THE ENGINEER SHALL BE NOTIFIED, IN WRITING, OF ANY ANY DISCREPANCIES OF CONFLICTS THAT MAY ARISE PERTAINING TO THESE DRAWINGS. NO DECISION SHALL BE MADE WITHOUT APPROVAL OR CONSENT FROM THE ENGINEER.
14. ALL CONSTRUCTION SHALL COMPLY WITH NYS FIRE PREVENTION AND BUILDING CODES, LOCAL BUILDING CODES AND REQUIREMENTS, AS WELL AS FEDERAL PLUMBING AND ELECTRICAL CODES.
15. THE CONTRACTOR SHALL PERFORM ALL WORK IN A PROFESSIONAL AND SKILLED MANNER.
16. ANY EXISTING CONDITIONS DAMAGED OR DESTROYED DURING THE COURSE OF WORK SHALL BE RESTORED AT THE CONTRACTORS EXPENSE TO "AS GOOD AS CONDITION" AS DETERMINED BY THE ENGINEER.
17. ACCESS TO ALL PUBLIC UTILITIES INCLUDING FIRE HYDRANTS AND ALARMS, POLICE CALL BOXES, STREET LIGHTS, MANHOLES, ETC. SHALL BE KEPT UNOBSTRUCTED DURING CONSTRUCTION.
18. ALL MATERIALS AND EQUIPMENT USED FOR THIS PROJECT AND/OR KEPT ON-SITE SHALL COMPLY WITH NYS ENERGY CONSERVATION CONSTRUCTION CODE.
19. THESE PLANS ARE CERTIFIED TO BE IN ACCORDANCE WITH NYS BUILDING CODES; THE ENGINEER ASSUMES NO RESPONSIBILITY FOR THE FINISHED PRODUCT.
20. THE CONTRACTOR SHALL PROVIDE PROTECTIVE BARRICADES, PROPER SIGNAGE AND LIGHTING TO PREVENT UNAUTHORIZED ENTRY TO THE WORK-SITE.
21. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL OSHA REGULATIONS AND NYS LABOR COMPLIANCE.
22. THE CONTRACTOR SHALL COMPLY WITH ALL DRAWING NOTES.
23. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING HIGHWAY WORK PERMIT BEFORE PERFORMING ANY WORK WITHIN RIGHT-OF-WAY.
24. APPROPRIATE TRAFFIC CONTROL DEVICES & PERSONEL SHALL BE PROVIDED IN ACCORDANCE WITH FEDERAL, STATE OR LOCAL REGULATIONS TO REGULATE, WARN & GUIDE TRAFFIC THROUGH THE WORK SITE.

OWNER / APPLICANT:

O'NEILL GROUP HAMILTON, LLC
241 HUDSON STREET
HACKENSACK, NEW JERSEY 07601

PREPARED BY:

ARICO ASSOCIATES
ENGINEERS
LAND PLANNERS
& CONSULTANTS



NOTE

UNAUTHORIZED ALTERATION OR ADDITION
TO THESE PLANS IS A VIOLATION OF
SECTION 7209, SUBDIVISION 2 OF THE
NEW YORK STATE EDUCATION LAW.

Fire Chief
Police Chief
Building Inspector
City Engineer
Commissioner of Public Works

CITY PLANNING BOARD
CITY OF POUGHKEEPSIE, NEW YORK

SITE PLAN APPROVAL

FILE NO.
PROJECT NAME:
PROJECT LOCATION:
APPLICANT:

OFFICIALLY APPROVED at the Planning Board
meeting held the ____ day of ____ 2____
PLANNING CHAIR:
DESIGNED:
DATE SIGNED:
CONDITION(S):

	CATCH BASIN
	HYDRANT
	LIGHT POLE
	SANITARY MANHOLE
	STORM MANHOLE
	ELECTRIC MANHOLE
	WATER VALVE
	TELEPHONE PEDESTAL
	ELECTRIC BOX
	ELECTRIC TRANSFORMER
	UTILITY POLE
	TELEPHONE MANHOLE
	TRAFFIC CONTROL BOX
	TRAFFIC SIGNAL POLE
	GAS LINE
	SANITARY LINE
	STORM LINE
	WATER LINE
	TELEPHONE LINE
	OVERHEAD WIRES
	SPOUT ELEVATION
	CAPPED IRON ROD FOUND
	IRON ROD FOUND
	IRON PIPE FOUND
	LANDSCAPED AREA
	ROOF DRAIN
	SOIL BORING
	MONITORING WELL
	GAS MARKER POST
	ELECTRIC METER
	GAS METER



LANDS N/F OF
GEORGE & ANN HARMUTH
BOOK 1967, PAGE 403
PARCEL 6161-30-182803

LANDS N/F OF
GEORGE & ANN HARMUTH
BOOK 1640, PAGE 54
PARCEL 6161-30-181794

LANDS N/F OF
GORDON & LIZABETH GEBERT
INSTR. 22003-5091
PARCEL 6161-30-174764

LANDS N/F OF
MARTIN SURUN
INSTR. 22018-5516
PARCEL 6161-30-166759

LANDS N/F OF
JEFFREY LINTEL
BOOK 1833, PAGE 809
PARCEL 6161-30-160759

LANDS N/F OF
NEW HORIZONS RESOURCES INC.
INSTR. 22005-228
PARCEL 6161-30-139757

LANDS N/F OF
HAMILTON GARDEN ESTATES, LLC
INSTR. 22022-50349
PARCEL 6161-30-152824

LANDS N/F OF
KRISTOFER BAIN
INSTR. 22023-51218
PARCEL 6161-30-138812

LANDS N/F OF
SHIRLEY ADAMS
INSTR. 22008-3021
PARCEL 6161-30-137803

SOUTH HAMILTON STREET

BENCHMARK
RR SPIKE
ELEV. = 188.64

SITE LOCATION PLAN: 1"=2000

PROJECT LOCATION

CITY OF POUGHKEEPSIE, DUTCHESS COUNTY, NEW YORK
TAX PARCEL 6161-30-151783

1. "SURVEY OF PROPERTY PREPARED FOR STEPHEN REIFLER," PREPARED BY JOHN MARANO, DATED JUNE 6, 1987.
2. SANITARY SEWER MAP NO. 18 ON FILE AT THE CITY OF POUGHKEEPSIE ENGINEERING DEPARTMENT.
3. STORM SEWER MAP NO. 18 ON FILE AT THE CITY OF POUGHKEEPSIE ENGINEERING DEPARTMENT.
4. WATER MAPS PAGE NOS. 118 AND 176 ON FILE AT THE CITY OF POUGHKEEPSIE ENGINEERING DEPARTMENT.

1. STEVEN REIFLER TO ONEILL GROUP HAMILTON LLC, DATED SEPTEMBER 13, 2022 AND RECORDED IN THE DUTCHESS COUNTY CLERKS OFFICE IN DEED INSTR. 22022-53570.

NORTH IS ORIENTED TO DEED REFERENCE 1.

VERTICAL DATUM IS ASSUMED. ADD 1.0 FEET TO ALL ELEVATIONS TO NAVD88 DATUM.

UNAUTHORIZED ALTERATION OR ADDITION TO A SURVEY MAP
BEARING A LICENSED LAND SURVEYOR'S SEAL IS A VIOLATION
OF SECTION 7209, SUBDIVISION 2 OF THE NEW YORK STATE
EDUCATION LAW.

ONLY COPIES FROM THE ORIGINAL OF THIS SURVEY MARKED
WITH AN ORIGINAL OF THE LAND SURVEYOR'S INKED SEAL OR
HIS EMBOSSED SEAL SHALL BE CONSIDERED TO BE VALID TRUE
COPIES.

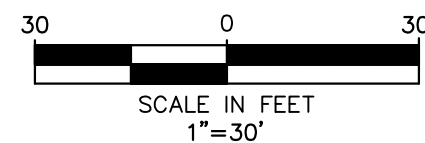
SURVEYED FROM RECORD DESCRIPTION AND AS IN POSSESSION

SURVEYED FROM RECORD DESCRIPTION AND AS IN POSSESSION
SUBJECT TO COVENANTS, EASEMENTS, RESTRICTIONS,
CONDITIONS AND AGREEMENTS OF RECORD.

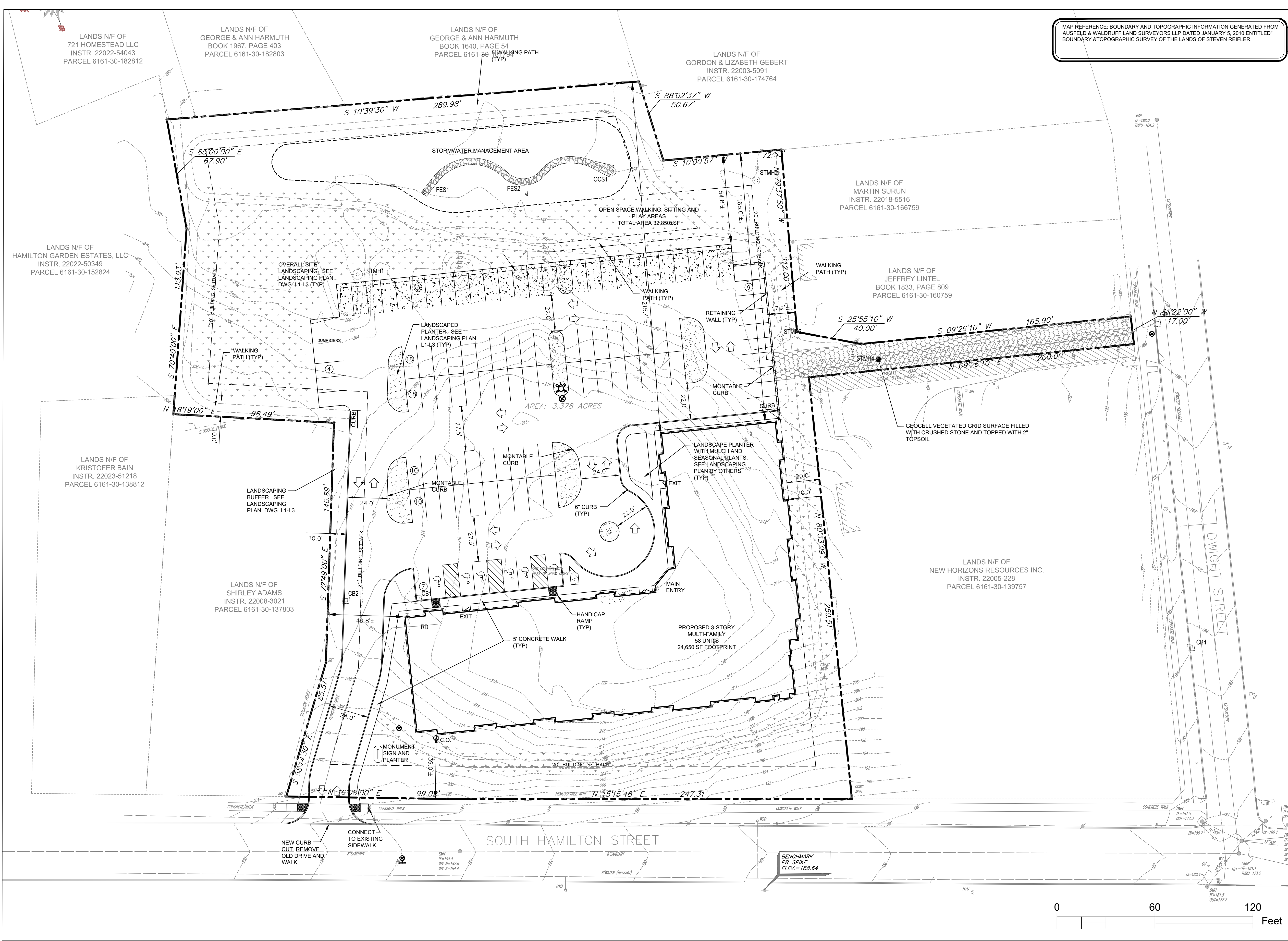
SURVEY SUBJECT TO ANY RIGHT, TITLE OR INTEREST THE PUBLIC MAY HAVE FOR HIGHWAY USE.

SURVEY SHOWN IS SUBJECT TO ANY SUBSURFACE CONDITIONS
THAT MAY EXIST, IF ANY.

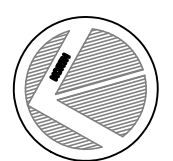
UNDERGROUND UTILITIES AND STRUCTURES SHOWN HEREON WERE TAKEN FROM DATA OBTAINED FROM PREVIOUS MAPS AND RECORD DRAWINGS. ALL ABOVE GROUND STRUCTURES AND SURFACE FEATURES SHOWN HEREON ARE THE RESULT OF A FIELD SURVEY UNLESS OTHERWISE NOTED. THERE MAY BE OTHER UNDERGROUND UTILITIES, THE EXISTENCE OF WHICH ARE NOT KNOWN OR CERTIFIED BY THE UNDERSIGNED. THE SIZE AND LOCATION OF ALL UNDERGROUND UTILITIES AND STRUCTURES MUST BE VERIFIED BY THE APPROPRIATE AUTHORITIES. THE UNDERGROUND FACILITIES PROTECTIVE ORGANIZATION MUST BE NOTIFIED PRIOR TO CONDUCTING TEST BORINGS, EXCAVATION AND CONSTRUCTION.



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SITE LAYOUT PLAN
SCALE: 1"= 30'



PRIOR TO START OF CONSTRUCTION OR EXCAVATION, TREES TO BE RETAINED SHALL BE PROTECTED BY SILT FENCE, SNOW FENCE, CONSTRUCTION FENCE, OR EQUAL, WHICH WILL BE PLACED AT THE DRIP LINE OR SEVERAL FEET BEYOND, HOWEVER, THE FENCING SHALL NOT CROSS PUBLIC SIDEWALKS. NO STORAGE OR STOCKPILING OF CONSTRUCTION MATERIALS, DEBRIS OR EQUIPMENT SHALL OCCUR WITHIN THE FENCED AREA.

Fire Chief	
Police Chief	
Building Inspector	
City Engineer	
Commissioner of Public Works	

CITY PLANNING BOARD CITY OF Poughkeepsie, New York	
SITE PLAN APPROVAL	
FILE NO.	
PROJECT NAME:	
PROJECT LOCATION:	
APPLICANT:	
OFFICIALLY APPROVED at the Planning Board meeting held the _____ day of _____, 2023.	
PLANNING CHAIR:	
DESIGNEE:	
DATE SIGNED:	
CONDITION(S):	

ISSUED FOR REVIEW

NOT FOR CONSTRUCTION

RECORD PLANS

DATE SUBMITTED:

NO.	DATE	DESCRIPTION
1	04.04.23	DFA 08.07.23
2	08.07.23	DFA 10.03.23
3	10.03.23	

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LAND PLANNERS
& CONSULTANTS

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518.573.6989
aricoassociates@gmail.com

CITY OF Poughkeepsie

COUNTY OF Dutchess

LAYOUT PLAN

SOUTH HAMILTON

DATE: JANUARY 2023

JOB NO. 22.10

SCALE: AS NOTED

DRAWN BY: DFA

CHECKED BY: DFA

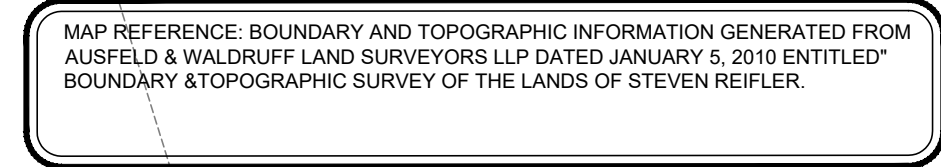
DRAWING NO.

S1

SHEET 1 OF 13

TAX MAP : 6161-30-151783

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☐ NOT FOR CONSTRUCTION
☐ RECORD PLANS

REVISIONS			
NO	DESCRIPTION	BY	DATE
1	PER CITY COMMENTS	DFA	05.13.23
2	REVISED: PARKING, LANDSCAPING	DFA	08.01.23
3	REVISED PER CITY COMMENTS	DFA	10.03.23

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DOMINICK F. ARICO, P.E.
LIC. NO. 066515



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LAND PLANNERS
& CONSULTANTS
ASSOCIATES

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518.573.6989 aricoassociates@gmail.com

CITY OF POUGHKEEPSIE COUNTY OF DUTCHESS
WATER & SEWER UTILITY PLAN
SOUTH HAMILTON

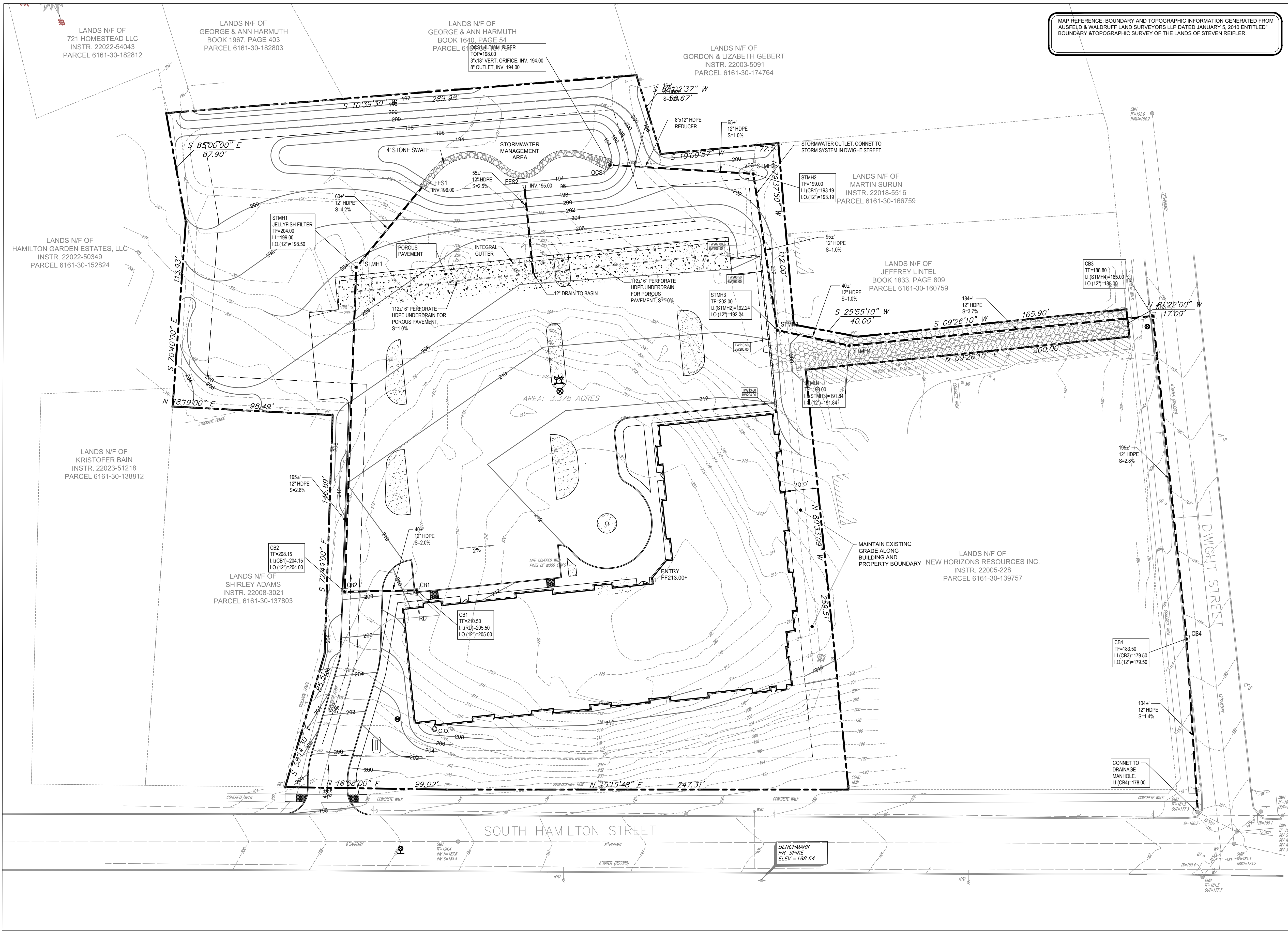
TAX MAP : 6161-30-151783

DATE: JANUARY 2023
JOB NO. 22.10
SCALE: AS NOTED
DRAWN BY: DFA
CHECKED BY: DFA
DRAWING NO.
S2
SHEET 2 OF 13

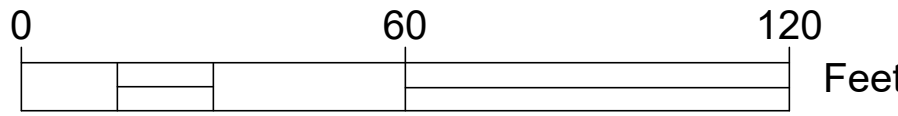
TAX MAP : 6161-30-151783

UTILITY PLAN
SCALE: 1"= 30'

	CITY PLANNING BOARD CITY OF POUGHKEEPSIE, NEW YORK
Fire Chief	SITE PLAN APPROVAL FILE NO: _____ PROJECT NAME: _____ PROJECT LOCATION: _____ APPLICANT: _____ OFFICIALLY APPROVED at the Planning Board meeting held the _____ day of _____ 2____. PLANNING CHAIR: _____ DESIGNEE: _____ DATE SIGNED: _____ CONDITION(S): _____
Police Chief	
Building Inspector	
City Engineer	
Commissioner of Public Works	



GRADING & STORM PLAN
SCALE: 1"= 30'



Fire Chief	CITY PLANNING BOARD CITY OF Poughkeepsie, New York SITE PLAN APPROVAL FILE NO.: PROJECT NAME: PROJECT LOCATION: APPLICANT: OFFICIALLY APPROVED at the Planning Board meeting held the _____ day of _____, 2023. PLANNING CHAIR: DESIGNED: DATE SIGNED: CONDITION(S):
Police Chief	
Building Inspector	
City Engineer	
Commissioner of Public Works	

ISSUED FOR REVIEW

NOT FOR CONSTRUCTION

RECORD PLANS

DATE SUBMITTED:

NO.	DATE	DESCRIPTION
1	05.11.23	DEA
2	08.01.23	DFA
3	10.03.23	DFA

REVISIONS

NO.	DATE	DESCRIPTION
1	05.11.23	DEA
2	08.01.23	DFA
3	10.03.23	DFA

PER CITY COMMENTS

REVISED PER CITY COMMENTS

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CITY OF Poughkeepsie

COUNTY OF Dutchess

GRADING & DRAINAGE PLAN

SOUTH HAMILTON

DATE: JANUARY 2023

JOB NO. 22.10

SCALE: AS NOTED

DRAWN BY: DFA

CHECKED BY: DFA

DRAWING NO.

SHEET 3 OF 13

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GENERAL NOTES:

BEFORE UNDERTAKING ANY CONSTRUCTION ACTIVITY, ALL CONTRACTORS AND SUBCONTRACTORS INVOLVED WITH SITE WORK THAT INVOLVES PHYSICAL GROUND DISTURBANCE ON THE PROJECT SITE SHALL SIGN AND DATE A COPY OF THE CERTIFICATION STATEMENT, WHICH IS LOCATED IN THE STORMWATER POLLUTION PREVENTION PLAN (SWPPP), PREPARED FOR THIS PROJECT.

1. ALL EROSION & SEDIMENT CONTROL MEASURES SHALL ADHERE TO THE PROJECT'S STORMWATER POLLUTION PREVENTION PLAN (SWPPP). ALL APPROPRIATE STORMWATER CONTROL MEASURES IDENTIFIED IN THE SWPPP SHALL BE IN PLACE BEFORE COMMENCING CONSTRUCTION OF ANY SEGMENT OF THE PROJECT THAT REQUIRES THESE MEASURES.

2. INSPECTIONS: THE OPERATOR SHALL HAVE A QUALIFIED PROFESSIONAL CONDUCT AN ASSESSMENT OF THE SITE PRIOR TO THE COMMENCEMENT OF CONSTRUCTION & CERTIFY IN AN INSPECTION REPORT THAT THE APPROPRIATE EROSION & SEDIMENT CONTROLS DESCRIBED IN THE SWPPP HAVE BEEN ADEQUATELY INSTALLED OR IMPLEMENTED TO ENSURE OVERALL PREPAREDNESS OF THE SITE FOR THE COMMENCEMENT OF CONSTRUCTION. FOLLOWING THE COMMENCEMENT OF CONSTRUCTION, SITE INSPECTIONS SHALL BE CONDUCTED BY THE QUALIFIED PROFESSIONAL AT LEAST EVERY 7 CALENDAR DAYS. INSPECTIONS SHALL LOOK FOR EVIDENCE OF SOIL EROSION ON THE SITE, POTENTIAL OF POLLUTANTS ENTERING DRAINAGE SYSTEMS, PROBLEMS AT DISCHARGE POINTS, I.E., TURBIDITY IN RECEIVING WATERS, & SIGNS OF SOIL & MUD TRANSPORT FROM THE SITE ONTO PUBLIC ROADWAYS. REPAIRS & CLEANING SHALL BE MADE IMMEDIATELY.

3. SITE LOG/CERTIFICATION: THE OPERATOR MUST MAINTAIN A RECORD OF ALL INSPECTION REPORTS IN A SITE LOG BOOK. THE SITE LOG BOOK MUST BE MAINTAINED ON-SITE & BE MADE AVAILABLE TO THE PERMITTING AUTHORITY UPON REQUEST.

4. STABILIZATION: THE OPERATOR SHALL INITIATE STABILIZATION MEASURES AS SOON AS PRACTICABLE ON PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITIES HAVE TEMPORARILY OR PERMANENTLY CEASED.

5. MAINTENANCE: SEDIMENT SHALL BE REMOVED FROM TEMPORARY SEDIMENT TRAPS OR PONDS WHENEVER THEIR CAPACITY HAS BEEN REDUCED BY 50% OF THE DESIGN CAPACITY. INSTALL ADDITIONAL SEDIMENT TRAPS & BARRIERS AS NEEDED DURING CONSTRUCTION.

6. CONTRACTOR SHALL ESTABLISH A TEMPORARY STABILIZED CONSTRUCTION ENTRANCE TO CAPTURE MUD & DEBRIS FROM CONSTRUCTION VEHICLES BEFORE THEY ENTER A PUBLIC ROADWAY, AS DETAILED. SEDIMENT TRACKED ONTO THE ROADWAYS SHALL BE REMOVED OR CLEANED ON A DAILY BASIS. ALL BARE AREAS, E.G., ENTRANCES, CONSTRUCTION ROUTES, & EQUIPMENT PARKING AREAS, SHALL BE STABILIZED IMMEDIATELY AS CONSTRUCTION TAKES PLACE. PLACE GRAVEL ON THESE AREAS OR MAINTAIN THE EXISTING VEGETATIVE COVER.

7. TEMPORARY CONSTRUCTION SILT FENCES, STRAW BALE DIKES, OR TRIANGULAR SILT DIKES SHALL BE PLACED ON OR PARALLEL TO CONTOURS WHERE THERE IS NO CONCENTRATION OF STORMWATER RUNOFF FLOWING TO THEM & WHERE EROSION OCCURS IN THE FORM OF SHEET EROSION, AS DETAILED. THE AREA BELOW THESE TEMPORARY STRUCTURAL MEASURES SHOULD BE UNDISTURBED/UNEXCAVATED EARTH.

TEMPORARY STABILIZATION MEASURES SHALL START AS SOON AS PRACTICAL ON PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITIES HAVE TEMPORARILY OR PERMANENTLY CEASED, BUT NOT MORE THAN (7) DAYS AFTER WORK HAS CEASED. ACCEPTABLE TEMPORARY STABILIZATION MEASURES INCLUDE, BUT MAY NOT BE LIMITED TO SEEDING MULCH, STRAW, EROSION CONTROL BLANKETS, SOIL STABILIZING EMULSION PRODUCTS, OR SOME FUNCTIONALLY EQUIVALENT MEASURE. TEMPORARY SEEDING SHALL BE ANNUAL RYE GRASS, APPLIED AT A RATE OF 30 LBS./ACRE.

8. DIVERT OFFSITE OR CLEAN STORMWATER RUNOFF FROM DISTURBED AREAS. CONVEY RUNOFF FROM HIGHLY ERODIBLE SOILS & STEEP SLOPES TO MORE SUITABLE STABLE AREAS. INSTALL TRIANGULAR SILT DIKES TO REDUCE THE VELOCITY OF CONCENTRATED RUNOFF.

9. LIMIT ALL CLEARING & EXCAVATION ACTIVITIES TO MAINTAIN EXISTING VEGETATIVE COVER AS MUCH AS PRACTICABLE. TOPSOIL SHALL BE STOCKPILED, PROTECTED, & STABILIZED IN AN AREA LOCATED AWAY FROM DRAINAGE CHANNELS.

10. WHEN ACTIVITIES TEMPORARILY CEASE DURING CONSTRUCTION, SOIL STOCKPILES & EXPOSED SOIL SHALL BE STABILIZED BY SEED, MULCH, OR OTHER APPROPRIATE MEASURES AS SOON AS POSSIBLE, BUT IN NO CASE MORE THAN 14 DAYS AFTER CONSTRUCTION ACTIVITY HAS CEASED. PERMANENT VEGETATION & LANDSCAPING SHALL BE INSTALLED IMMEDIATELY FOLLOWING FINAL GRADING.

11. NO GRADING, FILLING, OR OTHER DISTURBANCE IS ALLOWED WITHIN EXISTING DRAINAGE SWALES, UNLESS INDICATED ON THE GRADING PLANS.

12. BACKFILL & STABILIZE ALL UTILITY TRENCHES DAILY.

13. SEEDING:
A. SOW GRASS SEED BETWEEN APRIL 1 & MAY 15TH OR BETWEEN AUGUST 15TH & OCTOBER 15TH, EXCEPT AS OTHERWISE APPROVED IN WRITING BY THE ENGINEER. NO SEEDING SHALL OCCUR ON FROZEN GROUND, HIGHLY MUDDY SOILS, OR DURING EXTREME HEAT OR DROUGHT CONDITIONS.
B. SEED MIXTURE: 40% FENWAY RED FESCUE, 30% ABBEY KENTUCKY BLUEGRASS BLEND, 20% DEVINE PERENNIAL RYE, & 10% ENCHANTED PERENNIAL RYE. PROVIDE SEED MIXTURE EQUAL TO SCOTT'S PURE PREMIUM SUN & SHADE NORTH GRASS SEED MIXTURE, OR APPROVED EQUIVALENT. VARIETY MAY BE ALTERED DEPENDING ON AVAILABILITY OF SEED FROM MANUFACTURER.

14. TOPSOIL:
A. PROVIDE TOPSOIL FROM EXISTING STOCKPILES STRIPPED FROM THE PROJECT SITE & APPROVED BY THE ENGINEER. IF REQUIRED, PROVIDE ADDITIONAL TOPSOIL CONFORMING TO THE FOLLOWING:
B. ORIGINAL LOAM TOPSOIL WELL DRAINED HOMOGENEOUS TEXTURE & OF UNIFORM GRADE, WITHOUT THE ADMIXTURE OF SUBSOIL MATERIAL & ENTIRELY FREE OF DENSE MATERIAL, HARDPAN, SOD, OR ANY OTHER OBJECTIONABLE FOREIGN MATERIAL.
C. CONTAINING NOT LESS THAN 4% NOR MORE THAN 20% ORGANIC MATTER IN THAT PORTION OF A SAMPLE PASSING A 1/4 INCH SIEVE WHEN DETERMINED BY THE WET COMBUSTION METHOD ON A SAMPLE DRIED AT 105 DEGREES C.
D. CONTAINING A PH VALUE WITHIN THE RANGE OF 4.5 TO 7 ON THAT PORTION OF THE SAMPLE THAT PASSES A 1/4 INCH SIEVE.

15. FERTILIZER:
A. MIXED COMMERCIAL FERTILIZERS SHALL CONTAIN TOTAL NITROGEN, AVAILABLE PHOSPHORIC ACID & SOLUBLE POTASH IN THE RATIO OF 10-6-4 (50% N/U/F). 50% OF TOTAL NITROGEN SHALL BE DERIVED FROM UREAFORM FURNISHING A MINIMUM OF 3.5% WATER INSOLUBLE NITROGEN (3.5% WIN). THE BALANCE OF THE NITROGEN SHALL BE PRESENT AS METHYLENE UREA, WATER-SOLUBLE UREA, NITRATE & AMMONIACAL COMPOUNDS. OTHER FERTILIZERS MEETING NYSDOT SPECIFICATION SECTION 713-03 FERTILIZER CAN BE USED.

16. MULCH:
A. DRY APPLICATION, STRAW: STALKS OF OATS, WHEAT, RYE OR OTHER APPROVED CROPS THAT ARE FREE OF NOXIOUS WEED SEEDS. WEIGHT SHALL BE BASED ON A 15% MOISTURE CONTENT.
B. HYDRO APPLICATION: COLORED WOOD CELLULOSE FIBER PRODUCT SPECIFICALLY DESIGNED FOR USE AS A HYDRO-MECHANICAL APPLIED MULCH, BY CONVED HYDRO MULCH, CONVED FIBERS, OR APPROVED EQUIVALENT.

17. JUTE MESH:
A. HEAVY JUTE EROSION CLOTH WITH 12 INCH LENGTHS OF NO. 11 GAGE WIRE BENT TO FORM A "U" WIRE STAPLE, BY JUTE PRODUCTS, INC., OR APPROVED EQUIVALENT.

18. EROSION CONTROL BLANKETS:
A. FOR USE IN REPLACE OF MULCH ON SLOPES GREATER THAN 5% AS SHOWN ON THE PLANS, CONSISTING OF LIGHTWEIGHT PHOTODEGRADABLE POLYPROPYLENE NETS WITH AN APPROXIMATE WEIGHT OF 1.64 LBS/1,000 SQUARE FEET, DEGRADABLE THREAD, STRAW FIBER WITH AN APPROXIMATE WEIGHT OF 0.5 LBS/SQUARE YARD, WITH MANUFACTURER'S DOT STAPLE SYSTEM & 6 INCH BIOSTAKE'S BIODEGRADABLE BLANKET PIN FASTENERS. USE DEGRADABLE DOUBLE NET STRAW BLANKET, PRODUCT NO. S150 BY NORTH AMERICAN GREEN, OR APPROVED EQUIVALENT.

19. DUST SUPPRESSION MEASURES MAY INCLUDE: 1) USING A DEDICATED ON-SITE WATER TRUCK FOR ROAD AND SOILS WETTING, CAPABLE OF SPRAYING WATER DIRECTLY ONTO OFF-ROAD AREAS, INCLUDING EXCAVATIONS AND STOCKPILES; 2) CLEARING AND GRUBBING OF LARGER SITES WILL BE DONE IN STAGES TO LIMIT THE AREA OF EXPOSED, UNVEGETATED SOILS VULNERABLE TO DUST PRODUCTION; AND 3) GRAVEL WILL BE USED ON ROADWAYS TO PROVIDE A CLEAN AND DUST-FREE ROAD SURFACE.

SILT FENCE NOTES:

1. SILT FENCE SHALL BE PLACED ON THE DOWNSLOPE SIDE OF DISTURBED AREAS AND AROUND THE PERIMETER OF SOIL STOCKPILES.
2. SILT FENCE SHALL BE PLACED AROUND THE BOUNDARY OF WETLANDS ADJACENT TO THE WORK AREA, AND AT THE EDGE OF WETLANDS AFTER CONSTRUCTION IS COMPLETED.
3. SILT FENCE SHALL BE REPAIRED OR REPLACED WHEN THE ENDS ARE FRAYED OR WORN, AND WHEN THE FENCE IS NOT ANCHORED 6" INTO THE GROUND, WHEN THE ACCUMULATED SEDIMENT REACHES 30% OF THE SILT FENCE HEIGHT, THE SEDIMENT SHALL BE REMOVED AND DISPOSED OF IN AN APPROPRIATE UPLAND AREA.

DUST CONTROL NOTES:

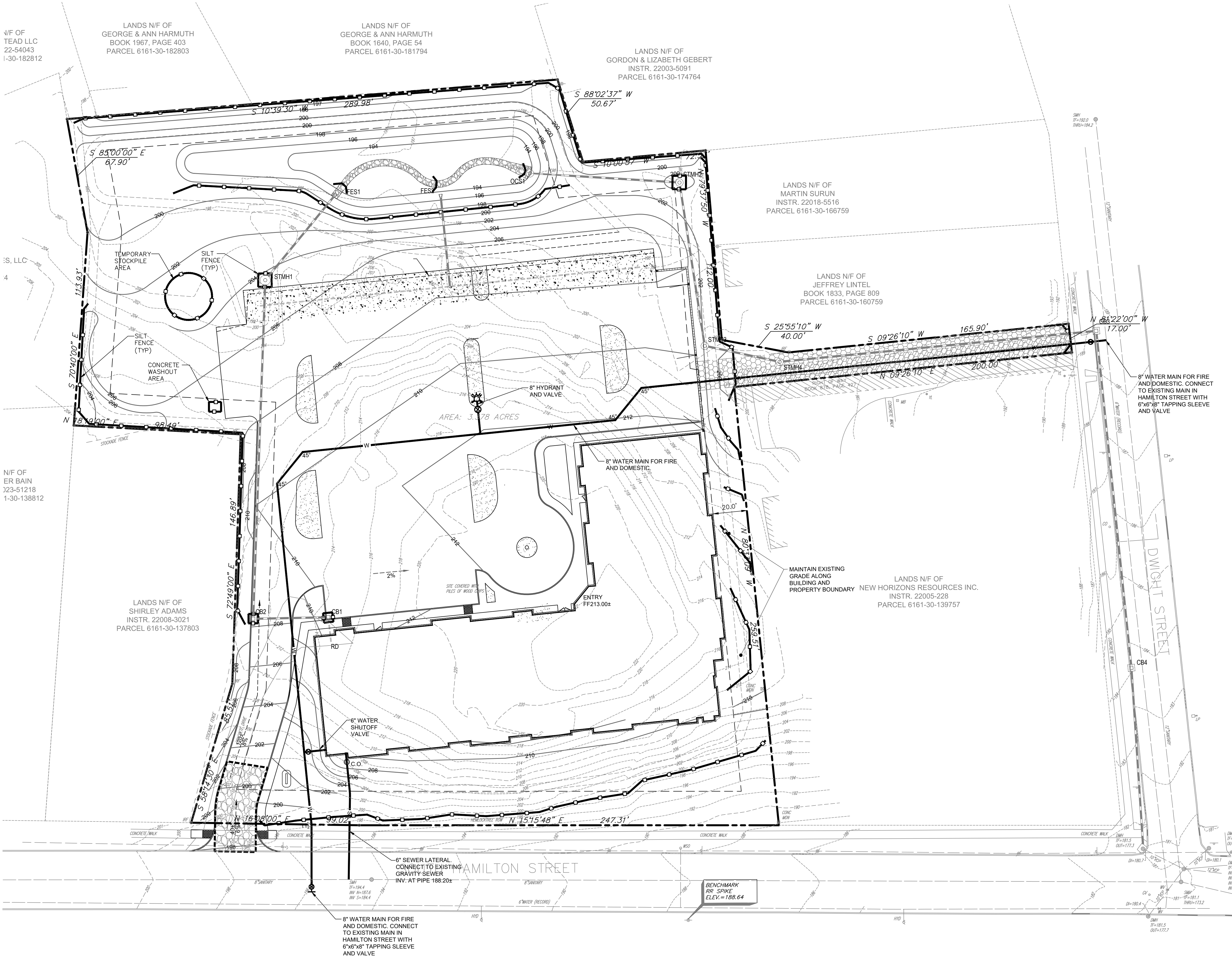
1. DUST SHALL BE CONTROLLED ON THIS PROJECT BY USE OF A WATER TRUCK.
2. THE QUALIFIED INSPECTOR WILL DETERMINE THE FREQUENCY OF WATER APPLICATION IN ORDER TO CONTROL DUST.
3. CHEMICALS OR OTHER METHODS OF DUST CONTROL ARE PROHIBITED TO BE USED ON THIS PROJECT, UNLESS APPROVED BY NYSDEC REGION 3.

STABILIZED CONSTRUCTION ACCESS NOTES:

1. STABILIZED CONSTRUCTION ACCESS SHALL BE INSTALLED WHERE NECESSARY TO PREVENT SEDIMENT FROM BEING TRACKED ONTO ROADWAYS.
2. PERMANENT TRAFFIC CORRIDORS SHALL BE ESTABLISHED AND "ROUTES OF CONVENIENCE" SHALL BE AVOIDED. CONSTRUCTION TRAFFIC SHALL NOT CROSS STREAMS OR DITCHES EXCEPT AT SUITABLE CROSSING FACILITIES, AND SHALL NOT OPERATE UNNECESSARILY WITHIN WATERWAYS OR DRAINAGE DITCHES.
3. IF INTERNAL CONSTRUCTION ROADS ARE DETERMINED TO BE A SOURCE OF SEDIMENT-LADEN RUNOFF TO SENSITIVE AREAS, THEY SHALL BE STABILIZED AS SOON AS PRACTICABLE.

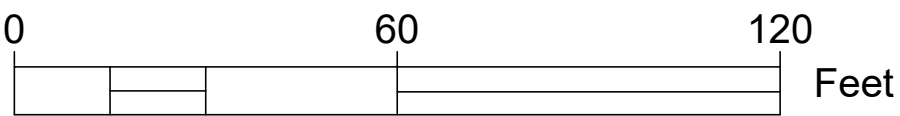
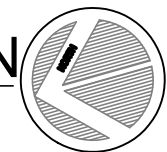
TEMPORARY SEEDING AND MULCHING NOTES:

1. AREAS TO RECEIVE TEMPORARY SEEDING AND MULCHING SHALL RECEIVE BOTH GRASS SEED AND MULCH.
2. A JUTE MESH SHALL BE PLACED OVER THE MULCH IN AREAS WHERE WIND OR WATER EROSION PREVENTS ESTABLISHMENT OF GRASS COVER.



EROSION CONTROL PLAN

SCALE: 1"= 30'



Fire Chief
Police Chief
Building Inspector
City Engineer
Commissioner of Public Works

CITY PLANNING BOARD CITY OF Poughkeepsie, New York
SITE PLAN APPROVAL
FILE NO. _____
PROJECT NAME: _____
PROJECT LOCATION: _____
APPLICANT: _____
OFFICIALLY APPROVED at the Planning Board meeting held the _____ day of _____, 2023.
PLANNING CHAIR: _____
DESIGNEE: _____
DATE SIGNED: _____
CONDITION(S): _____

☐ ISSUED FOR REVIEW
☐ NOT FOR CONSTRUCTION
☐ RECORD PLANS

DATE SUBMITTED: _____

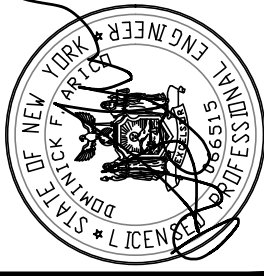
NO.	BY	DATE	REVISIONS
1	DFA	05.11.23	DESIGN: PARKING, LANDSCAPING
2	DFA	08.01.23	REVISED PER CITY COMMENTS
3	DFA	10.03.23	REVISED PER CITY COMMENTS

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DOMINICK S. ARCO, P.E.



ARICO & ASSOCIATES

ENGINEERS
LAND PLANNERS
& CONSULTANTS

1407 Route 9, Bldg 2, Suite 6, Clifton Park, New York 12065

518.573.8989

aricoassociates@gmail.com

CITY OF Poughkeepsie COUNTY OF Dutchess
EROSION CONTROL PLAN
SOUTH HAMILTON

DATE: JANUARY 2023
JOB NO. 22-10

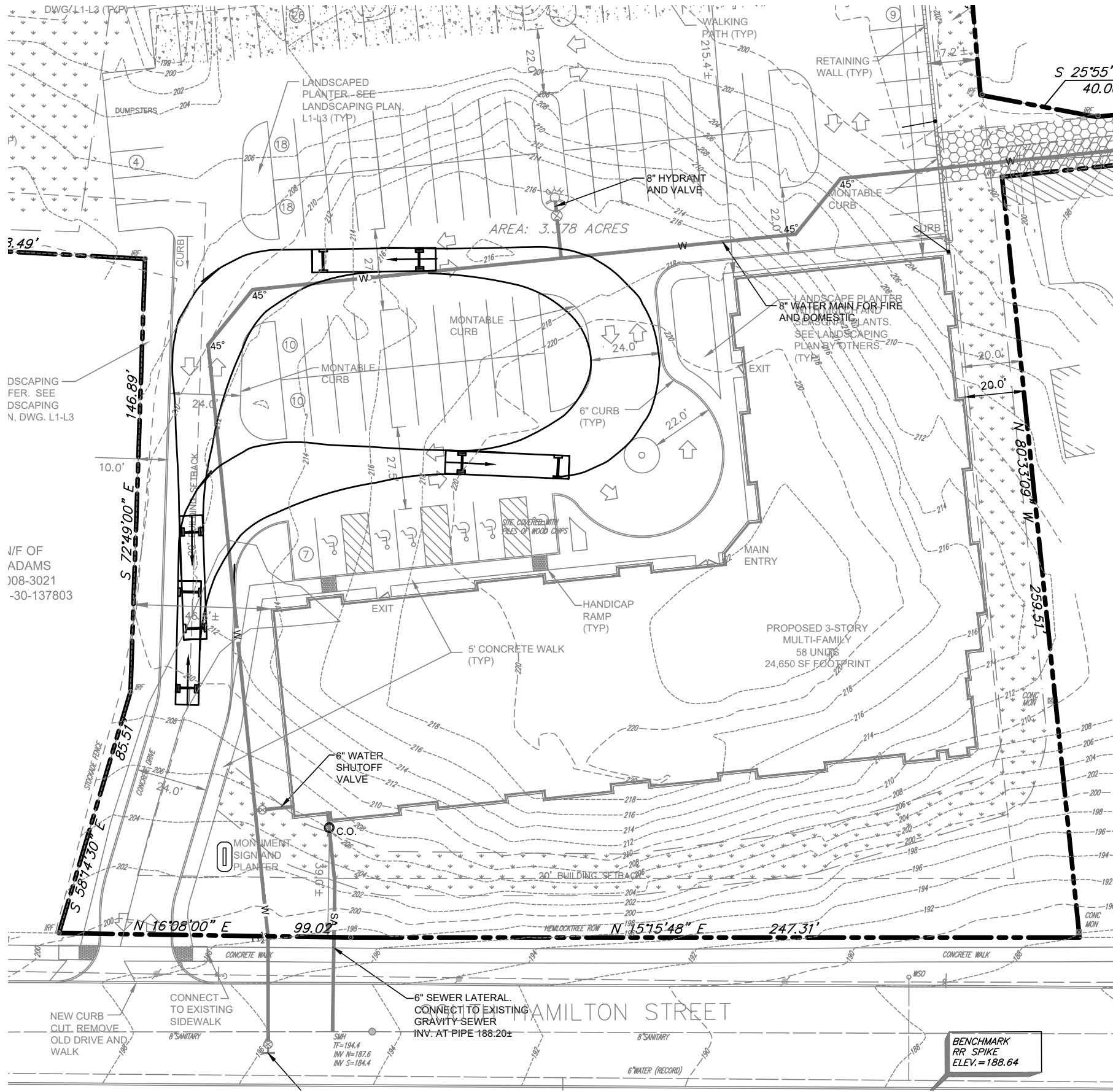
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DRAWN BY: DFA
CHECKED BY: DFA

DRAWING NO.

S4

SHEET 4 OF 13

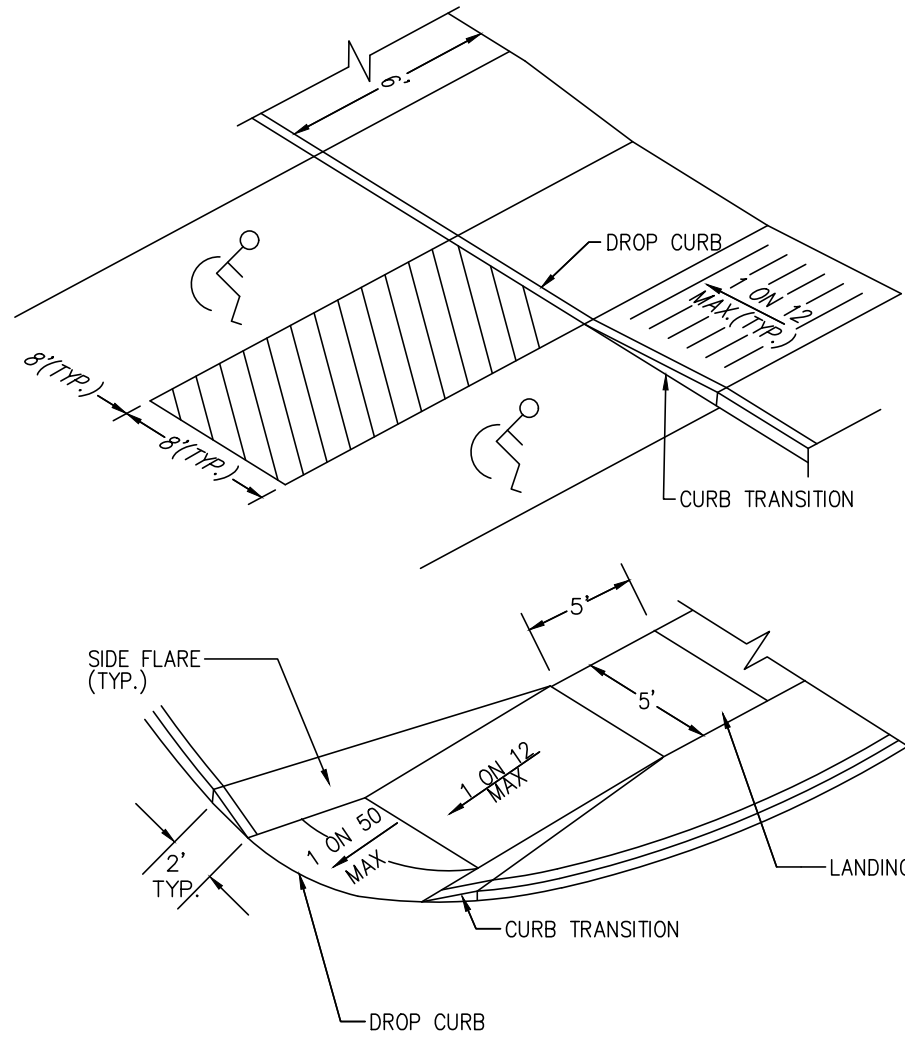
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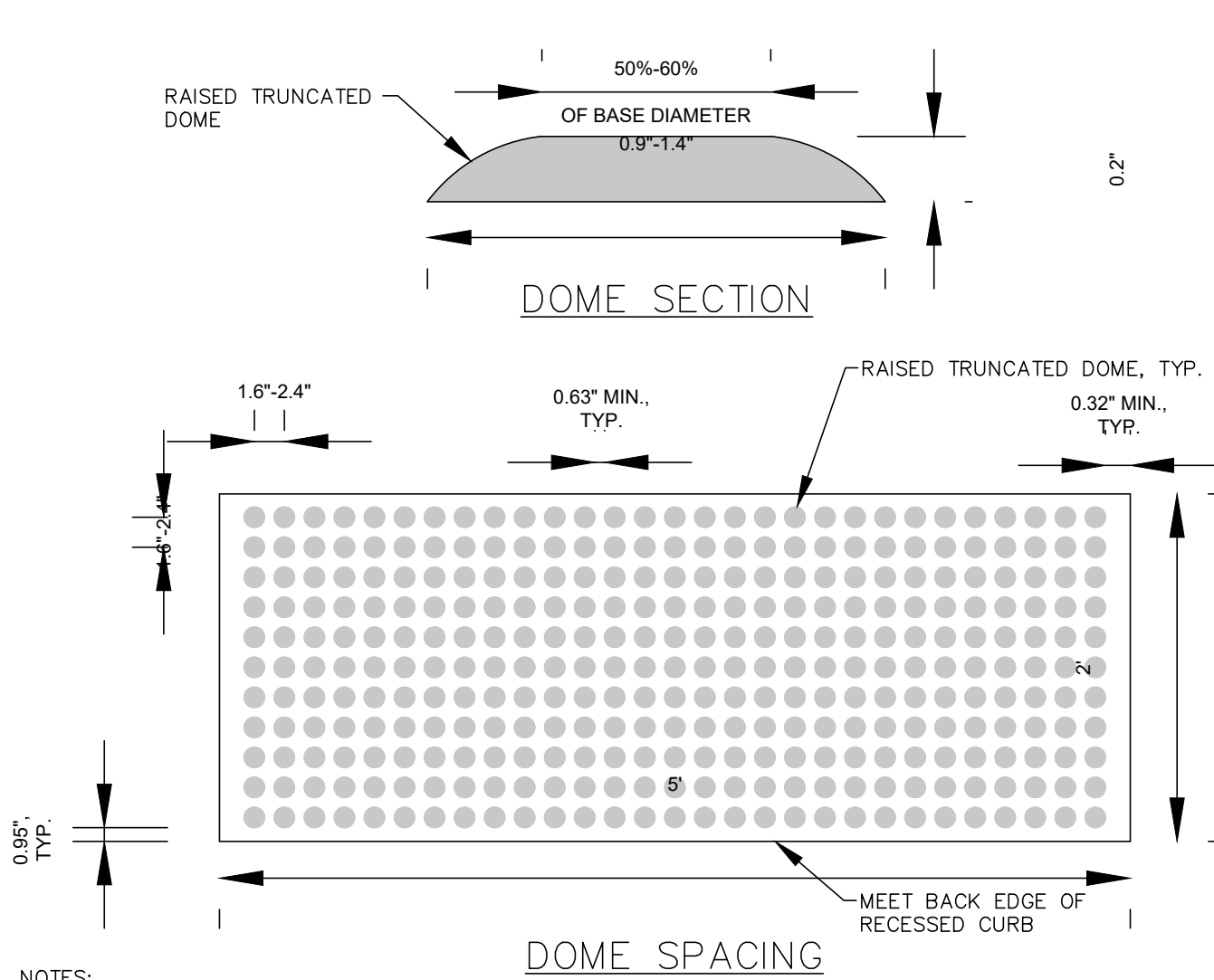
EMERGENCY VEHICLE TURNING MOVEMENT
SCALE: 1"= 40'

COMBINED TYPE LS CROSSWALK MARKING DETAIL

- NOTE:
1. ALL CROSSWALK MARKINGS SHALL BE WHITE.
- N.T.S.



SIDEWALK RAMP DETAILS
SCALE: NO SCALE



- NOTES:
- SIDEWALK SIDE FLARES ARE NOT REQUIRED FOR RAMPS IN LAWN AREAS, SEE PLANS FOR LOCATIONS. IN THESE AREAS, THE CURB SHALL TRANSITION FROM FLUSH TO FULL REVEAL IN 3 LF.
 - IN ACCORDANCE WITH THE AMERICAN WITH DISABILITY ACT (ADA) REQUIREMENTS, DETECTABLE WARNINGS SHALL BE INSTALLED WHENEVER SIDEWALK CURB RAMPS OR CUT-THROUGHS IN MEDIAN/REFUGE ISLANDS, & IN-STREET OR CROSS-SIDEWALK RAIL CROSSINGS ARE CONSTRUCTED OR ALTERED.
 - SURFACE APPLIED DETECTABLE WARNINGS ARE NOT ALLOWED. BRICK PAVERS, TILES, SHEETS & STAMPED CONCRETE METHODS ARE ALLOWABLE.
 - THE SIZE OF THE DETECTABLE WARNING FIELD SHALL BE 2' IN THE DIRECTION OF TRAVEL & SHALL EXTEND THE FULL WIDTH OF THE CURB RAMP OR FLUSH SURFACE, EXCLUSIVE OF SIDE FLARES.
 - DETECTABLE WARNINGS SHALL BE LOCATED SO THAT THE EDGE OF THE WARNING FIELD MEETS THE BACK EDGE OF THE RECESSED CURB.
 - THE COLORS FOR THE DETECTABLE WARNING FIELDS SHALL BE DARK GRAY (MUNSELL BOOK NOTATION BG-PB 3/5), DARK BROWN (MUNSELL BOOK NOTATION 10 YR. 3/2), DARK RED (MUNSELL BOOK NOTATION 10R 3/6), OR DARK GREEN (MUNSELL BOOK NOTATION 2.5G 3/6).
 - DOME SHALL BE ALIGNED ON A SQUARE GRID IN THE PREDOMINANT DIRECTION OF TRAVEL.

REINFORCED CONCRETE SIDEWALK RAMP DETAILS
SCALE: NO SCALE

- ☐ ISSUED FOR REVIEW
☐ NOT FOR CONSTRUCTION
☐ RECORD PLANS

DATE SUBMITTED:

REVISIONS		NO.	DATE	DESCRIPTION
1	BY: DATE: DPA 05.11.21	1	PER CITY COMMENTS	REVISIONS
2	DPA 08.01.21	2	REVISED: PARKING, LANDSCAPING	REVISIONS
3	DPA 10.03.21	3	REVISED PER CITY COMMENTS	REVISIONS

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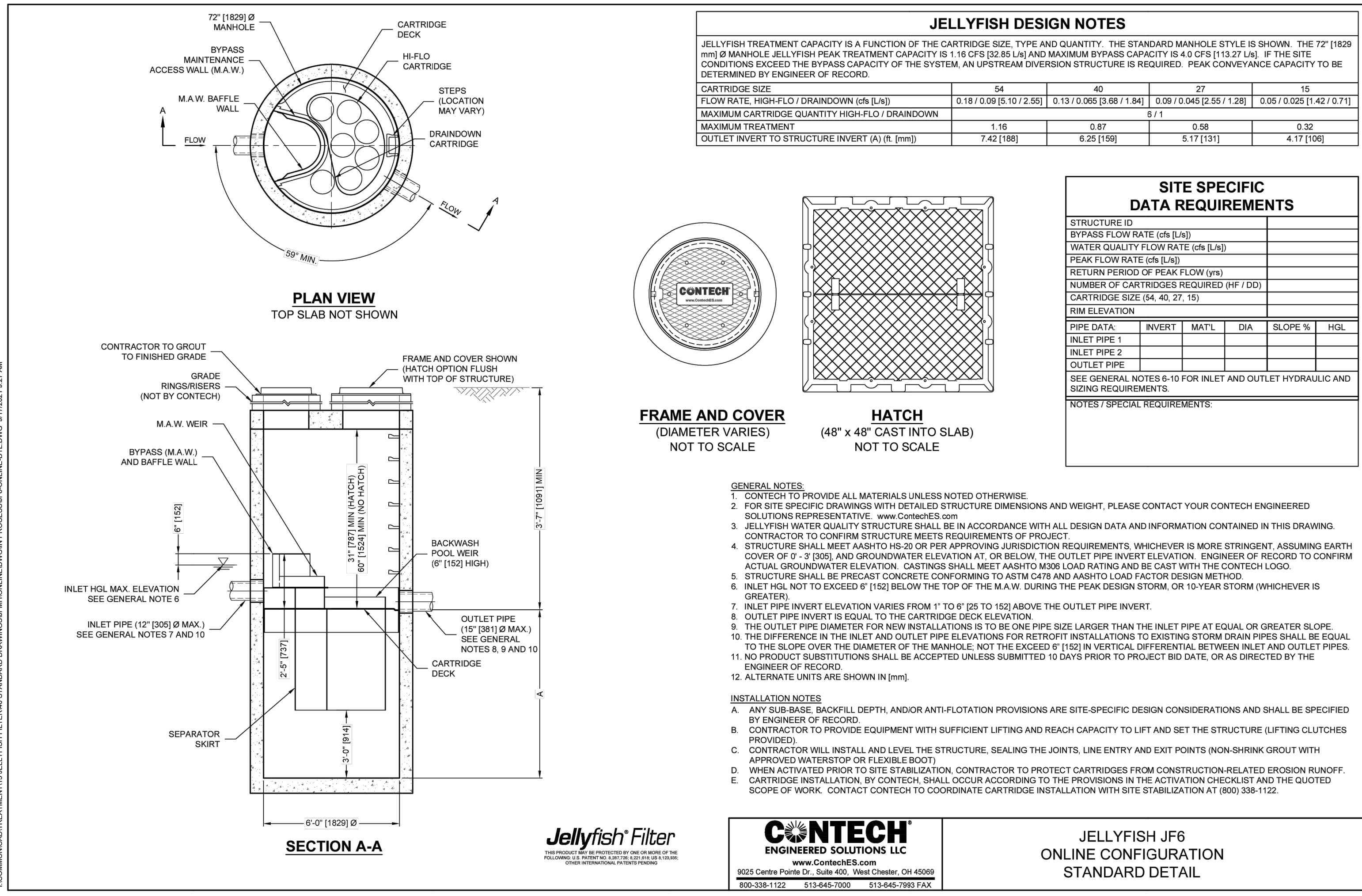
BY A LICENSED PROFESSIONAL ENGINEER IN THE STATE OF NEW YORK
DOMINICK E. ARICO, P.E.
EDUCATION LAW ARTICLE 145, SEC. 1603



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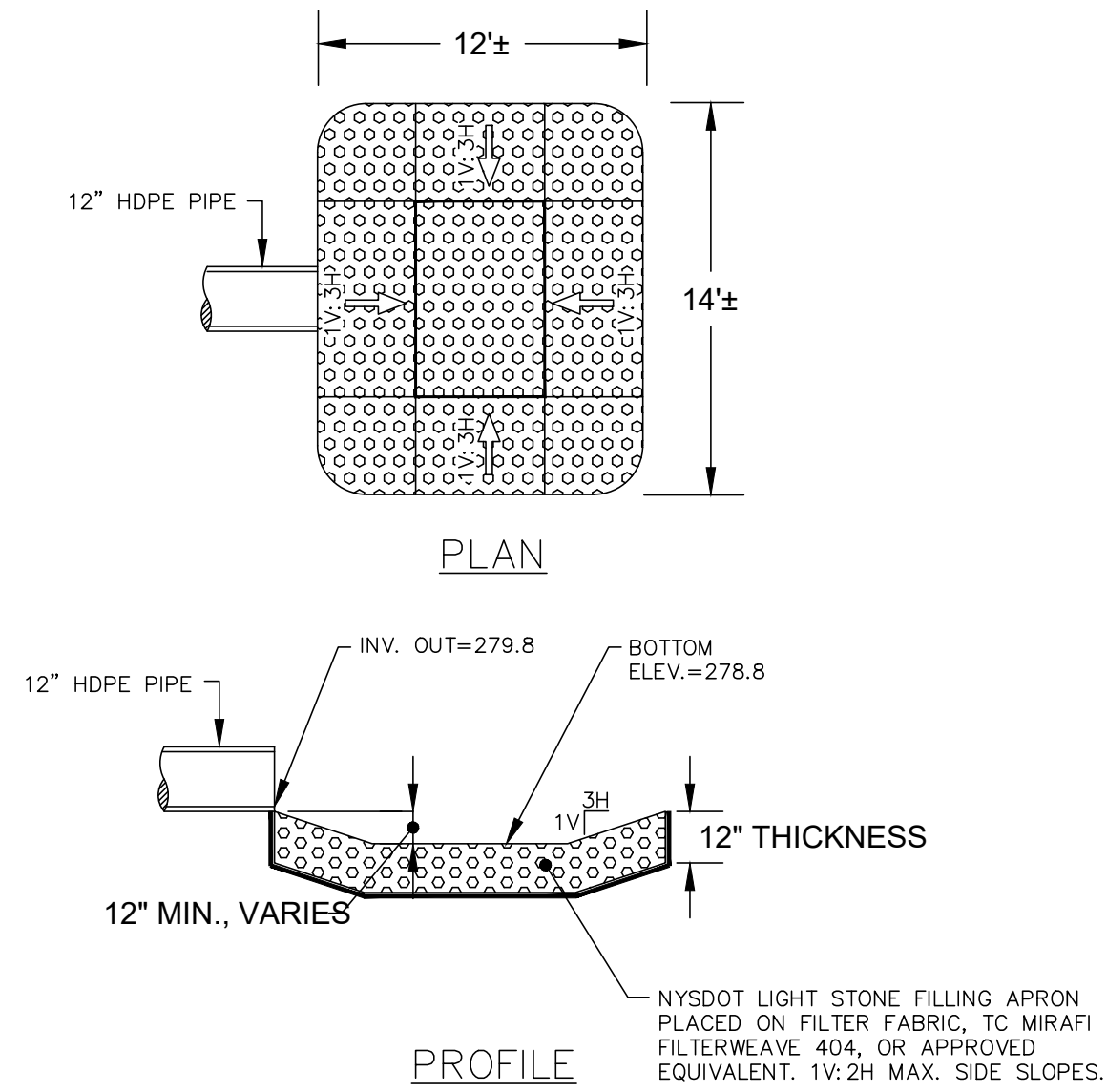
CITY OF Poughkeepsie COUNTY OF Dutchess
STANDARD DETAILS
SOUTH HAMILTON

DATE: JANUARY 2023
JOB NO. 22-10
SCALE: AS NOTED
DRAWN BY: DFA
CHECKED BY: DFA
DRAWING NO.
D1
SHEET 5 OF 13



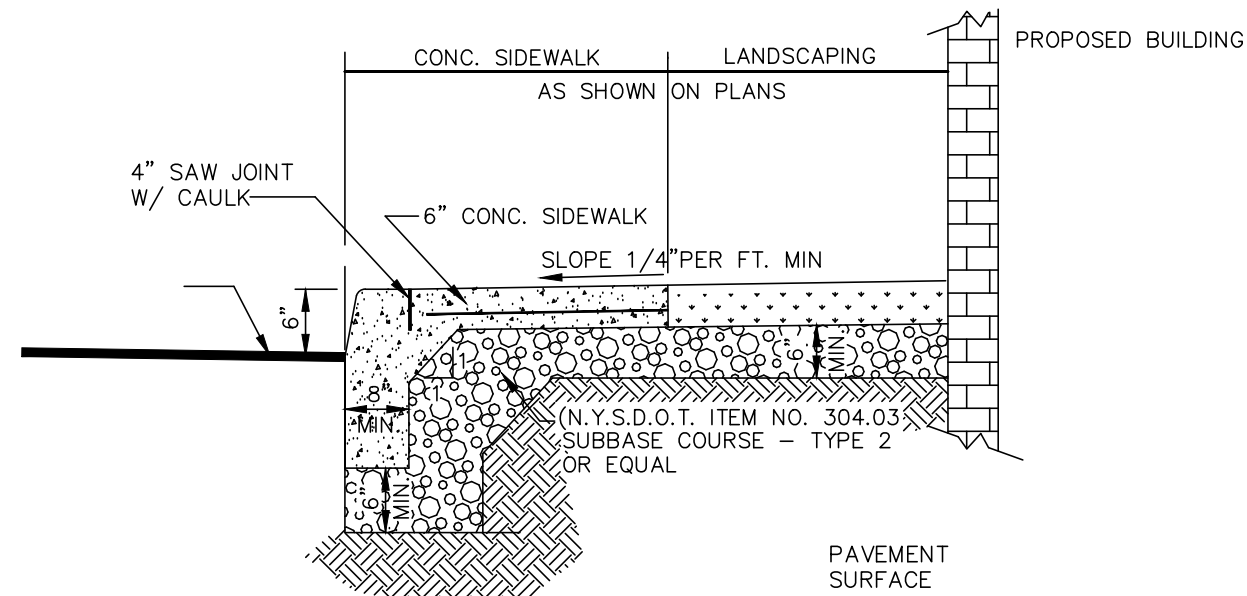
Fire Chief	CITY PLANNING BOARD CITY OF Poughkeepsie, NEW YORK
Police Chief	SITE PLAN APPROVAL FILE NO.: PROJECT NAME: PROJECT LOCATION: APPLICANT:
Building Inspector	OFFICIALLY APPROVED at the Planning Board meeting held the _____ day of _____, 20____.
City Engineer	PLANNING CHAIR: DESIGNEE: DATE SIGNED: CONDITION(S):
Commissioner of Public Works	

1. COMMUNICATIONS TREATMENT JELLYFISH FILTERS STANDARD DRAWINGS FOR INFORMATION PROCESSING ONLINE 071.DWG 01/2023 1:27 AM



EXPANDED SCOUR HOLE DETAIL FOR DETENTION POND OUTLET

SCALE: NO SCALE

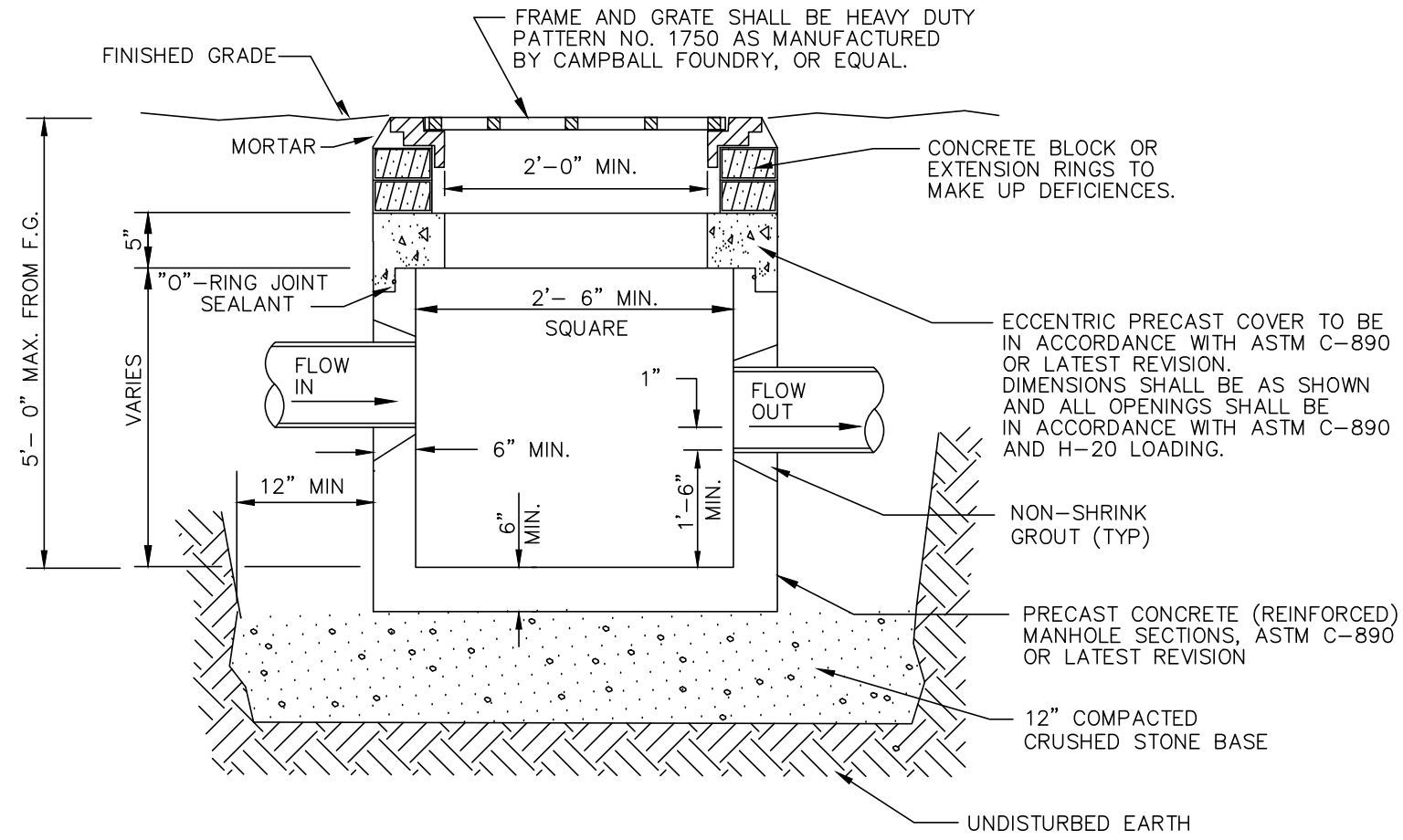


CONCRETE CURB & SIDEWALK DETAIL

SCALE: NONE

NOTES:
FOR DEPTHS GREATER THAN 5'-0",
A 4'-0" I.D. PRECAST CONCRETE
STRUCTURE SHALL BE USED.

STRUCTURES SHALL BE AS
MANUFACTURED BY THE FORT
MILLER COMPANY, OR EQUAL.



TYPE "A" CATCH BASIN

SCALE: NONE

DISINFECTION OF WATER MAINS:

- DISINFECTION METHODS IN ACCORDANCE WITH AWWA C651.
- CHLORINATION SHALL PROVIDE A MINIMUM OF 25 PPM RESIDUAL AFTER 24-HOURS CONTACT IN PIPELINE. IN GENERAL, THIS RESIDUAL MAY BE EXPECTED WITH AN APPLICATION OF 50 PPM ALTHOUGH SOME CONDITIONS MAY REQUIRE MORE.
- CHLORINE, IN A FORM OF A 1% SLURRY OF HIGH TEST CALCIUM HYPOCHLORITE (HTH, PERCHLORON, PITTOCHLOR, ETC.) SHALL BE FED INTO THE PIPELINE IN SUCH A MANNER AS TO MIX WITH THE WATER FLOWING IN THE PIPELINE. (A 1% SLURRY RESULTS FROM MIXING 1 POUND OF CALCIUM HYPOCHLORITE WITH 7.5 GALLONS OF WATER).
- THE FOLLOWING TABLE PROVIDES INFORMATION AS TO THE REQUIRED QUANTITY OF SLURRY TO BE USED PER 100 FEET OF PIPE TO PROVIDE A CHLORINE CONCENTRATION OF 50 PPM:

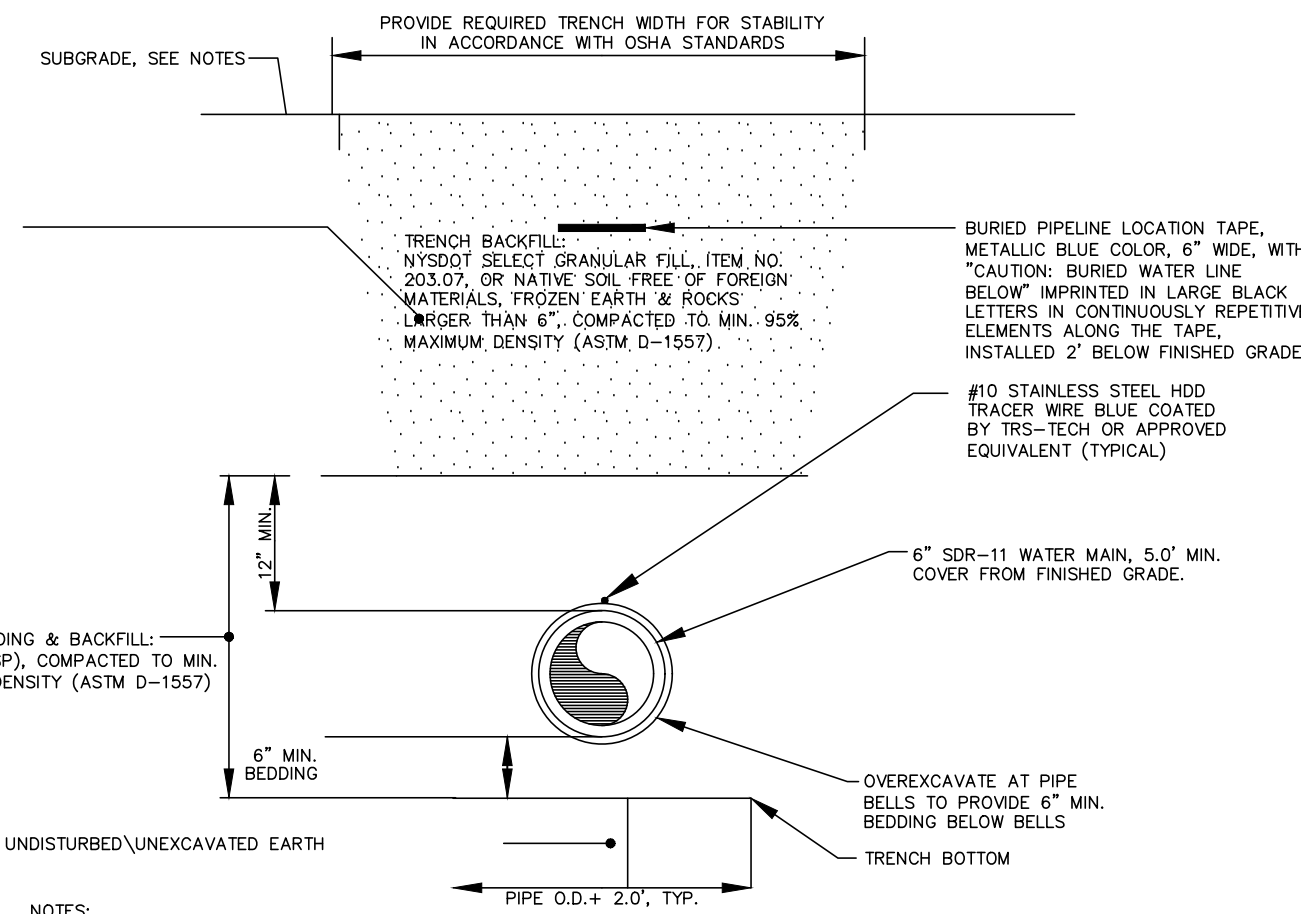
PIPE SIZE INCHES	VOL. OF 100 FT. LENGTH (GALLONS)	REQUIRED AMOUNT OF SLURRY (GALLONS)
4	65	0.33
6	147	0.74
8	261	1.3
10	408	2.0
12	588	3.0
16	1044	5.2

- DURING THE PROCESS OF CHLORINATING THE PIPELINE, ALL VALVES AND OTHER PIPELINE APPURTENANCES SHALL BE OPERATED SEVERAL TIMES TO PROVIDE SUFFICIENT CONTACT WITH THE CHLORINATING AGENT.

HYDROSTATIC WATER MAIN TEST:

- THE HYDROSTATIC TEST METHOD SHALL BE IN ACCORDANCE WITH ANSI/AWWA C600.
- ALL PIPELINES, SERVICES, AIR RELEASE VALVES AND APPURTENANCES SHALL BE HYDROSTATIC TESTED.
- ALL WATER MAINS SHALL BE GIVEN A HYDROSTATIC TEST OF AT LEAST 1.5 TIMES THE WATER SYSTEM OPERATING PRESSURE OR 150 POUNDS PER SQUARE INCH (PSI), WHICHEVER IS GREATER. THE PRESSURE MEASUREMENT SHALL BE MADE FROM THE LOWEST ELEVATION OF THE FORCE MAIN SECTION BEING TESTED. THE TEST PRESSURE SHALL NOT EXCEED THE PIPE OR VALVE RATED PRESSURES.
- THE HYDROSTATIC TEST DURATION OF EACH WATER/FIRE LINE TEST SECTION IS 2 HOURS (UNINTERRUPTED).
- THE MAXIMUM ALLOWABLE LEAKAGE OF FORCE MAIN SHALL BE BASED UPON THE FOLLOWING TABLE:

ALLOWABLE LEAKAGE OF DIP WATER MAINS IN GALLONS PER HOUR (GPH) AT 150 PSI									
DIA.	100 FT.	200 FT.	300 FT.	400 FT.	500 FT.	600 FT.	700 FT.	800 FT.	900 FT.
4	0.04	0.07	0.11	0.15	0.18	0.22	0.26	0.29	0.33
6	0.06	0.11	0.17	0.22	0.28	0.33	0.39	0.44	0.50
8	0.07	0.15	0.22	0.29	0.37	0.44	0.51	0.59	0.66
10	0.09	0.18	0.28	0.37	0.46	0.55	0.64	0.74	0.83
12	0.11	0.22	0.33	0.44	0.55	0.66	0.77	0.88	0.99

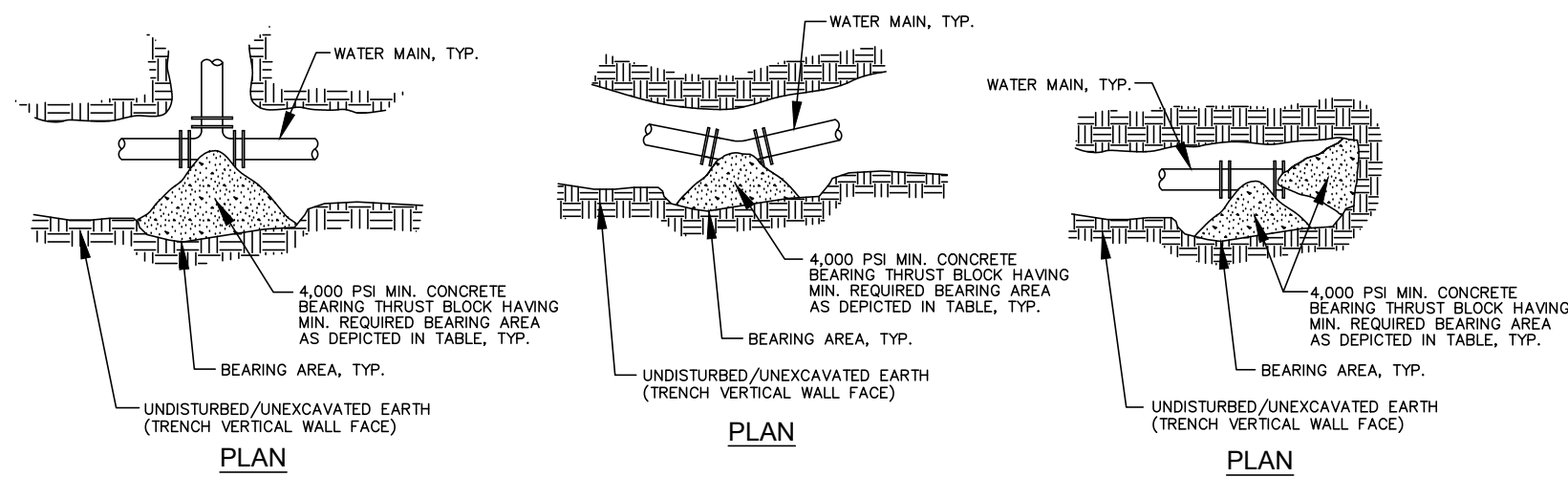


NOTES:

- THE TOWN OF NISKAYUNA SHALL BE NOTIFIED A MIN. OF 72 HOURS PRIOR TO THE COMMENCEMENT OF THE WATER MAIN CONSTRUCTION.
- WATER MAINS SHALL BE INSTALLED IN ACCORDANCE WITH THE REQUIREMENTS OF THE PLASTIC PIPE INSTITUTE, NYSOCH, NYSOC, AMERICAN WATER WORKS ASSOCIATION (AWWA), GREAT LAKES-UPPER MISSISSIPPI RIVER BOARD OF STATE & PROVINCIAL PUBLIC HEALTH & ENVIRONMENTAL MANAGERS' RECOMMENDED STANDARDS FOR WATER WORKS (GLUMRB-TEN STATE STANDARDS), AND THE TOWN OF NISKAYUNA, NY.
- ALL ITEMS DISTURBED BY THE CONTRACTOR, SHALL BE RESTORED TO THE FULL LIMIT OF DISTURBANCE AT THE CONTRACTOR'S EXPENSE.
- PAVEMENT AREAS ONLY: RESTORE DISTURBED AREAS IN ACCORDANCE WITH ASPHALT CONCRETE PAVEMENT DETAIL. B. LAWN AREAS SHALL RESTORE DISTURBED AREAS WITH MIN. 4" TOPSOIL, SEED, MULCH, & SUE MESH.
- ALL NEW WATER SYSTEMS SHALL BE FLUSHED THOROUGHLY AT MIN. 2.5 FT/SEC. FOR A DURATION ACCEPTABLE TO THE TOWN OF NISKAYUNA, THEN HYDROSTATIC PRESSURE TESTED IN ACCORDANCE WITH ANSI/AWWA C906-7 STANDARDS PRIOR TO PLACING IN SERVICE.
- AFTER HYDROSTATIC PRESSURE TESTING, WATER MAINS SHALL BE DISINFECTED IN ACCORDANCE WITH ANSI/AWWA C651 STANDARDS PRIOR TO PLACING IN SERVICE.
- PROVIDE (3) COPIES OF THE HYDROSTATIC PRESSURE, DISINFECTION, & BACTERIOLOGICAL TEST REPORTS TO THE TOWN OF NISKAYUNA AND THE SHERKNESTADT COUNTY DEPARTMENT OF HEALTH FOR APPROVAL.

WATER MAIN TRENCH DETAIL - H.D.P.E. PIPE

SCALE: NO SCALE



THRUST BLOCK DETAIL

SCALE: NO SCALE

MINIMUM REQUIRED BEARING AREAS FOR CONCRETE THRUST BLOCKS 225 PSIG & 2,000 PSF SOIL						
PIPE DIAMETER	DEAD END, TEE	90° BEND	45° BEND	22.5° BEND	11.25° BEND	5.625° BEND
6"	4.2	5.9	3.2	1.6	0.8	0.8

NOTES:

- REQUIRED BEARING AREAS ARE DUE TO THRUSTS CAUSED BY 225 PSI TOTAL INTERNAL PRESSURE (MAX. 150 PSIG WORKING PRESSURE PLUS 75 PSIG SURGE ALLOWANCE)
- CONCRETE THRUST BLOCK SHALL BE PLACED SO THAT THE PIPE & FITTING SHALL BE ACCESSIBLE FOR FUTURE REPAIR. FITTING AND HARDWARE SHALL BE WRAPPED WITH A TOTAL OF 12-IN. OF POLYETHYLENE PLASTIC PRIOR TO PLACING CONCRETE THRUST BLOCKS. THRUST BLOCKS ARE REQUIRED AT ALL HORIZONTAL & VERTICAL BENDS.
- REQUIRED BEARING AREAS ARE BASED ON ALLOWABLE SOIL BEARING CAPACITY OF 2,000 POUNDS PER SQUARE FOOT FOR SAND. DUE TO OTHER SOIL CONDITIONS THAT MAY BE ENCOUNTERED, BEARING AREAS MAY BE MODIFIED (BY THE ENGINEER ONLY) BY MULTIPLYING THE AREA GIVEN IN THE TABLE FOR THE APPROPRIATE PIPE SIZE AND FITTING BY THE CORRECTION FACTORS LISTED BELOW IN THE EVENT THAT ENCOUNTERED SOIL CONDITIONS ARE OTHER THAN SAND. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER OF SAID CONDITIONS, AND THE CONTRACTOR SHALL CONSTRUCT THE MODIFIED CONCRETE THRUST BLOCK(S) IN ACCORDANCE WITH THE ENGINEER'S WRITTEN INSTRUCTIONS. THE CONTRACTOR SHALL NOT MODIFY THE MINIMUM REQUIRED BEARING AREAS OF THE CONCRETE THRUST BLOCKS FROM THOSE DEPICTED ABOVE WITHOUT RECEIVING THE ENGINEER'S WRITTEN PERMISSION TO DO SO.
- IN MUCK, PEAT, OR RECENTLY PLACED FILL, ALL THRUSTS SHALL BE RESISTED BY PILES, OR BY INSTALLING THE RODS TO SOLID FOUNDATIONS, OR BY REMOVAL OF SUCH UNSTABLE MATERIAL AND REPLACEMENT WITH BALLAST OF SUFFICIENT STABILITY TO RESIST THE THRUSTS, ALL AS REQUIRED BY THE ENGINEER.
- ALL CONCRETE THRUST BLOCKS SHALL BE CONSTRUCTED OF MINIMUM 4,000 PSI COMPRESSIVE STRENGTH (f'c) CONCRETE (28-DAY STRENGTH) WITH 4% TO 6% AIR ENTRAINMENT, AND A MAXIMUM WATER-TO-CEMENT (W/C) RATIO OF 0.35. ALL THRUST BLOCKS SHALL BEAR AGAINST UNDISTURBED/UNEXCAVATED SOIL OF TRENCH VERTICAL WALL FACE.
- MINIMUM REQUIRED THRUST BLOCK BEARING AREA SHALL BE THE ACTUAL CONTACT AREA AGAINST THE VERTICAL FACE OF THE TRENCH WALL LOCATED BEHIND THE PIPE FITTING. BEARING AREA SHALL BE IN A SQUARE CONFIGURATION TO THE BEST EXTENT POSSIBLE.

THRUST BLOCK BEARING AREA TABLE & NOTES

Fire Chief
Police Chief
Building Inspector
City Engineer
Commissioner of Public Works

CITY PLANNING BOARD CITY OF Poughkeepsie, New York
SITE PLAN APPROVAL FILE NO: PROJECT NAME: PROJECT LOCATION: APPLICANT:
OFFICIALLY APPROVED at the Planning Board meeting held the ____ day of ____ 20__ PLANNING CHAIR: DESIGNEE: DATE SIGNED: CONDITION(S):

- ☐ ISSUED FOR REVIEW
☐ NOT FOR CONSTRUCTION
☐ RECORD PLANS

DATE SUBMITTED:

NO.	BY	DATE
1	DFA	05.11.21
2	DFA	08.01.21
3	DFA	10.05.21

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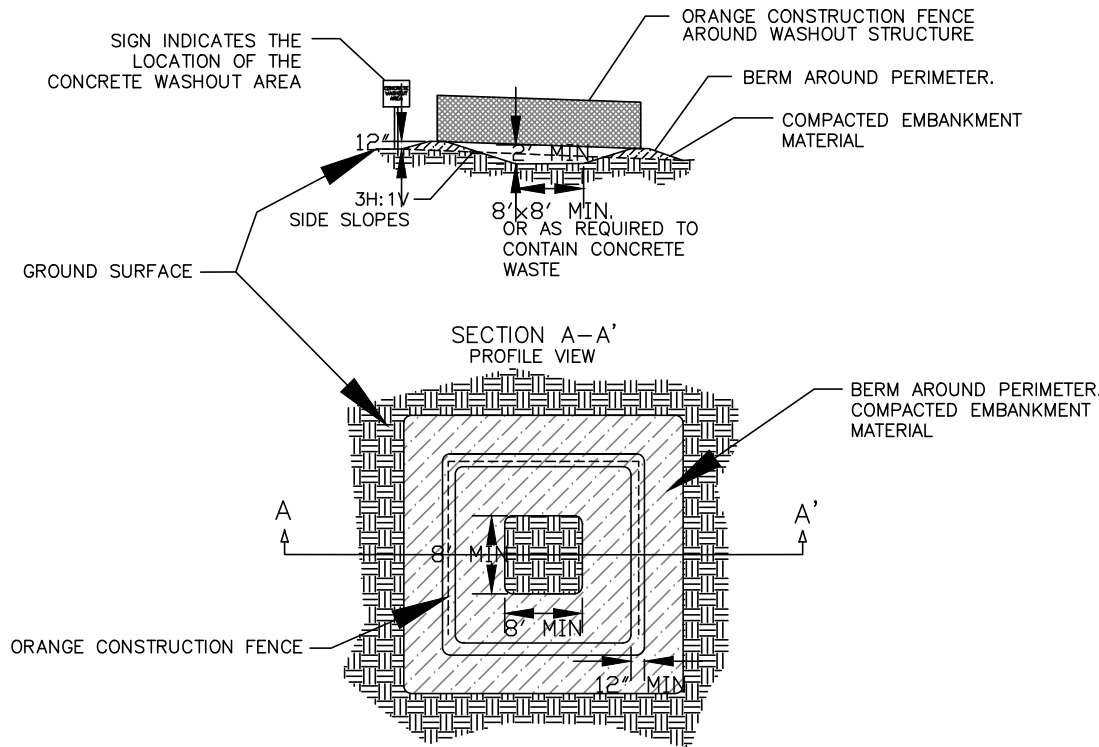
DOMINICK E. ARICO, P.E.
LICENSE NO. 00000000000000000000



ARICO ASSOCIATES
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& CONSULTANTS
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CITY OF Poughkeepsie COUNTY OF Dutchess
STANDARD DETAILS
SOUTH HAMILTON

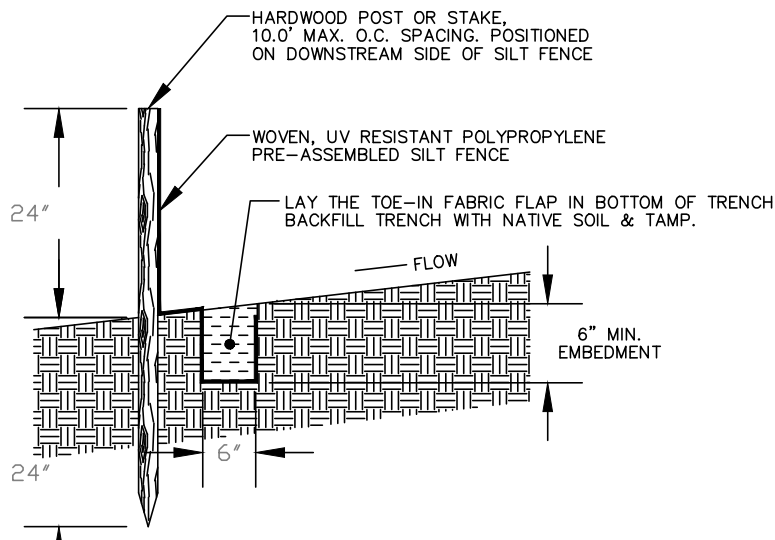
DATE: JANUARY 2023
JOB NO. 22-10
SCALE: AS NOTED
DRAWN BY: DFA
CHECKED BY: DFA
DRAWING NO.
D3
SHEET 7 OF 13



- NOTES:
1. CONCRETE WASHOUT AREA SHALL BE INSTALLED PRIOR TO ANY CONCRETE PLACEMENT ON SITE.
 2. VEHICLE TRACKING CONTROL IS REQUIRED AT CONCRETE WASHOUT ENTRANCE IF ACCESS TO CONCRETE WASHOUT AREA IS OFF PAVEMENT.
 3. THE CONCRETE WASHOUT AREA SHALL BE REPAIRED AND/OR ENLARGED AS NECESSARY TO MAINTAIN CAPACITY FOR WASTE CONCRETE.
 4. WASTE MATERIAL FROM CONCRETE WASHOUT OPERATIONS MUST BE REMOVED AND LEGALLY DISPOSED OF WHEN IT HAS ACCUMULATED TWO-THIRDS OF THE NET STORAGE CAPACITY OF THE STRUCTURE.
 5. AT THE END OF CONSTRUCTION, ALL CONCRETE SHALL BE REMOVED FROM THE SITE AND LEGALLY DISPOSED OF AT AN APPROVED WASTE SITE.
 6. WHEN THE CONCRETE WASHOUT AREA IS REMOVED, THE DISTURBED AREA SHALL BE SEEDD AND MULCHED OR OTHERWISE STABILIZED IN A MANNER ACCEPTED BY THE TOWN.

CONCRETE WASHOUT AREA DETAIL

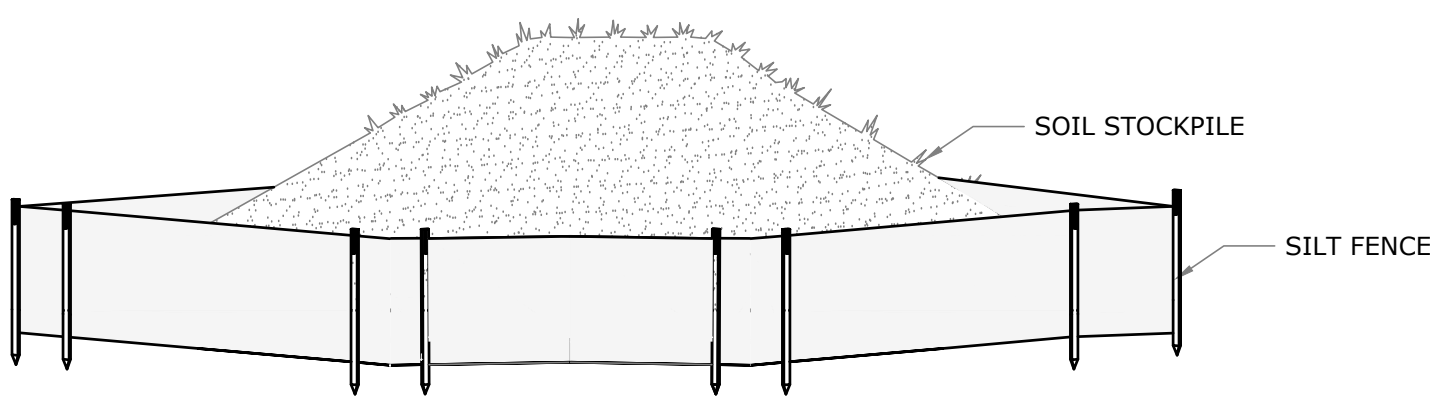
SCALE: NO SCALE



- NOTES:
1. THE LENGTH OF SLOPE ABOVE THE SILT FENCE SHALL NOT EXCEED THE FOLLOWING LIMITS:
 2. DRAINAGE AREA SHALL NOT BE GREATER THAN 0.25 ACRES PER 100 L.F. OF SILT FENCE.
 3. ALL SILT FENCES SHALL BE PLACED AS CLOSE TO THE WORK AREA AS POSSIBLE, AND THE AREA BELOW THE SILT FENCE MUST BE UNDISTURBED OR STABILIZED.
 4. WHERE ENDS OF FILTER CLOTH COME TOGETHER, THEY SHALL BE OVERLAPPED, FOLDED AND STAPLED TO PREVENT SEDIMENT BYPASS.
 5. SILT FENCES SHALL BE REMOVED WHEN THEY HAVE SERVED THEIR USEFULNESS TO PREVENT BLOCKING OR IMPEDING STORMWATER RUNOFF.
- | SLOPE | MAX. SLOPE LENGTH |
|--------------------------|-------------------|
| 1V:2H (50%) | 50 FEET |
| 1V:3H (33%) | 75 FEET |
| 1V:4H (25%) | 125 FEET |
| 1V:5H (20%) | 175 FEET |
| FLATTER THAN 1V:5H (20%) | 200 FEET |

TEMPORARY SILT FENCE DETAIL

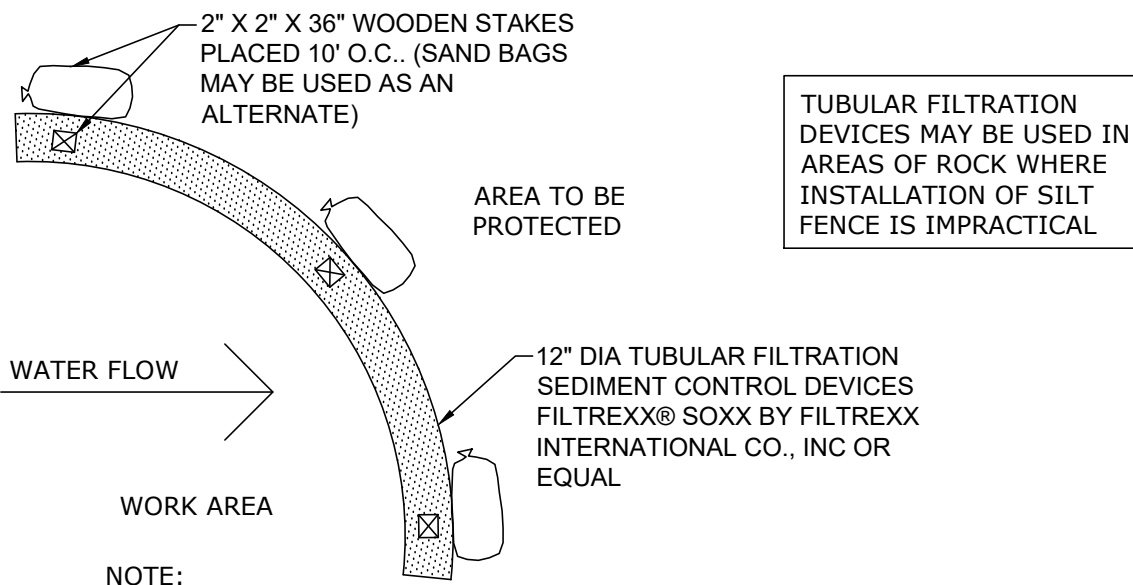
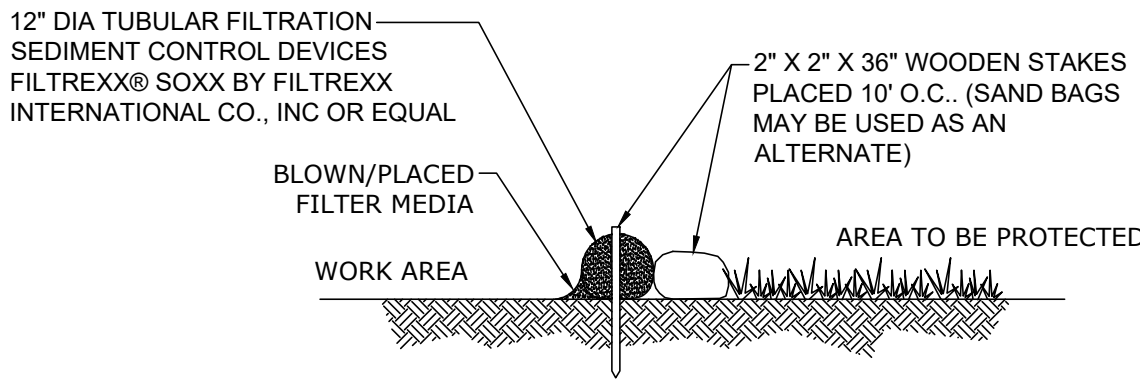
SCALE: NO SCALE



1. TOPSOIL AND FILL THAT IS ANTICIPATED TO REMAIN STOCKPILED ON-SITE FOR PERIODS GREATER THAN 30 DAYS SHALL BE STABILIZED BY SEEDING. PRIOR TO THE SEEDING OPERATION, THE STOCKPILED MATERIAL SHALL BE GRADED TO PERMIT THE USE OF CONVENTIONAL EQUIPMENT FOR SEEDBED PREPARATION, SEEDING, MULCH APPLICATION, AND MULCH ANCHORING. IN NO CASE SHALL ERODIBLE MATERIALS BE STOCKPILED WITHIN 25 FT OF ANY DITCH, STREAM, OR OTHER SURFACE WATERBODY.
2. SILT FENCE SHALL BE CONSTRUCTED AROUND ALL STOCKPILES OF FILL, TOPSOIL AND EXCAVATED OVERBURDEN THAT ARE TO REMAIN EXPOSED FOR PERIODS LESS THAN 30 DAYS. SILT FENCE SHALL BE ANCHORED AND MAINTAINED IN GOOD CONDITION UNTIL SUCH TIME AS SAID STOCKPILES ARE REMOVED AND STOCKPILING AREAS ARE BROUGHT TO FINAL GRADE AND PERMANENTLY STABILIZED.
3. TEMPORARY STABILIZATION MEASURES SHALL START AS SOON AS PRACTICAL ON PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITIES HAVE TEMPORARILY OR PERMANENTLY CEASED, BUT NOT MORE THAN 14 DAYS AFTER WORK HAS CEASED.

SOIL STOCKPILE DETAIL

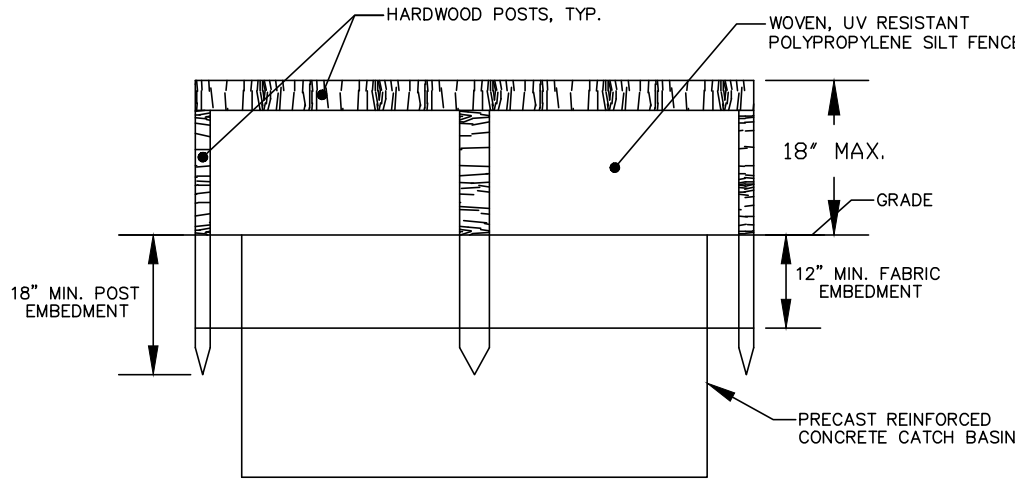
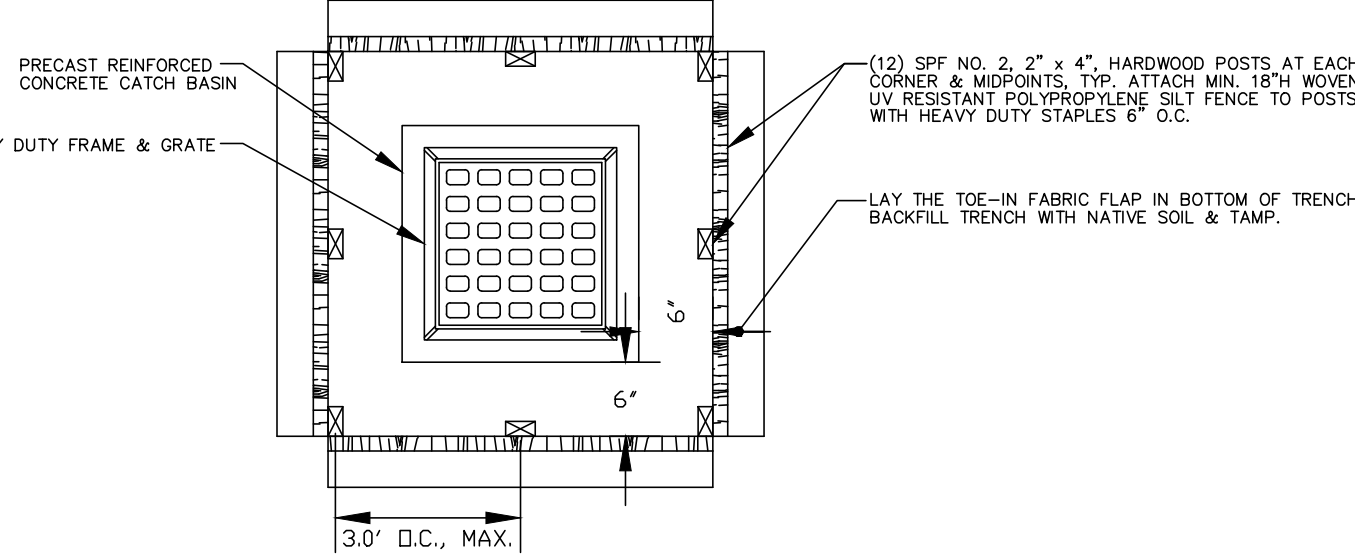
SCALE: NONE



- NOTE:
1. FILL TUBULAR FILTRATION SEDIMENT CONTROL DEVICE WITH FILTER MEDIA APPROVED BY NYSDEC FOR THIS APPLICATION.

TUBULAR FILTRATION SEDIMENT CONTROL DEVICE

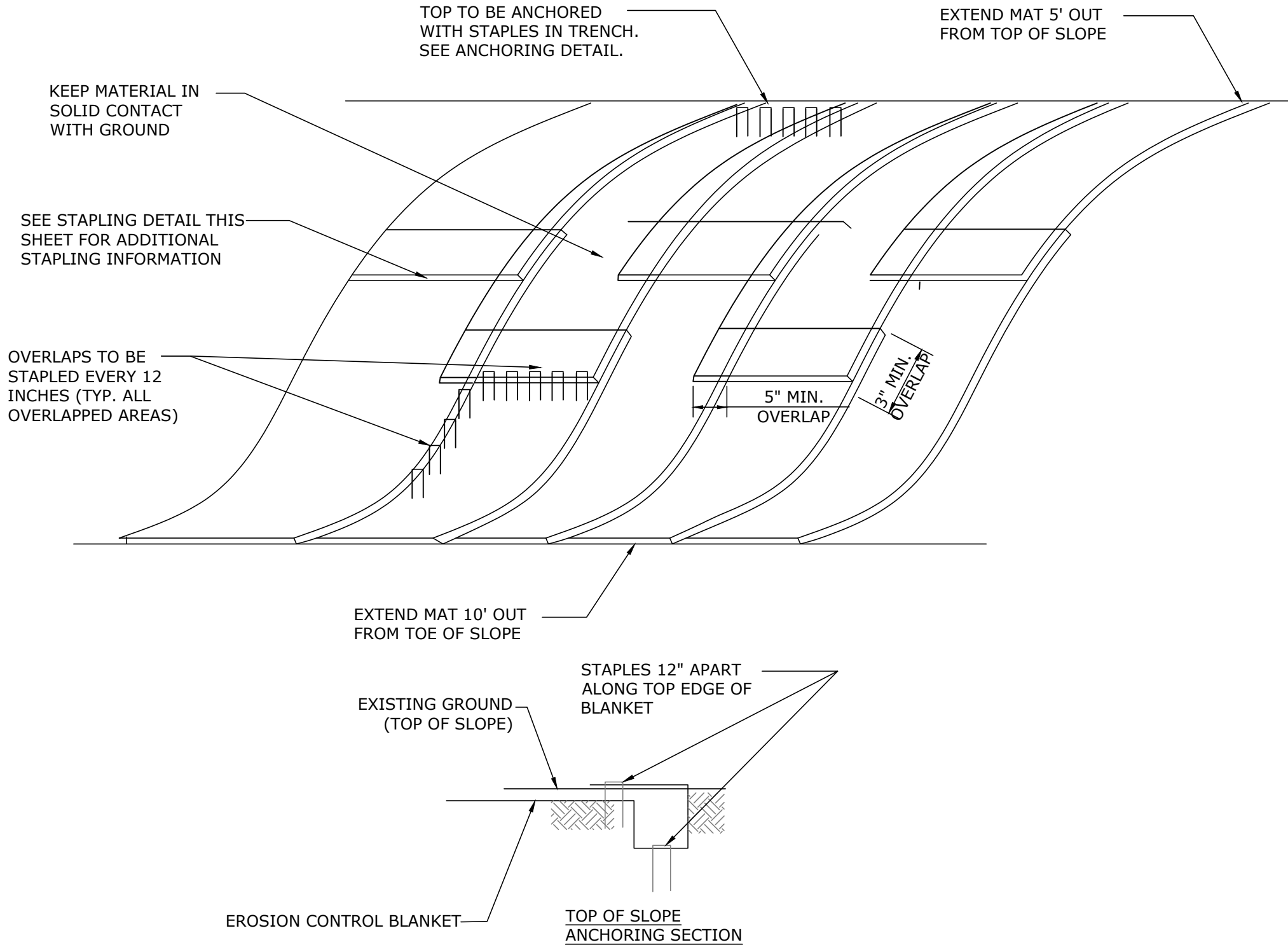
SCALE: NO SCALE



- NOTES:
1. CUT FABRIC FROM A CONTINUOUS ROLL TO ELIMINATE JOINTS. IF JOINTS ARE NEEDED THEY MUST BE OVERLAPPED TO THE NEXT STAKE.
 2. 1 ACRE MAXIMUM DRAINAGE AREA PER PROTECTION DEVICE.
 3. LAND AREA SLOPE IMMEDIATELY SURROUNDING THE PROTECTION DEVICE SHOULD NOT EXCEED 1%.
 4. REMOVE SEDIMENT FROM THE POOL AREA AS NECESSARY WITH CARE TO PREVENT UNDERCUTTING OR DAMAGING THE FILTER FABRIC.
 5. PROTECTION DEVICE SHALL BE KEPT IN PLACE UNTIL ASPHALT CONCRETE PLACEMENT IS COMMENCED.

TEMPORARY FILTER FABRIC CATCH BASIN PROTECTION DETAIL

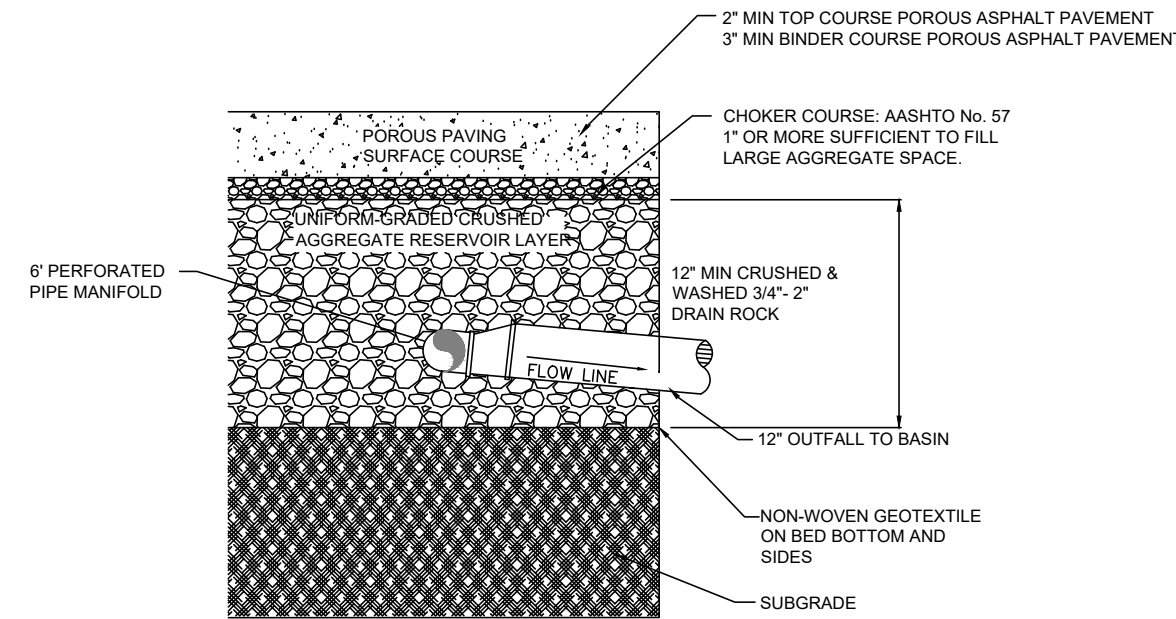
SCALE: NO SCALE



- NOTES:
- 1) APPLY APPLICATION OF ANY LIME, FERTILIZER, AND SEED BEFORE INSTALLING TURF REINFORCEMENT MAT.
 - 2) INSTALL USING ALL MANUFACTURERS INSTALLATION RECOMMENDATIONS AND GUIDELINES.
 - 3) BLANKET FOR 5:1 SLOPES - S75 BY NORTH AMERICAN GREEN
BLANKET FOR 2:1 TO 3:1 SLOPES - HYDRAMAX STEEP SLOPE MATRIX - NORTH AMERICAN GREEN

EROSION CONTROL BLANKET DETAIL

SCALE: NONE



- NOTES:
1. PRIVATE WATER QUANTITY CONTROL SYSTEM, NOT FOR PUBLIC RIGHT OF WAY
 2. PAVEMENT SURFACE WITH SIGNIFICANT PERMEABILITY (> 8\"/>

POROUS PAVEMENT DETAIL

SCALE: NO SCALE

- ☐ ISSUED FOR REVIEW
☐ NOT FOR CONSTRUCTION
☐ RECORD PLANS

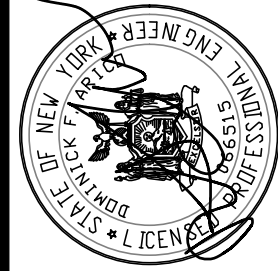
DATE SUBMITTED:

REVISIONS		NO.	PER CITY COMMENTS	DESCRIPTION
1	DATE	1	BY	DATE
2	DATE	2	BY	DATE
3	DATE	3	BY	DATE

THE ENGINEER, THE ARCHITECT AND DESIGNER HEREBY WARRANT THAT THE INFORMATION CONTAINED HEREIN IS TRUE AND CORRECT TO THE BEST OF THEIR KNOWLEDGE AND BELIEF. ANY INFORMATION NOT SO WARRANTED SHALL BE SO INDICATED. THIS WARRANTY IS LIMITED TO THE INFORMATION CONTAINED HEREIN AND DOES NOT EXTEND TO ANY OTHER INFORMATION OR TO ANY OTHER PART OF THE PROJECT. THE ENGINEER, THE ARCHITECT AND DESIGNER SHALL NOT BE RESPONSIBLE FOR ANY CONSEQUENCES OF THE USE OF THIS INFORMATION FOR ANY PURPOSE OTHER THAN THAT FOR WHICH IT WAS PROVIDED.

ALTERNATION OF THIS DOCUMENT, EXCEPT AS NOTED, SHALL BE INDICATED BY A LICENSED PROFESSIONAL ENGINEER OR ARCHITECT. THE ENGINEER, THE ARCHITECT AND DESIGNER SHALL BE RESPONSIBLE FOR THE ACCURACY OF THE INFORMATION CONTAINED HEREIN.

DOMINICK E. ARICO, P.E.
aricoassociates@gmail.com

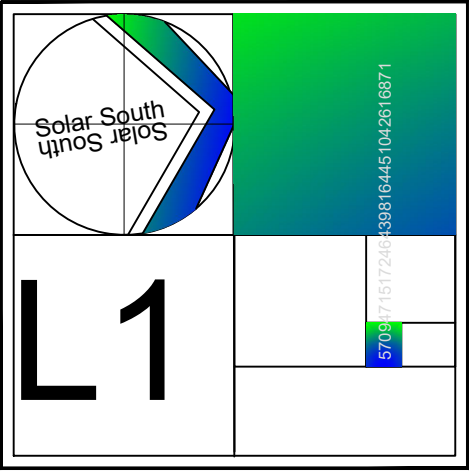
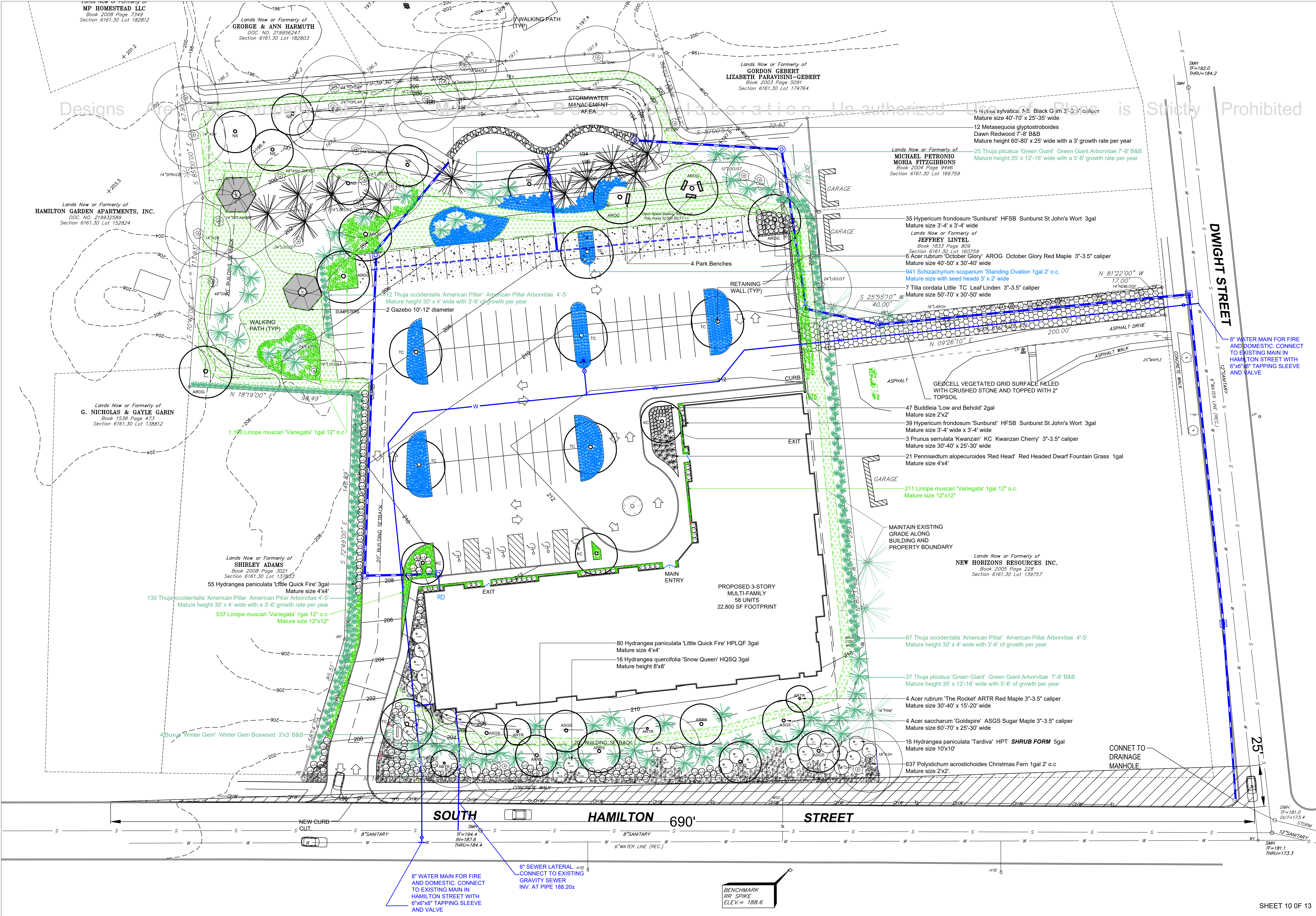


ARICO ASSOCIATES
ENGINEERS
LAND PLANNERS
& CONSULTANTS
1407 Route 9, Bldg. 2, Suite 6, Clifton Park, New York 12065
518.573.0959
aricoassociates@gmail.com

CITY OF Poughkeepsie COUNTY OF Dutchess
STANDARD DETAILS
SOUTH HAMILTON

DATE: JANUARY 2023
JOB NO. 22-10
SCALE: AS NOTED
DRAWN BY: DFA
CHECKED BY: DFA
DRAWING NO.
D4
SHEET 8 OF 13

Fire Chief	CITY PLANNING BOARD CITY OF Poughkeepsie, New York SITE PLAN APPROVAL FILE NO.: PROJECT NAME: PROJECT LOCATION: APPLICANT: OFFICIALLY APPROVED at the Planning Board meeting held the _____ day of _____, 20____. PLANNING CHAIR: DESIGNEE: DATE SIGNED: CONDITION(S):
Police Chief	
Building Inspector	
City Engineer	
Commissioner of Public Works	



845-702-1004

The Michael Boice Collaboration

DESIGNERS AND MAKERS OF FINE LANDSCAPES

One Key South Hamilton Street

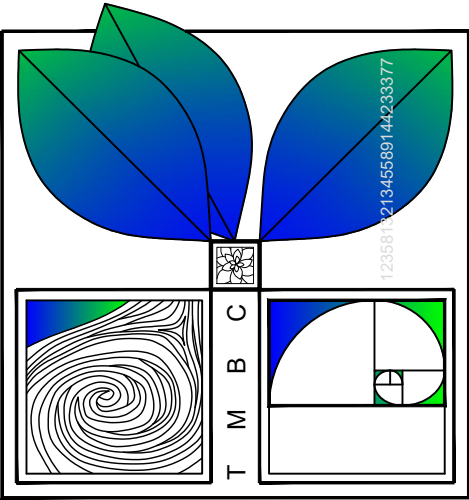
South Hamilton Street Poughkeepsie, New York

Scale | 1" = 25' - 0"

Date | September 21, 2023

Revisions |

11 South Fowlerhouse Road Wappinger Falls, New York 12590



PLANTING NOTES

Designs Are The Property of The Michael Boice Collaboration. Un-authorized Use of Plans is Strictly Prohibited

1. All plant material must conform to American Standard for Nursery Stock ANSI Z60.1-2014

2. Voluntary Alternates/Plant substitutions must be approved by the Designer/Landscape Architect

3. All plant material delivered to the site and or planted must be maintained by the Landscape Contractor until the scope of agreed Landscape work is complete

4. All wire baskets must be either removed from the root ball or cut from the root ball after placing the plant in the planting hole

5. All burlap must be removed from root balls or cut away from the root ball after placing the plant in the planting hole. All planting holes must be 50% wider than the root ball or container root mass.

6a. The indigenous soil or back fill for major trees must not be conditioned unless rocky or unsuitable conditions are present
- 6b. Incorporate 50% organic soil media and slow release fertilizer per instructions into indigenous soil used to back fill shrubs, perennials and ground cover. Ground cover areas in particular may require mechanical tilling in an effort to create a friable soil texture

6c. Any soil glazing created by the back of the shovel shall be removed by reversing the shovel and caving in the planting hole

7. A soil pH test is highly recommended before planting begins

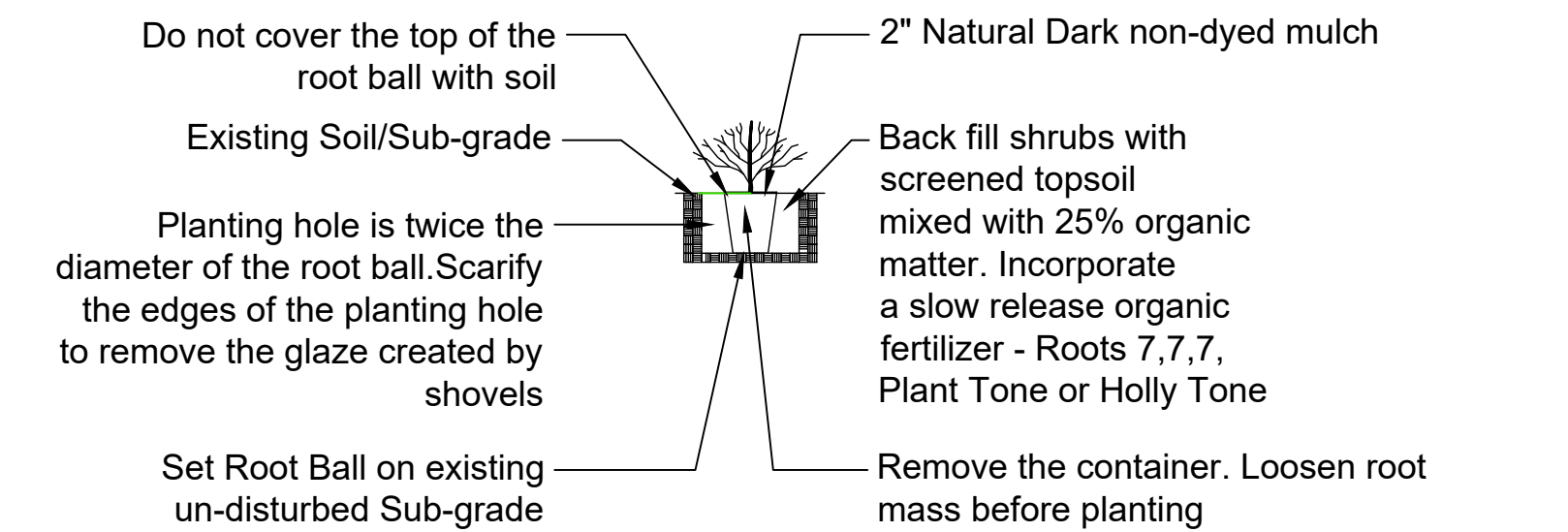
8. All existing unwanted plant material and weeds and their roots systems must be completely removed from planting beds prior to installing new plant material. The use of Round-Up may be required to eliminate invasive/stoloniferous weeds. Only a licensed professional may apply Round-Up

9. Only non-dyed natural mulch may be used. Install mulch to a depth of 2" - 3"

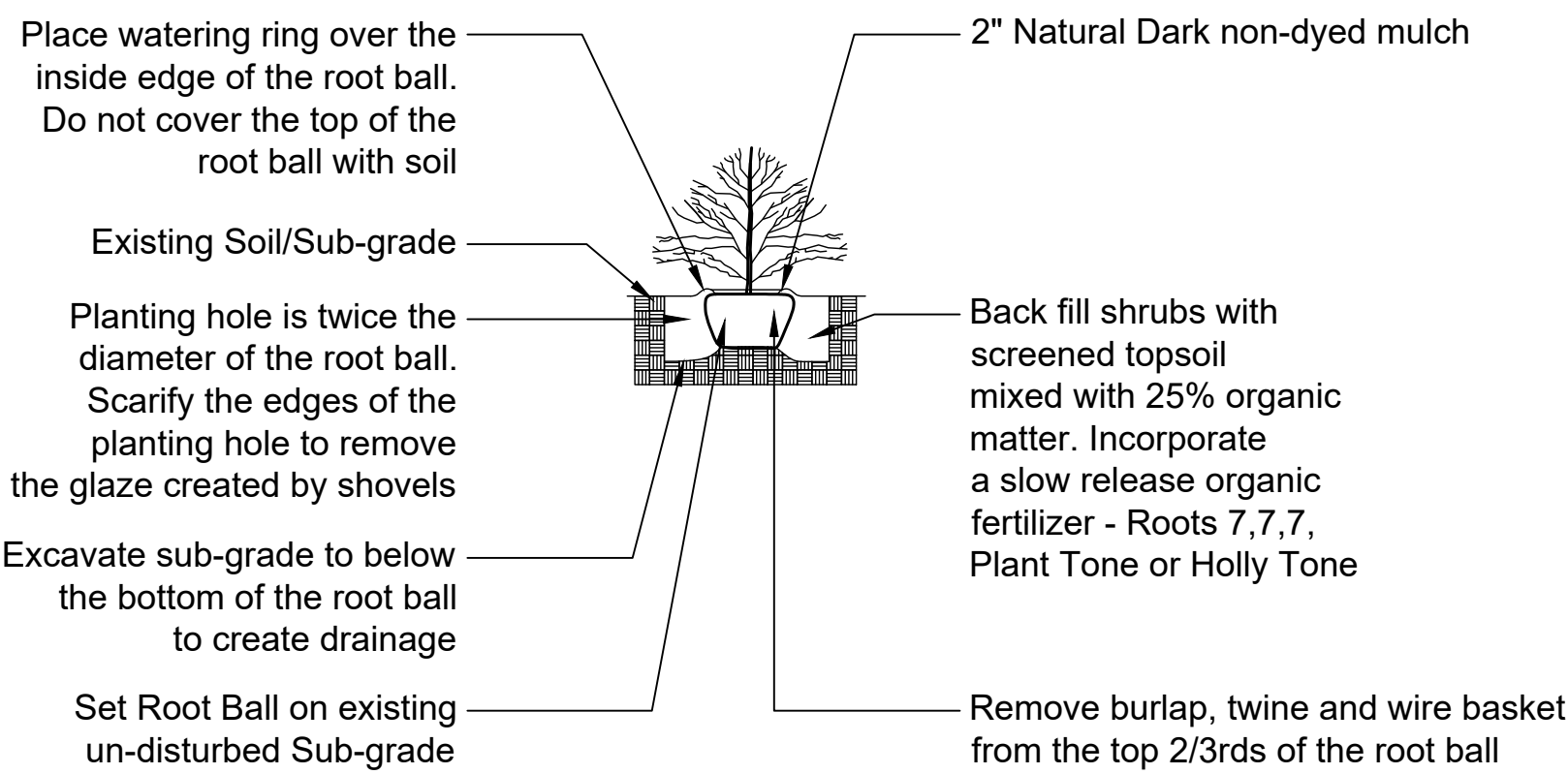
10. Stake any trees or plants that may be affected by high wind loads.

11. Where an automatic irrigation system is recommended, the Landscape Contractor shall provide proof of CID, CIC, CIT - Certified Irrigation Designer, Contractor, Technician through the Irrigation Association

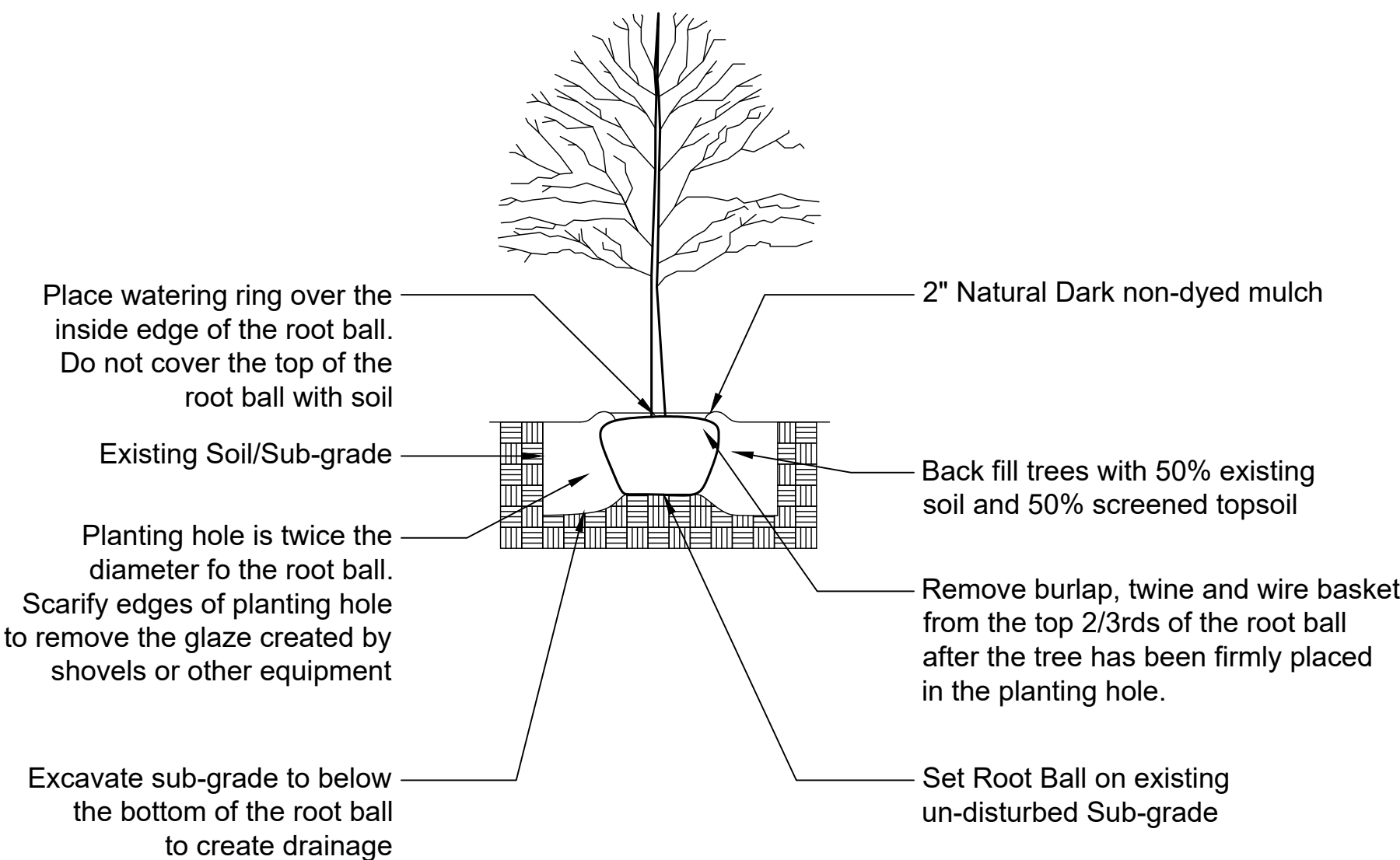
12. It is the Landscape Contractor's responsibility to comply with all local and state codes. Any permits required for the Landscape Contractor's work must be obtained by the Landscape Contractor
13. It is the Landscape Contractor's responsibility to coordinate the planting schedule with the owner
14. Any deviations from the design due to unknown site conditions must be brought to the attention of the Designer/Landscape Architect
15. All Grasses and perennials must be cut back either in fall or early spring - before new foliage emerges. Grasses must not be pruned lower than 12", 18" is ideal
16. All hydrangea paniculata can be pruned in fall, winter or early spring. Little Quick fire does not require pruning and will grow to 4'x4'
17. Hypericum can be pruned to 12"-18" in late fall or early spring
18. The Liriope can be cut with a typical hand lawn mower set at 3" in late fall or early spring before the new foliage appears



CONTAINER PLANTING SPECIFICATIONS

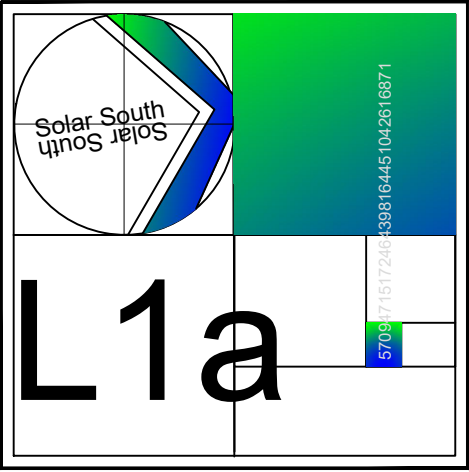


SHRUB PLANTING SPECIFICATIONS



TREE PLANTING SPECIFICATION

PLANTING LIST						
GENUS SPECIE	COMMON NAME	SIZE	KEY	QUANTITY	NOTES	Zone
Tilia tomentosa	Silver Leaf Linden	3"-3.5" cal. branched 5-6'	TT - Labeled	7	Native, fragrant pollinator friendly	4-7
Acer rubrum 'The Rocket'	The Rocket Red Maple	3"-3.5" cal. branched 5-6'	ARTR - Labeled	4	Native, red fall color upright habit	3-9
Acer saccharum 'Goldspire'	Goldspire Sugar Maple	3"-3.5" cal. branched 5-6'	ASGS - Labeled	4	Native, fragrant pollinator friendly	3-9
Acer rubrum 'October Glory'	October Glory Red Maple	3"-3.5" cal. branched 5-6'	AROG - Labeled	6	Native, wet soils, red/orange fall color	3-9
Metasequoia glyptostroboides	Dawn Redwood	7'-8'	Labeled	12	Introduced, wet soils, deciduous evergreen	5-10
Nyssa Sylvatica	Black Gum	3"-3.5" cal.	NS - Labeled	6	Native wetland tree, red fall color	3-9
Prunus serrulata 'Kwanzan'	Kwanzan Cherry	3"-3.5" cal. branched 5-6'	KC - Labeled	3	Introduced, pink flowers spring, yellow fall color	5-8
Thuja plicata 'Green Giant'	Green Giant Arborvitae	7'-8'	Labeled	62	Evergreen, deer resistant, grow 5'-6'/yr	5-8
Thuja occidentalis 'American Pillar'	American Pillar Arborvitae	4'-5' Larger sizes N/A	Labeled	229	Evergreen, deer resistant, grow 4'-5'/yr	2-7
Hydrangea paniculata 'Little Quick Fire'	Little Quick Fire Hydrangea	3 gal	HPLQF - Labeled	135	White flowers summer fade to red/pink in fall	4-8
Hypericum frondosum 'Sunburst'	Sunburst St John's Wort	3 gal	HFSB - Labeled	16	Yellow flowers summer native specie	5-8
Hydrangea quercifolia 'Snow Queen'	Snow Queen Oakleaf Hydrangea	3 al	HQSQ - Labeled	16	White flowers summer fade to pink in fall	4-8
Hydrangea paniculata 'Tardiva'	Tardiva Hydrangea	5 gal	HPT - Labeled	16	White flowers summer fade to pink in fall	4-8
Buddleia 'Low and Behold'	Low and Behold Buddleia	2 gal	BLB - Labeled	47	Lavender flowers all summer, fragrant	5-9
Pennisetum alopecuroides 'Red Head'	Red Headed Fountain Grass	1 gal	Labeled	21	Introduced Drought tolerant	5-9
Schizachyrium scoparium 'Standing Ovation'	Standing Ovation Little Bluestem	1 gal	Labeled	911	Native grass, intense blue color	3-10
Liriope spicata 'Variegata'	Variegated Liriope	1 gal	Labeled	1,916	Low Grassy texture, Purple Flowers	5-10
Polystichum acrostichoides	Christmas Fern	1 gal	Labeled	637	Evergreen fern	3-8



845-702-1004

11 South Fowlerhouse Road Wappinger Falls, New York 12590

The Michael Boice Collaboration

DESIGNERS AND MAKERS OF FINE LANDSCAPES

One Key South Hamilton Street

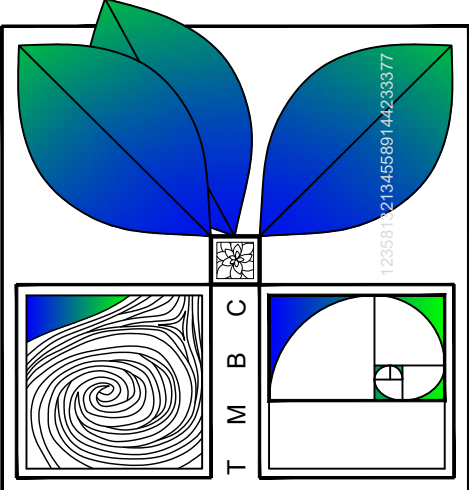
South Hamilton Street Poughkeepsie, New York

Scale | 1" = 25' - 0"

Date | September 27, 2023

Revisions |

mm September 26, 2023
mm September 26, 2023
mm September 26, 2023

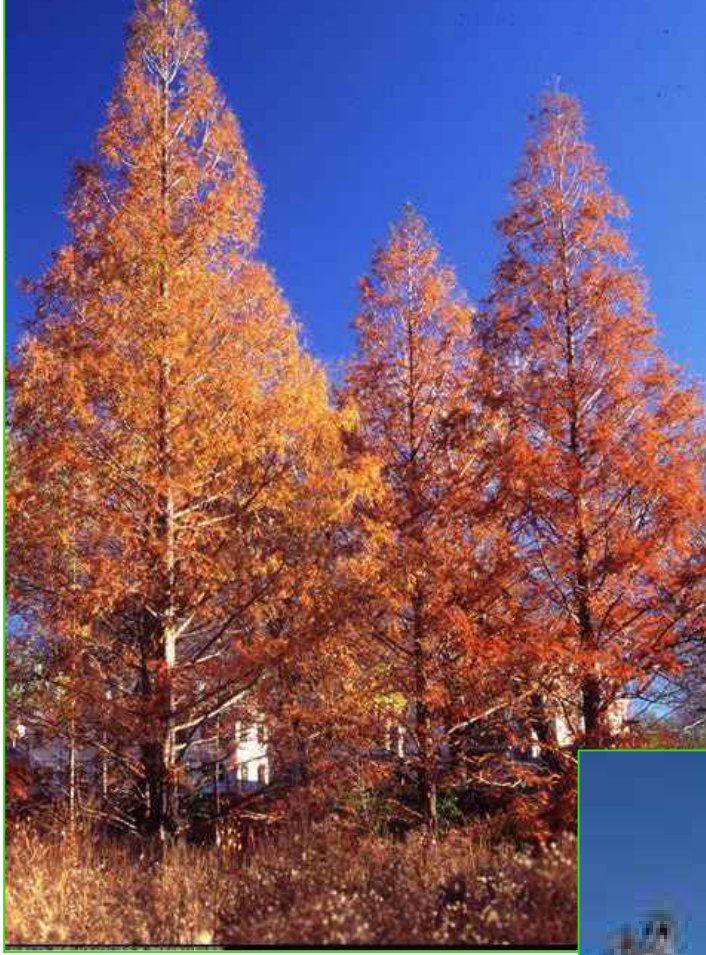




Acer rubrum 'October Glory' October Glory Red Maple



Thuja occidentalis 'American Pillar' American Pillar Arborvitea



Metasequoia glyptostroboides Dawn Redwood



Acer rubrum 'The Rocket' The Rocket Red Maple



Tilia cordata Little Leaf Linden



Acer saccharum 'Goldspire' Goldspire Sugar Maple



Nyssa sylvatica Black Gum



Prunus serrulata 'Kwanzan' Kwanzan Cherry



Thuja plicata 'Green Giant' Green Giant Arborvitae

Schizachyrium scoparium 'Standing Ovation' Standing Ovation Little Bluestem



Liriope muscari 'Variegata'



Pennisetum alopecuroides 'Red Head' Red Headed Dwarf Fountain Grass

Polystichum acrostichoides Christmas Fern



Hydrangea paniculata 'Tardiva'

Buddleia 'Low and Behold'



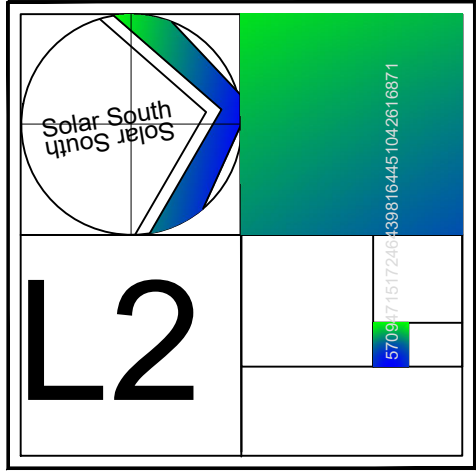
Hydrangea paniculata 'Little Quick Fire'



Hydrangea quercifolia 'Snow Queen'



Hypericum frondosum 'Sunburst' Sunburst St John's Wort



845-702-1004

11 South Fowlerhouse Road Wappinger Falls, New York 12590

The Michael Boice Collaboration

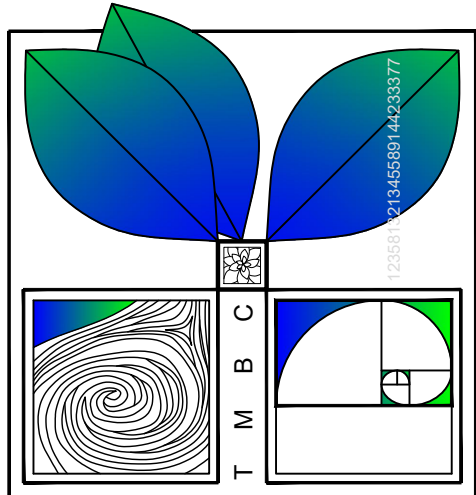
DESIGNERS AND MAKERS OF FINE LANDSCAPES

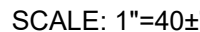
One Key South Hamilton Street
South Hamilton Street Poughkeepsie, New York

Scale | 1" = 25' - 0"

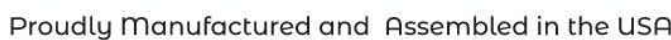
Date | September 27, 2023

Revisions |





Description	Symbol	Avg	Max	Min	Max/Min	Avg/Min
Calc Zone #2	+	1.1 fc	26.3 fc	0.0 fc	N/A	N/A



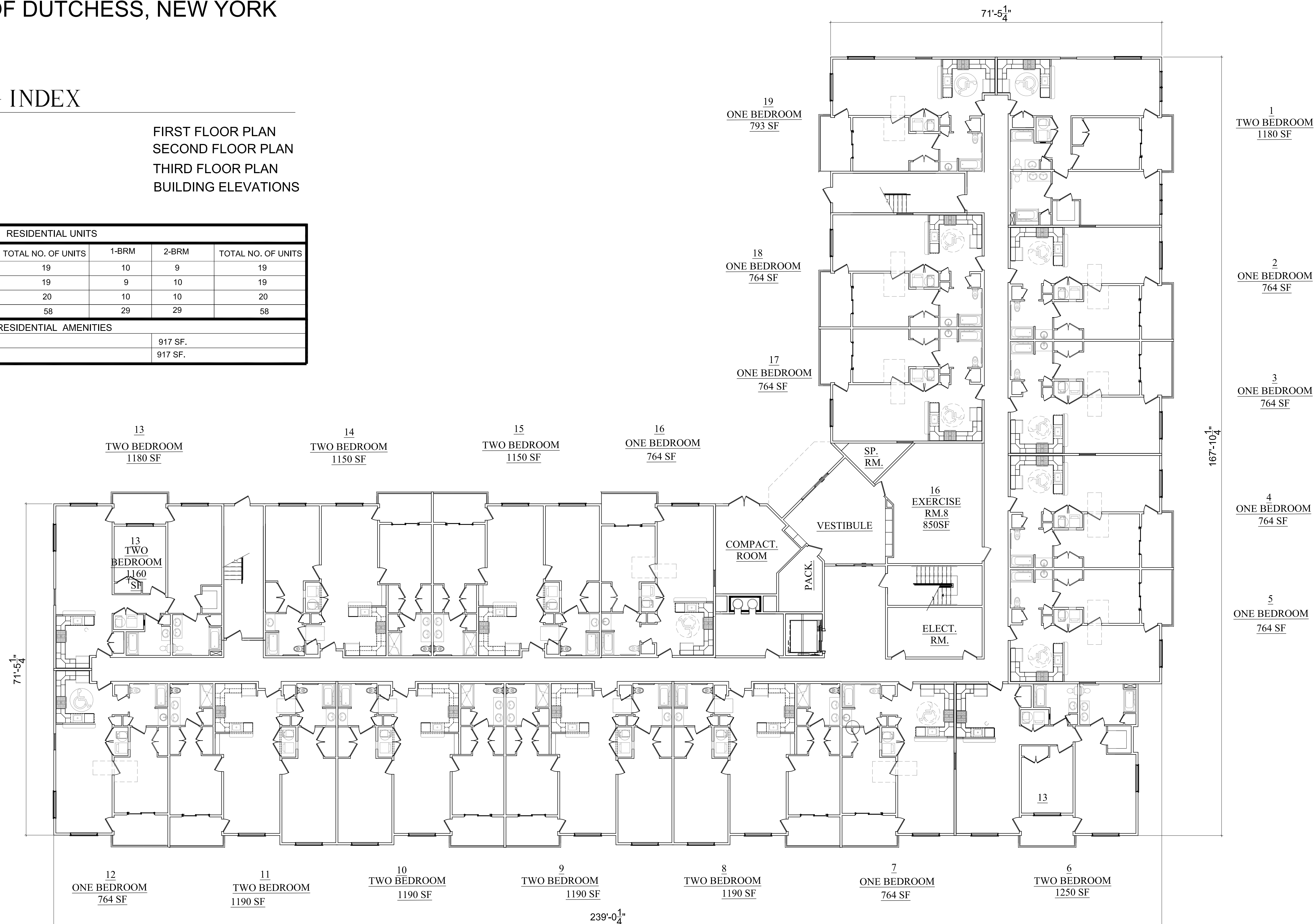
	CITY PLANNING BOARD CITY OF POUGHKEEPSIE, NEW YORK
Fire Chief	SITE PLAN APPROVAL
	FILE NO:
Police Chief	PROJECT NAME:
	PROJECT LOCATION:
Building Inspector	APPLICANT:
	OFFICIALLY APPROVED at the Planning Board meeting held the ____ day of _____ 2 ____
City Engineer	PLANNING CHAIR:
	DESIGNER:
Commissioner of Public Works	DATE SIGNED:
	CONDITION(S):

58- UNIT RESIDENTIAL BUILDING
100 SOUTH HAMILTON STREET
CITY OF POUGHKEEPSIE
COUNTY OF DUTCHESS, NEW YORK

DRAWING INDEX

SK-100	FIRST FLOOR PLAN
SK-101	SECOND FLOOR PLAN
SK-102	THIRD FLOOR PLAN
SK-103	BUILDING ELEVATIONS

RESIDENTIAL UNITS				
UNIT	TOTAL NO. OF UNITS	1-BRM	2-BRM	TOTAL NO. OF UNITS
FIRST FLOOR	19	10	9	19
SECOND FLOOR	19	9	10	19
THIRD FLOOR	20	10	10	20
TOTAL	58	29	29	58
RESIDENTIAL AMENITIES				
FITNESS CENTER (GYM)		917 SF.		
ACTIVITY RM		917 SF.		



FIRST FLOOR PLAN

SCALE: 3/32" = 1'-0"

ASSOCIATE ARCHITECTS



N.J. AC. 669

241 HUDSON STREET
HACKENSACK, NJ 07601
PHONE: (201) 925 5888
F A X : (201) 488-1020
MJAARCH@OPTONLINE.NET

ARCHITECT

NABIL N. MIJALLI, R.A.
NJ LIC. # A101665500
NY LIC. # 032031
PA LIC. # RA405317
CT LIC. # 13827
SC LIC. # 8469

PROJECT:

58- UNIT RESIDENTIAL BUILDING
100 SOUTH HAMILTON STREET
CITY OF POUGHKEEPSIE
COUNTY OF DUTCHESS

OWNER:

O'NEILL GROUP HAMILTON LLC
241 HUDSON STREET
HACKENSACK, NJ 07601

RELEASE STATUS OF DRAWING DATE:
FOR REVIEW 10-3-2013

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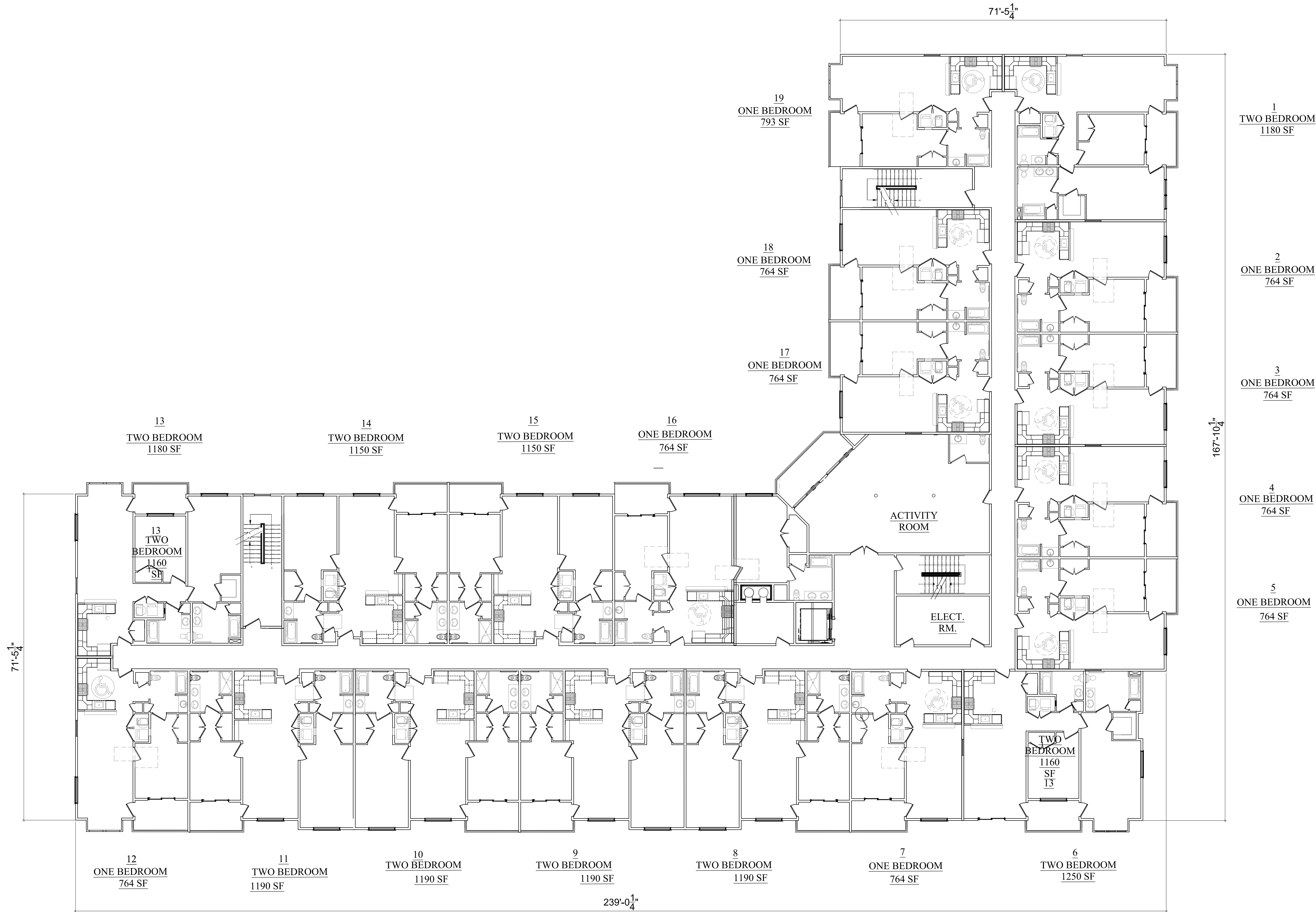
FIRST FLOOR

REVISIONS		
DATE	DWN/CHK	REMARKS
DO NOT SCALE DRAWINGS		

DATE:	8/30/2023
DRAWN :	RI
CHECKED:	NM
SCALE:	AS NOTED
PROJECT NUMBER:	0102023
DRAWING NO:	

SK-100

ZONING DOCUMENTS 10-03-2023



SECOND FLOOR PLAN

SCALE: 3/32" = 1'-0"

ASSOCIATE ARCHITECTS



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ARCHITECT

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NJ LIC. # A101665500
NY LIC. # 032031
PA LIC. # RA405317
CT LIC. # 13827
SC LIC. # 8469

PROJECT:
58- UNIT RESIDENTIAL BUILDING
100 SOUTH HAMILTON STREET
CITY OF POUGHKEEPSIE
COUNTY OF DUTCHESS

OWNER:
O'NEILL GROUP HAMILTON LLC
241 HUDSON STREET
HACKENSACK, NJ 07601

RELEASE STATUS OF DRAWING DATE:
FOR REVIEW 10-3-2013

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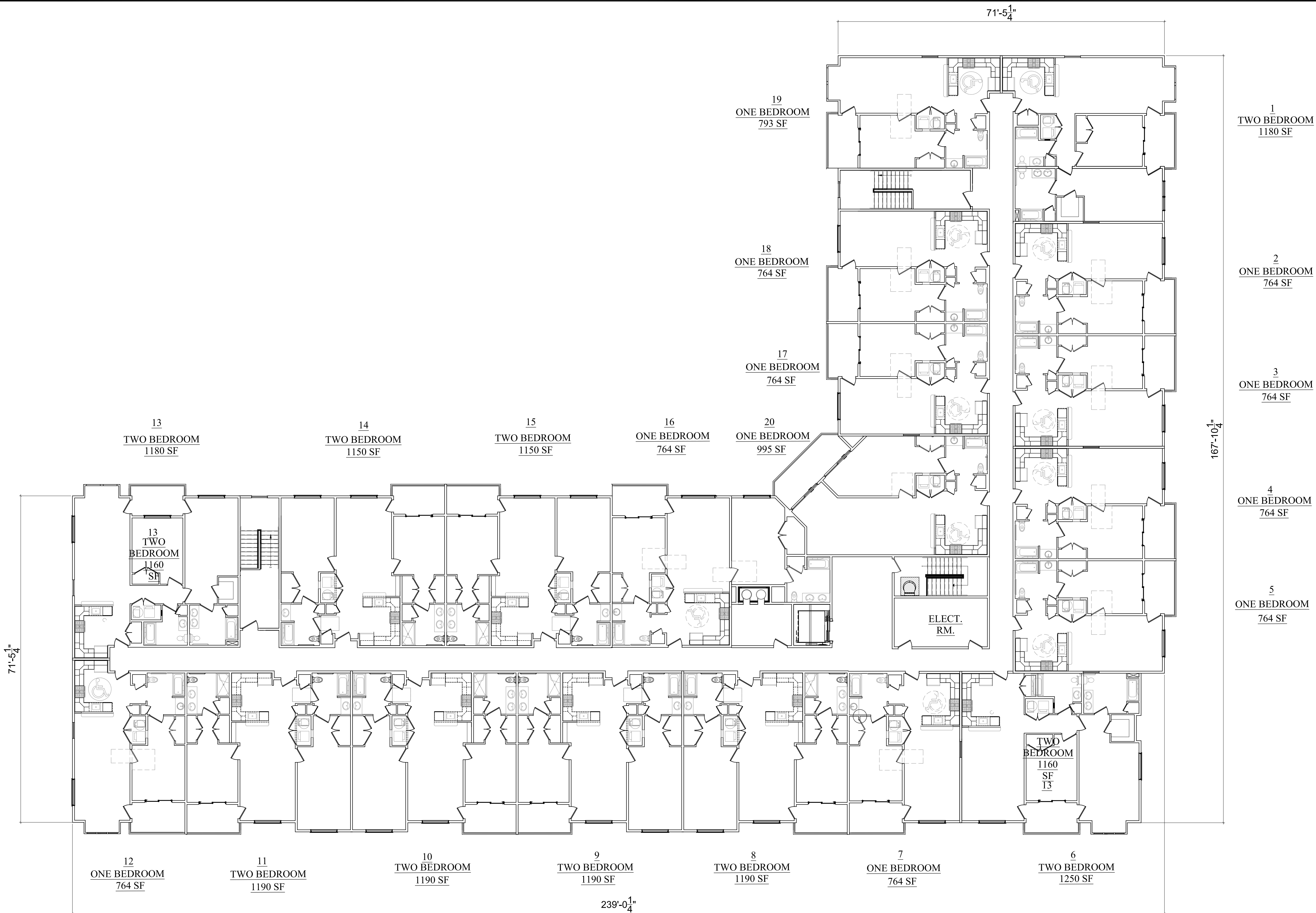
SECOND FLOOR

REVISIONS		
DATE	DWN CHK	REMARKS
DO NOT SCALE DRAWINGS		

DATE:	8/30/2023
DRAWN :	RI
CHECKED:	NM
SCALE:	AS NOTED
PROJECT NUMBER:	0102023
DRAWING NO:	

SK-101

ZONING DOCUMENTS 10-03-2023



THIRD FLOOR PLAN

SCALE: 3/32" = 1'-0"

ASSOCIATE ARCHITECTS



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PA LIC. # RA405317
CT LIC. # 13827
SC LIC. # 8469

PROJECT:
58- UNIT RESIDENTIAL BUILDING
100 SOUTH HAMILTON STREET
CITY OF POUGHKEEPSIE
COUNTY OF DUTCHESS

OWNER:
O'NEILL GROUP HAMILTON LLC
241 HUDSON STREET
HACKENSACK, NJ 07601

RELEASE STATUS OF DRAWING	DATE:
FOR REVIEW	10-3-2013

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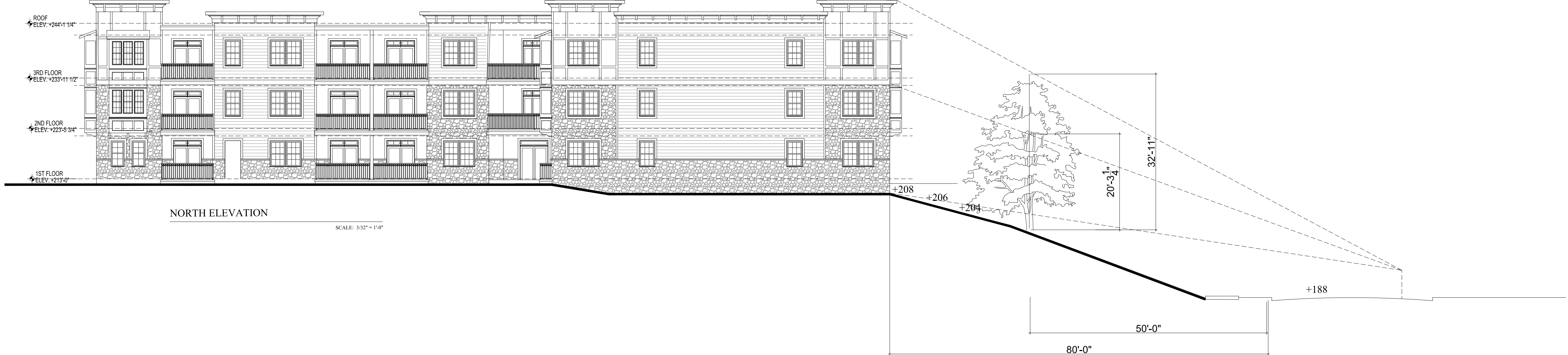
THIRD FLOOR

REVISIONS		
DATE	DWN/CHK	REMARKS
DO NOT SCALE DRAWINGS		

DATE:	8/30/2023
DRAWN :	RI
CHECKED:	NM
SCALE:	AS NOTED
PROJECT NUMBER:	0102023
DRAWING NO:	

SK-102

ZONING DOCUMENTS 10-03-2023



NORTH ELEVATION
SCALE: 3/32" = 1'-0"



EAST ELEVATION
SCALE: 3/32" = 1'-0"



WEST ELEVATION (S. HAMILTON STREET)
SCALE: 3/32" = 1'-0"



SOUTH ELEVATION
SCALE: 3/32" = 1'-0"

ASSOCIATE ARCHITECTS



N.J. AC. 669

241 HUDSON STREET
HACKENSACK, NJ 07601
PHONE: (201) 925 5888
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MIJAARCH@OPTONLINE.NET

ARCHITECT

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NJ LIC. # A101665500
NY LIC. # 032031
PA LIC. # RA405317
CT LIC. # 13827
SC LIC. # 8469

PROJECT:
58- UNIT RESIDENTIAL BUILDING
100 SOUTH HAMILTON STREET
CITY OF POUGHKEEPSIE
COUNTY OF DUTCHESS

OWNER:
O'NEILL GROUP HAMILTON LLC
241 HUDSON STREET
HACKENSACK, NJ 07601

RELEASE STATUS OF DRAWING DATE:
FOR REVIEW 10-3-2013

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DRAWING TITLE:

ELEVATIONS

REVISIONS			
DATE	DWN	CHK	REMARKS
DO NOT SCALE DRAWINGS			

DATE:	8/30/2023
DRAWN :	RI
CHECKED:	NM
SCALE:	AS NOTED
PROJECT NUMBER:	0102023
DRAWING NO:	

SK- 103

ZONING DOCUMENTS 10-03-2023

SOUTH HAMILTON MULTI-FAMILY DEVELOPMENT



241 HUDSON STREET
HACKENSACK, NJ 07601
P. (201) 525-3000
F. (201) 488-1020

MIJA ARCHITECTURE LLC

NABIL N. MIJALLI, ARCHITECT



SOUTH HAMILTON MULTI-FAMILY DEVELOPMENT



241 HUDSON STREET
HACKENSACK, NJ 07601
P. (201) 525-3000
F. (201) 488-1020

MIJA ARCHITECTURE LLC
NABIL N. MIJALLI, ARCHITECT



Full Environmental Assessment Form
Part 1 - Project and Setting

Instructions for Completing Part 1

Part 1 is to be completed by the applicant or project sponsor. Responses become part of the application for approval or funding, are subject to public review, and may be subject to further verification.

Complete Part 1 based on information currently available. If additional research or investigation would be needed to fully respond to any item, please answer as thoroughly as possible based on current information; indicate whether missing information does not exist, or is not reasonably available to the sponsor; and, when possible, generally describe work or studies which would be necessary to update or fully develop that information.

Applicants/sponsors must complete all items in Sections A & B. In Sections C, D & E, most items contain an initial question that must be answered either "Yes" or "No". If the answer to the initial question is "Yes", complete the sub-questions that follow. If the answer to the initial question is "No", proceed to the next question. Section F allows the project sponsor to identify and attach any additional information. Section G requires the name and signature of the applicant or project sponsor to verify that the information contained in Part 1 is accurate and complete.

A. Project and Applicant/Sponsor Information.

Name of Action or Project: SOUTH HAMILTON STREET		
Project Location (describe, and attach a general location map): SOUTH HAMILTON STREET OFF DWIGHT STREET - 41°41'42.82"W 73°55'28.39"		
Brief Description of Proposed Action (include purpose or need): THE PROJECT SITE IS LOCATED AT S. HAMILTON ST. AND CONSISTS OF 3.378 ACRES. THE TAX MAP NUMBER FOR THE PARCEL IS 13130-6161-30-151783. THE PROJECT CONSISTS OF 58 LUXURY RENTAL APARTMENTS CONSISTING OF 29 TWO-BEDROOM APARTMENTS AND 29 ONE-BEDROOM APARTMENTS SERVICED BY CENTRAL WATER AND SANITARY SEWER. IN WINTER OF 2021, THE SITE WAS PARTIALLY CLEARED. IN THE 1970'S THE SITE WAS USED AS RENTAL PROPERTY UNTIL IT WAS DESTROYED BY FIRE AND DEMOLISHED IN 1973. TODAY THE PROPERTY IS VACANT AND LARGELY COVERED WITH SMALL BRUSH AND DYING AND DECEASED TREES. THE FUTURE DEVELOPMENT WILL INCLUDE LANDSCAPING, LIGHTING, SURFACE PARKING AND SIDEWALKS TYPICALLY ASSOCIATED WITH A RESIDENTIAL DEVELOPMENT, AS WELL AS A STORMWATER DETENTION POND TO IMPLEMENT A STORM WATER POLLUTION PREVENTION PLAN.		
Name of Applicant/Sponsor: O'NEILL GROUP HAMILTON LLC		Telephone: 201-488-4455
		E-Mail: LONEILL@ONEKEYLLC.COM
Address: 241 HUDSON STREET		
City/PO: HACKENSACK	State: NEW JERSEY	Zip Code: 07601
Project Contact (if not same as sponsor; give name and title/role):		Telephone: 201-488-4455
		E-Mail: LONEILL@ONEKEYLLC.COM
Address:		
City/PO:	State:	Zip Code:
Property Owner (if not same as sponsor):		Telephone:
		E-Mail:
Address:		
City/PO:	State:	Zip Code:

B. Government Approvals

B. Government Approvals, Funding, or Sponsorship. ("Funding" includes grants, loans, tax relief, and any other forms of financial assistance.)		
Government Entity	If Yes: Identify Agency and Approval(s) Required	Application Date (Actual or projected)
a. City Counsel, Town Board, ☐ Yes ☒ No or Village Board of Trustees		
b. City, Town or Village Planning Board or Commission ☒ Yes ☐ No	SITE PLAN	
c. City, Town or Village Zoning Board of Appeals ☐ Yes ☒ No		
d. Other local agencies ☒ Yes ☐ No	POUGHKEEPSIE DEPT. OF PUBLIC WORKS	
e. County agencies ☒ Yes ☐ No	DUTCHESS DEPT OF HEALTH WATER & SEWER	
f. Regional agencies ☐ Yes ☒ No		
g. State agencies ☒ Yes ☐ No	NYSDEC - SPDES FROM CONSTRUCTION ACTIVITY	
h. Federal agencies ☐ Yes ☒ No		
i. Coastal Resources.		
i. Is the project site within a Coastal Area, or the waterfront area of a Designated Inland Waterway?		☐ Yes ☒ No
ii. Is the project site located in a community with an approved Local Waterfront Revitalization Program?		☐ Yes ☒ No
iii. Is the project site within a Coastal Erosion Hazard Area?		☐ Yes ☒ No

C. Planning and Zoning

C.1. Planning and zoning actions.	
Will administrative or legislative adoption, or amendment of a plan, local law, ordinance, rule or regulation be the only approval(s) which must be granted to enable the proposed action to proceed? ☐ Yes ☒ No	
- If Yes, complete sections C, F and G. - If No, proceed to question C.2 and complete all remaining sections and questions in Part 1	
C.2. Adopted land use plans.	
a. Do any municipally- adopted (city, town, village or county) comprehensive land use plan(s) include the site where the proposed action would be located? ☒ Yes ☐ No	
If Yes, does the comprehensive plan include specific recommendations for the site where the proposed action would be located? ☐ Yes ☒ No	
b. Is the site of the proposed action within any local or regional special planning district (for example: Greenway; Brownfield Opportunity Area (BOA); designated State or Federal heritage area; watershed management plan; or other?) ☐ Yes ☒ No	
If Yes, identify the plan(s): _____ _____ _____	
c. Is the proposed action located wholly or partially within an area listed in an adopted municipal open space plan, or an adopted municipal farmland protection plan? ☐ Yes ☒ No	
If Yes, identify the plan(s): _____ _____ _____	

C.3. Zoning	
a. Is the site of the proposed action located in a municipality with an adopted zoning law or ordinance. If Yes, what is the zoning classification(s) including any applicable overlay district? <u>MEDIUM HIGH-DENSITY RESIDENCE (R-4)</u>	☒ Yes ☐ No
b. Is the use permitted or allowed by a special or conditional use permit?	☒ Yes ☐ No
c. Is a zoning change requested as part of the proposed action? If Yes, i. What is the proposed new zoning for the site? _____	☐ Yes ☒ No
C.4. Existing community services.	
a. In what school district is the project site located? <u>CITY OF POUGHKEEPSIE SCHOOLS</u>	
b. What police or other public protection forces serve the project site? <u>CITY OF POUGHKEEPSIE POLICE DEPARTMENT</u>	
c. Which fire protection and emergency medical services serve the project site? <u>CITY OF POUGHKEEPSIE FIRE DEPARTMENT</u>	
d. What parks serve the project site? <u>NONE</u>	

D. Project Details

D.1. Proposed and Potential Development	
a. What is the general nature of the proposed action (e.g., residential, industrial, commercial, recreational; if mixed, include all components)? <u>RESIDENTIAL</u>	
b. a. Total acreage of the site of the proposed action?	<u>3.378</u> acres
b. Total acreage to be physically disturbed?	<u>3.0</u> acres
c. Total acreage (project site and any contiguous properties) owned or controlled by the applicant or project sponsor?	<u>3.378</u> acres
c. Is the proposed action an expansion of an existing project or use? ☐ Yes ☒ No	
i. If Yes, what is the approximate percentage of the proposed expansion and identify the units (e.g., acres, miles, housing units, square feet)? % _____ Units: _____	
d. Is the proposed action a subdivision, or does it include a subdivision? ☐ Yes ☒ No	
If Yes, i. Purpose or type of subdivision? (e.g., residential, industrial, commercial; if mixed, specify types) _____	
ii. Is a cluster/conservation layout proposed? ☐ Yes ☐ No	
iii. Number of lots proposed? _____	
iv. Minimum and maximum proposed lot sizes? Minimum _____ Maximum _____	
e. Will the proposed action be constructed in multiple phases? ☐ Yes ☒ No	
i. If No, anticipated period of construction: <u>18</u> months	
ii. If Yes:	
• Total number of phases anticipated	<u>1</u>
• Anticipated commencement date of phase 1 (including demolition)	<u>12</u> month <u>24</u> year
• Anticipated completion date of final phase	<u>6</u> month <u>26</u> year
• Generally describe connections or relationships among phases, including any contingencies where progress of one phase may determine timing or duration of future phases: _____	

f. Does the project include new residential uses? ☒ Yes ☐ No				
If Yes, show numbers of units proposed.				
	<u>One Family</u>	<u>Two Family</u>	<u>Three Family</u>	<u>Multiple Family (four or more)</u>
Initial Phase				58
At completion of all phases				58

g. Does the proposed action include new non-residential construction (including expansions)? ☐ Yes ☒ No	
If Yes,	
i. Total number of structures _____	
ii. Dimensions (in feet) of largest proposed structure: _____ height; _____ width; and _____ length	
iii. Approximate extent of building space to be heated or cooled: _____ square feet	

h. Does the proposed action include construction or other activities that will result in the impoundment of any liquids, such as creation of a water supply, reservoir, pond, lake, waste lagoon or other storage? ☐ Yes ☒ No	
If Yes,	
i. Purpose of the impoundment: _____	
ii. If a water impoundment, the principal source of the water: ☐ Ground water ☐ Surface water streams ☐ Other specify: _____	
iii. If other than water, identify the type of impounded/contained liquids and their source. _____	
iv. Approximate size of the proposed impoundment. Volume: _____ million gallons; surface area: _____ acres	
v. Dimensions of the proposed dam or impounding structure: _____ height; _____ length	
vi. Construction method/materials for the proposed dam or impounding structure (e.g., earth fill, rock, wood, concrete): _____	

D.2. Project Operations

a. Does the proposed action include any excavation, mining, or dredging, during construction, operations, or both? (Not including general site preparation, grading or installation of utilities or foundations where all excavated materials will remain onsite) ☒ Yes ☐ No	
If Yes:	
i. What is the purpose of the excavation or dredging? <u>CLEARING AND GRADING</u>	
ii. How much material (including rock, earth, sediments, etc.) is proposed to be removed from the site?	
• Volume (specify tons or cubic yards): _____ • Over what duration of time? _____	
iii. Describe nature and characteristics of materials to be excavated or dredged, and plans to use, manage or dispose of them. <u>ON SITE SOIL WILL BE EXCAVATED AND DISPOSED OF OFFSITE. TOP SOIL WILL BE STOCKPILED AND STORE FOR REUSE.</u>	
iv. Will there be onsite dewatering or processing of excavated materials? ☐ Yes ☒ No	
If yes, describe. _____	
v. What is the total area to be dredged or excavated? _____ 3 acres	
vi. What is the maximum area to be worked at any one time? _____ 3,378 acres	
vii. What would be the maximum depth of excavation or dredging? _____ feet	
viii. Will the excavation require blasting? ☐ Yes ☒ No	
ix. Summarize site reclamation goals and plan: _____ <u>ON SITE SOIL AND ROCK WILL BE EXCAVATED AND DISPOSED OF OFFSITE. TOP SOIL WILL BE STOCKPILED AND STORE FOR REUSE.</u>	

b. Would the proposed action cause or result in alteration of, increase or decrease in size of, or encroachment into any existing wetland, waterbody, shoreline, beach or adjacent area? ☐ Yes ☒ No	
If Yes:	
i. Identify the wetland or waterbody which would be affected (by name, water index number, wetland map number or geographic description): _____	

ii. Describe how the proposed action would affect that waterbody or wetland, e.g. excavation, fill, placement of structures, or alteration of channels, banks and shorelines. Indicate extent of activities, alterations and additions in square feet or acres:

iii. Will the proposed action cause or result in disturbance to bottom sediments? ☐ Yes ☐ No
If Yes, describe: _____

iv. Will the proposed action cause or result in the destruction or removal of aquatic vegetation? ☐ Yes ☐ No
If Yes:

- acres of aquatic vegetation proposed to be removed: _____
- expected acreage of aquatic vegetation remaining after project completion: _____
- purpose of proposed removal (e.g. beach clearing, invasive species control, boat access): _____
- proposed method of plant removal: _____
- if chemical/herbicide treatment will be used, specify product(s): _____

v. Describe any proposed reclamation/mitigation following disturbance: _____

c. Will the proposed action use, or create a new demand for water? ☒ Yes ☐ No
If Yes:

i. Total anticipated water usage/demand per day: 9,570 gallons/day

ii. Will the proposed action obtain water from an existing public water supply? ☒ Yes ☐ No
If Yes:

- Name of district or service area: CITY OF POUGHKEEPSIE WATER DISTRICT
- Does the existing public water supply have capacity to serve the proposal? ☒ Yes ☐ No
- Is the project site in the existing district? ☒ Yes ☐ No
- Is expansion of the district needed? ☐ Yes ☒ No
- Do existing lines serve the project site? ☒ Yes ☐ No

iii. Will line extension within an existing district be necessary to supply the project? ☐ Yes ☒ No
If Yes:

- Describe extensions or capacity expansions proposed to serve this project: _____
- Source(s) of supply for the district: _____

iv. Is a new water supply district or service area proposed to be formed to serve the project site? ☐ Yes ☒ No
If Yes:

- Applicant/sponsor for new district: _____
- Date application submitted or anticipated: _____
- Proposed source(s) of supply for new district: _____

v. If a public water supply will not be used, describe plans to provide water supply for the project: _____

vi. If water supply will be from wells (public or private), what is the maximum pumping capacity: _____ gallons/minute.

d. Will the proposed action generate liquid wastes? ☒ Yes ☐ No
If Yes:

i. Total anticipated liquid waste generation per day: 9,570 gallons/day

ii. Nature of liquid wastes to be generated (e.g., sanitary wastewater, industrial; if combination, describe all components and approximate volumes or proportions of each):
SANITARY WASTEWATER

iii. Will the proposed action use any existing public wastewater treatment facilities? ☒ Yes ☐ No
If Yes:

- Name of wastewater treatment plant to be used: CITY OF POUGHKEEPSIE WASTEWATER PLANT
- Name of district: CITY OF POUGHKEEPSIE SEWER DISTRICT
- Does the existing wastewater treatment plant have capacity to serve the project? ☒ Yes ☐ No
- Is the project site in the existing district? ☒ Yes ☐ No
- Is expansion of the district needed? ☐ Yes ☒ No

• Do existing sewer lines serve the project site? ☒ Yes ☐ No • Will a line extension within an existing district be necessary to serve the project? ☐ Yes ☒ No If Yes: • Describe extensions or capacity expansions proposed to serve this project: _____	
iv. Will a new wastewater (sewage) treatment district be formed to serve the project site? ☐ Yes ☒ No If Yes: • Applicant/sponsor for new district: _____ • Date application submitted or anticipated: _____ • What is the receiving water for the wastewater discharge? _____	
v. If public facilities will not be used, describe plans to provide wastewater treatment for the project, including specifying proposed receiving water (name and classification if surface discharge or describe subsurface disposal plans): _____	
vi. Describe any plans or designs to capture, recycle or reuse liquid waste: _____ _____	
e. Will the proposed action disturb more than one acre and create stormwater runoff, either from new point sources (i.e. ditches, pipes, swales, curbs, gutters or other concentrated flows of stormwater) or non-point source (i.e. sheet flow) during construction or post construction? ☒ Yes ☐ No If Yes: i. How much impervious surface will the project create in relation to total size of project parcel? _____ Square feet or <u>1.81</u> acres (impervious surface) _____ Square feet or <u>3.378</u> acres (parcel size) ii. Describe types of new point sources. <u>City of Poughkeepsie storm system</u>	
iii. Where will the stormwater runoff be directed (i.e. on-site stormwater management facility/structures, adjacent properties, groundwater, on-site surface water or off-site surface waters)? <u>Off site storm collection system</u> _____ • If to surface waters, identify receiving water bodies or wetlands: _____ _____ • Will stormwater runoff flow to adjacent properties? ☐ Yes ☒ No	
iv. Does the proposed plan minimize impervious surfaces, use pervious materials or collect and re-use stormwater? ☐ Yes ☒ No	
f. Does the proposed action include, or will it use on-site, one or more sources of air emissions, including fuel combustion, waste incineration, or other processes or operations? ☒ Yes ☐ No If Yes, identify: i. Mobile sources during project operations (e.g., heavy equipment, fleet or delivery vehicles) <u>CONSTRUCTION EQUIPMENT SUCH AS EXCAVATORS, GRADERS AND PAVERS</u> ii. Stationary sources during construction (e.g., power generation, structural heating, batch plant, crushers) <u>TEMPORARY POWER FROM GENERATORS</u> iii. Stationary sources during operations (e.g., process emissions, large boilers, electric generation) <u>HEATING AND COOLING EQUIPMENT</u>	
g. Will any air emission sources named in D.2.f (above), require a NY State Air Registration, Air Facility Permit, or Federal Clean Air Act Title IV or Title V Permit? ☐ Yes ☒ No If Yes: i. Is the project site located in an Air quality non-attainment area? (Area routinely or periodically fails to meet ambient air quality standards for all or some parts of the year) ☐ Yes ☐ No ii. In addition to emissions as calculated in the application, the project will generate: • _____ Tons/year (short tons) of Carbon Dioxide (CO₂) • _____ Tons/year (short tons) of Nitrous Oxide (N₂O) • _____ Tons/year (short tons) of Perfluorocarbons (PFCs) • _____ Tons/year (short tons) of Sulfur Hexafluoride (SF₆) • _____ Tons/year (short tons) of Carbon Dioxide equivalent of Hydrofluorocarbons (HFCs) • _____ Tons/year (short tons) of Hazardous Air Pollutants (HAPs)	

h. Will the proposed action generate or emit methane (including, but not limited to, sewage treatment plants, landfills, composting facilities)? ☐ Yes ☒ No

If Yes:

i. Estimate methane generation in tons/year (metric): _____

ii. Describe any methane capture, control or elimination measures included in project design (e.g., combustion to generate heat or electricity, flaring): _____

i. Will the proposed action result in the release of air pollutants from open-air operations or processes, such as quarry or landfill operations? ☐ Yes ☒ No

If Yes: Describe operations and nature of emissions (e.g., diesel exhaust, rock particulates/dust): _____

j. Will the proposed action result in a substantial increase in traffic above present levels or generate substantial new demand for transportation facilities or services? ☐ Yes ☒ No

If Yes:

i. When is the peak traffic expected (Check all that apply): ☐ Morning ☐ Evening ☐ Weekend
☐ Randomly between hours of _____ to _____.

ii. For commercial activities only, projected number of truck trips/day and type (e.g., semi trailers and dump trucks): _____

iii. Parking spaces: Existing _____ Proposed _____ Net increase/decrease _____

iv. Does the proposed action include any shared use parking? ☐ Yes ☐ No

v. If the proposed action includes any modification of existing roads, creation of new roads or change in existing access, describe: _____

vi. Are public/private transportation service(s) or facilities available within ½ mile of the proposed site? ☐ Yes ☐ No

vii. Will the proposed action include access to public transportation or accommodations for use of hybrid, electric or other alternative fueled vehicles? ☐ Yes ☐ No

viii. Will the proposed action include plans for pedestrian or bicycle accommodations for connections to existing pedestrian or bicycle routes? ☐ Yes ☐ No

k. Will the proposed action (for commercial or industrial projects only) generate new or additional demand for energy? ☒ Yes ☐ No

If Yes:

i. Estimate annual electricity demand during operation of the proposed action: _____
TBD

ii. Anticipated sources/suppliers of electricity for the project (e.g., on-site combustion, on-site renewable, via grid/local utility, or other):
Local utility _____

iii. Will the proposed action require a new, or an upgrade, to an existing substation? ☐ Yes ☒ No

l. Hours of operation. Answer all items which apply.

i. During Construction:		ii. During Operations:	
• Monday - Friday:	7-5	• Monday - Friday:	9-5
• Saturday:	8-3	• Saturday:	TBD
• Sunday:	CLOSED	• Sunday:	CLOSED
• Holidays:	CLOSED	• Holidays:	CLOSED

m. Will the proposed action produce noise that will exceed existing ambient noise levels during construction, operation, or both? ☐ Yes ☒ No

If yes:

i. Provide details including sources, time of day and duration:
NO UNREASONABLE NOISE WILL BE GENERATED AND THE PROJECT WILL COMPLY WITH THE STANDARDS ESTABLISHED UNDER CITY CODE SECTION 13 1/2.

ii. Will the proposed action remove existing natural barriers that could act as a noise barrier or screen? ☒ Yes ☐ No
Describe: AREAS OF VEGETATIVE COVER WILL BE REDUCED WHICH MAY LESSEN THE NOISE BARRIER. SEE LANDSCAPE PLAN FOR REVEGETATIVE PLANTING AND BUFFER DETAILS.

n. Will the proposed action have outdoor lighting? ☒ Yes ☐ No

If yes:

i. Describe source(s), location(s), height of fixture(s), direction/aim, and proximity to nearest occupied structures:
LIGHTING THAT COMPLIES WITH THE CITY CODE FOR SAFETY AND CONVENIENCE WILL BE KEPT FROM SPILLING ONTO NEIGHBORING PROPERTIES

ii. Will proposed action remove existing natural barriers that could act as a light barrier or screen? ☒ Yes ☐ No
Describe: PARTICULAR CARE WILL BE TAKEN TO ENSURE ADEQUATE SCREENING AND LIGHT BARRIER CONSISTENT WITH THE CITY CODE.

o. Does the proposed action have the potential to produce odors for more than one hour per day? ☐ Yes ☒ No
If Yes, describe possible sources, potential frequency and duration of odor emissions, and proximity to nearest occupied structures: _____

p. Will the proposed action include any bulk storage of petroleum (combined capacity of over 1,100 gallons) or chemical products 185 gallons in above ground storage or any amount in underground storage? ☐ Yes ☒ No

If Yes:

i. Product(s) to be stored _____

ii. Volume(s) _____ per unit time _____ (e.g., month, year)

iii. Generally, describe the proposed storage facilities: _____

q. Will the proposed action (commercial, industrial and recreational projects only) use pesticides (i.e., herbicides, insecticides) during construction or operation? ☐ Yes ☒ No

If Yes:

i. Describe proposed treatment(s): _____

ii. Will the proposed action use Integrated Pest Management Practices? ☐ Yes ☒ No

r. Will the proposed action (commercial or industrial projects only) involve or require the management or disposal of solid waste (excluding hazardous materials)? ☒ Yes ☐ No

If Yes:

i. Describe any solid waste(s) to be generated during construction or operation of the facility:

- Construction: 2.5 tons per MONTHLY (unit of time)
- Operation: 8.7 tons per MONTHLY (unit of time)

ii. Describe any proposals for on-site minimization, recycling or reuse of materials to avoid disposal as solid waste:

- Construction: UNUSED CONSTRUCTION MATERIAL, PARTICULARLY PIPES AND FITTINGS, ARE STORED FOR FUTURE REUSE OR SPARE PARTS.
- Operation: MATERIALS ARE NEVER THROWN AWAY BUT STORED OR STOCKPILED FOR FUTURE REUSE SUCH AS SPARE PARTS OR RETURNED TO SUPPLIERS WHEN POSSIBLE. COMPLIANCE WITH CITY/COUNTY REGS.

iii. Proposed disposal methods/facilities for solid waste generated on-site:

- Construction: NYSDEC LICENSED DISPOSAL CONTAINER CONTRACTORS AND FACILITIES ARE USED.
- Operation: COUNTY RESOURCE RECOVERY FACILITY.

s. Does the proposed action include construction or modification of a solid waste management facility? ☐ Yes ☒ No

If Yes:

i. Type of management or handling of waste proposed for the site (e.g., recycling or transfer station, composting, landfill, or other disposal activities): _____

ii. Anticipated rate of disposal/processing:

- _____ Tons/month, if transfer or other non-combustion/thermal treatment, or
- _____ Tons/hour, if combustion or thermal treatment

iii. If landfill, anticipated site life: _____ years

t. Will the proposed action at the site involve the commercial generation, treatment, storage, or disposal of hazardous waste? ☐ Yes ☒ No

If Yes:

i. Name(s) of all hazardous wastes or constituents to be generated, handled or managed at facility: _____

ii. Generally describe processes or activities involving hazardous wastes or constituents: _____

iii. Specify amount to be handled or generated _____ tons/month

iv. Describe any proposals for on-site minimization, recycling or reuse of hazardous constituents: _____

v. Will any hazardous wastes be disposed at an existing offsite hazardous waste facility? ☐ Yes ☐ No

If Yes: provide name and location of facility: _____

If No: describe proposed management of any hazardous wastes which will not be sent to a hazardous waste facility: _____

E. Site and Setting of Proposed Action

E.1. Land uses on and surrounding the project site

a. Existing land uses.

i. Check all uses that occur on, adjoining and near the project site.

☐ Urban ☐ Industrial ☒ Commercial ☒ Residential (suburban) ☐ Rural (non-farm)

☐ Forest ☐ Agriculture ☐ Aquatic ☐ Other (specify): _____

ii. If mix of uses, generally describe: _____

b. Land uses and covertypes on the project site.

Land use or Covertype	Current Acreage	Acreage After Project Completion	Change (Acres +/-)
• Roads, buildings, and other paved or impervious surfaces	.05	1.81	+1.76
• Forested	0	0	0
• Meadows, grasslands or brushlands (non-agricultural, including abandoned agricultural)	0	0	0
• Agricultural (includes active orchards, field, greenhouse etc.)	0	0	0
• Surface water features (lakes, ponds, streams, rivers, etc.)	0	0	0
• Wetlands (freshwater or tidal)	0	0	0
• Non-vegetated (bare rock, earth or fill)	0	0	0
• Other Describe: <u>Open/Grassed/Brush</u>	3.328	1.568	-1.76

c. Is the project site presently used by members of the community for public recreation? ☐ Yes ☒ No
i. If Yes: explain: _____

d. Are there any facilities serving children, the elderly, people with disabilities (e.g., schools, hospitals, licensed day care centers, or group homes) within 1500 feet of the project site? ☒ Yes ☐ No
If Yes,
i. Identify Facilities:
REFORMED CHURCH OF POUGHKEEPSIE, THE PINES AT POUGHKEEPSIE NURSING AND REHABILITATION CENTER, TABERNACLE BAPTIST CHURCH, AND NEW HORIZONS RESOURCES INC.

e. Does the project site contain an existing dam? ☐ Yes ☒ No
If Yes:
i. Dimensions of the dam and impoundment:
• Dam height: _____ feet
• Dam length: _____ feet
• Surface area: _____ acres
• Volume impounded: _____ gallons OR acre-feet
ii. Dam's existing hazard classification: _____
iii. Provide date and summarize results of last inspection: _____

f. Has the project site ever been used as a municipal, commercial or industrial solid waste management facility, or does the project site adjoin property which is now, or was at one time, used as a solid waste management facility? ☐ Yes ☒ No
If Yes:
i. Has the facility been formally closed? ☐ Yes ☐ No
• If yes, cite sources/documentation: _____
ii. Describe the location of the project site relative to the boundaries of the solid waste management facility: _____

iii. Describe any development constraints due to the prior solid waste activities: _____

g. Have hazardous wastes been generated, treated and/or disposed of at the site, or does the project site adjoin property which is now or was at one time used to commercially treat, store and/or dispose of hazardous waste? ☐ Yes ☒ No
If Yes:
i. Describe waste(s) handled and waste management activities, including approximate time when activities occurred: _____

h. Potential contamination history. Has there been a reported spill at the proposed project site, or have any remedial actions been conducted at or adjacent to the proposed site? ☐ Yes ☒ No
If Yes:
i. Is any portion of the site listed on the NYSDEC Spills Incidents database or Environmental Site Remediation database? Check all that apply: ☐ Yes ☒ No
☐ Yes – Spills Incidents database Provide DEC ID number(s): _____
☐ Yes – Environmental Site Remediation database Provide DEC ID number(s): _____
☐ Neither database
ii. If site has been subject of RCRA corrective activities, describe control measures: _____

iii. Is the project within 2000 feet of any site in the NYSDEC Environmental Site Remediation database? ☐ Yes ☒ No
If yes, provide DEC ID number(s): _____
iv. If yes to (i), (ii) or (iii) above, describe current status of site(s): _____

v. Is the project site subject to an institutional control limiting property uses? ☐ Yes ☒ No	
• If yes, DEC site ID number: _____ • Describe the type of institutional control (e.g., deed restriction or easement): _____ • Describe any use limitations: _____ • Describe any engineering controls: _____ • Will the project affect the institutional or engineering controls in place? ☐ Yes ☒ No • Explain: _____	
E.2. Natural Resources On or Near Project Site	
a. What is the average depth to bedrock on the project site?	3-5 feet
b. Are there bedrock outcroppings on the project site?	☒ Yes ☐ No
If Yes, what proportion of the site is comprised of bedrock outcroppings? 5 %	
c. Predominant soil type(s) present on project site:	<u>DUTCHESS-CARDIGAN-URBAN</u> 100 % _____ % _____ %
d. What is the average depth to the water table on the project site? Average:	> 8 feet
e. Drainage status of project site soils: ☐ Well Drained: _____ % of site	
☒ Moderately Well Drained: 100 % of site	
☐ Poorly Drained: _____ % of site	
f. Approximate proportion of proposed action site with slopes: ☒ 0-10%: 60 % of site	
☒ 10-15%: 35 % of site	
☒ 15% or greater: 5 % of site	
g. Are there any unique geologic features on the project site?	☐ Yes ☒ No
If Yes, describe: _____	
h. Surface water features.	
i. Does any portion of the project site contain wetlands or other waterbodies (including streams, rivers, ponds or lakes)?	☐ Yes ☒ No
ii. Do any wetlands or other waterbodies adjoin the project site?	☐ Yes ☒ No
If Yes to either i or ii, continue. If No, skip to E.2.i.	
iii. Are any of the wetlands or waterbodies within or adjoining the project site regulated by any federal, state or local agency?	☐ Yes ☒ No
iv. For each identified regulated wetland and waterbody on the project site, provide the following information:	
• Streams: Name _____ Classification _____ • Lakes or Ponds: Name _____ Classification _____ • Wetlands: Name _____ Approximate Size _____ • Wetland No. (if regulated by DEC) _____	
v. Are any of the above water bodies listed in the most recent compilation of NYS water quality-impaired waterbodies?	☐ Yes ☒ No
If yes, name of impaired water body/bodies and basis for listing as impaired: _____	
i. Is the project site in a designated Floodway?	☐ Yes ☒ No
j. Is the project site in the 100-year Floodplain?	☐ Yes ☒ No
k. Is the project site in the 500-year Floodplain?	☐ Yes ☒ No
l. Is the project site located over, or immediately adjoining, a primary, principal or sole source aquifer?	☐ Yes ☒ No
If Yes:	
i. Name of aquifer: _____	

m. Identify the predominant wildlife species that occupy or use the project site: _____ _____	
n. Does the project site contain a designated significant natural community? ☐ Yes ☒ No If Yes: i. Describe the habitat/community (composition, function, and basis for designation): _____ ii. Source(s) of description or evaluation: _____ iii. Extent of community/habitat: _____ • Currently: _____ acres • Following completion of project as proposed: _____ acres • Gain or loss (indicate + or -): _____ acres	
o. Does project site contain any species of plant or animal that is listed by the federal government or NYS as endangered or threatened, or does it contain any areas identified as habitat for an endangered or threatened species? ☒ Yes ☐ No If Yes: i. Species and listing (endangered or threatened): _____ Bald Eagle _____ _____	
p. Does the project site contain any species of plant or animal that is listed by NYS as rare, or as a species of special concern? ☐ Yes ☒ No If Yes: i. Species and listing: _____ _____	
q. Is the project site or adjoining area currently used for hunting, trapping, fishing or shell fishing? ☐ Yes ☒ No If yes, give a brief description of how the proposed action may affect that use: _____ _____	
E.3. Designated Public Resources On or Near Project Site	
a. Is the project site, or any portion of it, located in a designated agricultural district certified pursuant to Agriculture and Markets Law, Article 25-AA, Section 303 and 304? ☐ Yes ☒ No If Yes, provide county plus district name/number: _____	
b. Are agricultural lands consisting of highly productive soils present? ☐ Yes ☒ No i. If Yes: acreage(s) on project site? _____ ii. Source(s) of soil rating(s): _____	
c. Does the project site contain all or part of, or is it substantially contiguous to, a registered National Natural Landmark? ☐ Yes ☒ No If Yes: i. Nature of the natural landmark: ☐ Biological Community ☐ Geological Feature ii. Provide brief description of landmark, including values behind designation and approximate size/extent: _____ _____	
d. Is the project site located in or does it adjoin a state listed Critical Environmental Area? ☐ Yes ☒ No If Yes: i. CEA name: _____ ii. Basis for designation: _____ iii. Designating agency and date: _____	

e. Does the project site contain, or is it substantially contiguous to, a building, archaeological site, or district which is listed on the National or State Register of Historic Places, or that has been determined by the Commissioner of the NYS Office of Parks, Recreation and Historic Preservation to be eligible for listing on the State Register of Historic Places? ☒ Yes ☐ No	
If Yes: i. Nature of historic/archaeological resource: ☐ Archaeological Site ☒ Historic Building or District ii. Name: Reynolds House, South Hamilton Street Row, Dwight-Hooker Avenue Historic District, Carlies-Ritter-Hart House, Refor... iii. Brief description of attributes on which listing is based: HISTORIC DISTRICTS AND LOCATIONS ON EAST AND WEST SIDES OF PROJECT SITE AS WELL AS THE WEST SIDE OF PROJECT SITE.	
f. Is the project site, or any portion of it, located in or adjacent to an area designated as sensitive for archaeological sites on the NY State Historic Preservation Office (SHPO) archaeological site inventory? ☒ Yes ☐ No	
g. Have additional archaeological or historic site(s) or resources been identified on the project site? ☐ Yes ☒ No	
If Yes: i. Describe possible resource(s): _____ ii. Basis for identification: _____	
h. Is the project site within five miles of any officially designated and publicly accessible federal, state, or local scenic or aesthetic resource? ☒ Yes ☐ No	
If Yes: i. Identify resource: WALKWAY OVER THE HUDSON ii. Nature of, or basis for, designation (e.g., established highway overlook, state or local park, state historic trail or scenic byway, etc.): NEW YORK STATE PARK iii. Distance between project and resource: _____ 1.3 miles.	
i. Is the project site located within a designated river corridor under the Wild, Scenic and Recreational Rivers Program 6 NYCRR 666? ☐ Yes ☒ No	
If Yes: i. Identify the name of the river and its designation: _____ ii. Is the activity consistent with development restrictions contained in 6NYCRR Part 666? ☐ Yes ☐ No	

F. Additional Information

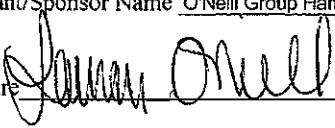
Attach any additional information which may be needed to clarify your project.

If you have identified any adverse impacts which could be associated with your proposal, please describe those impacts plus any measures which you propose to avoid or minimize them.

G. Verification

I certify that the information provided is true to the best of my knowledge.

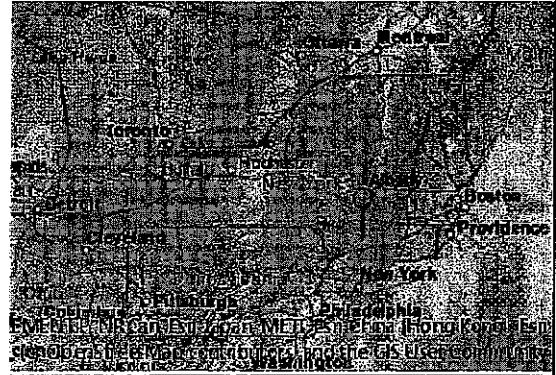
Applicant/Sponsor Name O'Neill Group Hamilton LLC Date 4/24/2023

Signature  Title Director of Business Development

PRINT FORM



Disclaimer: The EAF Mapper is a screening tool intended to assist project sponsors and reviewing agencies in preparing an environmental assessment form (EAF). Not all questions asked in the EAF are answered by the EAF Mapper. Additional information on any EAF question can be obtained by consulting the EAF Workbooks. Although the EAF Mapper provides the most up-to-date digital data available to DEC, you may also need to contact local or other data sources in order to obtain data not provided by the Mapper. Digital data is not a substitute for agency determinations.



B.i.i [Coastal or Waterfront Area]	No
B.i.ii [Local Waterfront Revitalization Area]	No
C.2.b. [Special Planning District]	Digital mapping data are not available or are incomplete. Refer to EAF Workbook.
E.1.h [DEC Spills or Remediation Site - Potential Contamination History]	Digital mapping data are not available or are incomplete. Refer to EAF Workbook.
E.1.h.i [DEC Spills or Remediation Site - Listed]	Digital mapping data are not available or are incomplete. Refer to EAF Workbook.
E.1.h.i [DEC Spills or Remediation Site - Environmental Site Remediation Database]	Digital mapping data are not available or are incomplete. Refer to EAF Workbook.
E.1.h.iii [Within 2,000' of DEC Remediation Site]	No
E.2.g [Unique Geologic Features]	No
E.2.h.i [Surface Water Features]	No
E.2.h.ii [Surface Water Features]	No
E.2.h.iii [Surface Water Features]	No
E.2.h.v [Impaired Water Bodies]	No
E.2.i. [Floodway]	No
E.2.j. [100 Year Floodplain]	No
E.2.k. [500 Year Floodplain]	No
E.2.l. [Aquifers]	No
E.2.n. [Natural Communities]	No
E.2.o. [Endangered or Threatened Species]	Yes
E.2.o. [Endangered or Threatened Species - Name]	Bald Eagle

E.2.g. [Rare Plants or Animals]	No
E.3.a. [Agricultural District]	No
E.3.c. [National Natural Landmark]	No
E.3.d [Critical Environmental Area]	No
E.3.e. [National or State Register of Historic Places or State Eligible Sites]	Yes - Digital mapping data for archaeological site boundaries are not available. Refer to EAF Workbook.
E.3.e.ii [National or State Register of Historic Places or State Eligible Sites - Name]	, Reynolds House, South Hamilton Street Row, Dwight-Hooker Avenue Historic District, Corlies-Ritter-Hart House, Reformed Dutch Church of Poughkeepsie
E.3.f. [Archeological Sites]	Yes
E.3.i. [Designated River Corridor]	No



**THE CITY OF POUGHKEEPSIE
NEW YORK**

DEVELOPMENT DEPARTMENT
62 CIVIC CENTER PLAZA, 2ND FLOOR
POUGHKEEPSIE, NY 12601
Phone: (845) 451-4263 Fax: (845) 451-4006

MEMORANDUM

To: O'Neill Group Hamilton LLC

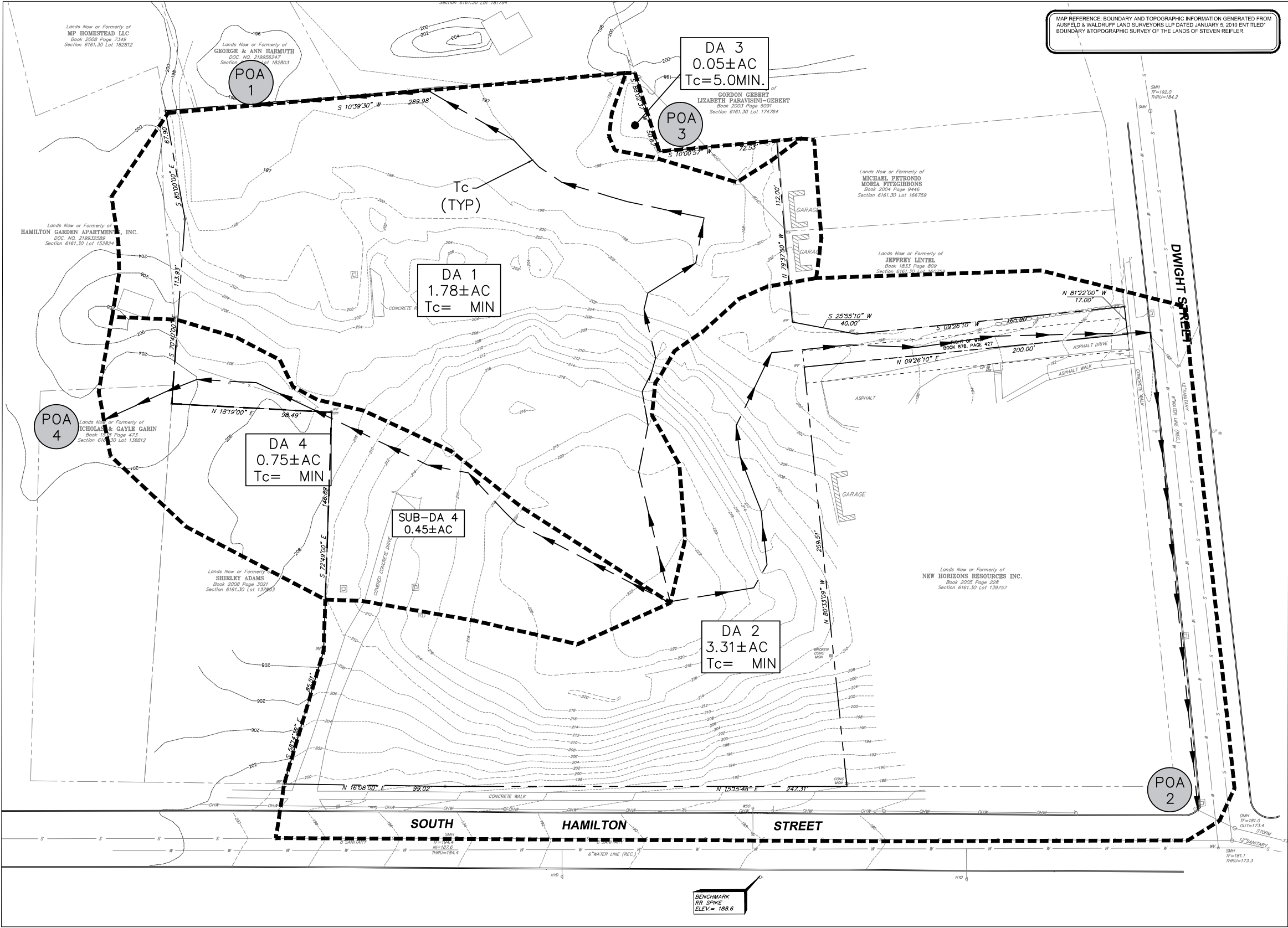
From: Tyler Maegerle, Junior Planner

Date: 6/12/2023

Re: Lot 151783 South Hamilton Avenue

Upon review of project materials provided by the applicant, including Full EAF and site plan application, it is my understanding that the proposed action to be undertaken includes construction of a three-story building consisting of 29 one-bedroom units and 29 two-bedroom units.

Based on the above description it is the determination of the planning division that the proposed action is classified as Type I Action with regards to SEQRA pursuant to 6 NYCRR Section 617.4 (b)(9) "any Unlisted action (unless the action is designed for the preservation of the facility or site), that exceeds 25 percent of any threshold established in this section, occurring wholly or partially within, or substantially contiguous to, any historic building, structure, facility, site or district or prehistoric site that is listed on the National Register of Historic Places (Volume 36 of the Code of Federal Regulations, parts 60 and 63, which is incorporated by reference pursuant to section 617.17 of this Part), or that is listed on the State Register of Historic Places or that has been determined by the Commissioner of the Office of Parks, Recreation and Historic Preservation to be eligible for listing on the State Register of Historic Places pursuant to sections 14.07 or 14.09 of the Parks, Recreation and Historic Preservation Law." and Section 617.4 (b)(5)(iii) "in a city, town or village having a population of 150,000 persons or less, 200 units to be connected (at the commencement of habitation) to existing community or public water and sewerage systems including sewage treatment works." The action is therefore subject to coordinated environmental review.



ISSUED FOR REVIEW
NOT FOR CONSTRUCTION
RECORD PLANS

DATE SUBMITTED:

NO.	REVISIONS	DESCRIPTION	BY	DATE

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CITY OF POUGHKEEPSIE COUNTY OF DUTCHESS
PRE-DEVELOPMENT DRAINAGE
SOUTH HAMILTON
LOCATED AT
100 SOUTH HAMILTON STREET
TAX MAP: 6161-30-151783

DATE: AUGUST 2023
JOB NO: 22.10
SCALE: AS NOTED
DRAWN BY: DFA
CHECKED BY: DFA
DRAWING NO:
SWPPP
1.0
SHEET 2 OF 3

STORMWATER POLLUTION PREVENTION PLAN (SWPPP)

FOR THE DEVELOPMENT OF
SOUTH HAMILTON

CITY OF POUGHKEEPSIE
DUTCHESS COUNTY, NEW YORK

PREPARED FOR:

O'NEILL GROUP HAMILTON, LLC
241 HUDSON AVENUE
HACKENSACK, NEW JERSEY 07601

OCTOBER 3, 2023

PREPARED BY:

Arico Associates
1407 Route 9, Bld.2, Suite 6
Clifton Park, New York 12065

ARICO ASSOCIATES PROJECT ID: 22.10



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APPENDIX C:	Inspection Report and Miscellaneous Forms
APPENDIX D:	Copy of SPDES General Permit for Stormwater Discharges from Construction Activity Permit No. GP-0-15-002
APPENDIX E:	Copy of MS4, Notice of Intent (draft), Notice of Termination
APPENDIX F:	Site Maps and Figures
APPENDIX G:	Supporting Data and Calculations
APPENDIX H:	Hydrographs and Supporting Data
APPENDIX I:	Soil Data and Classification Summary
APPENDIX J:	Construction BMP's
APPENDIX K:	Alternative Construction BMP's
APPENDIX L:	Operational and Maintenance Manual

1.0 INTRODUCTION

1.1 Background

In 1972, Congress passed the Federal Water Pollution Control Act (FWPCA), also known as the Clean Water Act (CWA), to restore and maintain the quality of the nation's waterways. The ultimate goal was to ensure that rivers and streams were fishable, swimmable, and drinkable. In 1987, the Water Quality Act (WQA) added provisions to the CWA that allowed the United States Environmental Protection Agency (EPA) to govern stormwater discharges from construction sites. In 1998, EPA published the final notice for General Permits for Stormwater Discharges from Construction Activities Disturbing 5 Acres or Greater (63 Federal Register 7898, February 14, 1998). The general permit includes provisions for development of a SWPPP to maximize the potential benefits of pollution prevention and sediment and erosion control measures at construction sites.

Development, implementation, and maintenance of the SWPPP will provide the General Contractor with the framework for reducing soil erosion and minimizing pollutants in stormwater during site work construction of South Hamilton (Project).

The SWPPP will:

- Define the characteristics of the site and the type of construction which will occur;
- Describe the site plan for the development to be constructed;
- Describe the practices that will be implemented to control erosion and the release of pollutants in stormwater;
- Create an implementation schedule to ensure that the practices described in this SWPPP are implemented and to evaluate the plan's effectiveness in reducing erosion, sediment, and pollutant levels in stormwater discharged from the site; and
- Describe the final stabilization/termination design to minimize erosion and prevent stormwater impacts after construction is complete.

1.2 SWPPP Content

This SWPPP has been prepared, based on the current level of design, in accordance with the regulations, guidelines, and conditions set forth in the Construction Stormwater General Permit, NYS Stormwater Pollution Prevention Plan (SWPPP) Design Manual, January 2015 and NYSDEC GP-0-20-001 Permit Requirements. The report is divided into sections with several appendices that include stormwater related reference materials.

This SWPPP includes the following:

- Identification of the SWPPP coordinator with a description of this person's duties;
- Description of the existing site conditions include existing land use for the site (i.e., wooded areas, open grassed areas, pavement, buildings, etc.), and if applicable the location of surface waters which are located on or immediately adjacent to the site (wetlands, streams, rivers, lakes, ponds, etc.);
- Identification of the water bodies which will receive runoff from the construction site, including the ultimate body of water that receives the stormwater;
- Identification of drainage areas and potential stormwater contaminants;
- Description of stormwater management controls and various Best Management Practices (BMP) necessary to reduce erosion, sediment and pollutants in stormwater discharge;
- Description of the facility monitoring plan and how controls will be coordinated with construction activities;
- Description of the implementation schedule and provisions for amendment of the plan.

Supporting documentation and standard forms are provided in the following Appendices:

- APPENDIX A: Certification Statements
- APPENDIX B: Site Location and USGS Quadrangle Maps
- APPENDIX C: Inspection Report and Miscellaneous Forms
- APPENDIX D: General Permit Copy of SPDES General Permit for Stormwater Discharges from Construction Activity Permit No. GP-0-20-001.
- APPENDIX E: Copy of MS4, Notice of Intent & Notice of Termination
- APPENDIX F: Site Maps and Figures
- APPENDIX G: Supporting Data and Calculations
- APPENDIX H: Hydrographs and Supporting Data
- APPENDIX I: Soil Data and Classification Summary
- APPENDIX J: Construction BMP's
- APPENDIX K: Alternative Construction BMP's
- APPENDIX L: Operational and Maintenance Manual

1.3 SWPPP COORDINATOR AND DUTIES

The construction site SWPPP coordinator will be determined following the acceptance of a General Contractor. Once the construction site SWPPP Coordinator is selected, the SWPPP will be revised and clearly detail the implementation responsibilities for each part of the site.

The SWPPP coordinator's duties include, but are not necessarily limited to the following:

- Implement the SWPPP plan with the aid of the SWPPP team;
- Oversee maintenance practices identified as BMPs in the SWPPP;
- Implement and oversee employee training;
- Conduct or provide for inspection and monitoring activities;
- Identify other potential pollutant sources and make sure they are added to the plan;
- Identify any deficiencies in the SWPPP and make sure they are corrected;
- Ensure that any changes in construction plans are addressed in the SWPPP; and
- To aid in the implementation of the SWPPP plan, the SWPPP coordinator will assign additional SWPPP team members as necessary.

2.0 PROJECT DESCRIPTION, GOALS AND ASSESSMENT

2.1 Site Location and Existing Conditions

The project applicant and current owner is O'Neill Group Hamilton LLC, located at 241 Hudson Avenue, Hackensack, New Jersey 07601. The applicant is pursuing approvals for a 58-unit multi-family residential apartment building to be known as South Hamilton.

The Project site is 3.378± acres, located on the east side of South Hamilton Street in the City of Poughkeepsie, Dutchess County, New York in the south-western portion of the City of Poughkeepsie. The site is bounded mostly by single-family residences and a community house to the south. The parcel is identified as Tax Map Number 13130-6161-30-151783 (NYSDEC Stormwater Mapper Coordinates: LAT 41.6952, LONG -73.9247).

The 3.378± acres site is currently wooded vacant land and was home to a single-family residence until 1973 when a fire completely destroyed the building and it was demolished. In the winter of 2021, the site was partially cleared and is now mainly covered with small brush and trees.

The Project is presently accessible from South Hamilton Street. Municipal water and sanitary sewer utilities are located within South Hamilton Street and are still available.

2.2 Soils and Topography

A geotechnical study and analysis were prepared by Kevin L. Patton, P.E., 36 Patton Road, Newburgh, N.Y. 12550, (845) 275-7732 on July 6, 2023. In summary, the observed subsurface conditions are described in the test pit logs and are summarized in the drawing attached under separate cover to this report. Most of the native soils and most of the existing fill consisted of fine-sandy silt with traces gravel, few cobbles and boulders. Some of the native soils were identified as glacial till, but most of the native material appears to be till that has been significantly reworked by frost, soil creep and similar factors. Well-graded granular soil (gravelly silty clayey sand) was only encountered in test pits below elevation 205. Bedrock was encountered in five of the six building area test pits at elevation of 200 to 207 feet. The other test pits indicate rock near elevation 200 becoming shallower toward the south and east.

The topography of the site is varied, with steep sections and flatter plateaus. Approximately 70% of the site drains to the east and the remaining area to the west. According to regulatory mapping, the project site is not located in the 100-year flood plain area, have state or federal wetlands and no endangered species have been identified.

2.3 Proposed Site Conditions

South Hamilton proposes the development of fifty-eight (58) apartment units consisting of one-bedroom and two-bedroom units. The Project will contain one (1) three-story apartment building, accessory parking for 102 parking spaces, open common space, sidewalks, landscaping, lighting, public water and sewer, stormwater management areas and utilities.

Access to the site will be from Hamilton Street connecting a 102-space open parking area. Municipal water and sanitary sewer utilities are located along South Hamilton Street and are owned and operated by the City of Poughkeepsie.

The infrastructure development for the project will consist of the construction of approximately 2,700± linear feet of a private access drive leading to a 102-space parking area, together with utility extensions for water, sanitary, electric, gas, telephone and cable improvements. On-site improvement and developments will additionally include of stormwater conveyance and management facilities, general landscaping, lighting, walking trail and other amenities that can complement the project.

The proposed water distribution system will consist of an eight (8) inch DIP water main as an extension of the existing municipal infrastructure connecting Dwight Street and Hamilton Street water mains. A 4-inch lateral from the 8-inch main will service the building. The pipes will be installed with a minimum of 5.5 feet of cover with provisions for proper thrust restraint and separation from sanitary and storm sewers.

The proposed sanitary sewer for the project will be a 6-inch gravity sewer connecting into the existing gravity system along S. Hamilton Street.

3.0 STORMWATER MANAGEMENT AND RUNOFF CHARACTERISTICS

3.1 Introduction

In order to accommodate the proposed expansion, it is necessary to add on-site stormwater collection system and management areas. The stormwater collection system will be provided using sheet drainage, catch basin drains and piping. Proposed control structures and basins will regulate the discharge rate by not exceeding the peak flows for existing development conditions. The following data has been generated and based on the Rational Runoff & Storage-Induction Methods for both existing and expansion conditions for the Water Quality and Quantity Controls according to the NYS Stormwater Pollution Prevention Plan (SWPPP) Design Manual, January 2015 and NYSDEC GP-0-20-001 Permit Requirements.

The stormwater management model used for the design of the stormwater management areas is based upon 1-year, 10-year, 50-year and 100-year peak frequency twenty-four-hour storm events for pre-development and 1-year, 10-year, 50-year and 100-year peak frequency twenty-four-hour storm events for post-development. The control structures, ponds, and direct runoff will be regulated to the discharge rates and volumes by not exceeding the peak flows and volumes for pre-developed conditions at a 1-year, 10-year, 50-year and 100-year storm frequency. The post-developed flows will be stored and released over time through Stormwater Management Practices. The following data has been generated and based on the SCS Runoff & Storage-Induction Methods for both pre-developed and post-developed conditions.

Runoff rates based on post-construction site conditions have been calculated for a 1-year, 10- year, 50-year and 100-year storm event, and are shown below. The site contains primarily Hydrological Soil Group B soils that are considered to be moderately-drained soils. The calculated weighted SCS Runoff Curve Number (CN) is 76 for the pre-development site and the resulting post-development weighted CN is 83 based on the underlying soils types and post-construction cover.

3.2 Stormwater Management & Drainage

The proposed land use will modify the existing site by increasing impervious area. Accordingly, 1.59± acres, or 47.1% of the entire project parcel area of 3.378± acres, will contain impervious area (buildings, concrete walks and asphalt pavement). To reduce the storm water run-off and provide stormwater quality control near the source, a series of Green Infrastructure Practices (GIP) are planned for this project for removal of stormwater pollutants. The intended GIP include detention areas, bioretention areas, grassed swales and porous pavements. All or nearly all the new impervious area from roofs, pavement and other impervious areas will be directed to the storm sewer collection system and management areas. The discharge from the stormwater management basin and all pre-development flow will be tributary to the same discharge point locations that are tributary to the surrounding area.

The flow data is generated and based on the Rational Runoff & Storage-Induction Methods for both pre-developed and post-developed conditions and the Water Quality and Quantity Controls according to the

NYS Stormwater Pollution Prevention Plan (SWPPP) Design Manual, January 2015 and NYSDEC General Permit for Stormwater Discharges from Construction Activity (GP-0-20-001) guidelines and regulations.

In the post-development condition, the overall drainage pattern does not change significantly, and reflects the pre-development patterns. Water quality management and stormwater detention will be provided by the Stormwater Management Areas, (SMA). There are four (4) proposed drainage areas identified as DA1, DA2, DA3 and DA4 for pre- and post-development areas. In assessing the drainage areas, the points of analysis (POA) for each discharge point will remain the same for the pre- and post-development areas, identified as POA1, POA2, POA3 and POA4. See Appendices for maps depicting the Pre-Development Drainage Areas and Post-Development Drainage Areas. There are no tributary areas to the project.

3.3 Green Infrastructure Techniques

Under the guidance of the Design Manual, proprietary stormwater treatment practices were selected to provide the Water Quality controls for the proposed project. Proprietary practices for consideration of Green Infrastructure Practices were considered for use or not implemented as follows:

- Planning. Practice and techniques used in the site design and layout were to incorporate pavement reduction; and soil restoration for areas that will remain as open/landscaped areas.
- Conservation of Natural Areas. Due to the nature of the project, this practice is not practical.
- Sheet Flow to Riparian Buffers or Filter Strips. Due to the nature of the project site as developed, the proposed configuration does not allow for the use of riparian buffers or filter strips.
- Vegetative Open Swale. Some vegetative swales are proposed to direct runoff, this was considered and was not taken for RRV calculations.
- Tree Planting/Tree Boxes. Although tree plantings will occur on the project site, this is considered insignificant and no credit was taken for RRV calculations.
- Disconnection of Rooftop Runoff. Due to the nature of the project, this practice is not practical.
- Soil Restoration. There is existing pavement on the property, therefore as well as these areas and those that will receive heavy machinery and traffic during construction shall be restored prior to placement of topsoil.
- Stream Day-lighting for Redevelopment Projects. There are not existing drainage structures; therefore, there is not opportunity for stream day-lighting.
- Rain Garden/Bioretenention. Though considered, the nature of the projects site the proposed configuration does not allow for the use of Bioretention or Rain Garden areas.
- Green Roof. Due to the nature of the project, this practice is not practical.
- Stormwater Planter. Due to the nature of the project, this practice is not practical.
- Rain Tank/Cistern. Due to the nature of the project, this practice is not practical.
- Porous Pavement. The site design and nature of the project it acceptable for porous pavements, which have been used for RRV calculations.
- Alternate SMP. The site design and nature of the project it acceptable for Hydrodynamic devices and/or Media Filters, and are being used for RRV and WQv calculations.

3.4 Proposed Management Practice(s)

Water quality management utilizing porous pavements and membrane filters (Jellyfish®) for runoff-reduction (RRv) and water quality (WQv), will be provided with a combination of a stormwater management basin to control water quantity controls for Overbank Flow (Qp) and Extreme Flood (Qf) from the site.

The porous pavement will collect sheet runoff from a portion of the paved parking and concrete walkways. The porous pavement will have an underdrain that will be directed towards the detention basin along the eastern portion of the property. The Jellyfish filtering system will collect the remaining portion of the parking and concrete walkways, as well as the entire roof area of 24,650 sf from the proposed building. The combination of the two (2) runoff area will collect all the impervious areas except for a small portion of the access drive at S. Hamilton Street.

In combination, the porous pavement and filtering system will reduce the Water Quality Volume (WQv) and add Runoff Reduction (RRv) credits to meet or exceed the required WQv of 0.123 ac-ft (5,336 cf) and the minimum RRv to be provided of 0.037 ac-ft (1,593 cf). The porous pavement will provide a RRv of 2,519 cf (0.058 ac-ft) leaving a minimal of 2,817 cf to be treated by the filtering system. (See Appendix G – Supportive Data and Calculations.)

The stormwater basin is comprised of an outlet control structure in combination with a broad-crested weir. Together the post-development discharge is maintained lower than the pre-development discharge rate for the 10-year (Qp) and 100-year (Qf) storm event and no further storage practice is implemented. The Channel Protection volume (CPv) is reduced to pre-developed volumes within the WQv and RRv techniques. Based on design data, the WQv+RRv provided meets the criteria for no further water quality management practices to be implemented.

3.5 Hydrologic & Hydraulic Model Input Parameters

The hydrologic parameters and runoff rates for pre- and post-construction site conditions have been calculated for a 1-year, 10- year, 50-year and 100-year storm event, and are shown below. The site contains primarily Hydrological Soil Group B soils that are considered to be moderately-drained soils. The calculated weighted SCS Runoff Curve Number (CN) is 76 for the pre-development site and the resulting post-development weighted CN is 83 based on the underlying soils types and post-construction cover.

To provide an evaluation of pre- and post- development flow rates and volumes, each drainage area is described below in more detail showing the comparison of the proposed development and surrounding areas to existing conditions. See Appendix H – Hydrographs & Supporting Data.

As depicted on the drainage area maps, the analysis point is POA1 and is to the eastern portion of the property. It should also be noted that there is a pre-existing condition, outside of the property limits, that creates off-site ponding during storm events due to grades that are lower than surrounding grades. The pre-development area for DA1 is 1.78± acres and the post-development area is broken down into two (2) sub-areas of DA1.1 (2.49± acres) and DA1 (0.08± acres). In pre-development conditions, the entire 1.78± acres contributed to the runoff, where the post-development conditions, only 0.08± acres

contribute to POA1. The remaining post-development area (DA1.1) is collected, treated and detained in the stormwater management basin. The discharge from the basin will be added to POA2 (see below) and discharge directly to the City of Poughkeepsie storm collection system in Dwight Street. In summary the total water volume to POA1 is significantly reduced, see Table POA1 below.

TABLE POA1

Storm Event	Q _{1-yr}		Q _{10-yr}		Q _{50-yr}		Q _{100-yr}	
	cfs	cf	cfs	cf	cfs	cf	cfs	cf
Pre-Developed	0.56	3,349	2.14	11,403	4.22	22,474	5.47	29,291
Post-Developed	0.07	176	0.22	563	0.42	1,082	0.54	1,397
% change	-87.5	-94.7	-89.7	-95.0	-90.0	-95.2	-90.1	-95.2

As depicted on the drainage area maps, the analysis point is POA2 is to the south and west portion of the property. The pre-development area for DA2 is 3.31± acres and the post-development area is broken down into two (2) sub-areas of 2.49 ± acres (DA1.1) and 2.76± acres (DA2). In order to reduce the discharge to POA1, the remaining area (DA1.1) is collected treated and stored in the detention basin. As noted above, the discharge from the basin will be added to POA2 and discharge directly to the City of Poughkeepsie storm collection system in Dwight Street. In summary, the total water volume to POA2 will be reduced utilizing the outlet control structure and storage volume to reduce the discharge at POA2, see Table POA2 below.

TABLE POA2

Storm Event	Q _{1-yr}		Q _{10-yr}		Q _{50-yr}		Q _{100-yr}	
	cfs	cf	cfs	cf	cfs	cf	cfs	cf
Pre-Developed	2.61	9,405	7.60	27,014	13.70	49,611	17.24	63,152
Post-Developed*	2.48	9,375	6.61	25,041	11.52	44,552	14.33	56,089
% change	-5.0	-0.32	-13.0	-7.3	-15.9	-10.2	-16.9	-11.2

*Based on release rate through SMP and discharge culverts

As depicted on the drainage area maps, the analysis point is POA3 and is to the eastern portion of the property. The pre-development area for DA3 is 0.05± acres and the post-development area is broken down into two (2) sub-areas of 0.03± acres (DA3) and 0.14± acres (portion of DA3.1). In pre-development conditions, the entire 0.05± acres contributed to the runoff, where the post-development conditions, only 0.03± acres contribute to POA3. The remaining post-development area is part of DA3.1 will collected by a catch basin and combined with the discharge from the detention basin. As noted above, this discharge from the basin will be added to POA2 and discharge directly to the City of Poughkeepsie storm collection system in Dwight Street. In summary the total water volume to POA3 is significantly reduced, see Table POA3 below.

TABLE POA3

Storm Event	Q _{1-yr}		Q _{10-yr}		Q _{50-yr}		Q _{100-yr}	
	cfs	cf	cfs	cf	cfs	cf	cfs	cf
Pre-Developed	0.04	96	0.13	325	0.25	639	0.32	832
Post-Developed	0.03	66	0.08	211	0.16	406	0.20	524
% change	-0.25	-31.2	-38.5	-35.1	-36.0	-36.5	-37.5	-37.0

As depicted on the drainage area maps, the analysis point is POA4 and is to the northern portion of the property. The pre-development area for DA4 is 0.75± acres and the post-development area is 0.35± acres. In post-development conditions, 0.40± acres are now contributing to POA1.1. The remaining 0.06± acres (SUB-DA4) of the project site will continue to be tributary to POA4. In summary the total water volume to POA4 is significantly reduced, see Table POA4 below.

TABLE POA4

Storm Event	Q 1-yr		Q 10-yr		Q 50-yr		Q 100-yr	
	cfs	cf	cfs	cf	cfs	cf	cfs	cf
Pre-Developed	0.29	1,522	1.05	5,021	2.03	9,771	2.62	12,681
Post-Developed	0.23	769	0.78	2,457	1.48	4,720	1.89	6,100
% change	-20.7	-49.5	-25.7	-51.1	-27.1	-51.7	-27.9	-51.9

In conclusion, based on drainage patterns and peak flows compared relatively to pre-developed and post-developed conditions, it is believed that the improvements being proposed will not affect present or future downstream conditions relative to flow or sedimentation.

4.0 CONSTRUCTION SEQUENCING AND PHASING

Soil disturbing activities associated with the site development will be phased according to the construction schedule. The Project is not a phased project, though the sequence of major activities is shown on Erosion and Sedimentation Control Plans found in Appendix F and is expected to be as follows:

Site Preparation. The limits of disturbance shall be identified. Erosion and sedimentation control systems shall be placed in accordance with the plans and/or where erosion and sedimentation may occur. Erosion and sediment control systems will be placed as dictated by site conditions in order to maintain the intent of the specifications of the SWPPP and/or contract documents. Clearing and grubbing and removal of any trees, other vegetation and/or excavation from areas to be disturbed may begin only when erosion and sediment control systems are in place and fully functional. Place construction trailers (if any) and portable toilets.

Erosion & Sediment Control Systems. A number of stormwater controls are proposed for both construction and permanent/long-term measures. During construction, stormwater management areas will be used in conjunction with silt fence, soil berms, silt bags and stone check dams. Detention basins will be installed as a permanent water quality measure. For dust and debris control, construction traffic must enter and exit the site at the stabilized construction entrance(s).

Excavation & Rough Grading. Large scale earth moving will occur in this phase in order to construct the proposed roadway, proposed stormwater management areas, underground utilities, and building pad locations. The topsoil stripping and stockpiling will occur during this phase. The topsoil will be used later in landscaped areas. Erosion and sediment controls including, but not limited to, silt fencing will be installed prior to, during and after earthwork activities. The stormwater detention basins and the storm sewer drainage system shall be constructed and be fully operational. Material(s) resulting from clearing,

grubbing and excavation activities shall be stockpiled and protected up-slope from erosion and sediment control systems. Excavation and rough grading for lots will not be started prior to the completion of the road construction.

Decompaction of Existing Soils. Upon removal of any existing pavement areas, heavily traveled areas and/or areas that appear to be over compacted shall undergo a de-compaction application to restore soil porosity and permeability.

Installation of Utilities. Utilities will be installed once rough grading activities are completed and all exposed slopes are temporarily stabilized. Fill areas shall be compacted and stabilized.

Foundation Site Preparation. The building pad locations will be final graded to establish the finished floor elevation.

Access Drive & Pavement Preparation. Finished subgrade elevation and subbase course grades will be established and verified before any roadway asphalt courses are installed.

Landscaping. Topsoil shall be spread on areas to be landscaped and disturbed areas will be planted/seeded in accordance with approved plans.

Temporary Seeding. Within 7-days after construction activity ceases on any particular area, all disturbed ground where there will not be construction for longer than 21-days must be seeded with fast-germinating temporary seed and protected with mulch.

Permanent Seeding. All areas at final grade must be seeded within 7-days after completion of the major construction activity. Except for small level spots, seeded areas should generally be protected with mulch. Permanently seed and mulch cut slopes as excavation proceeds to extent considered desirable and practicable. Slopes exposed by the Road Construction Phase are deemed fully stabilized and complete when turf grass cover provides permanent stabilization for at least 80% of the disturbed soil surface, exclusive of pavement areas.

5.0 EROSION AND SEDIMENT CONTROLS & POLLUTION PREVENTION

During construction, the Operator will comply with the measures provided in this SWPPP and conduct construction activities in such a manner that is in accordance with GP-0-20-001 conditions. It is the Operator's responsibility not to undertake more than that magnitude of work that can be safely and adequately controlled by the methods at their disposal. The Operator's approach must emphasize preventing erosion before it occurs as opposed to treating sediment- laden storm water runoff.

The Erosion Control Plans show the direction of storm water flow and approximate slopes before and after grading activities, areas of soil disturbance and areas that will not be disturbed, natural features to be preserved, locations of non-structural BMPs identified in the SWPPP and locations where storm water discharges to a surface water (where applicable.)

The Erosion Control Plan represents the suggested best management practices proposed for the project. The Contractor's approach to controlling storm water runoff from the site may vary; however, they must

update this SWPPP to reflect the changes and appropriate corresponding erosion control measures using the Progress Maps and the SWPPP Amendment form (provided in Appendix C).

The use of erosion and sedimentation controls is mandatory and must be employed to minimize impacts to adjacent areas during the construction. If sediment escapes the construction site, off-site accumulations of sediment must be removed at a frequency sufficient to minimize off-site impacts.

The control practices which are required to control storm water pollution during construction must remain functional until disturbed areas have been stabilized. Erosion control products are to be installed and maintained in accordance with manufacturer's specifications and good engineering practices. Stabilization measures must be instituted on disturbed areas as soon as practicable, but no more than 14 days after construction activity has temporarily or permanently ceased on any portion of the site.

5.1 Site Maps

Site Maps are located in Appendix F of this report. The Operator must update these plans in accordance (but not limited to) the following:

- Locations where earth-disturbing activities will occur, noting changes in phases.
- Approximate slopes before and after major grading activities. Note sloped areas greater than 15%.
- Locations where sediment, soils or other construction materials, demolitions debris will be stockpiled.
- Locations of any crossing of surface waters.
- Designated points on the site where vehicles will exit onto paved roads.
- Locations of structures and impervious surfaces upon completion of construction.
- Locations of construction support activity areas covered by this permit.
- Locations of all surface waters, wetlands on or in the immediate vicinity of the Site.
- Boundary of any natural buffers (some work will, or may, occur inside of 50-foot buffer.)
- Areas of federally listed critical habitat for endangered or threatened species.
- Topography of the site, existing vegetative cover (e.g., forest, pasture, pavement, structures), and drainage pattern(s) of stormwater and authorized non-stormwaterflow onto, over, and from the site property before and after major grading activities.
- Stormwater and allowable non-stormwater discharge locations, including:
- Locations of any inlets to municipal separate storm sewer systems (MS4s).
- Locations where stormwater or allowable non-stormwater will be discharged to surface waters on or near the site.
- Locations of all potential pollutant-generating activities.
- Locations of stormwater control measures.
- Locations where polymers, flocculants, or other treatment chemicals will be used and stored.

5.2 Progress Site Maps.

GP-0-20-001 requires that as conditions change at the construction site (such as the location of BMPs) the Contractor must update the SWPPP to reflect such changes. Progress Maps (11 x 17) are provided in Appendix F. In addition to tracking the changes associated with the BMPs, the Contractor will indicate and track the location of the following:

- Portable toilets
- Material storage areas
- Concrete washout areas
- Refueling areas

- Off-site material, waste, borrow, or equipment storage areas
- Dumpster or other trash and debris containers
- Spill kits
- Stockpiles
- Any other non-structural non-storm water management BMPs
- Any changes to structural BMPs
- Areas where final stabilization has been accomplished

If the Progress Maps become too full to easily read, it should be dated, folded, and put into the SWPPP in Appendix F for documentation and a new Progress Map should be started.

5.3 Construction Phasing and Best Management Practices (BMPS)

Construction activities will be sequenced in order to minimize site disturbance, protect sensitive natural features, and prevent soil erosion and sediment transport. The intended construction sequence and timing of major activities are identified below.

Stabilized Construction Exit: At the beginning a stabilized construction exit must be installed at the location where vehicles are expected to enter and/or exit the site in order to prevent the off-site tracking of sediment onto adjacent public roadways. The stabilized construction entrances will consist of compacted two to three inch (2"-4" thickness) crushed stone, placed over a layer of geotextile fabric (to provide separation from the underlying soil and prevent the stone from being ground down into the soil). The stabilized construction entrance must be wide enough to cover the entire width of the entrance/exit and allow two vehicles to pass comfortably, and it should be flared where it meets the public roadway to accommodate longer construction vehicles. The stabilized construction entrance must be long enough to allow mud and sediment to become dislodged from vehicle tires, and/or a minimum of fifty (50') in length.

Over the course of construction, the stabilized construction entrance will become filled with accumulated sediment. The Contractor must inspect the stabilized construction entrance and adjacent public roadways for off-site sediment tracking and repair the entrance as necessary (remove accumulated sediment and add new stone as necessary). If tracking onto public roadways does occur, the streets in the vicinity of the stabilized construction entrance shall be swept immediately. The stabilized construction entrance shall not be removed until just prior to project completion.

Mulch Tube/Silt Fence: At the beginning a combination of mulch tube and silt fence or just silt fence shall be installed to prevent sediment laden runoff from leaving the site. In addition, silt fence will be used on the down gradient sides of material stockpile areas.

The mulch tubes consist of a tube of mulch placed along a contour if possible. The tubes intercept and slow sheet flow runoff. Mulch tubes shall be firmly staked. Accumulated sediment must be removed from the tubes when it reaches ½ of the height of the tube. Silt fence is a sediment control BMP consisting of a length of geotextile fabric stretched between anchoring posts spaced at regular intervals along the site at low/down-slope areas. The geotextile fabric must be entrenched in the ground between the support posts. Silt fence is effective in treating low velocity sheet flow and is not intended for use in areas of concentrated or channelized flow. Silt fence must be inspected for rips, tears, and gaps between the fence and the ground. An adequate reserve of silt fence must be kept on site at all times for

emergency and/or routine replacement. Silt fence shall be entirely removed only after exposed soils in the contributing drainage area are stabilized. Silt fence can also be used as an effective perimeter control to contain stockpiles of topsoil or other erodible material.

Stockpile Management: Stockpiles of erodible material, including any topsoil salvaged during construction, must be surrounded by a perimeter sediment control such as silt fence to prevent storm water runoff from being contaminated by eroded sediment. Stockpiles of erodible material must be stabilized utilizing a temporary stabilization technique if they remain inactive for more than fourteen (14) days. Stockpiles must be located at least 100 feet from wetland resource areas (i.e. bordering vegetated wetlands and Buffer zone). Stockpile locations must be tracked using the Site Maps included in Appendix A.

Dust Control: Dust control BMPs are various means and methods of preventing soil erosion by wind. During all stages of the project generation of dust must be minimized to prevent air and water pollution as well as minimize risks to human health. Earthmoving activities are the primary source of dust generation during construction, but traffic on un-stabilized access roads and sediment transport by wind blowing across exposed soil surfaces can also be contributing factors. The most effective dust control BMPs for preventing wind erosion involve stabilizing (temporary or permanent) exposed soils. However, where soil stabilization is not practical techniques that increase soil moisture and encourage the formation of soil clods or reduce wind velocity at the soil surface are also effective. The following dust control BMPs are typically used on construction sites:

- **Watering/Irrigation:** Sprinkling the ground surface with water until it is moist.
- **Soil Stabilization:** Vegetative cover, mulch, riprap or any method that covers the soil surface reduces the potential for soil particles to become airborne.
- **Wind Breaks:** Wind breaks are barriers (either natural or constructed) that reduce wind velocity across exposed soil surfaces and reduces the potential for soil particles become airborne. Wind breaks can be trees or shrubs left in place during site clearing or constructed barriers such as a wind fence.
- **Soil Roughening:** Deep tillage in large areas of exposed soil brings soil clods to the surface preventing soil particles from becoming airborne.

Temporary Sediment Basins/Traps: If temporary sediment basins/traps/swales are needed at the beginning, basins must provide 3,600 cubic feet of storage per acre drained and disturbed. Temporary sediment basins are a sediment control BMP that consist of an excavated or natural depression that detains/retains storm water runoff allowing sediments to settle out of suspension prior to discharge via a suitably stabilized outlet. They also provide an opportunity for storm water infiltration. The temporary sediment basin's side-slopes and bottom must be appropriately stabilized prior to directing runoff to it. Accumulated sediment must be removed when it reaches 33% of the design volume capacity of the basin in order to maximize sediment settling potential and minimize the possibility of sediment washout during high intensity/long duration storm events. The basins will include a controlled outlet structure consisting of a perforated riser pipe packed in gravel which allow for further reduction of sediment prior to discharge. Traps will include a rip rap spillway.

Temporary Stabilization: Stabilization measures must be initiated as soon as practicable on portions of the site where construction activities have temporarily or permanently ceased, but in no case more than

14 days after the construction activity in that portion of the site has temporarily or permanently ceased. Temporary stabilization refers to a variety of erosion control BMPs that protect exposed soils from the erosive forces of precipitation (raindrop and sheet erosion) and/or prevent the formation of channelized flow (rill, gully and channel erosion). The Contractor must inspect temporarily stabilized areas to assess the effectiveness of temporary stabilization BMPs and replace/repair then as necessary. The following temporary stabilization BMPs are typically used on construction sites and may be used by the Contractor for this project:

Erosion Control Blankets: Erosion control blankets are erosion control BMPs consisting of natural or synthetic geotextile fabrics formed into long sheets or mats that are rolled out over exposed soils and fastened with stakes, pegs or staples. They are used in areas where high runoff velocity makes traditional mulching ineffective. Blankets are highly effective at stabilizing steep slopes (3:1 or greater) and can be used to stabilize areas of concentrated flow such as swales.

Soil Roughening: Soil roughening is an erosion control BMP that involves creating grooves or impressions in exposed soil surfaces with tracked construction equipment (bulldozer, excavator, etc.). Slopes that are not fine graded or smoothed but left in a roughened condition reduce erosion by decreasing slope length and runoff velocity, increasing infiltration, trapping sediment, and allowing seed to take hold and grow. It is critically important that the impressions be made perpendicular to the slope contours (never parallel to the contour); improper use of this technique can actually accelerate erosion. Soil roughening shall be used as a last resort.

Temporary Seeding: Temporary seeding is an erosion control BMP that consists of using select varieties of grasses to establish vegetative cover. Temporary seeding utilizes annual species that establish quickly, are not persistent or invasive, but provide long term temporary cover (as opposed to the perennial species used in permanent seeding for final stabilization).

Temporary Diversion Ditches: Temporary diversion ditches are an option to divert runoff away from construction area. Temporary drainage ditches are a runoff control BMP consisting of a ditch or excavation installed as a means of conveying storm water runoff to temporary sediment basins/traps (or other sediment control BMPs) while soil disturbing construction activities are ongoing. The temporary drainage ditch side-slopes and bottom must be appropriately stabilized prior to directing runoff to it. The temporary ditches will include stone check dams (see below). Temporary drainage ditches may be constructed as needed at locations determined by the Operator. This is done to account for unanticipated on-site field conditions.

Concrete Washout Area: Concrete washout areas consist of a prefabricated or site-built impermeable containment area sized to hold concrete wastes and wash water (including one (1) foot freeboard). Concrete washouts are used to contain concrete and liquids when the chutes of concrete mixers and hoppers of concrete pumps are rinsed out after delivery. The washout facility consolidates solids for easier disposal and prevent runoff of liquids. The wash water is alkaline and contains high levels of chromium, which can leach into the ground and contaminate groundwater. It can also migrate to a storm drain, which can increase the pH of area waters and harm aquatic life. Solids that are improperly disposed of can clog storm drain pipes and cause flooding. The concrete washouts must be constructed prior to placement of concrete on-site. The concrete washout area must be located in an area where its likelihood

of contributing to storm water discharges is negligible. Washouts shall be located outside of any wetland resource area and 100' from buffer zones to wetlands.

These specially designated areas should be properly signed, and onsite personnel instructed in their proper use. The hardened residue from the concrete wash out area will be disposed of in the same manner as other non-hazardous construction waste materials or may be broken up and used onsite as appropriate. It is the responsibility of the Contractor to ensure that these procedures are followed. The Contractor must track concrete washout locations on the Progress Map if they are moved or if additional concrete washouts need to be constructed.

Permanent Stabilization: Permanent stabilization refers to a variety of erosion control BMPs that allow a construction project to achieve "final stabilization." Final stabilization is defined in Appendix A of GP-0-20-001 as: a uniform, perennial vegetative cover with a density of eighty percent (80%) over the entire pervious surface has been established, or other equivalent permanent stabilization measures have been employed.

6.0 STORMWATER MANAGEMENT CONTROLS

The purpose of this section is to identify the types of temporary and permanent erosion and sediment controls that will be used during construction activities. The controls will provide soil stabilization for disturbed areas and structural controls to divert runoff and remove sediment. This section will also address control of other potential stormwater pollutant sources such as construction materials (paints, concrete dust, solvents, plaster), waste disposal, control of vehicle traffic, and sanitary waste disposal. A number of the BMPs included in this plan have been developed to serve as stormwater controls. Project wide erosion control measures are illustrated on the site plans.

6.1 Site Wide Control Measures

To prevent soil from washing into undisturbed areas of the site, the following BMPs will be implemented:

- Construction vehicles will be directed to avoid wetlands, steep slopes and areas to remain vegetated to the maximum extent practicable.
- Silt fencing and hay bale barriers will be placed along the perimeter of the area to be cleared and graded before any clearing or grading takes place. Super silt fencing will be used on steep slopes at appropriate locations.
- Within 14-days after clearing and grading, agricultural limestone and fertilizer will be applied to each acre to be stabilized.
- After fertilizing, all areas which will not be impacted by construction will be seeded.
- After seeding, each area will be mulched with straw. The straw mulch is to be tacked into place by a disk with blades set nearly straight.
- Top soil stock piles will be stabilized with temporary seed and mulch no later than 14-days from the last construction activities in that area.
- Areas of the site which are to be paved will be temporarily stabilized by applying geotextile where required and/or stone sub-base until asphalt is applied.

6.2 Construction Practices to Minimize Stormwater Contamination

All waste materials will be collected and stored in a securely lidded metal dumpster. All trash and construction debris from the site will be deposited in the dumpster. The dumpster will be emptied a minimum of twice per week and the trash will be hauled to a nearby solid waste landfill. No construction materials will be buried on site. All personnel will be instructed regarding the correct procedure for waste disposal. Good housekeeping and spill control practices will be followed during construction to minimize stormwater contamination from petroleum products, fertilizers, paints and concrete. Any housekeeping practices for each drainage area are listed below:

To prevent stormwater contamination, the following BMPs will be implemented:

- All vehicles on site will be monitored for leaks and receive regular preventive maintenance to reduce the chance of leakage.
- Petroleum products will be stored in tightly sealed containers which are clearly labeled.
- Spill kits will be included with all fueling sources and maintenance activities.
- Any asphalt substances used on site will be applied according to the manufacturer's recommendation.
- Sanitary waste will be collected from portable units as necessary to avoid overfilling.
- A covered dumpster will be used for all waste materials.
- All paint containers and curing compounds will be tightly sealed and stored when not required for use. Excess paint will not be discharged to the environment but will be properly disposed according to the manufacturer's instructions.
- Materials and equipment necessary for spill cleanup will be kept in the temporary material storage trailer onsite. Equipment will include, but not be limited to, brooms, dust pans, mops, rags, gloves, goggles, kitty litter, sand, saw dust and plastic and metal trash containers.
- Spray guns will be cleaned on a removable tarp.
- All spills will be cleaned up immediately upon discovery. Significant spills will be reported to the NYSDEC Spill Response Hotline at 1-800-457-7362. All spills will be detailed in the Spill Response Reports (**Appendix D**).
- Concrete trucks will not be allowed to wash out or discharge surplus concrete or drum wash water on the site.
- All ruts caused by equipment used for cutting and removing of trees will be graded.
- Form release oil used for decorative stone work will be applied over a pallet covered with an absorbent material to collect excess fluid. The absorbent material will be replaced and disposed of properly when saturated.
- When testing/cleaning of water supply lines, the discharge from the tested pipe will be collected and disposed of appropriately.

To prevent stormwater contamination at the site entrance, the following BMPs will be implemented:

- A stabilized construction entrance will be constructed to reduce vehicle tracking of sediments.
- Dump trucks hauling material from the construction site will be covered with a tarpaulin.

6.3 Coordination of BMPs with Construction Activities

Structural BMPs will be coordinated with construction activities so the BMP is in place before construction begins. The following BMPs will be coordinated with construction activities:

- The temporary perimeter controls (silt fences and hay bales) will be installed before any clearing and grading begins.
- Clearing and grading will not occur in an area until it is necessary for construction to proceed.
- Sandbag trench plugs will be placed in sloping open trenches during utility installation.
- The stabilized construction site entrance will be built before clearing and grading begins.
- Once construction ceases permanently in an area, that area will be stabilized with permanent seed and mulch.
- The temporary perimeter controls (silt fencing and hay bales) will not be removed until all construction activities at the site are complete and soils have been stabilized.

6.4 Certification of Compliance with Federal, State and Local Regulations

This SWPPP reflects the EPA, the New York State Department of Environmental Conservation (NYSDEC) regulations, and the Town of Clifton Park requirements for stormwater management, erosion and sediment control.

7.0 MAINTENANCE & INSPECTION PROCEEDURES

7.1 Inspections

Following the start of construction, site inspections will be conducted by a qualified professional at least every seven (7) days. The inspection will verify that the structural BMPs described in Section 4 of the SWPPP are in good condition and are minimizing erosion. The inspection will also verify that the procedures used to prevent stormwater contamination from construction materials and petroleum products are effective. The following inspection and maintenance practices will be used to maintain erosion and sediment controls:

- Built up sediment will be removed from silt fencing when it has reached one-third the height of the fence.
- Silt fences will be inspected for depth of sediment, for tears, to examine if the fabric is securely attached to the fence posts, and to ensure that the fence posts are firmly in the ground.
- Temporary and permanent seeding will be inspected for bare spots, washouts, and healthy growth.
- The stabilized construction entrance will be inspected for sediment tracked on the road, for clean gravel and to make sure that all traffic uses the stabilized entrance when leaving the site.

The maintenance inspection reports will be made after each inspection. Copies of the report forms to be completed by the SWPPP coordinator, or qualified SWPPP team member, are provided in **Appendix C** of this SWPPP. Completed forms will be maintained on site during the entire construction project. Following construction, the completed forms will be retained at the general contractor's office for a minimum of 5 years.

If construction activities or design modifications are made to the site plan which could impact stormwater, this SWPPP will be amended appropriately. The amended SWPPP will have a description of the new activities that contribute to the increased pollutant loading and the planned source control activities. The Plan Changes, Authorization and Change Certification is included in **Appendix C**.

7.2 Contractor/Employee Inspection Training

The contracting (construction) company will have employee(s) receive four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity. After receiving the initial training, the *trained contractor* shall receive four (4) hours of training every three (3) years.

Additionally, the contracting (construction) company can utilize a *qualified inspector* qualifications (e.g. licensed Professional, Engineer, Certified Professional in Erosion and Sediment Control (CPESC), Registered Landscape Architect, or someone working under the direct supervision of, and at the same company as, the licensed Professional Engineer or Registered Landscape Architect, provided they have received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity).

The *trained contractor* is responsible for the day-to-day implementation of the SWPPP.

7.3 Final stabilization and Notice of Termination

Once construction activity has been completed a Notice of Termination must be filed with the DEC within 30 days of final stabilization on all portions of the site for which the Operator was responsible. A blank NOT has been provided in Appendix D. However, before terminating permit coverage, the following items must be accomplished:

- Remove any construction debris and trash
- Remove temporary BMPs (such as silt fence). Remove any residual sediment as needed. Seed and mulch any small bare spots. BMPs that will decompose, including some fiber rolls and blankets, may be left in place.
- Check areas where erosion-control blanket or matting was installed. Cut away and remove all loose, exposed material, especially in areas where walking or mowing will occur. Reseed all bare soil areas.
- Ensure that 80 percent of vegetation coverage has been achieved or equivalent stabilization measures have been applied.
- Repair any remaining signs of erosion
- Inspect storm drain system and any permanent sedimentation/detention basins and clean accumulated sediment or debris
- Ensure that post-construction BMPs are in place and operational. Provide written maintenance requirements for all post-construction BMPs to the appropriate party.
- Check all drainage conveyances and outlets to ensure they were installed correctly and are operational. Inspect inlet areas to ensure complete stabilization and remove any brush or debris that could clog inlets.
- Seed and mulch or otherwise stabilize any areas where runoff flows might converge, or high velocity flows are expected.
- Ensure subcontractors have repaired their work areas before final closeout.

7.5 Post-Construction Maintenance

General

The Owner/Operating Manager of 21st Century Park shall overtake and become responsible for all inspecting, monitoring and maintaining erosion control features and drainage structures over the lifetime of the feature. Any SMP's that will be located within the Town right-of-way or individual parcel of land that will be dedicated to the Town of Clifton Park. The inspection of on-site stabilization measures will become part of routine and preventive maintenance practices developed by the owner and/or his representatives.

Inspection

The entire site should be inspected regularly for the presence of erosion gullies, cracking and wash-outs caused by heavy storm events. The cause of any irregularities should be identified and addressed in a timely manner. At minimum, eroded areas should be top soiled, seeded and mulched, as necessary, depending upon the identified irregularity.

The stormwater management system (i.e., roadway drainage ditches, stormwater management ponds, pipe inlets and outlets, etc.) shall be inspected at least yearly for sediment build-up, debris and structural integrity. Stormwater management pond outlet control structures should be checked more frequently for debris blockages. Vacuum truck services should be secured by the owner for periodic removal of sediment within the pond system, the outlet control structure and drainage structures.

The inspection should include, but not be limited to the following:

- Accumulation of pollutants such as grease and oils
- Stabilization and condition of vegetative ground cover areas
- Cracking, settlement, sliding or gully erosions of embankments
- Sedimentation of downstream water bodies, culvert or swales
- Sedimentation of lawns, pavement areas or catch basin sumps
- Presences of animal borrows, clogging or sedimentation at inlets or outlets
- Erosion or disruption of flow paths from stormwater management areas
- Structural integrity of spillways and obstruction of overflow
- Inlet and outlet riprap for scouring or dislodged stones or obstructions
- Forebays and aquatic benches to ensure proper and structural integrity.

Any sediment build-up should be removed and disposed of utilizing acceptable practices. Care should be exercised to avoid storing snow on any stormwater facilities, including the outfall swale.

Maintenance

The permittee shall at all times properly operate and maintain all facilities and systems of treatment and control, as well as related appurtenances, which are installed or to be used by the permittee to achieve compliance with the conditions of this permit and within the requirements of the SWPPP. Maintenance shall occur on a regular basis and should include, but not be limited to the following:

- Mowing of lawn areas at least three times per year to prevent over and vigorous growth

- Mowing of steep slopes, embankments, overflow spillways at least three times per year to prevent overgrowth
- Sweeping common paved areas at least three times per year, especially during early spring to remove excessive deicing material and sediment
- Removing litter and debris that has accumulated during mowing and sweeping operations
- Repairing catch basins, storm sewer components upon immediately addressing and identifying structural deficiency
- Clean catch basin sumps once accumulation is within six inches of the outlet pipe invert
- Repair of riprap lined swales, inlets and outlets, replacing stone where signs of scouring have occurred
- Remove trees and excessive growth from inlets and outlets
- Infiltration basins, bio-retention and rain gardens shall be monitored periodically for sediment accumulations. Once the accumulations have reduced the water quality volume by more than 25%, sediment shall be removed. Removal of sediment shall be in a manner to re-establish original grades as shown on the original construction plans.
- A simple maintenance agreement should identify where Soil Restoration is applied, where newly restored areas are/cannot be cleared, who the responsible parties are to ensure that routine vegetation improvements are made (i.e., thinning, invasive plant removal, etc.). Soil compost amendments within a filter strip or grass channel should be located in public right of way, or within a dedicated Stormwater or drainage easement.
- First year maintenance operations include:
 - Initial inspections for the first six months (once after each storm greater than half- inch)
- Reseeding to repair bare or eroding areas to assure grass stabilization
 - Water once every three days for first month, and then provide a half inch of water per week during first year. Irrigation plan may be adjusted according to the rain event.
 - Fertilization may be needed in the fall after the first growing season to increase plant vigor

7.6 Signatory Requirements and Certifications

All NOIs, NOTs SWPPPs, reports, certifications or information required by this permit or submitted pursuant to this permit shall be signed as follows:

1. All NOIs and NOTs shall be signed as follows:
 - a. For a corporation: by (1) a president, secretary, trustee, or vice president of the corporation in charge of a principal business function, or any other person authorized to and who performs similar policy or decision making functions for the corporation; or (2) the manager of one or more manufacturing, production or operating facilities employing more than 250 persons or having gross annual sales or expenditures exceeding \$25,000,000 (in second-quarter 1980 dollars) if authority to sign documents has been assigned or delegated to the manager in accordance with corporate procedures;
 - b. For a partnership or sole proprietorship; by general partner or the proprietor, respectively; or
 - c. For a municipality, state, federal, or other public agency: by either a principal executive officer or ranking official. For purposes of this section, a principal executive officer of a federal agency includes (1) the chief executive officer of the agency, or (2) a senior executive officer having

responsibility for the overall operations of a principal geographic unit of the agency (e.g., Regional Administrators of EPA).

2. The SWPPP and all reports required by the permit and other information requested by the department or local agency shall be signed by a person described above or by a duly authorized representative of that person. A person is a duly authorized representative only if:
 - a. The authorization is made in writing by a person described above and submitted to the Department
 - b. The authorization specifies either an individual or a position having responsibilities for the overall operation of the regulated facility or activity, such as the position of manager, operator, superintendent, or position of equivalent responsibility or an individual or position having overall responsibility for environmental matters for the company. (A duly authorized representative may thus be either a named individual or any individual occupying a named person)
 - c. Certification Statement – Except for NOIs and NOTs, any person signing documents in accordance with this Part shall make the certifications as attached in **Appendix A**.

◇ ◇ ◇ ◇ ◇ ◇ ◇ ◇

APPENDIX A



Owner/Operator Certification Form

SPDES General Permit For Stormwater Discharges From Construction Activity (GP-0-20-001)

Project/Site Name: South Hamilton

eNOI Submission Number: _____

eNOI Submitted by: ☐ Owner/Operator ☒ SWPPP Preparer ☐ Other

Certification Statement - Owner/Operator

I have read or been advised of the permit conditions and believe that I understand them. I also understand that, under the terms of the permit, there may be reporting requirements. I hereby certify that this document and the corresponding documents were prepared under my direction or supervision. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations. I further understand that coverage under the general permit will be identified in the acknowledgment that I will receive as a result of submitting this NOI and can be as long as sixty (60) business days as provided for in the general permit. I also understand that, by submitting this NOI, I am acknowledging that the SWPPP has been developed and will be implemented as the first element of construction, and agreeing to comply with all the terms and conditions of the general permit for which this NOI is being submitted.

Owner/Operator First Name M.I. Last Name

Signature

Date



SWPPP Preparer Certification Form

*SPDES General Permit for Stormwater
Discharges From Construction Activity
(GP-0-20-001)*

Project Site Information

Project/Site Name

South Hamilton

Owner/Operator Information

Owner/Operator (Company Name/Private Owner/Municipality Name)

O'Neill GTroup Hamilton, LLC

Certification Statement – SWPPP Preparer

I hereby certify that the Stormwater Pollution Prevention Plan (SWPPP) for this project has been prepared in accordance with the terms and conditions of the GP-0-20-001. Furthermore, I understand that certifying false, incorrect or inaccurate information is a violation of this permit and the laws of the State of New York and could subject me to criminal, civil and/or administrative proceedings.

Dominick

First name

F

MI

Arico

Last Name

Signature

10.03.23

Date

CONTRACTOR/SUBCONTRACTOR CERTIFICATION
STORMWATER POLLUTION PREVENTION PLAN

SPDES Project Number: _____

Project Title: _____

Operator: _____

As a subcontractor, you are required to comply with the Stormwater Pollution Prevention Plan (SWPPP) for any work that you perform on-site. Any person or group who violates any condition of the SWPPP may be subject to substantial penalties or loss of contract. You are encouraged to advise each of your employees working on this project of the requirements of the SWPPP. A copy of the SWPPP is available for your review at the office trailer.

Each subcontractor engaged in activities at the construction site that could impact stormwater must be identified and sign the following certification statement:

I hereby certify under penalty of law that I understand and agree to comply with the terms and conditions of the SWPPP and agree to implement any corrective actions identified by the *qualified inspector* during a site inspection. I also understand that the *owner or operator* must comply with the terms and conditions of the most current version of the New York State Pollutant Discharge Elimination System ("SPDES") general permit for stormwater *discharges* from *construction activities* and that it is unlawful for any person to cause or contribute to a violation of *water quality standards*. Furthermore, I am aware that there are significant penalties for submitting false information, that I do not believe to be true, including the possibility of fine and imprisonment of knowing violations.

This certification is hereby signed in reference to the above-named project:

Company: _____

Address: _____

Telephone Number: _____

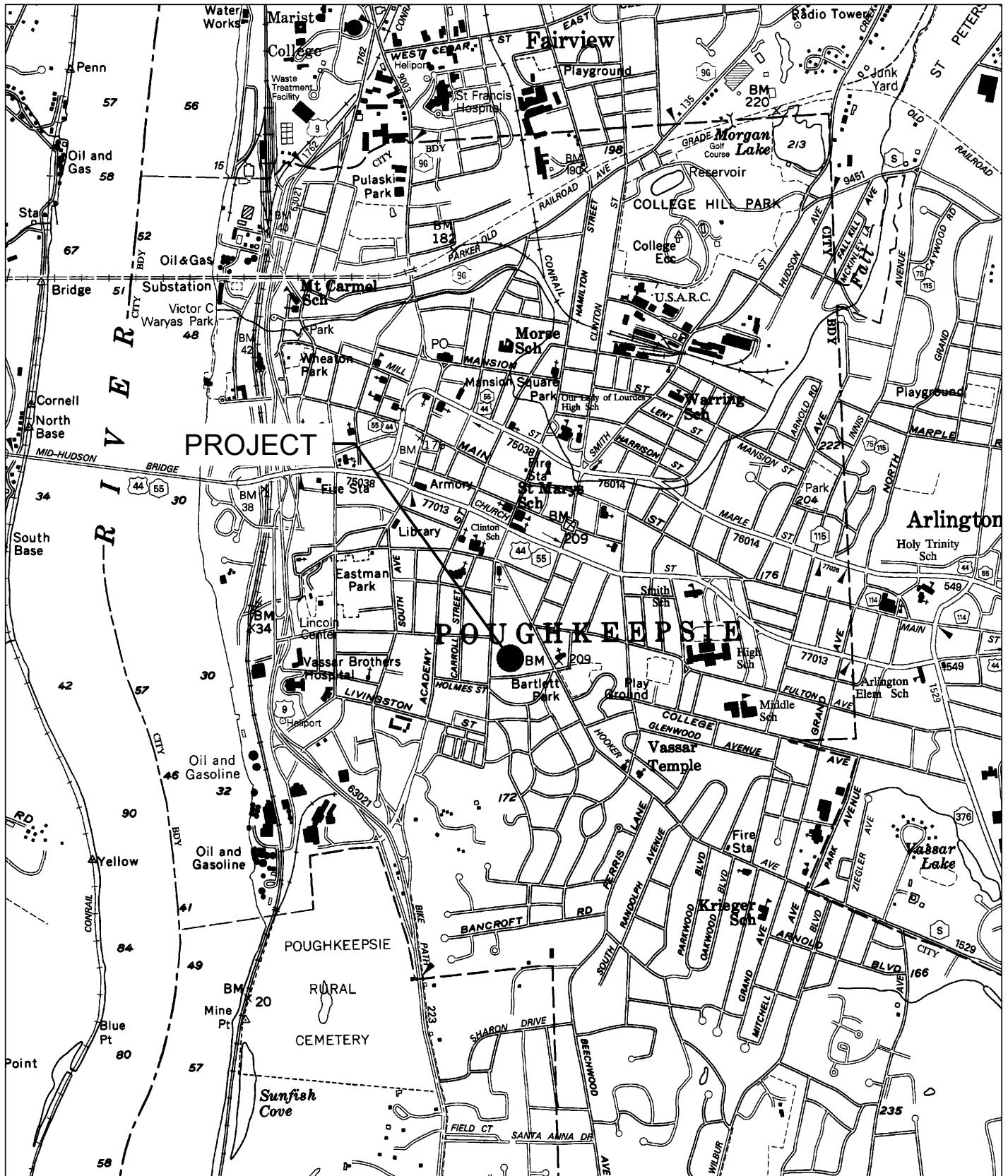
Description of Work: _____

Trained Contractor's Signature: _____

Title: _____

Date: _____

APPENDIX B



ARICO ASSOCIATES

1407 Route 9, Bldg. 2, Suite 6, Clifton Park, New York 12065
518.573.6989 aricoassociates@gmail.com

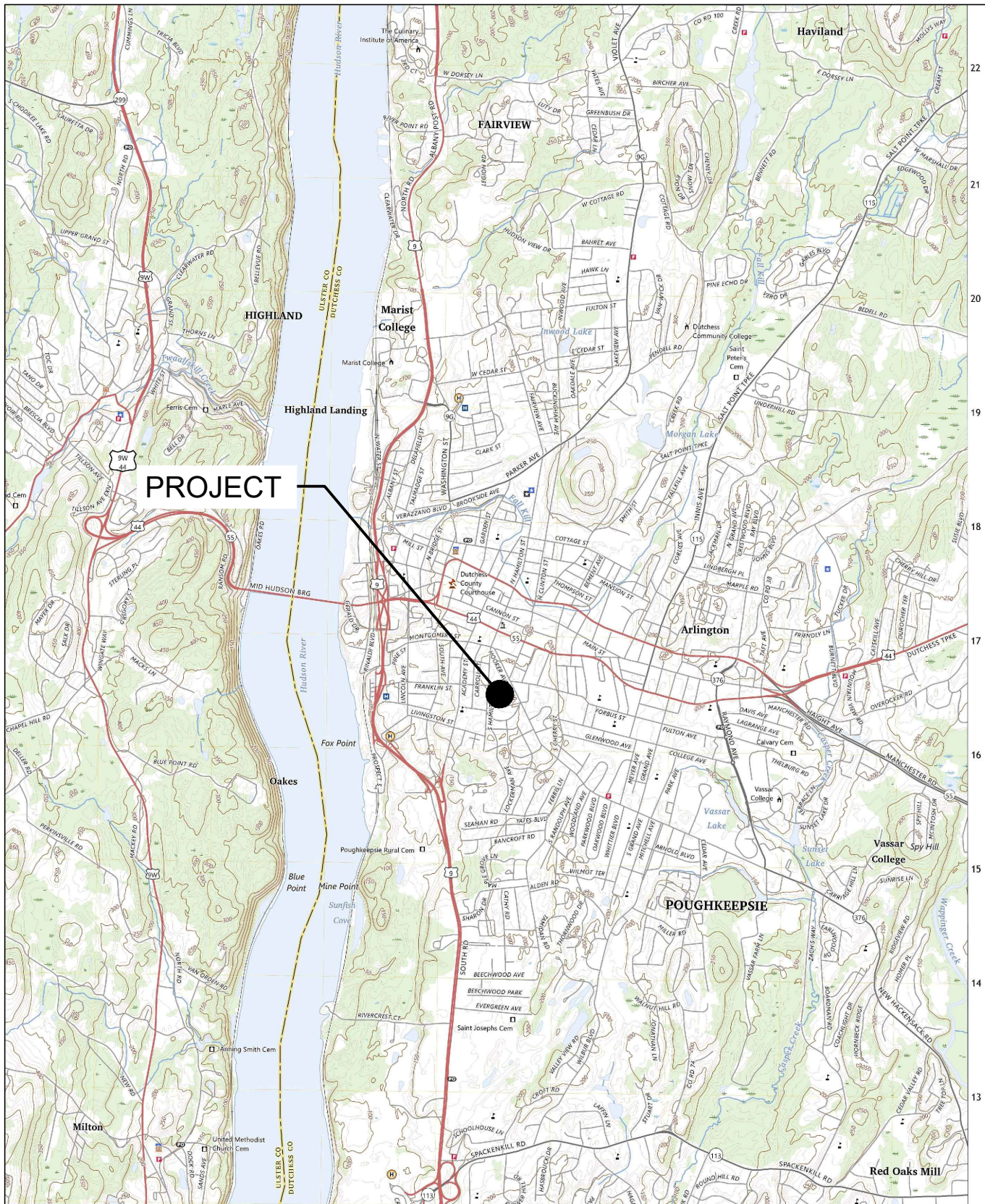
ENGINEERS
LAND PLANNERS
& CONSULTANTS

CITY OF POUGHKEEPSIE COUNTY OF DUTCHESS

LOCATION MAP
SOUTH HAMILTON
LOCATED AT
S. HAMILTON STREET

DATE: OCTOBER 2023

SCALE: 1"=2000'



ARICO ASSOCIATES
 ENGINEERS
 LAND PLANNERS
 & CONSULTANTS

1407 Route 9, Bldg. 2, Suite 6, Clifton Park, New York 12065
 518.573.6989 aricoassociates@gmail.com

CITY OF POUGHKEEPSIE COUNTY OF DUTCHESS

U.S.G.S MAP
SOUTH HAMILTON
 LOCATED AT
S. HAMILTON STREET

DATE: OCTOBER 2023

SCALE: 1"=4000'

APPENDIX C

SITE INSPECTION REPORT REQUIREMENTS

This information is to be provided on contracts covered by SPDES General Permit for Construction Activity (GP-0-10-001). The completed inspection form must be filed in the Engineer's Field Office.

1. Describe the condition of the runoff at all points of discharge from the construction site. Include *discharges* from conveyance systems (i.e. pipes, culverts, ditches, etc.) and overland flow.
2. Describe condition of any waterbodies within or adjacent to the construction site. Identify any sediment discharges to the surface waterbody.
3. Identify all erosion and sediment control practices that need repair or maintenance.
4. Identify all erosion and sediment control practices that were not installed properly or are not functioning as designed and need to be reinstalled or replaced.
5. Description and sketch of areas that are disturbed at the time of the inspection and areas that have been stabilized (temporary and/or final) since the last inspection.
6. Identify current phase of construction of all post-construction stormwater management practices and identify all construction that is not in conformance with the SWPPP and technical standards.
7. Identify corrective action(s) that must be taken to install, repair, replace or maintain erosion and sediment control practices; and to correct deficiencies identified with the construction of the post-construction stormwater management practice(s).
8. Digital photographs, with date stamp, that show the condition of all practices that have been identified as needing corrective actions.
 - a. Photographs must also be taken that show the practice(s) after the corrective action has been completed.
 - b. Color paper copies of the photographs must be included in the inspection report within 7 calendar days.
9. Inspection Notification:
Within One Business day of the inspection, the qualified inspector shall notify the owner or operator and appropriate contractor or subcontractor of any corrective actions required. .

STORMWATER POLLUTION PREVENTION PLAN INSPECTION REPORT

Project Name: _____ Project Number: _____
 Date of Inspection: _____ Time: _____ Day of the Week:

Mon	Tue	Wed	Thur	Fri	Sat	Sun
-----	-----	-----	------	-----	-----	-----

 Weather Conditions: _____ Soil Conditions: ☐ Dry ☐ Wet ☐ Saturated
 Temperature: _____ Reason for Visit: ☐ Weekly ☐ Special ☐ 5+ acres exposed
 Inspector Name: _____ Inspector's Signature: _____

Codes for erosion and sedimentation controls to be inspected:

- | | | |
|--------------------------|--------------------------------------|-----------------------------------|
| (1) Mulch | (6) Sediment Trap | (11) Soil Stabilizers |
| (2) Seeding and Mulch | (7) Turbidity Curtains | (12) Construction Entrance |
| (3) Check Dams | (8) Sedimentation Basins | (13) Pipe inlet/outlet protection |
| (4) Triangular Silt Dike | (9) Drain Structure Inlet Protection | (14) Water bars |
| (5) Silt Fence | (10) Rolled Erosion Control Products | (15) Other Control Measures |

Location	Disturbance		Measure		Remarks	Approximate Sediment Accumulation (%) of Depth	Maintenance Required Y or N (if Y describe)
	Existing Y or N	Next 7 days Y or N	Code #	Temp or Perm (T,P or NA)	Describe structure integrity and any erosion		

Comments: _____

—

—

**STATE POLLUTANT DISCHARGE ELIMINATION SYSTEM
FOR CONSTRUCTION ACTIVITIES**

CONSTRUCTION SITE LOG BOOK

Table of Contents

- I. Pre-Construction Meeting Documents.
 - a. Preamble to Site Assessment and Inspections
 - b. Operator's Certification
 - c. Qualified Professional's Credentials & Certification
 - d. Pre-Construction Site Assessment Checklist
- a. II. Construction Duration Inspections
 - a. Directions
 - b. Modification to the SWPPP
- III. Monthly Summary Reports
- IV. Monitoring, Reporting, and Three-Month Status Reports
 - a. Operator's Compliance Response Form
- a

Properly completing forms such as those contained in this document meet the inspection requirement of NYSDEC SPDES GP for Construction Activities. Completed forms shall be kept on site at all times and made available to authorities upon request.

I. PRE-CONSTRUCTION MEETING DOCUMENTS

Project Name _____

Permit No. _____ Date of Authorization _____

Name of Operator _____

Prime Contractor _____

a. Preamble to Site Assessment and Inspections -The Following Information To Be Read By All Person's Involved in The Construction of Stormwater Related Activities:

The Operator agrees to have a qualified professional¹ conduct an assessment of the site prior to the commencement of construction² and certify in this inspection report that the appropriate erosion and sediment controls described in the SWPPP have been adequately installed or implemented to ensure overall preparedness of the site for the commencement of construction.

Prior to the commencement of construction, the Operator shall certify in this site logbook that the SWPPP has been prepared in accordance with the State's standards and meets all Federal, State and local erosion and sediment control requirements.

When construction starts, site inspections shall be conducted by the qualified professional at least every 7 calendar days and within 24 hours of the end of a storm event of 0.5 inches or greater (Construction Duration Inspections). The Operator shall maintain a record of all inspection reports in this site logbook. The site logbook shall be maintained on site and be made available to the permitting authorities upon request. The Operator shall post at the site, in a publicly accessible location, a summary of the site inspection activities on a monthly basis (Monthly Summary Report).

The operator shall also prepare a written summary of compliance with this general permit at a minimum frequency of every three months (Operator's Compliance Response Form), while coverage exists. The summary should address the status of achieving each component of the SWPPP.

Prior to filing the Notice of Termination or the end of permit term, the Operator shall have a qualified professional perform a final site inspection. The qualified professional shall certify that the site has undergone final stabilization³ using either vegetative or structural stabilization methods and that all temporary erosion and sediment controls (such as silt fencing) not needed for long-term erosion control have been removed. In addition, the Operator must identify and certify that all permanent structures described in the SWPPP have been constructed and provide the owner(s) with an operation and maintenance plan that ensures the structure(s) continuously functions as designed.

1 "Qualified Professional means a person knowledgeable in the principles and practice of erosion and sediment controls, such as a Certified Professional in Erosion and Sediment Control (CPESC), soil scientist, licensed engineer or someone working under the direction and supervision of a licensed engineer (person must have experience in the principles and practices of erosion and sediment control).

2 "Commencement of construction" means the initial removal of vegetation and disturbance of soils associated with clearing, grading or excavating activities or other construction activities.

3 "Final stabilization" means that all soil-disturbing activities at the site have been completed and a uniform, perennial vegetative cover with a density of eighty (80) percent has been established or equivalent stabilization measures (such as the use of mulches or geotextiles) have been employed on all unpaved areas and areas not covered by permanent structures.

b. Operators Certification

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. Further, I hereby certify that the SWPPP meets all Federal, State, and local erosion and sediment control requirements. I am aware that false statements made herein are punishable as a class A misdemeanor pursuant to Section 210.45 of the Penal Law. "

Name (please print): _____

Title _____ **Date:** _____

Address: _____

Phone: _____ **Email:** _____

Signature: _____

c. Qualified Professional's Credentials & Certification

"I hereby certify that I meet the criteria set forth in the General Permit to conduct site inspections for this project and that the appropriate erosion and sediment controls described in the SWPPP and as described in the following Pre-construction Site Assessment Checklist have been adequately installed or implemented, ensuring the overall preparedness of this site for the commencement of construction."

Name (please print): _____

Title _____ **Date:** _____

Address: _____

Phone: _____ **Email:** _____

Signature: _____

d. Pre-construction Site Assessment Checklist (NOTE: Provide comments below as necessary)

1. Notice of Intent, SWPPP, and Contractors Certification:

Yes No NA

☐ ☐ ☐ Has a Notice of Intent been filed with the NYS Department of Conservation?

☐ ☐ ☐ Is the SWPPP on-site? Where? _____

☐ ☐ ☐ Is the Plan current? What is the latest revision date? _____

☐ ☐ ☐ Is a copy of the NOI (with brief description) onsite? Where? _____

☐ ☐ ☐ Have all contractors involved with stormwater related activities signed a contractor's certification?

Pre-construction Site Assessment Checklist (continued)

2. Resource Protection

Yes No NA

- ☐ ☐ ☐ Are construction limits clearly flagged or fenced?
- ☐ ☐ ☐ Important trees and associated rooting zones, on-site septic system absorption fields, existing vegetated areas suitable for filter strips, especially in perimeter areas, have been flagged for protection.
- ☐ ☐ ☐ Creek crossings installed prior to land-disturbing activity, including clearing and blasting.

3. Surface Water Protection

Yes No NA

- ☐ ☐ ☐ Clean stormwater runoff has been diverted from areas to be disturbed.
- ☐ ☐ ☐ Bodies of water located either on site or in the vicinity of the site have been identified and protected.
- ☐ ☐ ☐ Appropriate practices to protect on-site or downstream surface water are installed.
- ☐ ☐ ☐ Are clearing and grading operations divided into areas <5 acres?

4. Stabilized Construction Entrance

Yes No NA

- ☐ ☐ ☐ A temporary construction entrance to capture mud and debris from construction vehicles before they enter the public highway has been installed.
- ☐ ☐ ☐ Other access areas (entrances, construction routes, equipment parking areas) are stabilized immediately as work takes place with gravel or other cover.
- ☐ ☐ ☐ Sediment tracked onto public streets is removed or cleaned on a regular basis.

5. Perimeter Sediment Controls

Yes No NA

- ☐ ☐ ☐ Silt fence material and installation comply with the standard drawing and specifications.
- ☐ ☐ ☐ Silt fences are installed at appropriate spacing intervals
- ☐ ☐ ☐ Sediment/detention basin was installed as first land disturbing activity.
- ☐ ☐ ☐ Sediment traps and barriers are installed.

6. Pollution Prevention for Waste and Hazardous Materials

Yes No NA

- ☐ ☐ ☐ The Operator or designated representative has been assigned to implement the spill prevention avoidance and response plan.
- ☐ ☐ ☐ The plan is contained in the SWPPP on page _____
- ☐ ☐ ☐ Appropriate materials to control spills are onsite. Where? _____

II. CONSTRUCTION DURATION INSPECTIONS

a. Directions:

Inspection Forms will be filled out during the entire construction phase of the project.

Required Elements:

(1) On a site map, indicate the extent of all disturbed site areas and drainage pathways. Indicate site areas that are expected to undergo initial disturbance or significant site work within the next 14-day period;

(2) Indicate on a site map all areas of the site that have undergone temporary or permanent stabilization;

(3) Indicate all disturbed site areas that have not undergone active site work during the previous 14-day period;

Inspect all sediment control practices and record the approximate degree of sediment accumulation as a percentage of sediment storage volume (for example, 10 percent, 20 percent, 50 percent);

(5) Inspect all erosion and sediment control practices and record all maintenance requirements such as verifying the integrity of barrier or diversion systems (earthen berms or silt fencing) and containment systems (sediment basins and sediment traps). Identify any evidence of rill or gully erosion occurring on slopes and any loss of stabilizing vegetation or seeding/mulching. Document any excessive deposition of sediment or ponding water along barrier or diversion systems. Record the depth of sediment within containment structures, any erosion near outlet and overflow structures, and verify the ability of rock filters around perforated riser pipes to pass water; and

(6) Immediately report to the Operator any deficiencies that are identified with the implementation of the SWPPP.

SITE PLAN/SKETCH

Inspector (print name)

Date of Inspection

Qualified Professional (print name)

Qualified Professional Signature

The above signed acknowledges that, to the best of his/her knowledge, all information provided on the forms is accurate and complete.

Maintaining Water Quality

Yes No NA

- ☐ ☐ ☐ Is there an increase in turbidity causing a substantial visible contrast to natural conditions?
- ☐ ☐ ☐ Is there residue from oil and floating substances, visible oil film, or globules or grease?
- ☐ ☐ ☐ All disturbance is within the limits of the approved plans.
- ☐ ☐ ☐ Have receiving lake/bay, stream, and/or wetland been impacted by silt from project?

Housekeeping

1. General Site Conditions

Yes No NA

- ☐ ☐ ☐ Is construction site litter and debris appropriately managed?
- ☐ ☐ ☐ Are facilities and equipment necessary for implementation of erosion and sediment control in working order and/or properly maintained?
- ☐ ☐ ☐ Is construction impacting the adjacent property?
- ☐ ☐ ☐ Is dust adequately controlled?

2. Temporary Stream Crossing

Yes No NA

- ☐ ☐ ☐ Maximum diameter pipes necessary to span creek without dredging are installed.
- ☐ ☐ ☐ Installed non-woven geotextile fabric beneath approaches.
- ☐ ☐ ☐ Is fill composed of aggregate (no earth or soil)?
- ☐ ☐ ☐ Rock on approaches is clean enough to remove mud from vehicles & prevent sediment from entering stream during high flow.

Runoff Control Practices

1. Excavation Dewatering

Yes No NA

- ☐ ☐ ☐ Upstream and downstream berms (sandbags, inflatable dams, etc.) are installed per plan.
- ☐ ☐ ☐ Clean water from upstream pool is being pumped to the downstream pool.
- ☐ ☐ ☐ Sediment laden water from work area is being discharged to a silt-trapping device.
- ☐ ☐ ☐ Constructed upstream berm with one-foot minimum freeboard.

2. Level Spreader

Yes No NA

- ☐ ☐ ☐ Installed per plan.
- ☐ ☐ ☐ Constructed on undisturbed soil, not on fill, receiving only clear, non-sediment laden flow.
- ☐ ☐ ☐ Flow sheets out of level spreader without erosion on downstream edge.

3. Interceptor Dikes and Swales

Yes No NA

- ☐ ☐ ☐ Installed per plan with minimum side slopes 2H:1V or flatter.
- ☐ ☐ ☐ Stabilized by geotextile fabric, seed, or mulch with no erosion occurring.
- ☐ ☐ ☐ Sediment-laden runoff directed to sediment trapping structure

CONSTRUCTION DURATION INSPECTIONS
Runoff Control Practices (continued)

Page 3 of _____

4. Stone Check Dam

Yes No NA

- ☐ ☐ ☐ Is channel stable? (flow is not eroding soil underneath or around the structure).
☐ ☐ ☐ Check is in good condition (rocks in place and no permanent pools behind the structure).
☐ ☐ ☐ Has accumulated sediment been removed?.

5. Rock Outlet Protection

Yes No NA

- ☐ ☐ ☐ Installed per plan.
☐ ☐ ☐ Installed concurrently with pipe installation.

Soil Stabilization

1. Topsoil and Spoil Stockpiles

Yes No NA

- ☐ ☐ ☐ Stockpiles are stabilized with vegetation and/or mulch.
☐ ☐ ☐ Sediment control is installed at the toe of the slope.

2. Revegetation

Yes No NA

- ☐ ☐ ☐ Temporary seedings and mulch have been applied to idle areas.
☐ ☐ ☐ 4 inches minimum of topsoil has been applied under permanent seedings

Sediment Control

1. Stabilized Construction Entrance

Yes No NA

- ☐ ☐ ☐ Stone is clean enough to effectively remove mud from vehicles.
☐ ☐ ☐ Installed per standards and specifications?
☐ ☐ ☐ Does all traffic use the stabilized entrance to enter and leave site?
☐ ☐ ☐ Is adequate drainage provided to prevent ponding at entrance?

2. Silt Fence

Yes No NA

- ☐ ☐ ☐ Installed on Contour, 10 feet from toe of slope (not across conveyance channels).
☐ ☐ ☐ Joints constructed by wrapping the two ends together for continuous support.
☐ ☐ ☐ Fabric buried 6 inches minimum.
☐ ☐ ☐ Posts are stable, fabric is tight and without rips or frayed areas.
Sediment accumulation is ____% of design capacity.

Sediment Control (continued)

3. Storm Drain Inlet Protection (Use for Stone & Block; Filter Fabric; Curb; or, Excavated practices)

Yes No NA

- ☐ ☐ ☐ Installed concrete blocks lengthwise so open ends face outward, not upward.
 - ☐ ☐ ☐ Placed wire screen between No. 3 crushed stone and concrete blocks.
 - ☐ ☐ ☐ Drainage area is 1 acre or less.
 - ☐ ☐ ☐ Excavated area is 900 cubic feet.
 - ☐ ☐ ☐ Excavated side slopes should be 2:1.
 - ☐ ☐ ☐ 2" x 4" frame is constructed and structurally sound.
 - ☐ ☐ ☐ Posts 3-foot maximum spacing between posts.
 - ☐ ☐ ☐ Fabric is embedded 1 to 1.5 feet below ground and secured to frame/posts with staples at max 8-inch spacing.
 - ☐ ☐ ☐ Posts are stable, fabric is tight and without rips or frayed areas.
- Sediment accumulation ____% of design capacity.

4. Temporary Sediment Trap

Yes No NA

- ☐ ☐ ☐ Outlet structure is constructed per the approved plan or drawing.
 - ☐ ☐ ☐ Geotextile fabric has been placed beneath rock fill.
- Sediment accumulation is ____% of design capacity.

5. Temporary Sediment Basin

Yes No NA

- ☐ ☐ ☐ Basin and outlet structure constructed per the approved plan.
 - ☐ ☐ ☐ Basin side slopes are stabilized with seed/mulch.
 - ☐ ☐ ☐ Drainage structure flushed and basin surface restored upon removal of sediment basin facility.
- Sediment accumulation is ____% of design capacity.

Note: Not all erosion and sediment control practices are included in this listing. Add additional pages to this list as required by site specific design.

Construction inspection checklists for post-development stormwater management practices can be found in Appendix F of the New York Stormwater Management Design Manual.

b. Modifications to the SWPPP (To be completed as described below)

1. There is a significant change in design, construction, operation, or maintenance which may have a significant effect on the potential for the discharge of pollutants to the waters of the United States and which has not otherwise been addressed in the SWPPP; or

a. Eliminating or significantly minimizing pollutants from sources identified in the SWPPP and as required by this permit; or

3. Additionally, the SWPPP shall be amended to identify any new contractor or subcontractor that will implement any measure of the SWPPP.

[illegible]

III. Monthly Summary of Site Inspection Activities

Name of Permitted Facility:	Today's Date:	Reporting Month:
Location:	Permit Identification #:	
Name and Telephone Number of Site Inspector:		

Date of Inspection	Regular / Rainfall based Inspection	Name of Inspector	Items of Concern

Owner/Operator Certification:

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that false statements made herein are punishable as a class A misdemeanor pursuant to Section 210.45 of the Penal Law."

_____ Signature of Permittee or Duly Authorized Representative	_____ Name of Permittee or Duly Authorized Representative	_____ date
---	--	---------------

Duly authorized representatives must have written authorization, submitted to DEC, to sign any permit documents.

APPENDIX D



Department of
Environmental
Conservation

NEW YORK STATE
DEPARTMENT OF ENVIRONMENTAL CONSERVATION

SPDES GENERAL PERMIT
FOR STORMWATER DISCHARGES

From

CONSTRUCTION ACTIVITY

Permit No. GP- 0-20-001

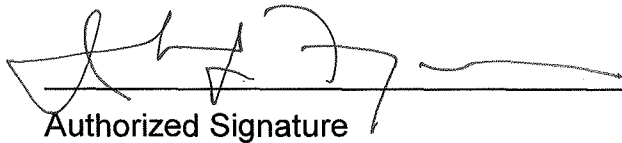
Issued Pursuant to Article 17, Titles 7, 8 and Article 70
of the Environmental Conservation Law

Effective Date: January 29, 2020

Expiration Date: January 28, 2025

John J. Ferguson

Chief Permit Administrator



Authorized Signature

1-23-20
Date

Address: NYS DEC
Division of Environmental Permits
625 Broadway, 4th Floor
Albany, N.Y. 12233-1750

PREFACE

Pursuant to Section 402 of the Clean Water Act (“CWA”), stormwater *discharges* from certain *construction activities* are unlawful unless they are authorized by a *National Pollutant Discharge Elimination System (“NPDES”)* permit or by a state permit program. New York administers the approved State Pollutant Discharge Elimination System (SPDES) program with permits issued in accordance with the New York State Environmental Conservation Law (ECL) Article 17, Titles 7, 8 and Article 70.

An *owner or operator* of a *construction activity* that is eligible for coverage under this permit must obtain coverage prior to the *commencement of construction activity*. Activities that fit the definition of “*construction activity*”, as defined under 40 CFR 122.26(b)(14)(x), (15)(i), and (15)(ii), constitute construction of a *point source* and therefore, pursuant to ECL section 17-0505 and 17-0701, the *owner or operator* must have coverage under a SPDES permit prior to *commencing construction activity*. The *owner or operator* cannot wait until there is an actual *discharge* from the *construction site* to obtain permit coverage.

***Note: The italicized words/phrases within this permit are defined in Appendix A.**

**NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION
SPDES GENERAL PERMIT FOR STORMWATER DISCHARGES FROM
CONSTRUCTION ACTIVITIES**

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Part 1. PERMIT COVERAGE AND LIMITATIONS

A. Permit Application

This permit authorizes stormwater *discharges* to *surface waters of the State* from the following *construction activities* identified within 40 CFR Parts 122.26(b)(14)(x), 122.26(b)(15)(i) and 122.26(b)(15)(ii), provided all of the eligibility provisions of this permit are met:

1. *Construction activities* involving soil disturbances of one (1) or more acres; including disturbances of less than one acre that are part of a *larger common plan of development or sale* that will ultimately disturb one or more acres of land; excluding *routine maintenance activity* that is performed to maintain the original line and grade, hydraulic capacity or original purpose of a facility;
2. *Construction activities* involving soil disturbances of less than one (1) acre where the Department has determined that a *SPDES* permit is required for stormwater *discharges* based on the potential for contribution to a violation of a *water quality standard* or for significant contribution of *pollutants* to *surface waters of the State*.
3. *Construction activities* located in the watershed(s) identified in Appendix D that involve soil disturbances between five thousand (5,000) square feet and one (1) acre of land.

B. Effluent Limitations Applicable to Discharges from Construction Activities

Discharges authorized by this permit must achieve, at a minimum, the effluent limitations in Part I.B.1. (a) – (f) of this permit. These limitations represent the degree of effluent reduction attainable by the application of best practicable technology currently available.

1. Erosion and Sediment Control Requirements - The *owner or operator* must select, design, install, implement and maintain control measures to *minimize* the *discharge of pollutants* and prevent a violation of the *water quality standards*. The selection, design, installation, implementation, and maintenance of these control measures must meet the non-numeric effluent limitations in Part I.B.1.(a) – (f) of this permit and be in accordance with the New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016, using sound engineering judgment. Where control measures are not designed in conformance with the design criteria included in the technical standard, the *owner or operator* must include in the *Stormwater Pollution Prevention Plan* (“SWPPP”) the reason(s) for the

deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the technical standard.

- a. **Erosion and Sediment Controls.** Design, install and maintain effective erosion and sediment controls to *minimize* the *discharge* of *pollutants* and prevent a violation of the *water quality standards*. At a minimum, such controls must be designed, installed and maintained to:
- (i) *Minimize* soil erosion through application of runoff control and soil stabilization control measure to *minimize pollutant discharges*;
 - (ii) Control stormwater *discharges*, including both peak flowrates and total stormwater volume, to *minimize* channel and *streambank* erosion and scour in the immediate vicinity of the *discharge* points;
 - (iii) *Minimize* the amount of soil exposed during *construction activity*;
 - (iv) *Minimize* the disturbance of *steep slopes*;
 - (v) *Minimize* sediment *discharges* from the site;
 - (vi) Provide and maintain *natural buffers* around surface waters, direct stormwater to vegetated areas and maximize stormwater infiltration to reduce *pollutant discharges*, unless *infeasible*;
 - (vii) *Minimize* soil compaction. Minimizing soil compaction is not required where the intended function of a specific area of the site dictates that it be compacted;
 - (viii) Unless *infeasible*, preserve a sufficient amount of topsoil to complete soil restoration and establish a uniform, dense vegetative cover; and
 - (ix) *Minimize* dust. On areas of exposed soil, *minimize* dust through the appropriate application of water or other dust suppression techniques to control the generation of pollutants that could be discharged from the site.
- b. **Soil Stabilization.** In areas where soil disturbance activity has temporarily or permanently ceased, the application of soil stabilization measures must be initiated by the end of the next business day and completed within fourteen (14) days from the date the current soil disturbance activity ceased. For construction sites that *directly discharge* to one of the 303(d) segments

listed in Appendix E or is located in one of the watersheds listed in Appendix C, the application of soil stabilization measures must be initiated by the end of the next business day and completed within seven (7) days from the date the current soil disturbance activity ceased. See Appendix A for definition of *Temporarily Ceased*.

- c. **Dewatering.** *Discharges* from *dewatering* activities, including *discharges* from *dewatering* of trenches and excavations, must be managed by appropriate control measures.
- d. **Pollution Prevention Measures.** Design, install, implement, and maintain effective pollution prevention measures to *minimize* the *discharge* of *pollutants* and prevent a violation of the *water quality standards*. At a minimum, such measures must be designed, installed, implemented and maintained to:
 - (i) *Minimize* the *discharge* of *pollutants* from equipment and vehicle washing, wheel wash water, and other wash waters. This applies to washing operations that use clean water only. Soaps, detergents and solvents cannot be used;
 - (ii) *Minimize* the exposure of building materials, building products, construction wastes, trash, landscape materials, fertilizers, pesticides, herbicides, detergents, sanitary waste, hazardous and toxic waste, and other materials present on the site to precipitation and to stormwater. Minimization of exposure is not required in cases where the exposure to precipitation and to stormwater will not result in a *discharge* of *pollutants*, or where exposure of a specific material or product poses little risk of stormwater contamination (such as final products and materials intended for outdoor use) ; and
 - (iii) Prevent the *discharge* of *pollutants* from spills and leaks and implement chemical spill and leak prevention and response procedures.
- e. **Prohibited Discharges.** The following *discharges* are prohibited:
 - (i) Wastewater from washout of concrete;
 - (ii) Wastewater from washout and cleanout of stucco, paint, form release oils, curing compounds and other construction materials;

- (iii) Fuels, oils, or other *pollutants* used in vehicle and equipment operation and maintenance;
 - (iv) Soaps or solvents used in vehicle and equipment washing; and
 - (v) Toxic or hazardous substances from a spill or other release.
- f. Surface Outlets. When discharging from basins and impoundments, the outlets shall be designed, constructed and maintained in such a manner that sediment does not leave the basin or impoundment and that erosion at or below the outlet does not occur.

C. Post-construction Stormwater Management Practice Requirements

1. The *owner or operator* of a *construction activity* that requires post-construction stormwater management practices pursuant to Part III.C. of this permit must select, design, install, and maintain the practices to meet the *performance criteria* in the New York State Stormwater Management Design Manual (“Design Manual”), dated January 2015, using sound engineering judgment. Where post-construction stormwater management practices (“SMPs”) are not designed in conformance with the *performance criteria* in the Design Manual, the *owner or operator* must include in the SWPPP the reason(s) for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the technical standard.
2. The *owner or operator* of a *construction activity* that requires post-construction stormwater management practices pursuant to Part III.C. of this permit must design the practices to meet the applicable *sizing criteria* in Part I.C.2.a., b., c. or d. of this permit.

a. Sizing Criteria for New Development

- (i) Runoff Reduction Volume (“RRv”): Reduce the total Water Quality Volume (“WQv”) by application of RR techniques and standard SMPs with RRv capacity. The total WQv shall be calculated in accordance with the criteria in Section 4.2 of the Design Manual.
- (ii) Minimum RRv and Treatment of Remaining Total WQv: Construction activities that cannot meet the criteria in Part I.C.2.a.(i) of this permit due to site limitations shall direct runoff from all newly constructed impervious areas to a RR technique or standard SMP with RRv capacity unless infeasible. The specific site limitations that prevent the reduction of 100% of the WQv shall be documented in the SWPPP.

For each impervious area that is not directed to a RR technique or standard SMP with RRV capacity, the SWPPP must include documentation which demonstrates that all options were considered and for each option explains why it is considered infeasible.

In no case shall the runoff reduction achieved from the newly constructed impervious areas be less than the Minimum RRV as calculated using the criteria in Section 4.3 of the Design Manual. The remaining portion of the total WQv that cannot be reduced shall be treated by application of standard SMPs.

- (iii) Channel Protection Volume (“Cpv”): Provide 24 hour extended detention of the post-developed 1-year, 24-hour storm event; remaining after runoff reduction. The Cpv requirement does not apply when:
 - (1) Reduction of the entire Cpv is achieved by application of runoff reduction techniques or infiltration systems, or
 - (2) The site discharges directly to tidal waters, or fifth order or larger streams.
- (iv) *Overbank* Flood Control Criteria (“Qp”): Requires storage to attenuate the post-development 10-year, 24-hour peak discharge rate (Qp) to predevelopment rates. The Qp requirement does not apply when:
 - (1) the site discharges directly to tidal waters or fifth order or larger streams, or
 - (2) A downstream analysis reveals that *overbank* control is not required.
- (v) Extreme Flood Control Criteria (“Qf”): Requires storage to attenuate the post-development 100-year, 24-hour peak discharge rate (Qf) to predevelopment rates. The Qf requirement does not apply when:
 - (1) the site discharges directly to tidal waters or fifth order or larger streams, or
 - (2) A downstream analysis reveals that *overbank* control is not required.

b. Sizing Criteria for New Development in Enhanced Phosphorus Removal Watershed

- (i) Runoff Reduction Volume (RRV): Reduce the total Water Quality Volume (WQv) by application of RR techniques and standard SMPs with RRV capacity. The total WQv is the runoff volume from the 1-year, 24 hour design storm over the post-developed watershed and shall be

calculated in accordance with the criteria in Section 10.3 of the Design Manual.

- (ii) Minimum RRv and Treatment of Remaining Total WQv: *Construction activities* that cannot meet the criteria in Part I.C.2.b.(i) of this permit due to *site limitations* shall direct runoff from all newly constructed *impervious areas* to a RR technique or standard SMP with RRv capacity unless *infeasible*. The specific *site limitations* that prevent the reduction of 100% of the WQv shall be documented in the SWPPP. For each *impervious area* that is not directed to a RR technique or standard SMP with RRv capacity, the SWPPP must include documentation which demonstrates that all options were considered and for each option explains why it is considered *infeasible*.

In no case shall the runoff reduction achieved from the newly constructed *impervious areas* be less than the Minimum RRv as calculated using the criteria in Section 10.3 of the Design Manual. The remaining portion of the total WQv that cannot be reduced shall be treated by application of standard SMPs.

- (iii) Channel Protection Volume (Cpv): Provide 24 hour extended detention of the post-developed 1-year, 24-hour storm event; remaining after runoff reduction. The Cpv requirement does not apply when:
 - (1) Reduction of the entire Cpv is achieved by application of runoff reduction techniques or infiltration systems, or
 - (2) The site *discharges* directly to tidal waters, or fifth order or larger streams.
- (iv) Overbank Flood Control Criteria (Qp): Requires storage to attenuate the post-development 10-year, 24-hour peak *discharge* rate (Qp) to predevelopment rates. The Qp requirement does not apply when:
 - (1) the site *discharges* directly to tidal waters or fifth order or larger streams, or
 - (2) A downstream analysis reveals that *overbank* control is not required.
- (v) Extreme Flood Control Criteria (Qf): Requires storage to attenuate the post-development 100-year, 24-hour peak *discharge* rate (Qf) to predevelopment rates. The Qf requirement does not apply when:
 - (1) the site *discharges* directly to tidal waters or fifth order or larger streams, or
 - (2) A downstream analysis reveals that *overbank* control is not required.

c. Sizing Criteria for Redevelopment Activity

- (i) Water Quality Volume (WQv): The WQv treatment objective for *redevelopment activity* shall be addressed by one of the following options. *Redevelopment activities* located in an Enhanced Phosphorus Removal Watershed (see Part III.B.3. and Appendix C of this permit) shall calculate the WQv in accordance with Section 10.3 of the Design Manual. All other *redevelopment activities* shall calculate the WQv in accordance with Section 4.2 of the Design Manual.
 - (1) Reduce the existing *impervious cover* by a minimum of 25% of the total disturbed, *impervious area*. The Soil Restoration criteria in Section 5.1.6 of the Design Manual must be applied to all newly created pervious areas, or
 - (2) Capture and treat a minimum of 25% of the WQv from the disturbed, *impervious area* by the application of standard SMPs; or reduce 25% of the WQv from the disturbed, *impervious area* by the application of RR techniques or standard SMPs with RRV capacity., or
 - (3) Capture and treat a minimum of 75% of the WQv from the disturbed, *impervious area* as well as any additional runoff from tributary areas by application of the alternative practices discussed in Sections 9.3 and 9.4 of the Design Manual., or
 - (4) Application of a combination of 1, 2 and 3 above that provide a weighted average of at least two of the above methods. Application of this method shall be in accordance with the criteria in Section 9.2.1(B) (IV) of the Design Manual.

If there is an existing post-construction stormwater management practice located on the site that captures and treats runoff from the *impervious area* that is being disturbed, the WQv treatment option selected must, at a minimum, provide treatment equal to the treatment that was being provided by the existing practice(s) if that treatment is greater than the treatment required by options 1 – 4 above.

- (ii) Channel Protection Volume (Cpv): Not required if there are no changes to hydrology that increase the *discharge* rate from the project site.
- (iii) Overbank Flood Control Criteria (Qp): Not required if there are no changes to hydrology that increase the *discharge* rate from the project site.
- (iv) Extreme Flood Control Criteria (Qf): Not required if there are no changes to hydrology that increase the *discharge* rate from the project site

d. Sizing Criteria for Combination of Redevelopment Activity and New Development

Construction projects that include both New Development and Redevelopment Activity shall provide post-construction stormwater management controls that meet the sizing criteria calculated as an aggregate of the Sizing Criteria in Part I.C.2.a. or b. of this permit for the New Development portion of the project and Part I.C.2.c of this permit for Redevelopment Activity portion of the project.

D. Maintaining Water Quality

The Department expects that compliance with the conditions of this permit will control *discharges* necessary to meet applicable *water quality standards*. It shall be a violation of the *ECL* for any discharge to either cause or contribute to a violation of *water quality standards* as contained in Parts 700 through 705 of Title 6 of the Official Compilation of Codes, Rules and Regulations of the State of New York, such as:

1. There shall be no increase in turbidity that will cause a substantial visible contrast to natural conditions;
2. There shall be no increase in suspended, colloidal or settleable solids that will cause deposition or impair the waters for their best usages; and
3. There shall be no residue from oil and floating substances, nor visible oil film, nor globules of grease.

If there is evidence indicating that the stormwater *discharges* authorized by this permit are causing, have the reasonable potential to cause, or are contributing to a violation of the *water quality standards*; the *owner or operator* must take appropriate corrective action in accordance with Part IV.C.5. of this general permit and document in accordance with Part IV.C.4. of this general permit. To address the *water quality standard* violation the *owner or operator* may need to provide additional information, include and implement appropriate controls in the SWPPP to correct the problem, or obtain an individual SPDES permit.

If there is evidence indicating that despite compliance with the terms and conditions of this general permit it is demonstrated that the stormwater *discharges* authorized by this permit are causing or contributing to a violation of *water quality standards*, or if the Department determines that a modification of the permit is necessary to prevent a violation of *water quality standards*, the authorized *discharges* will no longer be eligible for coverage under this permit. The Department may require the *owner or operator* to obtain an individual SPDES permit to continue discharging.

E. Eligibility Under This General Permit

1. This permit may authorize all *discharges* of stormwater from *construction activity to surface waters of the State* and *groundwaters* except for ineligible *discharges* identified under subparagraph F. of this Part.
2. Except for non-stormwater *discharges* explicitly listed in the next paragraph, this permit only authorizes stormwater *discharges*; including stormwater runoff, snowmelt runoff, and surface runoff and drainage, from *construction activities*.
3. Notwithstanding paragraphs E.1 and E.2 above, the following non-stormwater discharges are authorized by this permit: those listed in 6 NYCRR 750-1.2(a)(29)(vi), with the following exception: “Discharges from firefighting activities are authorized only when the firefighting activities are emergencies/unplanned”; waters to which other components have not been added that are used to control dust in accordance with the SWPPP; and uncontaminated *discharges* from *construction site* de-watering operations. All non-stormwater discharges must be identified in the SWPPP. Under all circumstances, the *owner or operator* must still comply with *water quality standards* in Part I.D of this permit.
4. The *owner or operator* must maintain permit eligibility to *discharge* under this permit. Any *discharges* that are not compliant with the eligibility conditions of this permit are not authorized by the permit and the *owner or operator* must either apply for a separate permit to cover those ineligible *discharges* or take steps necessary to make the *discharge* eligible for coverage.

F. Activities Which Are Ineligible for Coverage Under This General Permit

All of the following are **not** authorized by this permit:

1. *Discharges after construction activities* have been completed and the site has undergone *final stabilization*;
2. *Discharges* that are mixed with sources of non-stormwater other than those expressly authorized under subsection E.3. of this Part and identified in the SWPPP required by this permit;
3. *Discharges* that are required to obtain an individual SPDES permit or another SPDES general permit pursuant to Part VII.K. of this permit;
4. *Construction activities or discharges from construction activities* that may adversely affect an *endangered or threatened species* unless the *owner or*

operator has obtained a permit issued pursuant to 6 NYCRR Part 182 for the project or the Department has issued a letter of non-jurisdiction for the project. All documentation necessary to demonstrate eligibility shall be maintained on site in accordance with Part II.D.2 of this permit;

5. *Discharges* which either cause or contribute to a violation of *water quality standards* adopted pursuant to the *ECL* and its accompanying regulations;
6. *Construction activities* for residential, commercial and institutional projects:
 - a. Where the *discharges* from the *construction activities* are tributary to waters of the state classified as AA or AA-s; and
 - b. Which are undertaken on land with no existing *impervious cover*; and
 - c. Which disturb one (1) or more acres of land designated on the current United States Department of Agriculture ("USDA") Soil Survey as Soil Slope Phase "D", (provided the map unit name is inclusive of slopes greater than 25%), or Soil Slope Phase "E" or "F" (regardless of the map unit name), or a combination of the three designations.
7. *Construction activities* for linear transportation projects and linear utility projects:
 - a. Where the *discharges* from the *construction activities* are tributary to waters of the state classified as AA or AA-s; and
 - b. Which are undertaken on land with no existing *impervious cover*; and
 - c. Which disturb two (2) or more acres of land designated on the current USDA Soil Survey as Soil Slope Phase "D" (provided the map unit name is inclusive of slopes greater than 25%), or Soil Slope Phase "E" or "F" (regardless of the map unit name), or a combination of the three designations.

8. *Construction activities* that have the potential to affect an *historic property*, unless there is documentation that such impacts have been resolved. The following documentation necessary to demonstrate eligibility with this requirement shall be maintained on site in accordance with Part II.D.2 of this permit and made available to the Department in accordance with Part VII.F of this permit:
- a. Documentation that the *construction activity* is not within an archeologically sensitive area indicated on the sensitivity map, and that the *construction activity* is not located on or immediately adjacent to a property listed or determined to be eligible for listing on the National or State Registers of Historic Places, and that there is no new permanent building on the *construction site* within the following distances from a building, structure, or object that is more than 50 years old, or if there is such a new permanent building on the *construction site* within those parameters that NYS Office of Parks, Recreation and Historic Preservation (OPRHP), a Historic Preservation Commission of a Certified Local Government, or a qualified preservation professional has determined that the building, structure, or object more than 50 years old is not historically/archeologically significant.
 - 1-5 acres of disturbance - 20 feet
 - 5-20 acres of disturbance - 50 feet
 - 20+ acres of disturbance - 100 feet, or
 - b. DEC consultation form sent to OPRHP, and copied to the NYS DEC Agency Historic Preservation Officer (APO), and
 - (i) the State Environmental Quality Review (SEQR) Environmental Assessment Form (EAF) with a negative declaration or the Findings Statement, with documentation of OPRHP's agreement with the resolution; or
 - (ii) documentation from OPRHP that the *construction activity* will result in No Impact; or
 - (iii) documentation from OPRHP providing a determination of No Adverse Impact; or
 - (iv) a Letter of Resolution signed by the owner/operator, OPRHP and the DEC APO which allows for this *construction activity* to be eligible for coverage under the general permit in terms of the State Historic Preservation Act (SHPA); or
 - c. Documentation of satisfactory compliance with Section 106 of the National Historic Preservation Act for a coterminous project area:

- (i) No Affect
- (ii) No Adverse Affect
- (iii) Executed Memorandum of Agreement, or

d. Documentation that:

- (i) SHPA Section 14.09 has been completed by NYS DEC or another state agency.

9. *Discharges from construction activities* that are subject to an existing SPDES individual or general permit where a SPDES permit for *construction activity* has been terminated or denied; or where the *owner or operator* has failed to renew an expired individual permit.

Part II. PERMIT COVERAGE

A. How to Obtain Coverage

1. An *owner or operator* of a *construction activity* that is not subject to the requirements of a regulated, traditional land use control MS4 must first prepare a SWPPP in accordance with all applicable requirements of this permit and then submit a completed Notice of Intent (NOI) to the Department to be authorized to discharge under this permit.
2. An *owner or operator* of a *construction activity* that is subject to the requirements of a *regulated, traditional land use control MS4* must first prepare a SWPPP in accordance with all applicable requirements of this permit and then have the SWPPP reviewed and accepted by the *regulated, traditional land use control MS4* prior to submitting the NOI to the Department. The *owner or operator* shall have the “MS4 SWPPP Acceptance” form signed in accordance with Part VII.H., and then submit that form along with a completed NOI to the Department.
3. The requirement for an *owner or operator* to have its SWPPP reviewed and accepted by the *regulated, traditional land use control MS4* prior to submitting the NOI to the Department does not apply to an *owner or operator* that is obtaining permit coverage in accordance with the requirements in Part II.F. (Change of Owner or Operator) or where the *owner or operator* of the *construction activity* is the *regulated, traditional land use control MS4*. This exemption does not apply to *construction activities* subject to the New York City Administrative Code.

B. Notice of Intent (NOI) Submittal

1. Prior to December 21, 2020, an owner or operator shall use either the electronic (eNOI) or paper version of the NOI that the Department prepared. Both versions of the NOI are located on the Department's website (<http://www.dec.ny.gov/>). The paper version of the NOI shall be signed in accordance with Part VII.H. of this permit and submitted to the following address:

**NOTICE OF INTENT
NYS DEC, Bureau of Water Permits
625 Broadway, 4th Floor
Albany, New York 12233-3505**

2. Beginning December 21, 2020 and in accordance with EPA's 2015 NPDES Electronic Reporting Rule (40 CFR Part 127), the *owner or operator* must submit the NOI electronically using the *Department's* online NOI.
3. The *owner or operator* shall have the SWPPP preparer sign the "SWPPP Preparer Certification" statement on the NOI prior to submitting the form to the Department.
4. As of the date the NOI is submitted to the Department, the *owner or operator* shall make the NOI and SWPPP available for review and copying in accordance with the requirements in Part VII.F. of this permit.

C. Permit Authorization

1. An *owner or operator* shall not *commence construction activity* until their authorization to *discharge* under this permit goes into effect.
2. Authorization to *discharge* under this permit will be effective when the *owner or operator* has satisfied all of the following criteria:
 - a. project review pursuant to the State Environmental Quality Review Act ("SEQRA") have been satisfied, when SEQRA is applicable. See the Department's website (<http://www.dec.ny.gov/>) for more information,
 - b. where required, all necessary Department permits subject to the *Uniform Procedures Act* ("UPA") (see 6 NYCRR Part 621), or the equivalent from another New York State agency, have been obtained, unless otherwise notified by the Department pursuant to 6 NYCRR 621.3(a)(4). *Owners or operators of construction activities* that are required to obtain UPA permits

must submit a preliminary SWPPP to the appropriate DEC Permit Administrator at the Regional Office listed in Appendix F at the time all other necessary *UPA* permit applications are submitted. The preliminary SWPPP must include sufficient information to demonstrate that the *construction activity* qualifies for authorization under this permit,

- c. the final SWPPP has been prepared, and
 - d. a complete NOI has been submitted to the Department in accordance with the requirements of this permit.
3. An *owner or operator* that has satisfied the requirements of Part II.C.2 above will be authorized to *discharge* stormwater from their *construction activity* in accordance with the following schedule:
- a. For *construction activities* that are not subject to the requirements of a *regulated, traditional land use control MS4*:
 - (i) Five (5) business days from the date the Department receives a complete electronic version of the NOI (eNOI) for *construction activities* with a SWPPP that has been prepared in conformance with the design criteria in the technical standard referenced in Part III.B.1 and the *performance criteria* in the technical standard referenced in Parts III.B., 2 or 3, for *construction activities* that require post-construction stormwater management practices pursuant to Part III.C.; or
 - (ii) Sixty (60) business days from the date the Department receives a complete NOI (electronic or paper version) for *construction activities* with a SWPPP that has not been prepared in conformance with the design criteria in technical standard referenced in Part III.B.1. or, for *construction activities* that require post-construction stormwater management practices pursuant to Part III.C., the *performance criteria* in the technical standard referenced in Parts III.B., 2 or 3, or;
 - (iii) Ten (10) business days from the date the Department receives a complete paper version of the NOI for *construction activities* with a SWPPP that has been prepared in conformance with the design criteria in the technical standard referenced in Part III.B.1 and the *performance criteria* in the technical standard referenced in Parts III.B., 2 or 3, for *construction activities* that require post-construction stormwater management practices pursuant to Part III.C.

- b. For *construction activities* that are subject to the requirements of a *regulated, traditional land use control MS4*:
 - (i) Five (5) business days from the date the Department receives both a complete electronic version of the NOI (eNOI) and signed “MS4 SWPPP Acceptance” form, or
 - (ii) Ten (10) business days from the date the Department receives both a complete paper version of the NOI and signed “MS4 SWPPP Acceptance” form.
4. Coverage under this permit authorizes stormwater *discharges* from only those areas of disturbance that are identified in the NOI. If an *owner or operator* wishes to have stormwater *discharges* from future or additional areas of disturbance authorized, they must submit a new NOI that addresses that phase of the development, unless otherwise notified by the Department. The *owner or operator* shall not *commence construction activity* on the future or additional areas until their authorization to *discharge* under this permit goes into effect in accordance with Part II.C. of this permit.

D. General Requirements For Owners or Operators With Permit Coverage

1. The *owner or operator* shall ensure that the provisions of the SWPPP are implemented from the *commencement of construction activity* until all areas of disturbance have achieved *final stabilization* and the Notice of Termination (“NOT”) has been submitted to the Department in accordance with Part V. of this permit. This includes any changes made to the SWPPP pursuant to Part III.A.4. of this permit.
2. The *owner or operator* shall maintain a copy of the General Permit (GP-0-20-001), NOI, *NOI Acknowledgment Letter*, SWPPP, MS4 SWPPP Acceptance form, inspection reports, responsible contractor’s or subcontractor’s certification statement (see Part III.A.6.), and all documentation necessary to demonstrate eligibility with this permit at the *construction site* until all disturbed areas have achieved *final stabilization* and the NOT has been submitted to the Department. The documents must be maintained in a secure location, such as a job trailer, on-site construction office, or mailbox with lock. The secure location must be accessible during normal business hours to an individual performing a compliance inspection.
3. The *owner or operator* of a *construction activity* shall not disturb greater than five (5) acres of soil at any one time without prior written authorization from the Department or, in areas under the jurisdiction of a *regulated, traditional land*

use control MS4, the regulated, traditional land use control MS4 (provided the regulated, traditional land use control MS4 is not the owner or operator of the construction activity). At a minimum, the owner or operator must comply with the following requirements in order to be authorized to disturb greater than five (5) acres of soil at any one time:

- a. The *owner or operator* shall have a *qualified inspector* conduct **at least** two (2) site inspections in accordance with Part IV.C. of this permit every seven (7) calendar days, for as long as greater than five (5) acres of soil remain disturbed. The two (2) inspections shall be separated by a minimum of two (2) full calendar days.
 - b. In areas where soil disturbance activity has temporarily or permanently ceased, the application of soil stabilization measures must be initiated by the end of the next business day and completed within seven (7) days from the date the current soil disturbance activity ceased. The soil stabilization measures selected shall be in conformance with the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016.
 - c. The *owner or operator* shall prepare a phasing plan that defines maximum disturbed area per phase and shows required cuts and fills.
 - d. The *owner or operator* shall install any additional site-specific practices needed to protect water quality.
 - e. The *owner or operator* shall include the requirements above in their SWPPP.
4. In accordance with statute, regulations, and the terms and conditions of this permit, the Department may suspend or revoke an *owner's or operator's* coverage under this permit at any time if the Department determines that the SWPPP does not meet the permit requirements or consistent with Part VII.K..
 5. Upon a finding of significant non-compliance with the practices described in the SWPPP or violation of this permit, the Department may order an immediate stop to all activity at the site until the non-compliance is remedied. The stop work order shall be in writing, describe the non-compliance in detail, and be sent to the *owner or operator*.
 6. For *construction activities* that are subject to the requirements of a *regulated, traditional land use control MS4*, the *owner or operator* shall notify the

regulated, traditional land use control MS4 in writing of any planned amendments or modifications to the post-construction stormwater management practice component of the SWPPP required by Part III.A. 4. and 5. of this permit. Unless otherwise notified by the *regulated, traditional land use control MS4*, the *owner or operator* shall have the SWPPP amendments or modifications reviewed and accepted by the *regulated, traditional land use control MS4* prior to commencing construction of the post-construction stormwater management practice.

E. Permit Coverage for Discharges Authorized Under GP-0-15-002

1. Upon renewal of SPDES General Permit for Stormwater Discharges from *Construction Activity* (Permit No. GP-0-15-002), an *owner or operator* of a *construction activity* with coverage under GP-0-15-002, as of the effective date of GP- 0-20-001, shall be authorized to *discharge* in accordance with GP- 0-20-001, unless otherwise notified by the Department.

An *owner or operator* may continue to implement the technical/design components of the post-construction stormwater management controls provided that such design was done in conformance with the technical standards in place at the time of initial project authorization. However, they must comply with the other, non-design provisions of GP-0-20-001.

F. Change of Owner or Operator

1. When property ownership changes or when there is a change in operational control over the construction plans and specifications, the original *owner or operator* must notify the new *owner or operator*, in writing, of the requirement to obtain permit coverage by submitting a NOI with the Department. For *construction activities* subject to the requirements of a *regulated, traditional land use control MS4*, the original *owner or operator* must also notify the MS4, in writing, of the change in ownership at least 30 calendar days prior to the change in ownership.
2. Once the new *owner or operator* obtains permit coverage, the original *owner or operator* shall then submit a completed NOT with the name and permit identification number of the new *owner or operator* to the Department at the address in Part II.B.1. of this permit. If the original *owner or operator* maintains ownership of a portion of the *construction activity* and will disturb soil, they must maintain their coverage under the permit.
3. Permit coverage for the new *owner or operator* will be effective as of the date the Department receives a complete NOI, provided the original *owner or*

operator was not subject to a sixty (60) business day authorization period that has not expired as of the date the Department receives the NOI from the new *owner or operator*.

Part III. STORMWATER POLLUTION PREVENTION PLAN (SWPPP)

A. General SWPPP Requirements

1. A SWPPP shall be prepared and implemented by the *owner or operator* of each *construction activity* covered by this permit. The SWPPP must document the selection, design, installation, implementation and maintenance of the control measures and practices that will be used to meet the effluent limitations in Part I.B. of this permit and where applicable, the post-construction stormwater management practice requirements in Part I.C. of this permit. The SWPPP shall be prepared prior to the submittal of the NOI. The NOI shall be submitted to the Department prior to the *commencement of construction activity*. A copy of the completed, final NOI shall be included in the SWPPP.
2. The SWPPP shall describe the erosion and sediment control practices and where required, post-construction stormwater management practices that will be used and/or constructed to reduce the *pollutants* in stormwater *discharges* and to assure compliance with the terms and conditions of this permit. In addition, the SWPPP shall identify potential sources of pollution which may reasonably be expected to affect the quality of stormwater *discharges*.
3. All SWPPPs that require the post-construction stormwater management practice component shall be prepared by a *qualified professional* that is knowledgeable in the principles and practices of stormwater management and treatment.
4. The *owner or operator* must keep the SWPPP current so that it at all times accurately documents the erosion and sediment controls practices that are being used or will be used during construction, and all post-construction stormwater management practices that will be constructed on the site. At a minimum, the *owner or operator* shall amend the SWPPP, including construction drawings:
 - a. whenever the current provisions prove to be ineffective in minimizing *pollutants* in stormwater *discharges* from the site;

- b. whenever there is a change in design, construction, or operation at the *construction site* that has or could have an effect on the *discharge* of *pollutants*;
 - c. to address issues or deficiencies identified during an inspection by the *qualified inspector*, the Department or other regulatory authority; and
 - d. to document the final construction conditions.
5. The Department may notify the *owner or operator* at any time that the SWPPP does not meet one or more of the minimum requirements of this permit. The notification shall be in writing and identify the provisions of the SWPPP that require modification. Within fourteen (14) calendar days of such notification, or as otherwise indicated by the Department, the *owner or operator* shall make the required changes to the SWPPP and submit written notification to the Department that the changes have been made. If the *owner or operator* does not respond to the Department's comments in the specified time frame, the Department may suspend the *owner's or operator's* coverage under this permit or require the *owner or operator* to obtain coverage under an individual SPDES permit in accordance with Part II.D.4. of this permit.
6. Prior to the *commencement of construction activity*, the *owner or operator* must identify the contractor(s) and subcontractor(s) that will be responsible for installing, constructing, repairing, replacing, inspecting and maintaining the erosion and sediment control practices included in the SWPPP; and the contractor(s) and subcontractor(s) that will be responsible for constructing the post-construction stormwater management practices included in the SWPPP. The *owner or operator* shall have each of the contractors and subcontractors identify at least one person from their company that will be responsible for implementation of the SWPPP. This person shall be known as the *trained contractor*. The *owner or operator* shall ensure that at least one *trained contractor* is on site on a daily basis when soil disturbance activities are being performed.

The *owner or operator* shall have each of the contractors and subcontractors identified above sign a copy of the following certification statement below before they commence any *construction activity*:

"I hereby certify under penalty of law that I understand and agree to comply with the terms and conditions of the SWPPP and agree to implement any corrective actions identified by the *qualified inspector* during a site inspection. I also understand that the *owner or operator* must comply with

the terms and conditions of the most current version of the New York State Pollutant Discharge Elimination System ("SPDES") general permit for stormwater *discharges* from *construction activities* and that it is unlawful for any person to cause or contribute to a violation of *water quality standards*. Furthermore, I am aware that there are significant penalties for submitting false information, that I do not believe to be true, including the possibility of fine and imprisonment for knowing violations"

In addition to providing the certification statement above, the certification page must also identify the specific elements of the SWPPP that each contractor and subcontractor will be responsible for and include the name and title of the person providing the signature; the name and title of the *trained contractor* responsible for SWPPP implementation; the name, address and telephone number of the contracting firm; the address (or other identifying description) of the site; and the date the certification statement is signed. The *owner or operator* shall attach the certification statement(s) to the copy of the SWPPP that is maintained at the *construction site*. If new or additional contractors are hired to implement measures identified in the SWPPP after construction has commenced, they must also sign the certification statement and provide the information listed above.

7. For projects where the Department requests a copy of the SWPPP or inspection reports, the *owner or operator* shall submit the documents in both electronic (PDF only) and paper format within five (5) business days, unless otherwise notified by the Department.

B. Required SWPPP Contents

1. Erosion and sediment control component - All SWPPPs prepared pursuant to this permit shall include erosion and sediment control practices designed in conformance with the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016. Where erosion and sediment control practices are not designed in conformance with the design criteria included in the technical standard, the *owner or operator* must demonstrate *equivalence* to the technical standard. At a minimum, the erosion and sediment control component of the SWPPP shall include the following:
 - a. Background information about the scope of the project, including the location, type and size of project

- b. A site map/construction drawing(s) for the project, including a general location map. At a minimum, the site map shall show the total site area; all improvements; areas of disturbance; areas that will not be disturbed; existing vegetation; on-site and adjacent off-site surface water(s); floodplain/floodway boundaries; wetlands and drainage patterns that could be affected by the *construction activity*; existing and final contours ; locations of different soil types with boundaries; material, waste, borrow or equipment storage areas located on adjacent properties; and location(s) of the stormwater *discharge(s)*;
- c. A description of the soil(s) present at the site, including an identification of the Hydrologic Soil Group (HSG);
- d. A construction phasing plan and sequence of operations describing the intended order of *construction activities*, including clearing and grubbing, excavation and grading, utility and infrastructure installation and any other activity at the site that results in soil disturbance;
- e. A description of the minimum erosion and sediment control practices to be installed or implemented for each *construction activity* that will result in soil disturbance. Include a schedule that identifies the timing of initial placement or implementation of each erosion and sediment control practice and the minimum time frames that each practice should remain in place or be implemented;
- f. A temporary and permanent soil stabilization plan that meets the requirements of this general permit and the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016, for each stage of the project, including initial land clearing and grubbing to project completion and achievement of *final stabilization*;
- g. A site map/construction drawing(s) showing the specific location(s), size(s), and length(s) of each erosion and sediment control practice;
- h. The dimensions, material specifications, installation details, and operation and maintenance requirements for all erosion and sediment control practices. Include the location and sizing of any temporary sediment basins and structural practices that will be used to divert flows from exposed soils;
- i. A maintenance inspection schedule for the contractor(s) identified in Part III.A.6. of this permit, to ensure continuous and effective operation of the erosion and sediment control practices. The maintenance inspection

schedule shall be in accordance with the requirements in the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016;

- j. A description of the pollution prevention measures that will be used to control litter, construction chemicals and construction debris from becoming a *pollutant* source in the stormwater *discharges*;
 - k. A description and location of any stormwater *discharges* associated with industrial activity other than construction at the site, including, but not limited to, stormwater *discharges* from asphalt plants and concrete plants located on the *construction site*; and
 - l. Identification of any elements of the design that are not in conformance with the design criteria in the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016. Include the reason for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the technical standard.
2. Post-construction stormwater management practice component – The *owner or operator* of any construction project identified in Table 2 of Appendix B as needing post-construction stormwater management practices shall prepare a SWPPP that includes practices designed in conformance with the applicable *sizing criteria* in Part I.C.2.a., c. or d. of this permit and the *performance criteria* in the technical standard, New York State Stormwater Management Design Manual dated January 2015

Where post-construction stormwater management practices are not designed in conformance with the *performance criteria* in the technical standard, the *owner or operator* must include in the SWPPP the reason(s) for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the technical standard.

The post-construction stormwater management practice component of the SWPPP shall include the following:

- a. Identification of all post-construction stormwater management practices to be constructed as part of the project. Include the dimensions, material specifications and installation details for each post-construction stormwater management practice;

- b. A site map/construction drawing(s) showing the specific location and size of each post-construction stormwater management practice;
- c. A Stormwater Modeling and Analysis Report that includes:
 - (i) Map(s) showing pre-development conditions, including watershed/subcatchments boundaries, flow paths/routing, and design points;
 - (ii) Map(s) showing post-development conditions, including watershed/subcatchments boundaries, flow paths/routing, design points and post-construction stormwater management practices;
 - (iii) Results of stormwater modeling (i.e. hydrology and hydraulic analysis) for the required storm events. Include supporting calculations (model runs), methodology, and a summary table that compares pre and post-development runoff rates and volumes for the different storm events;
 - (iv) Summary table, with supporting calculations, which demonstrates that each post-construction stormwater management practice has been designed in conformance with the *sizing criteria* included in the Design Manual;
 - (v) Identification of any *sizing criteria* that is not required based on the requirements included in Part I.C. of this permit; and
 - (vi) Identification of any elements of the design that are not in conformance with the *performance criteria* in the Design Manual. Include the reason(s) for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the Design Manual;
- d. Soil testing results and locations (test pits, borings);
- e. Infiltration test results, when required; and
- f. An operations and maintenance plan that includes inspection and maintenance schedules and actions to ensure continuous and effective operation of each post-construction stormwater management practice. The plan shall identify the entity that will be responsible for the long term operation and maintenance of each practice.

3. Enhanced Phosphorus Removal Standards - All construction projects identified in Table 2 of Appendix B that are located in the watersheds identified in Appendix C shall prepare a SWPPP that includes post-construction stormwater management practices designed in conformance with the applicable *sizing criteria* in Part I.C.2. b., c. or d. of this permit and the *performance criteria*, Enhanced Phosphorus Removal Standards included in the Design Manual. At a minimum, the post-construction stormwater management practice component of the SWPPP shall include items 2.a - 2.f. above.

C. Required SWPPP Components by Project Type

Unless otherwise notified by the Department, *owners or operators of construction activities* identified in Table 1 of Appendix B are required to prepare a SWPPP that only includes erosion and sediment control practices designed in conformance with Part III.B.1 of this permit. *Owners or operators of the construction activities* identified in Table 2 of Appendix B shall prepare a SWPPP that also includes post-construction stormwater management practices designed in conformance with Part III.B.2 or 3 of this permit.

Part IV. INSPECTION AND MAINTENANCE REQUIREMENTS

A. General Construction Site Inspection and Maintenance Requirements

1. The *owner or operator* must ensure that all erosion and sediment control practices (including pollution prevention measures) and all post-construction stormwater management practices identified in the SWPPP are inspected and maintained in accordance with Part IV.B. and C. of this permit.
2. The terms of this permit shall not be construed to prohibit the State of New York from exercising any authority pursuant to the ECL, common law or federal law, or prohibit New York State from taking any measures, whether civil or criminal, to prevent violations of the laws of the State of New York or protect the public health and safety and/or the environment.

B. Contractor Maintenance Inspection Requirements

1. The *owner or operator* of each *construction activity* identified in Tables 1 and 2 of Appendix B shall have a *trained contractor* inspect the erosion and sediment control practices and pollution prevention measures being implemented within the active work area daily to ensure that they are being maintained in effective operating condition at all times. If deficiencies are identified, the contractor shall

begin implementing corrective actions within one business day and shall complete the corrective actions in a reasonable time frame.

2. For construction sites where soil disturbance activities have been temporarily suspended (e.g. winter shutdown) and *temporary stabilization* measures have been applied to all disturbed areas, the *trained contractor* can stop conducting the maintenance inspections. The *trained contractor* shall begin conducting the maintenance inspections in accordance with Part IV.B.1. of this permit as soon as soil disturbance activities resume.
3. For construction sites where soil disturbance activities have been shut down with partial project completion, the *trained contractor* can stop conducting the maintenance inspections if all areas disturbed as of the project shutdown date have achieved *final stabilization* and all post-construction stormwater management practices required for the completed portion of the project have been constructed in conformance with the SWPPP and are operational.

C. Qualified Inspector Inspection Requirements

The *owner or operator* shall have a *qualified inspector* conduct site inspections in conformance with the following requirements:

[Note: The *trained contractor* identified in Part III.A.6. and IV.B. of this permit **cannot** conduct the *qualified inspector* site inspections unless they meet the *qualified inspector* qualifications included in Appendix A. In order to perform these inspections, the *trained contractor* would have to be a:

- licensed Professional Engineer,
- Certified Professional in Erosion and Sediment Control (CPESC),
- New York State Erosion and Sediment Control Certificate Program holder
- Registered Landscape Architect, or
- someone working under the direct supervision of, and at the same company as, the licensed Professional Engineer or Registered Landscape Architect, provided they have received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity].

1. A *qualified inspector* shall conduct site inspections for all *construction activities* identified in Tables 1 and 2 of Appendix B, with the exception of:
 - a. the construction of a single family residential subdivision with 25% or less *impervious cover* at total site build-out that involves a soil disturbance of one (1) or more acres of land but less than five (5) acres and is not located

in one of the watersheds listed in Appendix C and not directly discharging to one of the 303(d) segments listed in Appendix E;

- b. the construction of a single family home that involves a soil disturbance of one (1) or more acres of land but less than five (5) acres and is not located in one of the watersheds listed in Appendix C and not directly discharging to one of the 303(d) segments listed in Appendix E;
 - c. construction on agricultural property that involves a soil disturbance of one (1) or more acres of land but less than five (5) acres; and
 - d. *construction activities* located in the watersheds identified in Appendix D that involve soil disturbances between five thousand (5,000) square feet and one (1) acre of land.
2. Unless otherwise notified by the Department, the *qualified inspector* shall conduct site inspections in accordance with the following timetable:
- a. For construction sites where soil disturbance activities are on-going, the *qualified inspector* shall conduct a site inspection at least once every seven (7) calendar days.
 - b. For construction sites where soil disturbance activities are on-going and the *owner or operator* has received authorization in accordance with Part II.D.3 to disturb greater than five (5) acres of soil at any one time, the *qualified inspector* shall conduct at least two (2) site inspections every seven (7) calendar days. The two (2) inspections shall be separated by a minimum of two (2) full calendar days.
 - c. For construction sites where soil disturbance activities have been temporarily suspended (e.g. winter shutdown) and *temporary stabilization* measures have been applied to all disturbed areas, the *qualified inspector* shall conduct a site inspection at least once every thirty (30) calendar days. The *owner or operator* shall notify the DOW Water (SPDES) Program contact at the Regional Office (see contact information in Appendix F) or, in areas under the jurisdiction of a *regulated, traditional land use control MS4*, the *regulated, traditional land use control MS4* (provided the *regulated, traditional land use control MS4* is not the *owner or operator* of the *construction activity*) in writing prior to reducing the frequency of inspections.

- d. For construction sites where soil disturbance activities have been shut down with partial project completion, the *qualified inspector* can stop conducting inspections if all areas disturbed as of the project shutdown date have achieved *final stabilization* and all post-construction stormwater management practices required for the completed portion of the project have been constructed in conformance with the SWPPP and are operational. The *owner or operator* shall notify the DOW Water (SPDES) Program contact at the Regional Office (see contact information in Appendix F) or, in areas under the jurisdiction of a *regulated, traditional land use control MS4*, the *regulated, traditional land use control MS4* (provided the *regulated, traditional land use control MS4* is not the *owner or operator* of the *construction activity*) in writing prior to the shutdown. If soil disturbance activities are not resumed within 2 years from the date of shutdown, the *owner or operator* shall have the *qualified inspector* perform a final inspection and certify that all disturbed areas have achieved *final stabilization*, and all temporary, structural erosion and sediment control measures have been removed; and that all post-construction stormwater management practices have been constructed in conformance with the SWPPP by signing the “*Final Stabilization*” and “*Post-Construction Stormwater Management Practice*” certification statements on the NOT. The *owner or operator* shall then submit the completed NOT form to the address in Part II.B.1 of this permit.
 - e. For construction sites that directly *discharge* to one of the 303(d) segments listed in Appendix E or is located in one of the watersheds listed in Appendix C, the *qualified inspector* shall conduct at least two (2) site inspections every seven (7) calendar days. The two (2) inspections shall be separated by a minimum of two (2) full calendar days.
3. At a minimum, the *qualified inspector* shall inspect all erosion and sediment control practices and pollution prevention measures to ensure integrity and effectiveness, all post-construction stormwater management practices under construction to ensure that they are constructed in conformance with the SWPPP, all areas of disturbance that have not achieved *final stabilization*, all points of *discharge* to natural surface waterbodies located within, or immediately adjacent to, the property boundaries of the *construction site*, and all points of *discharge* from the *construction site*.
 4. The *qualified inspector* shall prepare an inspection report subsequent to each and every inspection. At a minimum, the inspection report shall include and/or address the following:

- a. Date and time of inspection;
- b. Name and title of person(s) performing inspection;
- c. A description of the weather and soil conditions (e.g. dry, wet, saturated) at the time of the inspection;
- d. A description of the condition of the runoff at all points of *discharge* from the *construction site*. This shall include identification of any *discharges* of sediment from the *construction site*. Include *discharges* from conveyance systems (i.e. pipes, culverts, ditches, etc.) and overland flow;
- e. A description of the condition of all natural surface waterbodies located within, or immediately adjacent to, the property boundaries of the *construction site* which receive runoff from disturbed areas. This shall include identification of any *discharges* of sediment to the surface waterbody;
- f. Identification of all erosion and sediment control practices and pollution prevention measures that need repair or maintenance;
- g. Identification of all erosion and sediment control practices and pollution prevention measures that were not installed properly or are not functioning as designed and need to be reinstalled or replaced;
- h. Description and sketch of areas with active soil disturbance activity, areas that have been disturbed but are inactive at the time of the inspection, and areas that have been stabilized (temporary and/or final) since the last inspection;
- i. Current phase of construction of all post-construction stormwater management practices and identification of all construction that is not in conformance with the SWPPP and technical standards;
- j. Corrective action(s) that must be taken to install, repair, replace or maintain erosion and sediment control practices and pollution prevention measures; and to correct deficiencies identified with the construction of the post-construction stormwater management practice(s);
- k. Identification and status of all corrective actions that were required by previous inspection; and

- I. Digital photographs, with date stamp, that clearly show the condition of all practices that have been identified as needing corrective actions. The *qualified inspector* shall attach paper color copies of the digital photographs to the inspection report being maintained onsite within seven (7) calendar days of the date of the inspection. The *qualified inspector* shall also take digital photographs, with date stamp, that clearly show the condition of the practice(s) after the corrective action has been completed. The *qualified inspector* shall attach paper color copies of the digital photographs to the inspection report that documents the completion of the corrective action work within seven (7) calendar days of that inspection.
5. Within one business day of the completion of an inspection, the *qualified inspector* shall notify the *owner or operator* and appropriate contractor or subcontractor identified in Part III.A.6. of this permit of any corrective actions that need to be taken. The contractor or subcontractor shall begin implementing the corrective actions within one business day of this notification and shall complete the corrective actions in a reasonable time frame.
6. All inspection reports shall be signed by the *qualified inspector*. Pursuant to Part II.D.2. of this permit, the inspection reports shall be maintained on site with the SWPPP.

Part V. TERMINATION OF PERMIT COVERAGE

A. Termination of Permit Coverage

1. An *owner or operator* that is eligible to terminate coverage under this permit must submit a completed NOT form to the address in Part II.B.1 of this permit. The NOT form shall be one which is associated with this permit, signed in accordance with Part VII.H of this permit.
2. An *owner or operator* may terminate coverage when one or more the following conditions have been met:
 - a. Total project completion - All *construction activity* identified in the SWPPP has been completed; and all areas of disturbance have achieved *final stabilization*; and all temporary, structural erosion and sediment control measures have been removed; and all post-construction stormwater management practices have been constructed in conformance with the SWPPP and are operational;

- b. Planned shutdown with partial project completion - All soil disturbance activities have ceased; and all areas disturbed as of the project shutdown date have achieved *final stabilization*; and all temporary, structural erosion and sediment control measures have been removed; and all post-construction stormwater management practices required for the completed portion of the project have been constructed in conformance with the SWPPP and are operational;
 - c. A new *owner or operator* has obtained coverage under this permit in accordance with Part II.F. of this permit.
 - d. The *owner or operator* obtains coverage under an alternative SPDES general permit or an individual SPDES permit.
3. For *construction activities* meeting subdivision 2a. or 2b. of this Part, the *owner or operator* shall have the *qualified inspector* perform a final site inspection prior to submitting the NOT. The *qualified inspector* shall, by signing the “*Final Stabilization*” and “Post-Construction Stormwater Management Practice certification statements on the NOT, certify that all the requirements in Part V.A.2.a. or b. of this permit have been achieved.
4. For *construction activities* that are subject to the requirements of a *regulated, traditional land use control MS4* and meet subdivision 2a. or 2b. of this Part, the *owner or operator* shall have the *regulated, traditional land use control MS4* sign the “MS4 Acceptance” statement on the NOT in accordance with the requirements in Part VII.H. of this permit. The *regulated, traditional land use control MS4* official, by signing this statement, has determined that it is acceptable for the *owner or operator* to submit the NOT in accordance with the requirements of this Part. The *regulated, traditional land use control MS4* can make this determination by performing a final site inspection themselves or by accepting the *qualified inspector’s* final site inspection certification(s) required in Part V.A.3. of this permit.
5. For *construction activities* that require post-construction stormwater management practices and meet subdivision 2a. of this Part, the *owner or operator* must, prior to submitting the NOT, ensure one of the following:
- a. the post-construction stormwater management practice(s) and any right-of-way(s) needed to maintain such practice(s) have been deeded to the municipality in which the practice(s) is located,

- b. an executed maintenance agreement is in place with the municipality that will maintain the post-construction stormwater management practice(s),
- c. for post-construction stormwater management practices that are privately owned, the *owner or operator* has a mechanism in place that requires operation and maintenance of the practice(s) in accordance with the operation and maintenance plan, such as a deed covenant in the *owner or operator's* deed of record,
- d. for post-construction stormwater management practices that are owned by a public or private institution (e.g. school, university, hospital), government agency or authority, or public utility; the *owner or operator* has policy and procedures in place that ensures operation and maintenance of the practices in accordance with the operation and maintenance plan.

Part VI. REPORTING AND RETENTION RECORDS

A. Record Retention

The *owner or operator* shall retain a copy of the NOI, NOI Acknowledgment Letter, SWPPP, MS4 SWPPP Acceptance form and any inspection reports that were prepared in conjunction with this permit for a period of at least five (5) years from the date that the Department receives a complete NOT submitted in accordance with Part V. of this general permit.

B. Addresses

With the exception of the NOI, NOT, and MS4 SWPPP Acceptance form (which must be submitted to the address referenced in Part II.B.1 of this permit), all written correspondence requested by the Department, including individual permit applications, shall be sent to the address of the appropriate DOW Water (SPDES) Program contact at the Regional Office listed in Appendix F.

Part VII. STANDARD PERMIT CONDITIONS

A. Duty to Comply

The *owner or operator* must comply with all conditions of this permit. All contractors and subcontractors associated with the project must comply with the terms of the SWPPP. Any non-compliance with this permit constitutes a violation of the Clean Water

Act (CWA) and the ECL and is grounds for an enforcement action against the *owner or operator* and/or the contractor/subcontractor; permit revocation, suspension or modification; or denial of a permit renewal application. Upon a finding of significant non-compliance with this permit or the applicable SWPPP, the Department may order an immediate stop to all *construction activity* at the site until the non-compliance is remedied. The stop work order shall be in writing, shall describe the non-compliance in detail, and shall be sent to the *owner or operator*.

If any human remains or archaeological remains are encountered during excavation, the *owner or operator* must immediately cease, or cause to cease, all *construction activity* in the area of the remains and notify the appropriate Regional Water Engineer (RWE). *Construction activity* shall not resume until written permission to do so has been received from the RWE.

B. Continuation of the Expired General Permit

This permit expires five (5) years from the effective date. If a new general permit is not issued prior to the expiration of this general permit, an *owner or operator* with coverage under this permit may continue to operate and *discharge* in accordance with the terms and conditions of this general permit, if it is extended pursuant to the State Administrative Procedure Act and 6 NYCRR Part 621, until a new general permit is issued.

C. Enforcement

Failure of the *owner or operator*, its contractors, subcontractors, agents and/or assigns to strictly adhere to any of the permit requirements contained herein shall constitute a violation of this permit. There are substantial criminal, civil, and administrative penalties associated with violating the provisions of this permit. Fines of up to \$37,500 per day for each violation and imprisonment for up to fifteen (15) years may be assessed depending upon the nature and degree of the offense.

D. Need to Halt or Reduce Activity Not a Defense

It shall not be a defense for an *owner or operator* in an enforcement action that it would have been necessary to halt or reduce the *construction activity* in order to maintain compliance with the conditions of this permit.

E. Duty to Mitigate

The *owner or operator* and its contractors and subcontractors shall take all reasonable steps to *minimize* or prevent any *discharge* in violation of this permit which has a reasonable likelihood of adversely affecting human health or the environment.

F. Duty to Provide Information

The *owner or operator* shall furnish to the Department, within a reasonable specified time period of a written request, all documentation necessary to demonstrate eligibility and any information to determine compliance with this permit or to determine whether cause exists for modifying or revoking this permit, or suspending or denying coverage under this permit, in accordance with the terms and conditions of this permit. The NOI, SWPPP and inspection reports required by this permit are public documents that the *owner or operator* must make available for review and copying by any person within five (5) business days of the *owner or operator* receiving a written request by any such person to review these documents. Copying of documents will be done at the requester's expense.

G. Other Information

When the *owner or operator* becomes aware that they failed to submit any relevant facts, or submitted incorrect information in the NOI or in any of the documents required by this permit, or have made substantive revisions to the SWPPP (e.g. the scope of the project changes significantly, the type of post-construction stormwater management practice(s) changes, there is a reduction in the sizing of the post-construction stormwater management practice, or there is an increase in the disturbance area or *impervious area*), which were not reflected in the original NOI submitted to the Department, they shall promptly submit such facts or information to the Department using the contact information in Part II.A. of this permit. Failure of the *owner or operator* to correct or supplement any relevant facts within five (5) business days of becoming aware of the deficiency shall constitute a violation of this permit.

H. Signatory Requirements

1. All NOIs and NOTs shall be signed as follows:
 - a. For a corporation these forms shall be signed by a responsible corporate officer. For the purpose of this section, a responsible corporate officer means:

- (i) a president, secretary, treasurer, or vice-president of the corporation in charge of a principal business function, or any other person who performs similar policy or decision-making functions for the corporation; or
 - (ii) the manager of one or more manufacturing, production or operating facilities, provided the manager is authorized to make management decisions which govern the operation of the regulated facility including having the explicit or implicit duty of making major capital investment recommendations, and initiating and directing other comprehensive measures to assure long term environmental compliance with environmental laws and regulations; the manager can ensure that the necessary systems are established or actions taken to gather complete and accurate information for permit application requirements; and where authority to sign documents has been assigned or delegated to the manager in accordance with corporate procedures;
 - b. For a partnership or sole proprietorship these forms shall be signed by a general partner or the proprietor, respectively; or
 - c. For a municipality, State, Federal, or other public agency these forms shall be signed by either a principal executive officer or ranking elected official. For purposes of this section, a principal executive officer of a Federal agency includes:
 - (i) the chief executive officer of the agency, or
 - (ii) a senior executive officer having responsibility for the overall operations of a principal geographic unit of the agency (e.g., Regional Administrators of EPA).
2. The SWPPP and other information requested by the Department shall be signed by a person described in Part VII.H.1. of this permit or by a duly authorized representative of that person. A person is a duly authorized representative only if:
- a. The authorization is made in writing by a person described in Part VII.H.1. of this permit;
 - b. The authorization specifies either an individual or a position having responsibility for the overall operation of the regulated facility or activity, such as the position of plant manager, operator of a well or a well field,

superintendent, position of *equivalent* responsibility, or an individual or position having overall responsibility for environmental matters for the company. (A duly authorized representative may thus be either a named individual or any individual occupying a named position) and,

- c. The written authorization shall include the name, title and signature of the authorized representative and be attached to the SWPPP.
3. All inspection reports shall be signed by the *qualified inspector* that performs the inspection.
4. The MS4 SWPPP Acceptance form shall be signed by the principal executive officer or ranking elected official from the *regulated, traditional land use control MS4*, or by a duly authorized representative of that person.

It shall constitute a permit violation if an incorrect and/or improper signatory authorizes any required forms, SWPPP and/or inspection reports.

I. Property Rights

The issuance of this permit does not convey any property rights of any sort, nor any exclusive privileges, nor does it authorize any injury to private property nor any invasion of personal rights, nor any infringement of Federal, State or local laws or regulations. *Owners or operators* must obtain any applicable conveyances, easements, licenses and/or access to real property prior to *commencing construction activity*.

J. Severability

The provisions of this permit are severable, and if any provision of this permit, or the application of any provision of this permit to any circumstance, is held invalid, the application of such provision to other circumstances, and the remainder of this permit shall not be affected thereby.

K. Requirement to Obtain Coverage Under an Alternative Permit

1. The Department may require any owner or operator authorized by this permit to apply for and/or obtain either an individual SPDES permit or another SPDES general permit. When the Department requires any discharger authorized by a general permit to apply for an individual SPDES permit, it shall notify the discharger in writing that a permit application is required. This notice shall

include a brief statement of the reasons for this decision, an application form, a statement setting a time frame for the owner or operator to file the application for an individual SPDES permit, and a deadline, not sooner than 180 days from owner or operator receipt of the notification letter, whereby the authorization to discharge under this general permit shall be terminated. Applications must be submitted to the appropriate Permit Administrator at the Regional Office. The Department may grant additional time upon demonstration, to the satisfaction of the Department, that additional time to apply for an alternative authorization is necessary or where the Department has not provided a permit determination in accordance with Part 621 of this Title.

2. When an individual SPDES permit is issued to a discharger authorized to *discharge* under a general SPDES permit for the same *discharge(s)*, the general permit authorization for outfalls authorized under the individual SPDES permit is automatically terminated on the effective date of the individual permit unless termination is earlier in accordance with 6 NYCRR Part 750.

L. Proper Operation and Maintenance

The *owner or operator* shall at all times properly operate and maintain all facilities and systems of treatment and control (and related appurtenances) which are installed or used by the *owner or operator* to achieve compliance with the conditions of this permit and with the requirements of the SWPPP.

M. Inspection and Entry

The *owner or operator* shall allow an authorized representative of the Department, EPA, applicable county health department, or, in the case of a *construction site* which *discharges* through an *MS4*, an authorized representative of the *MS4* receiving the discharge, upon the presentation of credentials and other documents as may be required by law, to:

1. Enter upon the owner's or operator's premises where a regulated facility or activity is located or conducted or where records must be kept under the conditions of this permit;
2. Have access to and copy at reasonable times, any records that must be kept under the conditions of this permit; and

3. Inspect at reasonable times any facilities or equipment (including monitoring and control equipment), practices or operations regulated or required by this permit.
4. Sample or monitor at reasonable times, for purposes of assuring permit compliance or as otherwise authorized by the Act or ECL, any substances or parameters at any location.

N. Permit Actions

This permit may, at any time, be modified, suspended, revoked, or renewed by the Department in accordance with 6 NYCRR Part 621. The filing of a request by the *owner or operator* for a permit modification, revocation and reissuance, termination, a notification of planned changes or anticipated noncompliance does not limit, diminish and/or stay compliance with any terms of this permit.

O. Definitions

Definitions of key terms are included in Appendix A of this permit.

P. Re-Opener Clause

1. If there is evidence indicating potential or realized impacts on water quality due to any stormwater discharge associated with construction activity covered by this permit, the owner or operator of such discharge may be required to obtain an individual permit or alternative general permit in accordance with Part VII.K. of this permit or the permit may be modified to include different limitations and/or requirements.
2. Any Department initiated permit modification, suspension or revocation will be conducted in accordance with 6 NYCRR Part 621, 6 NYCRR 750-1.18, and 6 NYCRR 750-1.20.

Q. Penalties for Falsification of Forms and Reports

In accordance with 6NYCRR Part 750-2.4 and 750-2.5, any person who knowingly makes any false material statement, representation, or certification in any application, record, report or other document filed or required to be maintained under this permit, including reports of compliance or noncompliance shall, upon conviction, be punished in accordance with ECL §71-1933 and or Articles 175 and 210 of the New York State Penal Law.

R. Other Permits

Nothing in this permit relieves the *owner or operator* from a requirement to obtain any other permits required by law.

APPENDIX A – Acronyms and Definitions

Acronyms

APO – Agency Preservation Officer
BMP – Best Management Practice
CPESC – Certified Professional in Erosion and Sediment Control
Cpv – Channel Protection Volume
CWA – Clean Water Act (or the Federal Water Pollution Control Act, 33 U.S.C. §1251 et seq)
DOW – Division of Water
EAF – Environmental Assessment Form
ECL - Environmental Conservation Law
EPA – U. S. Environmental Protection Agency
HSG – Hydrologic Soil Group
MS4 – Municipal Separate Storm Sewer System
NOI – Notice of Intent
NOT – Notice of Termination
NPDES – National Pollutant Discharge Elimination System
OPRHP – Office of Parks, Recreation and Historic Places
Qf – Extreme Flood
Qp – Overbank Flood
RRv – Runoff Reduction Volume
RWE – Regional Water Engineer
SEQR – State Environmental Quality Review
SEQRA - State Environmental Quality Review Act
SHPA – State Historic Preservation Act
SPDES – State Pollutant Discharge Elimination System
SWPPP – Stormwater Pollution Prevention Plan
TMDL – Total Maximum Daily Load
UPA – Uniform Procedures Act
USDA – United States Department of Agriculture
WQv – Water Quality Volume

Definitions

All definitions in this section are solely for the purposes of this permit.

Agricultural Building – a structure designed and constructed to house farm implements, hay, grain, poultry, livestock or other horticultural products; excluding any structure designed, constructed or used, in whole or in part, for human habitation, as a place of employment where agricultural products are processed, treated or packaged, or as a place used by the public.

Agricultural Property – means the land for construction of a barn, *agricultural building*, silo, stockyard, pen or other structural practices identified in Table II in the “Agricultural Management Practices Catalog for Nonpoint Source Pollution in New York State” prepared by the Department in cooperation with agencies of New York Nonpoint Source Coordinating Committee (dated June 2007).

Alter Hydrology from Pre to Post-Development Conditions - means the post-development peak flow rate(s) has increased by more than 5% of the pre-developed condition for the design storm of interest (e.g. 10 yr and 100 yr).

Combined Sewer - means a sewer that is designed to collect and convey both “sewage” and “stormwater”.

Commence (Commencement of) Construction Activities - means the initial disturbance of soils associated with clearing, grading or excavation activities; or other construction related activities that disturb or expose soils such as demolition, stockpiling of fill material, and the initial installation of erosion and sediment control practices required in the SWPPP. See definition for “*Construction Activity(ies)*” also.

Construction Activity(ies) - means any clearing, grading, excavation, filling, demolition or stockpiling activities that result in soil disturbance. Clearing activities can include, but are not limited to, logging equipment operation, the cutting and skidding of trees, stump removal and/or brush root removal. Construction activity does not include routine maintenance that is performed to maintain the original line and grade, hydraulic capacity, or original purpose of a facility.

Construction Site – means the land area where *construction activity(ies)* will occur. See definition for “*Commence (Commencement of) Construction Activities*” and “*Larger Common Plan of Development or Sale*” also.

Dewatering – means the act of draining rainwater and/or groundwater from building foundations, vaults or excavations/trenches.

Direct Discharge (to a specific surface waterbody) - means that runoff flows from a *construction site* by overland flow and the first point of discharge is the specific surface waterbody, or runoff flows from a *construction site* to a separate storm sewer system

and the first point of discharge from the separate storm sewer system is the specific surface waterbody.

Discharge(s) - means any addition of any pollutant to waters of the State through an outlet or *point source*.

Embankment – means an earthen or rock slope that supports a road/highway.

Endangered or Threatened Species – see 6 NYCRR Part 182 of the Department’s rules and regulations for definition of terms and requirements.

Environmental Conservation Law (ECL) - means chapter 43-B of the Consolidated Laws of the State of New York, entitled the Environmental Conservation Law.

Equivalent (Equivalence) – means that the practice or measure meets all the performance, longevity, maintenance, and safety objectives of the technical standard and will provide an equal or greater degree of water quality protection.

Final Stabilization - means that all soil disturbance activities have ceased and a uniform, perennial vegetative cover with a density of eighty (80) percent over the entire pervious surface has been established; or other equivalent stabilization measures, such as permanent landscape mulches, rock rip-rap or washed/crushed stone have been applied on all disturbed areas that are not covered by permanent structures, concrete or pavement.

General SPDES permit - means a SPDES permit issued pursuant to 6 NYCRR Part 750-1.21 and Section 70-0117 of the ECL authorizing a category of discharges.

Groundwater(s) - means waters in the saturated zone. The saturated zone is a subsurface zone in which all the interstices are filled with water under pressure greater than that of the atmosphere. Although the zone may contain gas-filled interstices or interstices filled with fluids other than water, it is still considered saturated.

Historic Property – means any building, structure, site, object or district that is listed on the State or National Registers of Historic Places or is determined to be eligible for listing on the State or National Registers of Historic Places.

Impervious Area (Cover) - means all impermeable surfaces that cannot effectively infiltrate rainfall. This includes paved, concrete and gravel surfaces (i.e. parking lots, driveways, roads, runways and sidewalks); building rooftops and miscellaneous impermeable structures such as patios, pools, and sheds.

Infeasible – means not technologically possible, or not economically practicable and achievable in light of best industry practices.

Larger Common Plan of Development or Sale - means a contiguous area where multiple separate and distinct *construction activities* are occurring, or will occur, under one plan. The term “plan” in “larger common plan of development or sale” is broadly defined as any announcement or piece of documentation (including a sign, public notice or hearing, marketing plan, advertisement, drawing, permit application, State Environmental Quality Review Act (SEQRA) environmental assessment form or other documents, zoning request, computer design, etc.) or physical demarcation (including boundary signs, lot stakes, surveyor markings, etc.) indicating that *construction activities* may occur on a specific plot.

For discrete construction projects that are located within a larger common plan of development or sale that are at least 1/4 mile apart, each project can be treated as a separate plan of development or sale provided any interconnecting road, pipeline or utility project that is part of the same “common plan” is not concurrently being disturbed.

Minimize – means reduce and/or eliminate to the extent achievable using control measures (including best management practices) that are technologically available and economically practicable and achievable in light of best industry practices.

Municipal Separate Storm Sewer (MS4) - a conveyance or system of conveyances (including roads with drainage systems, municipal streets, catch basins, curbs, gutters, ditches, man-made channels, or storm drains):

- (i) Owned or operated by a State, city, town, borough, county, parish, district, association, or other public body (created by or pursuant to State law) having jurisdiction over disposal of sewage, industrial wastes, stormwater, or other wastes, including special districts under State law such as a sewer district, flood control district or drainage district, or similar entity, or an Indian tribe or an authorized Indian tribal organization, or a designated and approved management agency under section 208 of the CWA that discharges to surface waters of the State;
- (ii) Designed or used for collecting or conveying stormwater;
- (iii) Which is not a *combined sewer*; and
- (iv) Which is not part of a Publicly Owned Treatment Works (POTW) as defined at 40 CFR 122.2.

National Pollutant Discharge Elimination System (NPDES) - means the national system for the issuance of wastewater and stormwater permits under the Federal Water Pollution Control Act (Clean Water Act).

Natural Buffer – means an undisturbed area with natural cover running along a surface water (e.g. wetland, stream, river, lake, etc.).

New Development – means any land disturbance that does not meet the definition of Redevelopment Activity included in this appendix.

New York State Erosion and Sediment Control Certificate Program – a certificate program that establishes and maintains a process to identify and recognize individuals who are capable of developing, designing, inspecting and maintaining erosion and sediment control plans on projects that disturb soils in New York State. The certificate program is administered by the New York State Conservation District Employees Association.

NOI Acknowledgment Letter - means the letter that the Department sends to an owner or operator to acknowledge the Department's receipt and acceptance of a complete Notice of Intent. This letter documents the owner's or operator's authorization to discharge in accordance with the general permit for stormwater discharges from *construction activity*.

Nonpoint Source - means any source of water pollution or pollutants which is not a discrete conveyance or *point source* permitted pursuant to Title 7 or 8 of Article 17 of the Environmental Conservation Law (see ECL Section 17-1403).

Overbank –means flow events that exceed the capacity of the stream channel and spill out into the adjacent floodplain.

Owner or Operator - means the person, persons or legal entity which owns or leases the property on which the *construction activity* is occurring; an entity that has operational control over the construction plans and specifications, including the ability to make modifications to the plans and specifications; and/or an entity that has day-to-day operational control of those activities at a project that are necessary to ensure compliance with the permit conditions.

Performance Criteria – means the design criteria listed under the “Required Elements” sections in Chapters 5, 6 and 10 of the technical standard, New York State Stormwater Management Design Manual, dated January 2015. It does not include the Sizing Criteria (i.e. WQv, RRv, Cpv, Qp and Qf) in Part I.C.2. of the permit.

Point Source - means any discernible, confined and discrete conveyance, including but not limited to any pipe, ditch, channel, tunnel, conduit, well, discrete fissure, container, rolling stock, concentrated animal feeding operation, vessel or other floating craft, or landfill leachate collection system from which *pollutants* are or may be discharged.

Pollutant - means dredged spoil, filter backwash, solid waste, incinerator residue, sewage, garbage, sewage sludge, munitions, chemical wastes, biological materials, radioactive materials, heat, wrecked or discarded equipment, rock, sand and industrial, municipal, agricultural waste and ballast discharged into water; which may cause or might reasonably be expected to cause pollution of the waters of the state in contravention of the standards or guidance values adopted as provided in 6 NYCRR Parts 700 et seq .

Qualified Inspector - means a person that is knowledgeable in the principles and practices of erosion and sediment control, such as a licensed Professional Engineer, Certified Professional in Erosion and Sediment Control (CPESC), Registered Landscape Architect, New York State Erosion and Sediment Control Certificate Program holder or other Department endorsed individual(s).

It can also mean someone working under the direct supervision of, and at the same company as, the licensed Professional Engineer or Registered Landscape Architect, provided that person has training in the principles and practices of erosion and sediment control. Training in the principles and practices of erosion and sediment control means that the individual working under the direct supervision of the licensed Professional Engineer or Registered Landscape Architect has received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity. After receiving the initial training, the individual working under the direct supervision of the licensed Professional Engineer or Registered Landscape Architect shall receive four (4) hours of training every three (3) years.

It can also mean a person that meets the *Qualified Professional* qualifications in addition to the *Qualified Inspector* qualifications.

Note: Inspections of any post-construction stormwater management practices that include structural components, such as a dam for an impoundment, shall be performed by a licensed Professional Engineer.

Qualified Professional - means a person that is knowledgeable in the principles and practices of stormwater management and treatment, such as a licensed Professional Engineer, Registered Landscape Architect or other Department endorsed individual(s). Individuals preparing SWPPPs that require the post-construction stormwater management practice component must have an understanding of the principles of hydrology, water quality management practice design, water quantity control design, and, in many cases, the principles of hydraulics. All components of the SWPPP that involve the practice of engineering, as defined by the NYS Education Law (see Article 145), shall be prepared by, or under the direct supervision of, a professional engineer licensed to practice in the State of New York.

Redevelopment Activity(ies) – means the disturbance and reconstruction of existing impervious area, including impervious areas that were removed from a project site within five (5) years of preliminary project plan submission to the local government (i.e. site plan, subdivision, etc.).

Regulated, Traditional Land Use Control MS4 - means a city, town or village with land use control authority that is authorized to discharge under New York State DEC's

SPDES General Permit For Stormwater Discharges from Municipal Separate Stormwater Sewer Systems (MS4s) or the City of New York's Individual SPDES Permit for their Municipal Separate Storm Sewer Systems (NY-0287890).

Routine Maintenance Activity - means *construction activity* that is performed to maintain the original line and grade, hydraulic capacity, or original purpose of a facility, including, but not limited to:

- Re-grading of gravel roads or parking lots,
- Cleaning and shaping of existing roadside ditches and culverts that maintains the approximate original line and grade, and hydraulic capacity of the ditch,
- Cleaning and shaping of existing roadside ditches that does not maintain the approximate original grade, hydraulic capacity and purpose of the ditch if the changes to the line and grade, hydraulic capacity or purpose of the ditch are installed to improve water quality and quantity controls (e.g. installing grass lined ditch),
- Placement of aggregate shoulder backing that stabilizes the transition between the road shoulder and the ditch or *embankment*,
- Full depth milling and filling of existing asphalt pavements, replacement of concrete pavement slabs, and similar work that does not expose soil or disturb the bottom six (6) inches of subbase material,
- Long-term use of equipment storage areas at or near highway maintenance facilities,
- Removal of sediment from the edge of the highway to restore a previously existing sheet-flow drainage connection from the highway surface to the highway ditch or *embankment*,
- Existing use of Canal Corp owned upland disposal sites for the canal, and
- Replacement of curbs, gutters, sidewalks and guide rail posts.

Site limitations – means site conditions that prevent the use of an infiltration technique and or infiltration of the total WQv. Typical site limitations include: seasonal high groundwater, shallow depth to bedrock, and soils with an infiltration rate less than 0.5 inches/hour. The existence of site limitations shall be confirmed and documented using actual field testing (i.e. test pits, soil borings, and infiltration test) or using information from the most current United States Department of Agriculture (USDA) Soil Survey for the County where the project is located.

Sizing Criteria – means the criteria included in Part I.C.2 of the permit that are used to size post-construction stormwater management control practices. The criteria include; Water Quality Volume (WQv), Runoff Reduction Volume (RRv), Channel Protection Volume (Cpv), *Overbank Flood* (Qp), and *Extreme Flood* (Qf).

State Pollutant Discharge Elimination System (SPDES) - means the system established pursuant to Article 17 of the ECL and 6 NYCRR Part 750 for issuance of permits authorizing discharges to the waters of the state.

Steep Slope – means land area designated on the current United States Department of Agriculture (“USDA”) Soil Survey as Soil Slope Phase “D”, (provided the map unit name is inclusive of slopes greater than 25%) , or Soil Slope Phase E or F, (regardless of the map unit name), or a combination of the three designations.

Streambank – as used in this permit, means the terrain alongside the bed of a creek or stream. The bank consists of the sides of the channel, between which the flow is confined.

Stormwater Pollution Prevention Plan (SWPPP) – means a project specific report, including construction drawings, that among other things: describes the construction activity(ies), identifies the potential sources of pollution at the *construction site*; describes and shows the stormwater controls that will be used to control the pollutants (i.e. erosion and sediment controls; for many projects, includes post-construction stormwater management controls); and identifies procedures the *owner or operator* will implement to comply with the terms and conditions of the permit. See Part III of the permit for a complete description of the information that must be included in the SWPPP.

Surface Waters of the State - shall be construed to include lakes, bays, sounds, ponds, impounding reservoirs, springs, rivers, streams, creeks, estuaries, marshes, inlets, canals, the Atlantic ocean within the territorial seas of the state of New York and all other bodies of surface water, natural or artificial, inland or coastal, fresh or salt, public or private (except those private waters that do not combine or effect a junction with natural surface waters), which are wholly or partially within or bordering the state or within its jurisdiction. Waters of the state are further defined in 6 NYCRR Parts 800 to 941.

Temporarily Ceased – means that an existing disturbed area will not be disturbed again within 14 calendar days of the previous soil disturbance.

Temporary Stabilization - means that exposed soil has been covered with material(s) as set forth in the technical standard, New York Standards and Specifications for Erosion and Sediment Control, to prevent the exposed soil from eroding. The materials can include, but are not limited to, mulch, seed and mulch, and erosion control mats (e.g. jute twisted yarn, excelsior wood fiber mats).

Total Maximum Daily Loads (TMDLs) - A TMDL is the sum of the allowable loads of a single pollutant from all contributing point and *nonpoint sources*. It is a calculation of the maximum amount of a pollutant that a waterbody can receive on a daily basis and still meet *water quality standards*, and an allocation of that amount to the pollutant's sources. A TMDL stipulates wasteload allocations (WLAs) for *point source* discharges, load allocations (LAs) for *nonpoint sources*, and a margin of safety (MOS).

Trained Contractor - means an employee from the contracting (construction) company, identified in Part III.A.6., that has received four (4) hours of Department endorsed

training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity. After receiving the initial training, the *trained contractor* shall receive four (4) hours of training every three (3) years.

It can also mean an employee from the contracting (construction) company, identified in Part III.A.6., that meets the *qualified inspector* qualifications (e.g. licensed Professional Engineer, Certified Professional in Erosion and Sediment Control (CPESC), Registered Landscape Architect, New York State Erosion and Sediment Control Certificate Program holder, or someone working under the direct supervision of, and at the same company as, the licensed Professional Engineer or Registered Landscape Architect, provided they have received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity).

The *trained contractor* is responsible for the day to day implementation of the SWPPP.

Uniform Procedures Act (UPA) Permit - means a permit required under 6 NYCRR Part 621 of the Environmental Conservation Law (ECL), Article 70.

Water Quality Standard - means such measures of purity or quality for any waters in relation to their reasonable and necessary use as promulgated in 6 NYCRR Part 700 et seq.

APPENDIX B – Required SWPPP Components by Project Type

Table 1
Construction Activities that Require the Preparation of a SWPPP That Only Includes Erosion and Sediment Controls

The following construction activities that involve soil disturbances of one (1) or more acres of land, but less than five (5) acres: • Single family home <u>not</u> located in one of the watersheds listed in Appendix C or <u>not directly discharging</u> to one of the 303(d) segments listed in Appendix E • Single family residential subdivisions with 25% or less impervious cover at total site build-out and <u>not</u> located in one of the watersheds listed in Appendix C and <u>not</u> directly discharging to one of the 303(d) segments listed in Appendix E • Construction of a barn or other <i>agricultural building</i>, silo, stock yard or pen.
The following construction activities that involve soil disturbances between five thousand (5000) square feet and one (1) acre of land: All construction activities located in the watersheds identified in Appendix D that involve soil disturbances between five thousand (5,000) square feet and one (1) acre of land.
The following construction activities that involve soil disturbances of one (1) or more acres of land: • Installation of underground, linear utilities; such as gas lines, fiber-optic cable, cable TV, electric, telephone, sewer mains, and water mains • Environmental enhancement projects, such as wetland mitigation projects, stormwater retrofits and stream restoration projects • Pond construction • Linear bike paths running through areas with vegetative cover, including bike paths surfaced with an impervious cover • Cross-country ski trails and walking/hiking trails • Sidewalk, bike path or walking path projects, surfaced with an impervious cover, that are not part of residential, commercial or institutional development; • Sidewalk, bike path or walking path projects, surfaced with an impervious cover, that include incidental shoulder or curb work along an existing highway to support construction of the sidewalk, bike path or walking path. • Slope stabilization projects • Slope flattening that changes the grade of the site, but does not significantly change the runoff characteristics

**Table 1 (Continued) CONSTRUCTION ACTIVITIES THAT REQUIRE THE PREPARATION OF A SWPPP
THAT ONLY INCLUDES EROSION AND SEDIMENT CONTROLS**

The following construction activities that involve soil disturbances of one (1) or more acres of land:

- Spoil areas that will be covered with vegetation
- Vegetated open space projects (i.e. recreational parks, lawns, meadows, fields, downhill ski trails) excluding projects that *alter hydrology from pre to post development* conditions,
- Athletic fields (natural grass) that do not include the construction or reconstruction of *impervious area* and do not *alter hydrology from pre to post development* conditions
- Demolition project where vegetation will be established, and no redevelopment is planned
- Overhead electric transmission line project that does not include the construction of permanent access roads or parking areas surfaced with *impervious cover*
- Structural practices as identified in Table II in the “Agricultural Management Practices Catalog for Nonpoint Source Pollution in New York State”, excluding projects that involve soil disturbances of greater than five acres and construction activities that include the construction or reconstruction of impervious area
- Temporary access roads, median crossovers, detour roads, lanes, or other temporary impervious areas that will be restored to pre-construction conditions once the construction activity is complete

Table 2
CONSTRUCTION ACTIVITIES THAT REQUIRE THE PREPARATION OF A SWPPP THAT INCLUDES
POST-CONSTRUCTION STORMWATER MANAGEMENT PRACTICES

The following construction activities that involve soil disturbances of one (1) or more acres of land:

- Single family home located in one of the watersheds listed in Appendix C or *directly discharging* to one of the 303(d) segments listed in Appendix E
- Single family home that disturbs five (5) or more acres of land
- Single family residential subdivisions located in one of the watersheds listed in Appendix C or *directly discharging* to one of the 303(d) segments listed in Appendix E
- Single family residential subdivisions that involve soil disturbances of between one (1) and five (5) acres of land with greater than 25% impervious cover at total site build-out
- Single family residential subdivisions that involve soil disturbances of five (5) or more acres of land, and single family residential subdivisions that involve soil disturbances of less than five (5) acres that are part of a larger common plan of development or sale that will ultimately disturb five or more acres of land
- Multi-family residential developments; includes duplexes, townhomes, condominiums, senior housing complexes, apartment complexes, and mobile home parks
- Airports
- Amusement parks
- Breweries, cideries, and wineries, including establishments constructed on agricultural land
- Campgrounds
- Cemeteries that include the construction or reconstruction of impervious area (>5% of disturbed area) or *alter the hydrology from pre to post development conditions*
- Commercial developments
- Churches and other places of worship
- Construction of a barn or other *agricultural building* (e.g. silo) and structural practices as identified in Table II in the "Agricultural Management Practices Catalog for Nonpoint Source Pollution in New York State" that include the construction or reconstruction of *impervious area*, excluding projects that involve soil disturbances of less than five acres.
- Golf courses
- Institutional development; includes hospitals, prisons, schools and colleges
- Industrial facilities; includes industrial parks
- Landfills
- Municipal facilities; includes highway garages, transfer stations, office buildings, POTW's, water treatment plants, and water storage tanks
- Office complexes
- Playgrounds that include the construction or reconstruction of impervious area
- Sports complexes
- Racetracks; includes racetracks with earthen (dirt) surface
- Road construction or reconstruction, including roads constructed as part of the construction activities listed in Table 1

Table 2 (Continued)

**CONSTRUCTION ACTIVITIES THAT REQUIRE THE PREPARATION OF A SWPPP THAT INCLUDES
POST-CONSTRUCTION STORMWATER MANAGEMENT PRACTICES**

The following construction activities that involve soil disturbances of one (1) or more acres of land:

- Parking lot construction or reconstruction, including parking lots constructed as part of the construction activities listed in Table 1
- Athletic fields (natural grass) that include the construction or reconstruction of impervious area (>5% of disturbed area) or *alter the hydrology from pre to post development conditions*
- Athletic fields with artificial turf
- Permanent access roads, parking areas, substations, compressor stations and well drilling pads, surfaced with *impervious cover*, and constructed as part of an over-head electric transmission line project, wind-power project, cell tower project, oil or gas well drilling project, sewer or water main project or other linear utility project
- Sidewalk, bike path or walking path projects, surfaced with an impervious cover, that are part of a residential, commercial or institutional development
- Sidewalk, bike path or walking path projects, surfaced with an impervious cover, that are part of a highway construction or reconstruction project
- All other construction activities that include the construction or reconstruction of *impervious area* or *alter the hydrology from pre to post development conditions*, and are not listed in Table 1

APPENDIX C – Watersheds Requiring Enhanced Phosphorus Removal

Watersheds where *owners or operators* of construction activities identified in Table 2 of Appendix B must prepare a SWPPP that includes post-construction stormwater management practices designed in conformance with the Enhanced Phosphorus Removal Standards included in the technical standard, New York State Stormwater Management Design Manual (“Design Manual”).

- Entire New York City Watershed located east of the Hudson River - Figure 1
- Onondaga Lake Watershed - Figure 2
- Greenwood Lake Watershed -Figure 3
- Oscawana Lake Watershed – Figure 4
- Kinderhook Lake Watershed – Figure 5

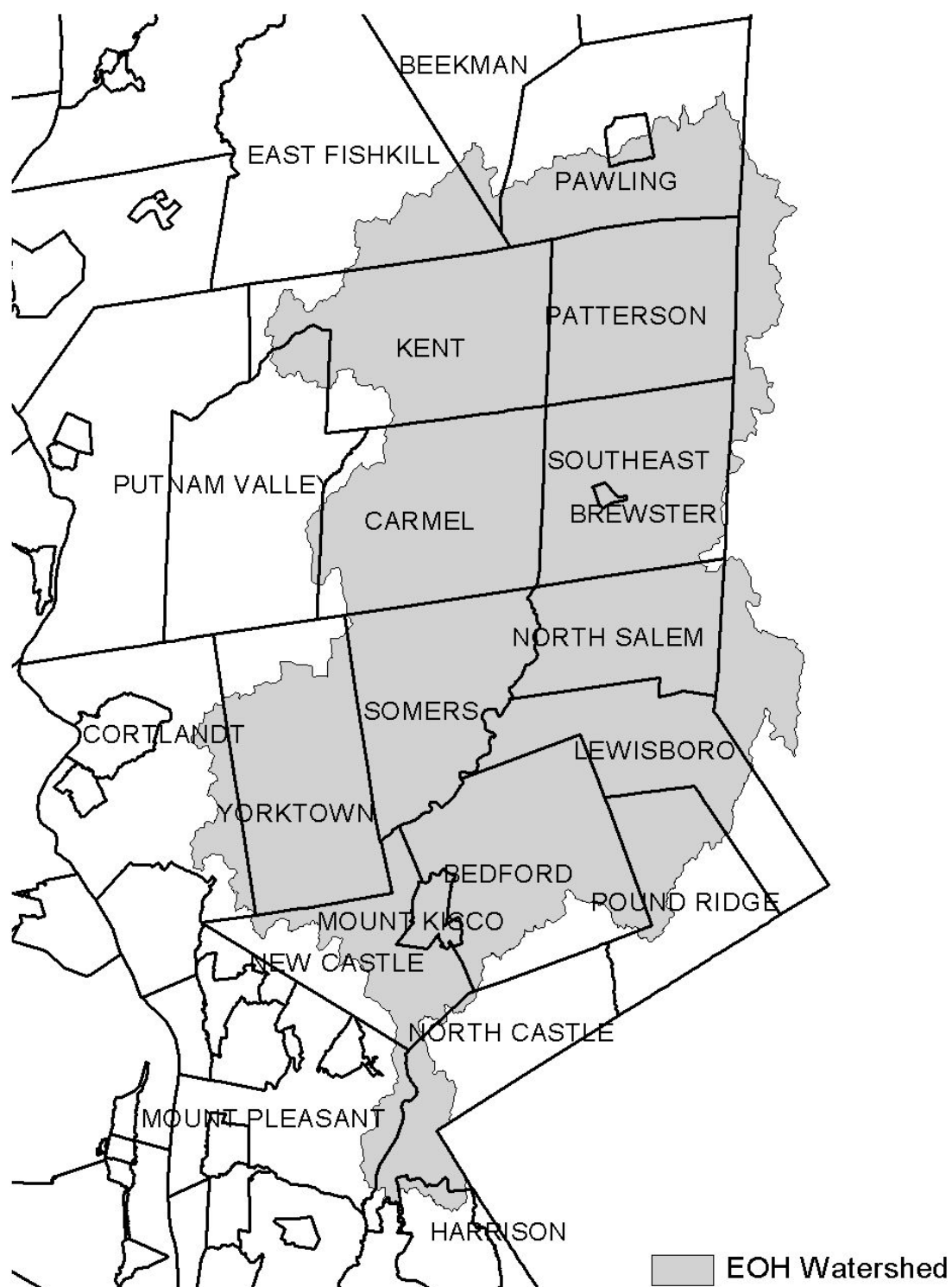
Figure 1 - New York City Watershed East of the Hudson

Figure 2 - Onondaga Lake Watershed

Figure 3 - Greenwood Lake Watershed

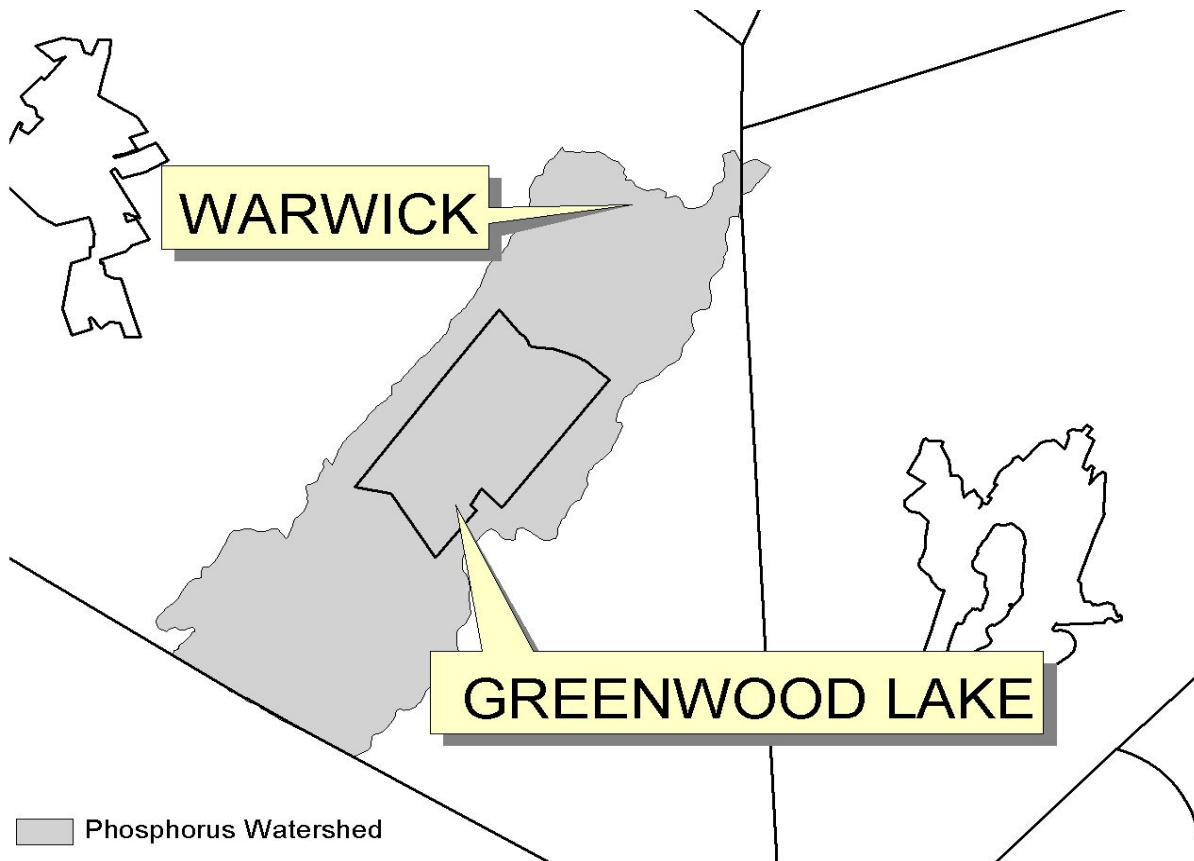


Figure 4 - Oscawana Lake Watershed

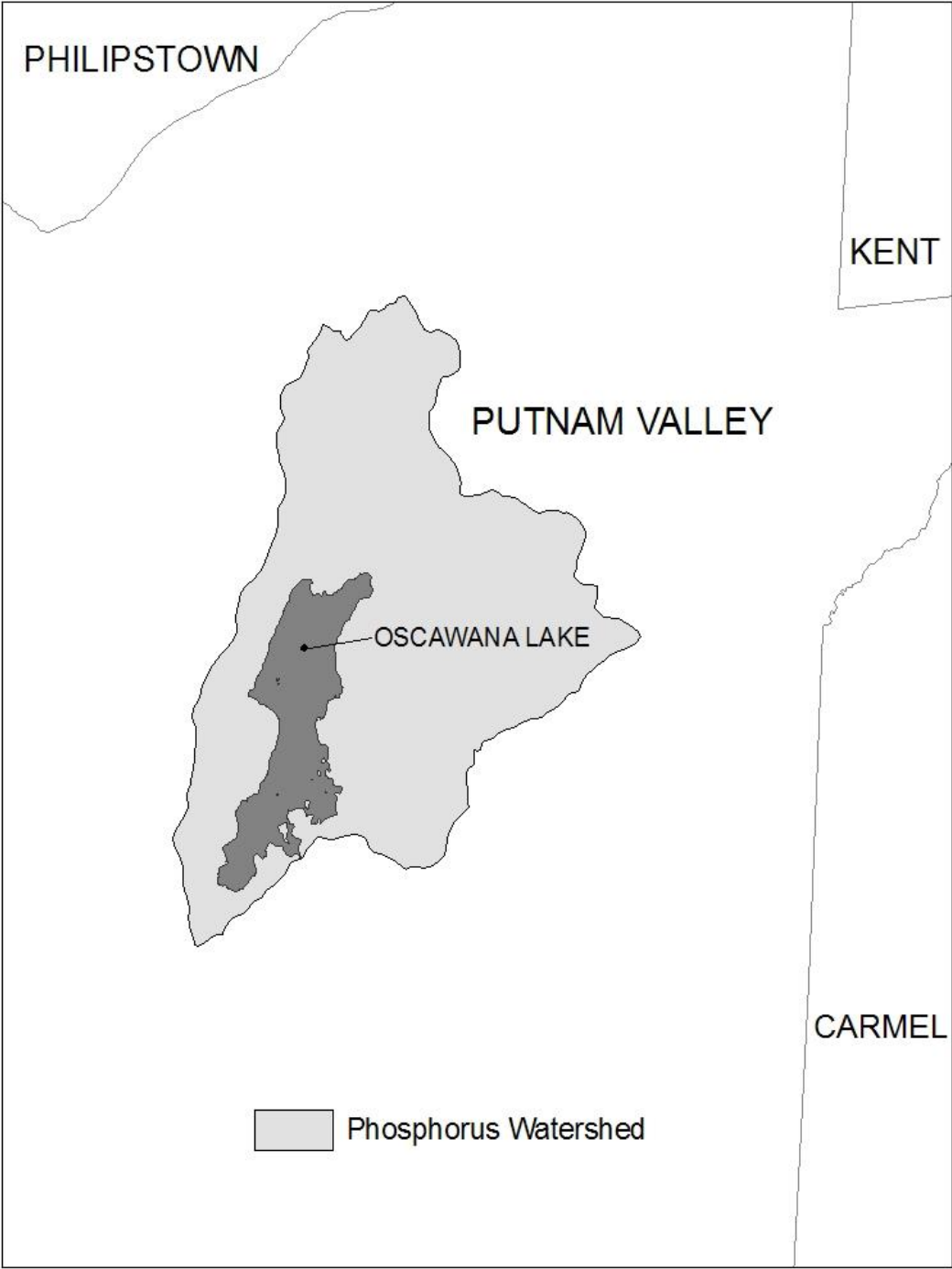
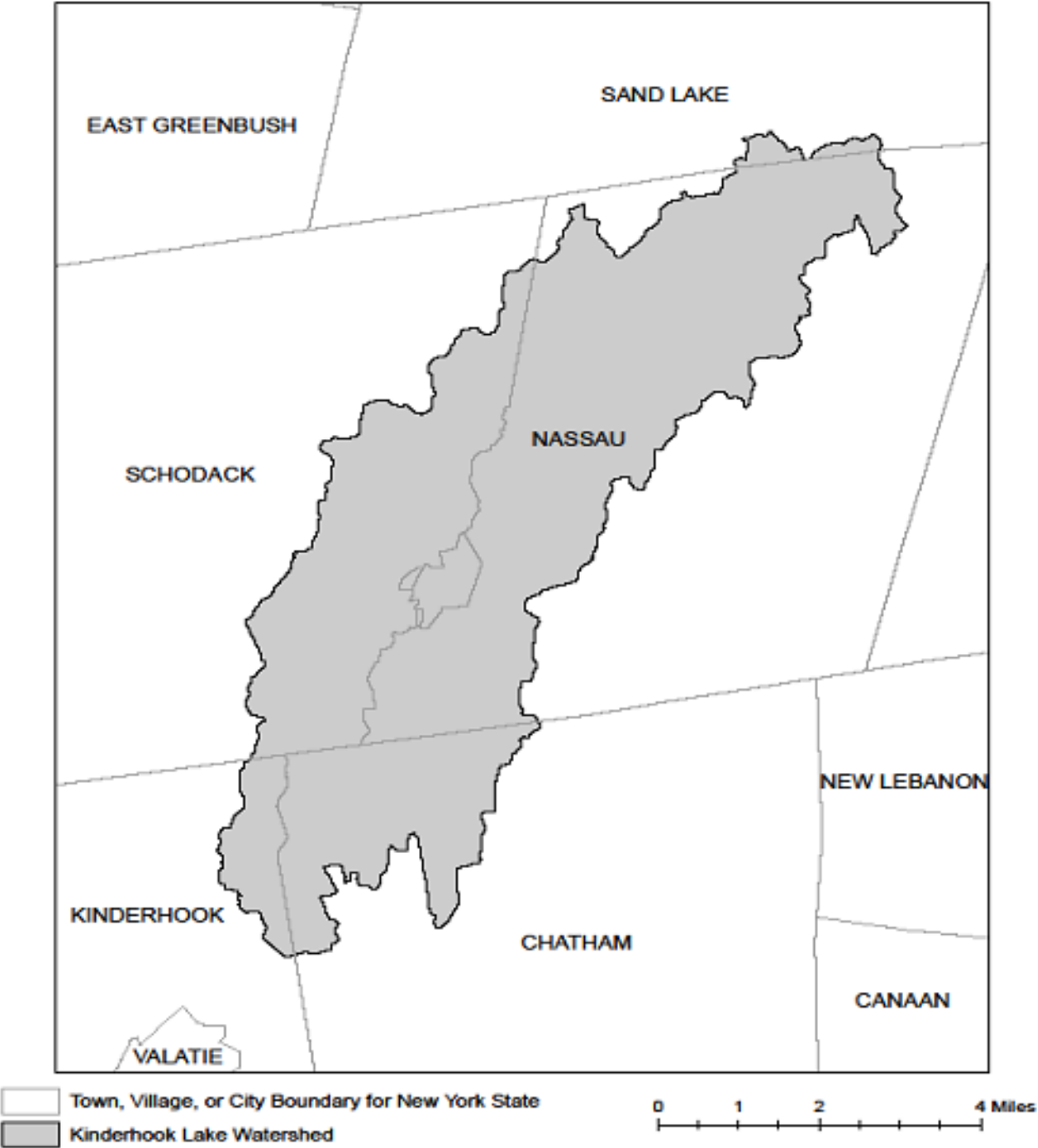


Figure 5 - Kinderhook Lake Watershed



APPENDIX D – Watersheds with Lower Disturbance Threshold

Watersheds where *owners or operators* of construction activities that involve soil disturbances between five thousand (5000) square feet and one (1) acre of land must obtain coverage under this permit.

Entire New York City Watershed that is located east of the Hudson River - See Figure 1 in Appendix C
--

APPENDIX E – 303(d) Segments Impaired by Construction Related Pollutant(s)

List of 303(d) segments impaired by pollutants related to *construction activity* (e.g. silt, sediment or nutrients). The list was developed using "The Final New York State 2016 Section 303(d) List of Impaired Waters Requiring a TMDL/Other Strategy" dated November 2016. *Owners or operators* of single family home and single family residential subdivisions with 25% or less total impervious cover at total site build-out that involve soil disturbances of one or more acres of land, but less than 5 acres, and *directly discharge* to one of the listed segments below shall prepare a SWPPP that includes post-construction stormwater management practices designed in conformance with the New York State Stormwater Management Design Manual ("Design Manual"), dated January 2015.

COUNTY	WATERBODY	POLLUTANT
Albany	Ann Lee (Shakers) Pond, Stump Pond	Nutrients
Albany	Basic Creek Reservoir	Nutrients
Allegany	Amity Lake, Saunders Pond	Nutrients
Bronx	Long Island Sound, Bronx	Nutrients
Bronx	Van Cortlandt Lake	Nutrients
Broome	Fly Pond, Deer Lake, Sky Lake	Nutrients
Broome	Minor Tribs to Lower Susquehanna (north)	Nutrients
Broome	Whitney Point Lake/Reservoir	Nutrients
Cattaraugus	Allegheny River/Reservoir	Nutrients
Cattaraugus	Beaver (Alma) Lake	Nutrients
Cattaraugus	Case Lake	Nutrients
Cattaraugus	Linlyco/Club Pond	Nutrients
Cayuga	Duck Lake	Nutrients
Cayuga	Little Sodus Bay	Nutrients
Chautauqua	Bear Lake	Nutrients
Chautauqua	Chadakoin River and tribs	Nutrients
Chautauqua	Chautauqua Lake, North	Nutrients
Chautauqua	Chautauqua Lake, South	Nutrients
Chautauqua	Findley Lake	Nutrients
Chautauqua	Hulburt/Clymer Pond	Nutrients
Clinton	Great Chazy River, Lower, Main Stem	Silt/Sediment
Clinton	Lake Champlain, Main Lake, Middle	Nutrients
Clinton	Lake Champlain, Main Lake, North	Nutrients
Columbia	Kinderhook Lake	Nutrients
Columbia	Robinson Pond	Nutrients
Cortland	Dean Pond	Nutrients

303(d) Segments Impaired by Construction Related Pollutant(s)

Dutchess	Fall Kill and tribs	Nutrients
Dutchess	Hillside Lake	Nutrients
Dutchess	Wappingers Lake	Nutrients
Dutchess	Wappingers Lake	Silt/Sediment
Erie	Beeman Creek and tribs	Nutrients
Erie	Ellicott Creek, Lower, and tribs	Silt/Sediment
Erie	Ellicott Creek, Lower, and tribs	Nutrients
Erie	Green Lake	Nutrients
Erie	Little Sister Creek, Lower, and tribs	Nutrients
Erie	Murder Creek, Lower, and tribs	Nutrients
Erie	Rush Creek and tribs	Nutrients
Erie	Scajaquada Creek, Lower, and tribs	Nutrients
Erie	Scajaquada Creek, Middle, and tribs	Nutrients
Erie	Scajaquada Creek, Upper, and tribs	Nutrients
Erie	South Branch Smoke Cr, Lower, and tribs	Silt/Sediment
Erie	South Branch Smoke Cr, Lower, and tribs	Nutrients
Essex	Lake Champlain, Main Lake, South	Nutrients
Essex	Lake Champlain, South Lake	Nutrients
Essex	Willsboro Bay	Nutrients
Genesee	Bigelow Creek and tribs	Nutrients
Genesee	Black Creek, Middle, and minor tribs	Nutrients
Genesee	Black Creek, Upper, and minor tribs	Nutrients
Genesee	Bowen Brook and tribs	Nutrients
Genesee	LeRoy Reservoir	Nutrients
Genesee	Oak Orchard Cr, Upper, and tribs	Nutrients
Genesee	Tonawanda Creek, Middle, Main Stem	Nutrients
Greene	Schoharie Reservoir	Silt/Sediment
Greene	Sleepy Hollow Lake	Silt/Sediment
Herkimer	Steele Creek tribs	Silt/Sediment
Herkimer	Steele Creek tribs	Nutrients
Jefferson	Moon Lake	Nutrients
Kings	Hendrix Creek	Nutrients
Kings	Prospect Park Lake	Nutrients
Lewis	Mill Creek/South Branch, and tribs	Nutrients
Livingston	Christie Creek and tribs	Nutrients
Livingston	Conesus Lake	Nutrients
Livingston	Mill Creek and minor tribs	Silt/Sediment
Monroe	Black Creek, Lower, and minor tribs	Nutrients
Monroe	Buck Pond	Nutrients
Monroe	Cranberry Pond	Nutrients

303(d) Segments Impaired by Construction Related Pollutant(s)

Monroe	Lake Ontario Shoreline, Western	Nutrients
Monroe	Long Pond	Nutrients
Monroe	Mill Creek and tribs	Nutrients
Monroe	Mill Creek/Blue Pond Outlet and tribs	Nutrients
Monroe	Minor Tribs to Irondequoit Bay	Nutrients
Monroe	Rochester Embayment - East	Nutrients
Monroe	Rochester Embayment - West	Nutrients
Monroe	Shipbuilders Creek and tribs	Nutrients
Monroe	Thomas Creek/White Brook and tribs	Nutrients
Nassau	Beaver Lake	Nutrients
Nassau	Camaans Pond	Nutrients
Nassau	East Meadow Brook, Upper, and tribs	Silt/Sediment
Nassau	East Rockaway Channel	Nutrients
Nassau	Grant Park Pond	Nutrients
Nassau	Hempstead Bay	Nutrients
Nassau	Hempstead Lake	Nutrients
Nassau	Hewlett Bay	Nutrients
Nassau	Hog Island Channel	Nutrients
Nassau	Long Island Sound, Nassau County Waters	Nutrients
Nassau	Massapequa Creek and tribs	Nutrients
Nassau	Milburn/Parsonage Creeks, Upp, and tribs	Nutrients
Nassau	Reynolds Channel, west	Nutrients
Nassau	Tidal Tribs to Hempstead Bay	Nutrients
Nassau	Tribs (fresh) to East Bay	Nutrients
Nassau	Tribs (fresh) to East Bay	Silt/Sediment
Nassau	Tribs to Smith/Halls Ponds	Nutrients
Nassau	Woodmere Channel	Nutrients
New York	Harlem Meer	Nutrients
New York	The Lake in Central Park	Nutrients
Niagara	Bergholtz Creek and tribs	Nutrients
Niagara	Hyde Park Lake	Nutrients
Niagara	Lake Ontario Shoreline, Western	Nutrients
Niagara	Lake Ontario Shoreline, Western	Nutrients
Oneida	Ballou, Nail Creeks and tribs	Nutrients
Onondaga	Harbor Brook, Lower, and tribs	Nutrients
Onondaga	Ley Creek and tribs	Nutrients
Onondaga	Minor Tribs to Onondaga Lake	Nutrients
Onondaga	Ninemile Creek, Lower, and tribs	Nutrients
Onondaga	Onondaga Creek, Lower, and tribs	Nutrients
Onondaga	Onondaga Creek, Middle, and tribs	Nutrients

303(d) Segments Impaired by Construction Related Pollutant(s)

Onondaga	Onondaga Lake, northern end	Nutrients
Onondaga	Onondaga Lake, southern end	Nutrients
Ontario	Great Brook and minor tribs	Silt/Sediment
Ontario	Great Brook and minor tribs	Nutrients
Ontario	Hemlock Lake Outlet and minor tribs	Nutrients
Ontario	Honeoye Lake	Nutrients
Orange	Greenwood Lake	Nutrients
Orange	Monhagen Brook and tribs	Nutrients
Orange	Orange Lake	Nutrients
Orleans	Lake Ontario Shoreline, Western	Nutrients
Orleans	Lake Ontario Shoreline, Western	Nutrients
Oswego	Lake Neatahwanta	Nutrients
Oswego	Pleasant Lake	Nutrients
Putnam	Bog Brook Reservoir	Nutrients
Putnam	Boyd Corners Reservoir	Nutrients
Putnam	Croton Falls Reservoir	Nutrients
Putnam	Diverting Reservoir	Nutrients
Putnam	East Branch Reservoir	Nutrients
Putnam	Lake Carmel	Nutrients
Putnam	Middle Branch Reservoir	Nutrients
Putnam	Oscawana Lake	Nutrients
Putnam	Palmer Lake	Nutrients
Putnam	West Branch Reservoir	Nutrients
Queens	Bergen Basin	Nutrients
Queens	Flushing Creek/Bay	Nutrients
Queens	Jamaica Bay, Eastern, and tribs (Queens)	Nutrients
Queens	Kissena Lake	Nutrients
Queens	Meadow Lake	Nutrients
Queens	Willow Lake	Nutrients
Rensselaer	Nassau Lake	Nutrients
Rensselaer	Snyders Lake	Nutrients
Richmond	Grasmere Lake/Bradys Pond	Nutrients
Rockland	Congers Lake, Swartout Lake	Nutrients
Rockland	Rockland Lake	Nutrients
Saratoga	Ballston Lake	Nutrients
Saratoga	Dwaas Kill and tribs	Silt/Sediment
Saratoga	Dwaas Kill and tribs	Nutrients
Saratoga	Lake Lonely	Nutrients
Saratoga	Round Lake	Nutrients
Saratoga	Tribs to Lake Lonely	Nutrients

303(d) Segments Impaired by Construction Related Pollutant(s)

Schenectady	Collins Lake	Nutrients
Schenectady	Duane Lake	Nutrients
Schenectady	Mariaville Lake	Nutrients
Schoharie	Engleville Pond	Nutrients
Schoharie	Summit Lake	Nutrients
Seneca	Reeder Creek and tribs	Nutrients
St.Lawrence	Black Lake Outlet/Black Lake	Nutrients
St.Lawrence	Fish Creek and minor tribs	Nutrients
Steuben	Smith Pond	Nutrients
Suffolk	Agawam Lake	Nutrients
Suffolk	Big/Little Fresh Ponds	Nutrients
Suffolk	Canaan Lake	Silt/Sediment
Suffolk	Canaan Lake	Nutrients
Suffolk	Flanders Bay, West/Lower Sawmill Creek	Nutrients
Suffolk	Fresh Pond	Nutrients
Suffolk	Great South Bay, East	Nutrients
Suffolk	Great South Bay, Middle	Nutrients
Suffolk	Great South Bay, West	Nutrients
Suffolk	Lake Ronkonkoma	Nutrients
Suffolk	Long Island Sound, Suffolk County, West	Nutrients
Suffolk	Mattituck (Marratooka) Pond	Nutrients
Suffolk	Meetinghouse/Terrys Creeks and tribs	Nutrients
Suffolk	Mill and Seven Ponds	Nutrients
Suffolk	Millers Pond	Nutrients
Suffolk	Moriches Bay, East	Nutrients
Suffolk	Moriches Bay, West	Nutrients
Suffolk	Peconic River, Lower, and tidal tribs	Nutrients
Suffolk	Quantuck Bay	Nutrients
Suffolk	Shinnecock Bay and Inlet	Nutrients
Suffolk	Tidal tribs to West Moriches Bay	Nutrients
Sullivan	Bodine, Montgomery Lakes	Nutrients
Sullivan	Davies Lake	Nutrients
Sullivan	Evens Lake	Nutrients
Sullivan	Pleasure Lake	Nutrients
Tompkins	Cayuga Lake, Southern End	Nutrients
Tompkins	Cayuga Lake, Southern End	Silt/Sediment
Tompkins	Owasco Inlet, Upper, and tribs	Nutrients
Ulster	Ashokan Reservoir	Silt/Sediment
Ulster	Esopus Creek, Upper, and minor tribs	Silt/Sediment
Warren	Hague Brook and tribs	Silt/Sediment

303(d) Segments Impaired by Construction Related Pollutant(s)

Warren	Huddle/Finkle Brooks and tribs	Silt/Sediment
Warren	Indian Brook and tribs	Silt/Sediment
Warren	Lake George	Silt/Sediment
Warren	Tribs to L.George, Village of L George	Silt/Sediment
Washington	Cossayuna Lake	Nutrients
Washington	Lake Champlain, South Bay	Nutrients
Washington	Tribs to L.George, East Shore	Silt/Sediment
Washington	Wood Cr/Champlain Canal and minor tribs	Nutrients
Wayne	Port Bay	Nutrients
Westchester	Amawalk Reservoir	Nutrients
Westchester	Blind Brook, Upper, and tribs	Silt/Sediment
Westchester	Cross River Reservoir	Nutrients
Westchester	Lake Katonah	Nutrients
Westchester	Lake Lincolndale	Nutrients
Westchester	Lake Meahagh	Nutrients
Westchester	Lake Mohegan	Nutrients
Westchester	Lake Shenorock	Nutrients
Westchester	Long Island Sound, Westchester (East)	Nutrients
Westchester	Mamaroneck River, Lower	Silt/Sediment
Westchester	Mamaroneck River, Upper, and minor tribs	Silt/Sediment
Westchester	Muscoot/Upper New Croton Reservoir	Nutrients
Westchester	New Croton Reservoir	Nutrients
Westchester	Peach Lake	Nutrients
Westchester	Reservoir No.1 (Lake Isle)	Nutrients
Westchester	Saw Mill River, Lower, and tribs	Nutrients
Westchester	Saw Mill River, Middle, and tribs	Nutrients
Westchester	Sheldrake River and tribs	Silt/Sediment
Westchester	Sheldrake River and tribs	Nutrients
Westchester	Silver Lake	Nutrients
Westchester	Teatown Lake	Nutrients
Westchester	Titicus Reservoir	Nutrients
Westchester	Truesdale Lake	Nutrients
Westchester	Wallace Pond	Nutrients
Wyoming	Java Lake	Nutrients
Wyoming	Silver Lake	Nutrients

APPENDIX F – List of NYS DEC Regional Offices

<u>Region</u>	<u>COVERING THE FOLLOWING COUNTIES:</u>	<u>DIVISION OF ENVIRONMENTAL PERMITS (DEP) PERMIT ADMINISTRATORS</u>	<u>DIVISION OF WATER (DOW) WATER (SPDES) PROGRAM</u>
1	NASSAU AND SUFFOLK	50 CIRCLE ROAD STONY BROOK, NY 11790 TEL. (631) 444-0365	50 CIRCLE ROAD STONY BROOK, NY 11790-3409 TEL. (631) 444-0405
2	BRONX, KINGS, NEW YORK, QUEENS AND RICHMOND	1 HUNTERS POINT PLAZA, 47-40 21ST ST. LONG ISLAND CITY, NY 11101-5407 TEL. (718) 482-4997	1 HUNTERS POINT PLAZA, 47-40 21ST ST. LONG ISLAND CITY, NY 11101-5407 TEL. (718) 482-4933
3	DUTCHESS, ORANGE, PUTNAM, ROCKLAND, SULLIVAN, ULSTER AND WESTCHESTER	21 SOUTH PUTT CORNERS ROAD NEW PALTZ, NY 12561-1696 TEL. (845) 256-3059	100 HILLSIDE AVENUE, SUITE 1W WHITE PLAINS, NY 10603 TEL. (914) 428 - 2505
4	ALBANY, COLUMBIA, DELAWARE, GREENE, MONTGOMERY, OTSEGO, RENSSELAER, SCHENECTADY AND SCHOHARIE	1150 NORTH WESTCOTT ROAD SCHENECTADY, NY 12306-2014 TEL. (518) 357-2069	1130 NORTH WESTCOTT ROAD SCHENECTADY, NY 12306-2014 TEL. (518) 357-2045
5	CLINTON, ESSEX, FRANKLIN, FULTON, HAMILTON, SARATOGA, WARREN AND WASHINGTON	1115 STATE ROUTE 86, Po Box 296 RAY BROOK, NY 12977-0296 TEL. (518) 897-1234	232 GOLF COURSE ROAD WARRENSBURG, NY 12885-1172 TEL. (518) 623-1200
6	HERKIMER, JEFFERSON, LEWIS, ONEIDA AND ST. LAWRENCE	STATE OFFICE BUILDING 317 WASHINGTON STREET WATERTOWN, NY 13601-3787 TEL. (315) 785-2245	STATE OFFICE BUILDING 207 GENESEE STREET UTICA, NY 13501-2885 TEL. (315) 793-2554
7	BROOME, CAYUGA, CHENANGO, CORTLAND, MADISON, ONONDAGA, OSWEGO, TIOGA AND TOMPKINS	615 ERIE BLVD. WEST SYRACUSE, NY 13204-2400 TEL. (315) 426-7438	615 ERIE BLVD. WEST SYRACUSE, NY 13204-2400 TEL. (315) 426-7500
8	CHEMUNG, GENESEE, LIVINGSTON, MONROE, ONTARIO, ORLEANS, SCHUYLER, SENECA, STEUBEN, WAYNE AND YATES	6274 EAST AVON-LIMA ROADAVON, NY 14414-9519 TEL. (585) 226-2466	6274 EAST AVON-LIMA RD. AVON, NY 14414-9519 TEL. (585) 226-2466
9	ALLEGANY, CATTARAUGUS, CHAUTAUQUA, ERIE, NIAGARA AND WYOMING	270 MICHIGAN AVENUE BUFFALO, NY 14203-2999 TEL. (716) 851-7165	270 MICHIGAN AVENUE BUFFALO, NY 14203-2999 TEL. (716) 851-7070

APPENDIX E



Department of
Environmental
Conservation

NYS Department of Environmental Conservation
Division of Water
625 Broadway, 4th Floor
Albany, New York 12233-3505

MS4 Stormwater Pollution Prevention Plan (SWPPP) Acceptance Form

for

Construction Activities Seeking Authorization Under SPDES General Permit

*(NOTE: Attach Completed Form to Notice Of Intent and Submit to Address Above)

I. Project Owner/Operator Information

- | | |
|-------------------------|------------------------------|
| 1. Owner/Operator Name: | O'Neill Group Hamilton, LLC |
| 2. Contact Person: | George Brandt |
| 3. Street Address: | 241 Hudson Avenue |
| 4. City/State/Zip: | Hackensack, New Jersey 07601 |

II. Project Site Information

- | | |
|-----------------------|-----------------------|
| 5. Project/Site Name: | South Hamilton |
| 6. Street Address: | South Hamilton Street |
| 7. City/State/Zip: | South Hamilton |

III. Stormwater Pollution Prevention Plan (SWPPP) Review and Acceptance Information

- | | |
|---|------------------------|
| 8. SWPPP Reviewed by: | City of Poughkeepsie |
| 9. Title/Position: | Stormwater Coordinator |
| 10. Date Final SWPPP Reviewed and Accepted: | |

IV. Regulated MS4 Information

- | | |
|---|----------------------|
| 11. Name of MS4: | City of Poughkeepsie |
| 12. MS4 SPDES Permit Identification Number: | NYR20A |
| 13. Contact Person: | |
| 14. Street Address: | |
| 15. City/State/Zip: | |
| 16. Telephone Number: | |

MS4 SWPPP Acceptance Form - continued

V. Certification Statement - MS4 Official (principal executive officer or ranking elected official) or Duly Authorized Representative

I hereby certify that the final Stormwater Pollution Prevention Plan (SWPPP) for the construction project identified in question 5 has been reviewed and meets the substantive requirements in the SPDES General Permit For Stormwater Discharges from Municipal Separate Storm Sewer Systems (MS4s). Note: The MS4, through the acceptance of the SWPPP, assumes no responsibility for the accuracy and adequacy of the design included in the SWPPP. In addition, review and acceptance of the SWPPP by the MS4 does not relieve the owner/operator or their SWPPP preparer of responsibility or liability for errors or omissions in the plan.

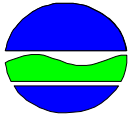
Printed Name:

Title/Position:

Signature:

Date:

VI. Additional Information

NOTICE OF INTENT**New York State Department of Environmental Conservation****Division of Water****625 Broadway, 4th Floor****Albany, New York 12233-3505**
NYR
 (for DEC use only)

**Stormwater Discharges Associated with Construction Activity Under State
 Pollutant Discharge Elimination System (SPDES) General Permit # GP-0-20-001**

All sections must be completed unless otherwise noted. Failure to complete all items may result in this form being returned to you, thereby delaying your coverage under this General Permit. Applicants must read and understand the conditions of the permit and prepare a Stormwater Pollution Prevention Plan prior to submitting this NOI. Applicants are responsible for identifying and obtaining other DEC permits that may be required.

**-IMPORTANT-
 RETURN THIS FORM TO THE ADDRESS ABOVE**

OWNER/OPERATOR MUST SIGN FORM

Owner/Operator Information

Owner/Operator (Company Name/Private Owner Name/Municipality Name)

Owner/Operator Contact Person Last Name (NOT CONSULTANT)

Owner/Operator Contact Person First Name

Owner/Operator Mailing Address

City

State

Zip

-

Phone (Owner/Operator)

 - -

Fax (Owner/Operator)

 - -

Email (Owner/Operator)

FED TAX ID

-

(not required for individuals)

Project Site Information																											
Project/Site Name																											
Street Address (NOT P.O. BOX)																											
Side of Street																											
☐ North ☐ South ☐ East ☐ West																											
City/Town/Village (THAT ISSUES BUILDING PERMIT)																											
State							Zip							County							DEC Region						
N Y																											
Name of Nearest Cross Street																											
Distance to Nearest Cross Street (Feet)														Project In Relation to Cross Street													
														☐ North ☐ South ☐ East ☐ West													
Tax Map Numbers Section-Block-Parcel														Tax Map Numbers													

1. Provide the Geographic Coordinates for the project site. To do this, go to the NYSDEC Stormwater Interactive Map on the DEC website at:

<https://giservices.dec.ny.gov/gis/stormwater/>

Zoom into your Project Location such that you can accurately click on the centroid of your site. Once you have located the centroid of your project site, go to the bottom right hand corner of the map for the X, Y coordinates. Enter the coordinates into the boxes below. For problems with the interactive map use the help function.

X Coordinates (Easting)

-7					
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Ex. -73.749

Y Coordinates (Northing)

4						
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Ex. 42.652

2. What is the nature of this construction project?

- ☐ New Construction
- ☐ Redevelopment with increase in impervious area
- ☐ Redevelopment with no increase in impervious area

SELECT ONLY ONE CHOICE FOR EACH

Post-Development Future Land Use

- ☐ FOREST
- ☐ PASTURE/OPEN LAND
- ☐ CULTIVATED LAND
- ☐ SINGLE FAMILY HOME
- ☐ SINGLE FAMILY SUBDIVISION
- ☐ TOWN HOME RESIDENTIAL
- ☐ MULTIFAMILY RESIDENTIAL
- ☐ INSTITUTIONAL/SCHOOL
- ☐ INDUSTRIAL
- ☐ COMMERCIAL
- ☐ ROAD/HIGHWAY
- ☐ RECREATIONAL/SPORTS FIELD
- ☐ BIKE PATH/TRAIL
- ☐ LINEAR UTILITY
- ☐ PARKING LOT
- ☐ OTHER

[illegible]

	Number of Lots
☐ SINGLE FAMILY HOME	
☐ SINGLE FAMILY SUBDIVISION	
☐ TOWN HOME RESIDENTIAL	
☐ MULTIFAMILY RESIDENTIAL	
☐ INSTITUTIONAL/SCHOOL	
☐ INDUSTRIAL	
☐ COMMERCIAL	
☐ MUNICIPAL	
☐ ROAD/HIGHWAY	
☐ RECREATIONAL/SPORTS FIELD	
☐ BIKE PATH/TRAIL	
☐ LINEAR UTILITY (water, sewer, gas, etc.)	
☐ PARKING LOT	
☐ CLEARING/GRADING ONLY	
☐ DEMOLITION, NO REDEVELOPMENT	
☐ WELL DRILLING ACTIVITY *(Oil, Gas, etc.)	
☐ OTHER	

[illegible]

***Note:** for gas well drilling, non-high volume hydraulic fractured wells only

4. In accordance with the larger common plan of development or sale, enter the total project site area; the total area to be disturbed; existing impervious area to be disturbed (for redevelopment activities); and the future impervious area constructed within the disturbed area. (Round to the nearest tenth of an acre.)

Total Site Area	Total Area To Be Disturbed	Existing Impervious Area To Be Disturbed	Future Impervious Area Within Disturbed Area
.	.	.	.

5. Do you plan to disturb more than 5 acres of soil at any one time? ☐ Yes ☐ No

6. Indicate the percentage of each Hydrologic Soil Group(HSG) at the site.

A

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 %

B

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 %

C

--	--	--

 %

D

--	--	--

 %

7. Is this a phased project? ☐ Yes ☐ No

8. Enter the planned start and end dates of the disturbance activities.

Start Date **End Date**
 / / - / /

[illegible]

☐ Wetland / State Jurisdiction On Site (Answer 9b)

☐ Wetland / State Jurisdiction Off Site

☐ Wetland / Federal Jurisdiction On Site (Answer 9b)

☐ Wetland / Federal Jurisdiction Off Site

☐ Stream / Creek On Site

☐ Stream / Creek Off Site

☐ River On Site

☐ River Off Site

☐ Lake On Site

☐ Lake Off Site

☐ Other Type On Site

☐ Other Type Off Site

☐ Regulatory Map

☐ Delineated by Consultant

☐ Delineated by Army Corps of Engineers

☐ Other (identify)

11. Is this project located in one of the Watersheds identified in Appendix C of GP-0-20-001? ☐ Yes ☐ No

13. Does this construction activity disturb land with no existing impervious cover and where the Soil Slope Phase is identified as an E or F on the USDA Soil Survey? ☐ Yes ☐ No

If Yes, what is the acreage to be disturbed?

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14. Will the project disturb soils within a State regulated wetland or the protected 100 foot adjacent area? ☐ Yes ☐ No

15. Does the site runoff enter a separate storm sewer system (including roadside drains, swales, ditches, culverts, etc)? ☐ Yes ☐ No ☐ Unknown

- [illegible]

17. Does any runoff from the site enter a sewer classified as a Combined Sewer? ☐ Yes ☐ No ☐ Unknown

18. Will future use of this site be an agricultural property as defined by the NYS Agriculture and Markets Law? ☐ Yes ☐ No

19. Is this property owned by a state authority, state agency, federal government or local government? ☐ Yes ☐ No

20. Is this a remediation project being done under a Department approved work plan? (i.e. CERCLA, RCRA, Voluntary Cleanup Agreement, etc.) ☐ Yes ☐ No

21. Has the required Erosion and Sediment Control component of the SWPPP been developed in conformance with the current NYS Standards and Specifications for Erosion and Sediment Control (aka Blue Book)? ☐ Yes ☐ No

22. Does this construction activity require the development of a SWPPP that includes the post-construction stormwater management practice component (i.e. Runoff Reduction, Water Quality and Quantity Control practices/techniques)? ☐ Yes ☐ No
- If No, skip questions 23 and 27-39.**

23. Has the post-construction stormwater management practice component of the SWPPP been developed in conformance with the current NYS Stormwater Management Design Manual? ☐ Yes ☐ No

24. The Stormwater Pollution Prevention Plan (SWPPP) was prepared by:

- ☐ Professional Engineer (P.E.)
- ☐ Soil and Water Conservation District (SWCD)
- ☐ Registered Landscape Architect (R.L.A.)
- ☐ Certified Professional in Erosion and Sediment Control (CPESC)
- ☐ Owner/Operator
- ☐ Other

[illegible]

SWPPP Preparer

[illegible]

Contact Name (Last, Space, First)

[illegible]

Mailing Address

[illegible]

City

[illegible]

State Zip

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Phone

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Fax

-

-

Email

[illegible][illegible]

SWPPP Preparer Certification

I hereby certify that the Stormwater Pollution Prevention Plan (SWPPP) for this project has been prepared in accordance with the terms and conditions of the GP-0-20-001. Furthermore, I understand that certifying false, incorrect or inaccurate information is a violation of this permit and the laws of the State of New York and could subject me to criminal, civil and/or administrative proceedings.

First Name

[illegible]

MI

--	--

Last Name

[illegible]

Signature

--

Date _____

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☐ Yes ☐ No

26. Select **all** of the erosion and sediment control practices that will be employed on the project site:

Temporary Structural

- ☐ Check Dams
- ☐ Construction Road Stabilization
- ☐ Dust Control
- ☐ Earth Dike
- ☐ Level Spreader
- ☐ Perimeter Dike/Swale
- ☐ Pipe Slope Drain
- ☐ Portable Sediment Tank
- ☐ Rock Dam
- ☐ Sediment Basin
- ☐ Sediment Traps
- ☐ Silt Fence
- ☐ Stabilized Construction Entrance
- ☐ Storm Drain Inlet Protection
- ☐ Straw/Hay Bale Dike
- ☐ Temporary Access Waterway Crossing
- ☐ Temporary Stormdrain Diversion
- ☐ Temporary Swale
- ☐ Turbidity Curtain
- ☐ Water bars

Biotechnical

- ☐ Brush Matting
- ☐ Wattling

Other

[illegible]

Vegetative Measures

- ☐ Brush Matting
- ☐ Dune Stabilization
- ☐ Grassed Waterway
- ☐ Mulching
- ☐ Protecting Vegetation
- ☐ Recreation Area Improvement
- ☐ Seeding
- ☐ Sodding
- ☐ Straw/Hay Bale Dike
- ☐ Streambank Protection
- ☐ Temporary Swale
- ☐ Topsoiling
- ☐ Vegetating Waterways

Permanent Structural

- ☐ Debris Basin
- ☐ Diversion
- ☐ Grade Stabilization Structure
- ☐ Land Grading
- ☐ Lined Waterway (Rock)
- ☐ Paved Channel (Concrete)
- ☐ Paved Flume
- ☐ Retaining Wall
- ☐ Riprap Slope Protection
- ☐ Rock Outlet Protection
- ☐ Streambank Protection

Post-construction Stormwater Management Practice (SMP) Requirements

Important: Completion of Questions 27-39 is not required if response to Question 22 is No.

27. Identify all site planning practices that were used to prepare the final site plan/layout for the project.

- ☐ Preservation of Undisturbed Areas
- ☐ Preservation of Buffers
- ☐ Reduction of Clearing and Grading
- ☐ Locating Development in Less Sensitive Areas
- ☐ Roadway Reduction
- ☐ Sidewalk Reduction
- ☐ Driveway Reduction
- ☐ Cul-de-sac Reduction
- ☐ Building Footprint Reduction
- ☐ Parking Reduction

27a. Indicate which of the following soil restoration criteria was used to address the requirements in Section 5.1.6("Soil Restoration") of the Design Manual (2010 version).

- ☐ All disturbed areas will be restored in accordance with the Soil Restoration requirements in Table 5.3 of the Design Manual (see page 5-22).
- ☐ Compacted areas were considered as impervious cover when calculating the **WQv Required**, and the compacted areas were assigned a post-construction Hydrologic Soil Group (HSG) designation that is one level less permeable than existing conditions for the hydrology analysis.

28. Provide the total Water Quality Volume (WQv) required for this project (based on final site plan/layout).

Total WQv Required

. acre-feet

29. Identify the RR techniques (Area Reduction), RR techniques (Volume Reduction) and Standard SMPs with RRV Capacity in Table 1 (See Page 9) that were used to reduce the Total WQv Required (#28).

Also, provide in Table 1 the total impervious area that contributes runoff to each technique/practice selected. For the Area Reduction Techniques, provide the total contributing area (includes pervious area) and, if applicable, the total impervious area that contributes runoff to the technique/practice.

Note: Redevelopment projects shall use Tables 1 and 2 to identify the SMPs used to treat and/or reduce the WQv required. If runoff reduction techniques will not be used to reduce the required WQv, skip to question 33a after identifying the SMPs.

Table 1 - Runoff Reduction (RR) Techniques
and Standard Stormwater Management
Practices (SMPs)

RR Techniques (Area Reduction)	Total Contributing Area (acres)	Total Contributing Impervious Area (acres)
☐ Conservation of Natural Areas (RR-1) ...	.	and/or .
☐ Sheetflow to Riparian Buffers/Filters Strips (RR-2)	.	and/or .
☐ Tree Planting/Tree Pit (RR-3)	.	and/or .
☐ Disconnection of Rooftop Runoff (RR-4) ..	.	and/or .
RR Techniques (Volume Reduction)		
☐ Vegetated Swale (RR-5)	.	.
☐ Rain Garden (RR-6)	.	.
☐ Stormwater Planter (RR-7)	.	.
☐ Rain Barrel/Cistern (RR-8)	.	.
☐ Porous Pavement (RR-9)	.	.
☐ Green Roof (RR-10)	.	.
Standard SMPs with RRv Capacity		
☐ Infiltration Trench (I-1)	.	.
☐ Infiltration Basin (I-2)	.	.
☐ Dry Well (I-3)	.	.
☐ Underground Infiltration System (I-4)	.	.
☐ Bioretention (F-5)	.	.
☐ Dry Swale (O-1)	.	.
Standard SMPs		
☐ Micropool Extended Detention (P-1)	.	.
☐ Wet Pond (P-2)	.	.
☐ Wet Extended Detention (P-3)	.	.
☐ Multiple Pond System (P-4)	.	.
☐ Pocket Pond (P-5)	.	.
☐ Surface Sand Filter (F-1)	.	.
☐ Underground Sand Filter (F-2)	.	.
☐ Perimeter Sand Filter (F-3)	.	.
☐ Organic Filter (F-4)	.	.
☐ Shallow Wetland (W-1)	.	.
☐ Extended Detention Wetland (W-2)	.	.
☐ Pond/Wetland System (W-3)	.	.
☐ Pocket Wetland (W-4)	.	.
☐ Wet Swale (O-2)	.	.

Note: Redevelopment projects which do not use RR techniques, shall use questions 28, 29, 33 and 33a to provide SMPs used, total WQv required and total WQv provided for the project.

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acre-feet

If Yes, go to question 36.
If No, go to question 32.

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acre-feet

33. Identify the Standard SMPs in Table 1 and, if applicable, the Alternative SMPs in Table 2 that were used to treat the remaining total WQv(=Total WQv Required in 28 - Total RRV Provided in 30).

Also, provide in Table 1 and 2 the total impervious area that contributes runoff to each practice selected.

Note: Use Tables 1 and 2 to identify the SMPs used on Redevelopment projects.

- 33a. Indicate the Total WQv provided (i.e. WQv treated) by the SMPs identified in question #33 and Standard SMPs with RRV Capacity identified in question 29.

WQv Provided

. acre-feet

Note: For the standard SMPs with RRV capacity, the WQv provided by each practice = the WQv calculated using the contributing drainage area to the practice - RRV provided by the practice. (See Table 3.5 in Design Manual)

34. Provide the sum of the Total RRV provided (#30) and the WQv provided (#33a).

.

35. Is the sum of the RRV provided (#30) and the WQv provided (#33a) greater than or equal to the total WQv required (#28)? ☐ Yes ☐ No

If Yes, go to question 36.

If No, sizing criteria has not been met, so NOI can not be processed. SWPPP preparer must modify design to meet sizing criteria.

36. Provide the total Channel Protection Storage Volume (CPv) required and provided or select waiver (36a), if applicable.

CPv Required

. acre-feet

CPv Provided

. acre-feet

- 36a. The need to provide channel protection has been waived because:

- ☐ Site discharges directly to tidal waters or a fifth order or larger stream.
- ☐ Reduction of the total CPv is achieved on site through runoff reduction techniques or infiltration systems.

37. Provide the Overbank Flood (Qp) and Extreme Flood (Qf) control criteria or select waiver (37a), if applicable.

Total Overbank Flood Control Criteria (Qp)

Pre-Development

. CFS

Post-development

. CFS

Total Extreme Flood Control Criteria (Qf)

Pre-Development

. CFS

Post-development

. CFS

37a. The need to meet the Qp and Qf criteria has been waived because:

- ☐ Site discharges directly to tidal waters or a fifth order or larger stream.
- ☐ Downstream analysis reveals that the Qp and Qf controls are not required

- ☐ Site discharges directly to tidal waters or a fifth order or larger stream.
- ☐ Downstream analysis reveals that the Qp and Qf controls are not required

☐ **Yes** ☐ **No**

[illegible]

39. Use this space to summarize the specific site limitations and justification for not reducing 100% of WQv required(#28). (See question 32a)
This space can also be used for other pertinent project information.

40. Identify other DEC permits, existing and new, that are required for this project/facility.

- ☐ Air Pollution Control
- ☐ Coastal Erosion
- ☐ Hazardous Waste
- ☐ Long Island Wells
- ☐ Mined Land Reclamation
- ☐ Solid Waste
- ☐ Navigable Waters Protection / Article 15
- ☐ Water Quality Certificate
- ☐ Dam Safety
- ☐ Water Supply
- ☐ Freshwater Wetlands/Article 24
- ☐ Tidal Wetlands
- ☐ Wild, Scenic and Recreational Rivers
- ☐ Stream Bed or Bank Protection / Article 15
- ☐ Endangered or Threatened Species(Incidental Take Permit)
- ☐ Individual SPDES
- ☐ SPDES Multi-Sector GP

N	Y	R							
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- ☐ Other

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- ☐ None

41. Does this project require a US Army Corps of Engineers Wetland Permit?

☐ Yes ☐ No

If Yes, Indicate Size of Impact.

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42. Is this project subject to the requirements of a regulated, traditional land use control MS4?
(If No, skip question 43)

☐ Yes ☐ No

43. Has the "MS4 SWPPP Acceptance" form been signed by the principal executive officer or ranking elected official and submitted along with this NOI?

☐ Yes ☐ No

44. If this NOI is being submitted for the purpose of continuing or transferring coverage under a general permit for stormwater runoff from construction activities, please indicate the former SPDES number assigned.

N	Y	R							
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Owner/Operator Certification

I have read or been advised of the permit conditions and believe that I understand them. I also understand that, under the terms of the permit, there may be reporting requirements. I hereby certify that this document and the corresponding documents were prepared under my direction or supervision. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations. I further understand that coverage under the general permit will be identified in the acknowledgment that I will receive as a result of submitting this NOI and can be as long as sixty (60) business days as provided for in the general permit. I also understand that, by submitting this NOI, I am acknowledging that the SWPPP has been developed and will be implemented as the first element of construction, and agreeing to comply with all the terms and conditions of the general permit for which this NOI is being submitted.

Print First Name

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Print Last Name

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Owner/Operator Signature

MI

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Date

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**New York State Department of Environmental Conservation
Division of Water
625 Broadway, 4th Floor
Albany, New York 12233-3505**

(NOTE: Submit completed form to address above)

NOTICE OF TERMINATION for Storm Water Discharges Authorized
under the SPDES General Permit for Construction Activity

Please indicate your permit identification number: NYR ____ _

I. Owner or Operator Information

1. Owner/Operator Name: O'Neill Group Hamilton, LLC

2. Street Address: 241 Hudson Avenue

3. City/State/Zip: Hackensack, New Jersey 07601

4. Contact Person: George Brandt

4a. Telephone: 201-488-4455

4b. Contact Person E-Mail: wawrla@gmail.com

II. Project Site Information

5. Project/Site Name: South Hamilton

6. Street Address: South Hamilton Street

7. City/Zip: City of Poughkeepsie, NY 12061

8. County: Dutchess

III. Reason for Termination

9a. ☐ All disturbed areas have achieved final stabilization in accordance with the general permit and SWPPP. ***Date final stabilization completed** (month/year): _____

9b. ☐ Permit coverage has been transferred to new owner/operator. Indicate new owner/operator's permit identification number: NYR ____ _

(Note: Permit coverage can not be terminated by owner identified in I.1. above until new owner/operator obtains coverage under the general permit)

9c. ☐ Other (Explain on Page 2)

IV. Final Site Information:

10a. Did this construction activity require the development of a SWPPP that includes post-construction stormwater management practices? ☐ yes ☐ no (If no, go to question 10f.)

10b. Have all post-construction stormwater management practices included in the final SWPPP been constructed? ☐ yes ☐ no (If no, explain on Page 2)

10c. Identify the entity responsible for long-term operation and maintenance of practice(s)?

**NOTICE OF TERMINATION for Storm Water Discharges Authorized under the
SPDES General Permit for Construction Activity - continued**

10d. Has the entity responsible for long-term operation and maintenance been given a copy of the operation and maintenance plan required by the general permit? ☐ yes ☐ no

10e. Indicate the method used to ensure long-term operation and maintenance of the post-construction stormwater management practice(s):

- ☐ Post-construction stormwater management practice(s) and any right-of-way(s) needed to maintain practice(s) have been deeded to the municipality.
- ☐ Executed maintenance agreement is in place with the municipality that will maintain the post-construction stormwater management practice(s).
- ☐ For post-construction stormwater management practices that are privately owned, a mechanism is in place that requires operation and maintenance of the practice(s) in accordance with the operation and maintenance plan, such as a deed covenant in the owner or operator's deed of record.
- ☐ For post-construction stormwater management practices that are owned by a public or private institution (e.g. school, university or hospital), government agency or authority, or public utility; policy and procedures are in place that ensures operation and maintenance of the practice(s) in accordance with the operation and maintenance plan.

10f. Provide the total area of impervious surface (i.e. roof, pavement, concrete, gravel, etc.) constructed within the disturbance area? _____
(acres)

11. Is this project subject to the requirements of a regulated, traditional land use control MS4? ☐ yes
☐ no
(If Yes, complete section VI - "MS4 Acceptance" statement)

V. Additional Information/Explanation:
(Use this section to answer questions 9c. and 10b., if applicable)

VI. MS4 Acceptance - MS4 Official (principal executive officer or ranking elected official) or Duly Authorized Representative (Note: Not required when 9b. is checked -transfer of coverage)

I have determined that it is acceptable for the owner or operator of the construction project identified in question 5 to submit the Notice of Termination at this time.

Printed Name:

Title/Position:

Signature:

Date:

NOTICE OF TERMINATION for Storm Water Discharges Authorized under the
SPDES General Permit for Construction Activity - continued

VII. Qualified Inspector Certification - Final Stabilization:

I hereby certify that all disturbed areas have achieved final stabilization as defined in the current version of the general permit, and that all temporary, structural erosion and sediment control measures have been removed. Furthermore, I understand that certifying false, incorrect or inaccurate information is a violation of the referenced permit and the laws of the State of New York and could subject me to criminal, civil and/or administrative proceedings.

Printed Name:

Title/Position:

Signature:

Date:

VIII. Qualified Inspector Certification - Post-construction Stormwater Management Practice(s):

I hereby certify that all post-construction stormwater management practices have been constructed in conformance with the SWPPP. Furthermore, I understand that certifying false, incorrect or inaccurate information is a violation of the referenced permit and the laws of the State of New York and could subject me to criminal, civil and/or administrative proceedings.

Printed Name:

Title/Position:

Signature:

Date:

IX. Owner or Operator Certification

I hereby certify that this document was prepared by me or under my direction or supervision. My determination, based upon my inquiry of the person(s) who managed the construction activity, or those persons directly responsible for gathering the information, is that the information provided in this document is true, accurate and complete. Furthermore, I understand that certifying false, incorrect or inaccurate information is a violation of the referenced permit and the laws of the State of New York and could subject me to criminal, civil and/or administrative proceedings.

Printed Name:

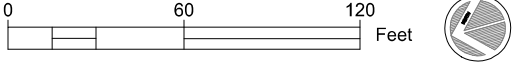
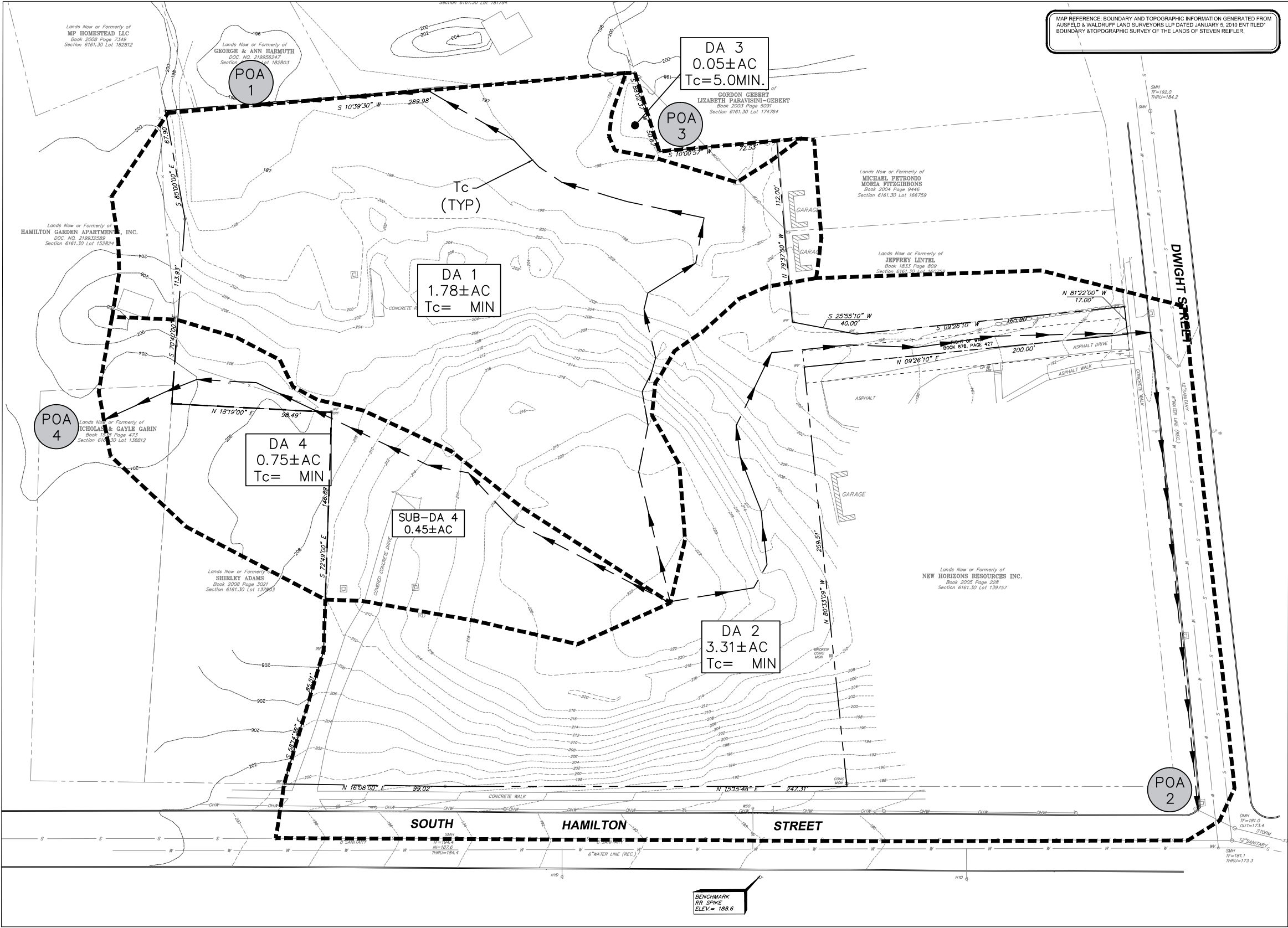
Title/Position:

Signature:

Date:

(NYS DEC Notice of Termination - January 2015)

APPENDIX F



☐ ISSUED FOR REVIEW
☐ NOT FOR CONSTRUCTION
☐ RECORD PLANS

DATE SUBMITTED:

NO.	REVISIONS	DESCRIPTION	BY	DATE

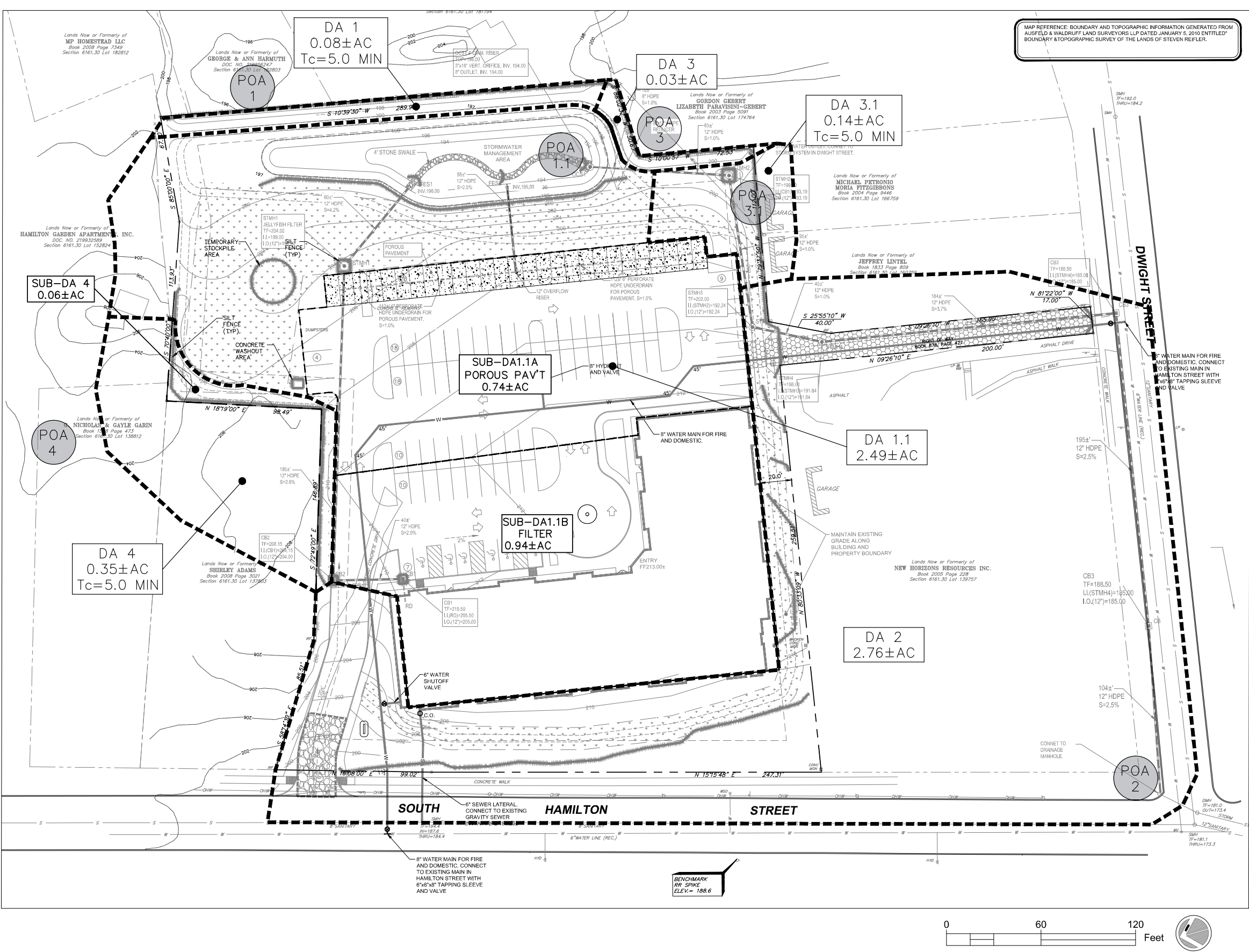
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ARICO ENGINEERS & CONSULTANTS ASSOCIATES
1407 ROUTE 28, Bldg 2 Suite 6, Clifton Park, New York 12065
518.257.3685 aricoassociates@gmail.com

QTY OF POUGHKEEPSIE COUNTY OF DUTCHESS
PRE-DEVELOPMENT DRAINAGE SOUTH HAMILTON
LOCATED AT
100 SOUTH HAMILTON STREET
TAX MAP: 6161-30-151783

DATE: AUGUST 2023
JOB NO: 22.10
SCALE: AS NOTED
DRAWN BY: DFA
CHECKED BY: DFA
DRAWING NO:
SWPPP 1.0
SHEET 2 OF 3



MAP REFERENCE: BOUNDARY AND TOPOGRAPHIC INFORMATION GENERATED FROM AUSFELD & WALDRUFF LAND SURVEYORS LLP DATED JANUARY 5, 2010 ENTITLED "BOUNDARY & TOPOGRAPHIC SURVEY OF THE LANDS OF STEVEN REIFLER."

- ☐ ISSUED FOR REVIEW
- ☐ NOT FOR CONSTRUCTION
- ☐ RECORD PLANS

DATE SUBMITTED:

NO.	REVISIONS	DESCRIPTION	DATE	BY
1		PER CITY COMMENTS	05.15.23	DFA

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1407 ROUTE 30, Suite 200, Clifton Park, New York 12065
518.267.3685
aricoassociates@arico.com

CITY OF POUGHKEEPSIE COUNTY OF DUTCHESS
POST-DEVELOPMENT DRAINAGE SOUTH HAMILTON
LOCATED AT
100 SOUTH HAMILTON STREET
TAX MAP: 6161.30-151763

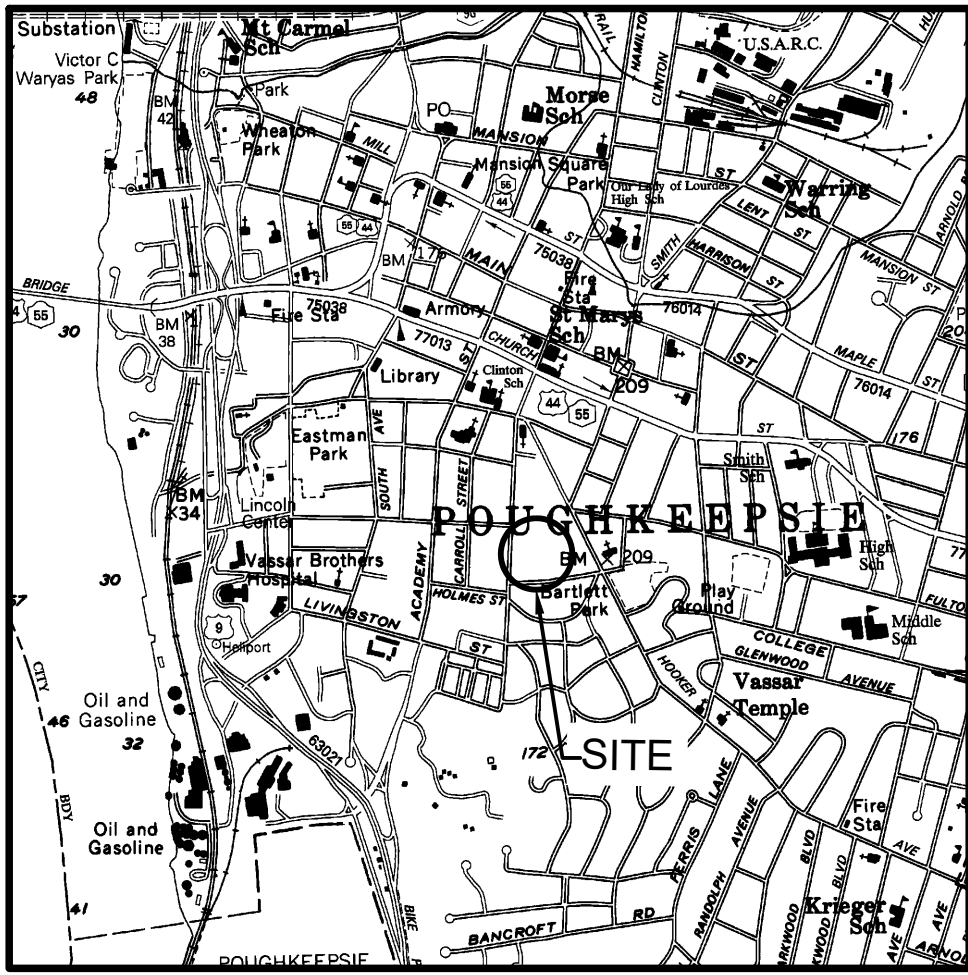
DATE: AUGUST 2023
JOB NO. 22.10
SCALE: AS NOTED
DRAWN BY: DFA
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DRAWING NO.
SWPPP 2.0

SHEET 1 OF 1

SOUTH HAMILTON

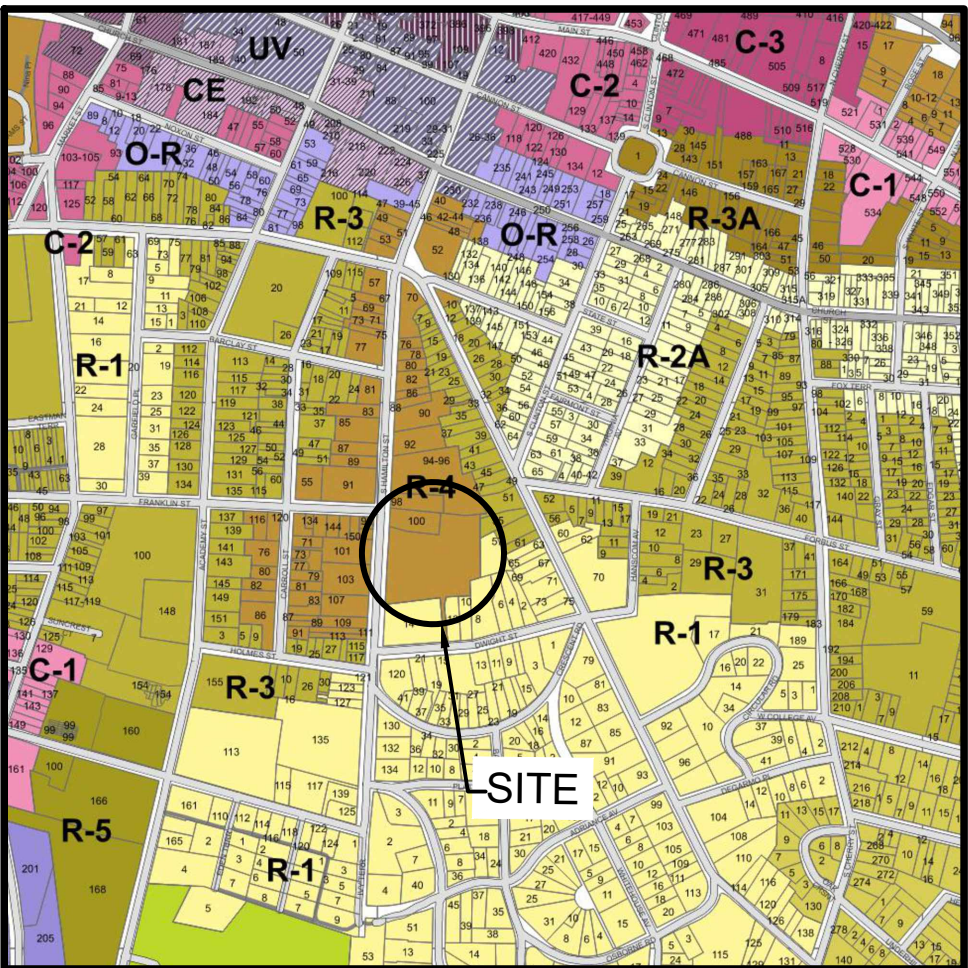
CITY OF POUGHKEEPSIE
COUNTY OF DUTCHESS STATE OF NEW YORK

OCTOBER 3, 2023



SITE LOCATION MAP

SCALE: 1"=2000'



ZONING MAP

SCALE: 1"=2000'

MAP REFERENCE: BOUNDARY AND TOPOGRAPHIC INFORMATION GENERATED FROM
AUSFELD & WALDRUFF LAND SURVEYORS LLP DATED JANUARY 5, 2010 ENTITLED "BOUNDARY & TOPOGRAPHIC SURVEY OF THE LANDS OF STEVEN REIFLER."

CITY OF POUGHKEEPSIE ZONING DISTRICT INFORMATION

DESCRIPTION	REQUIREMENT	PROPOSED
R4 - MEDIUM/HIGH DENSITY RESIDENTIAL		
MINIMUM LOT AREA	145,000 SF (+3 BEDROOM UNITS)	147,145.7 SF (26 UNITS)
MINIMUM LOT WIDTH	50 FT	346.3 FT
MINIMUM LOT FRONTAGE	30 FT	346.3 FT
FRONT SETBACK	35 FT	39.0 FT
SIDE SETBACK	20 FT (0.5 MAX HEIGHT OF 40 FT)	20.0 FT (LEFT SIDE) 65.0 FT (RIGHT SIDE)
REAR SETBACK	20 FT (0.5 MAX HEIGHT OF 40 FT)	215.4 FT
USABLE OPEN SPACE	40%	52.2%
MAX. BUILDING HEIGHT	40 FT	40.0 FT
LOT COVERAGE	40%	15.4%

* - SEE SITE AREA CALCULATIONS TABLE

SITE STATISTICS

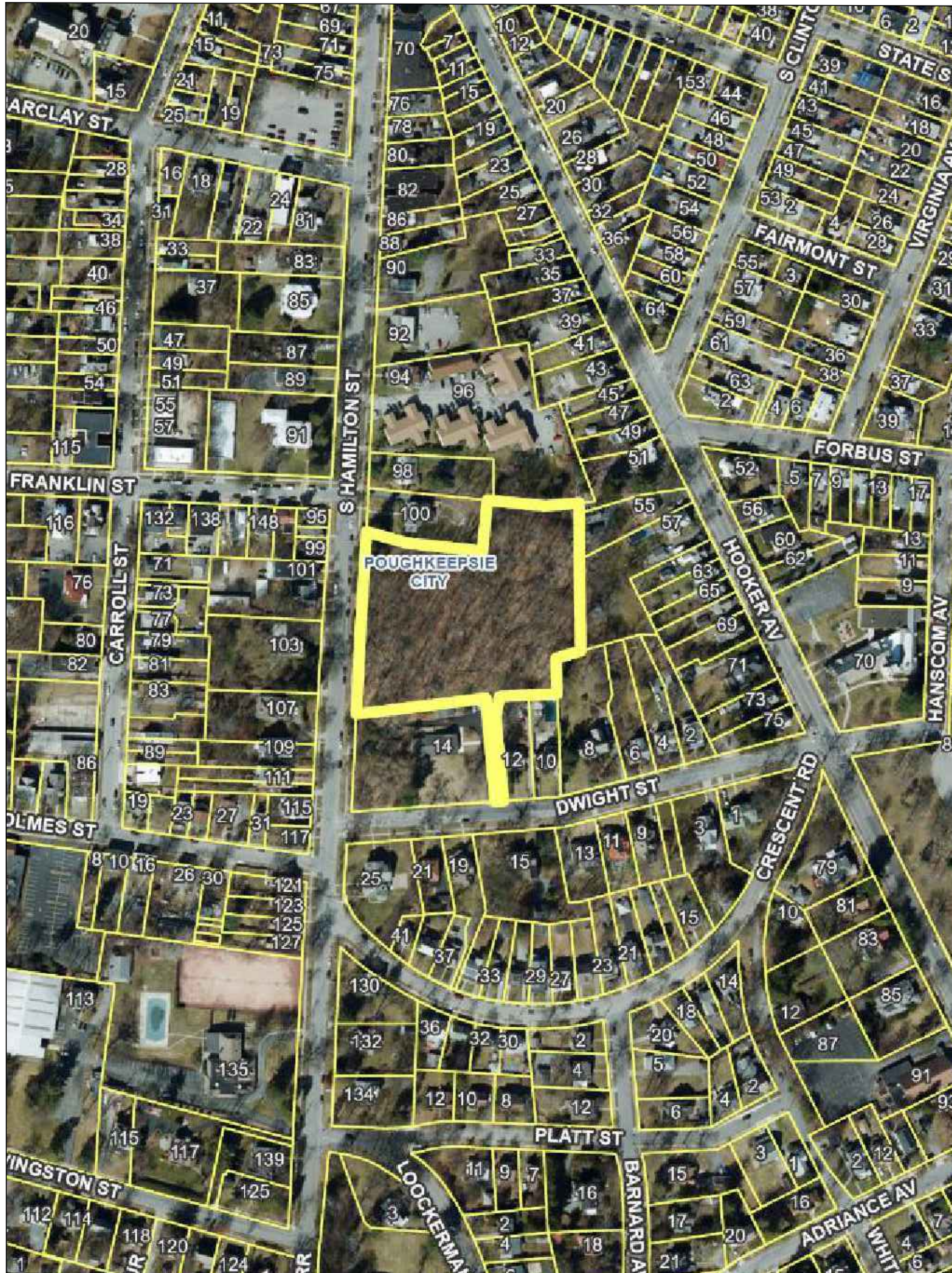
TOTAL PROJECT AREA 147,145.7 SF 3.378 AC	EXISTING			PROPOSED		
	SQUARE FEET	ACRES	COVERAGE	SQUARE FEET	ACRES	COVERAGE
BUILDING AREA	0.00	0.00	0.00%	24,850 SF	0.566	16.5%
PAVEMENT AREA	2,000.04	0.0504	1.5%	46,835.04	1.074	31.7%
GREEN/OPEN AREA	145,145.74	3.3284	98.5%	75,660.74	1.754	51.8%

PROJECT INFORMATION

ADDRESS	100 SOUTH HAMILTON STREET
TAX MAP No.	6161-30-151783
ZONING DISTRICT	R-4 MEDIUM HIGH DENSITY RESIDENTIAL
SITE COORDINATES	N 41°14'42.82" W 73°55'28.39"
TOTAL PROJECT AREA	3.378+ ACRES
EXISTING USE	RESIDENTIAL / VACANT
PROPOSED USE	MULTI-FAMILY
PARCEL FRONTAGE	346.3 LF (S. HAMILTON STREET) 17.0 LF (DOWIGHT STREET)
WATER SUPPLY	CITY OF POUGHKEEPSIE
SANITARY SERVICE	CITY OF POUGHKEEPSIE
FIRE PROTECTION	CITY OF POUGHKEEPSIE
SCHOOL DISTRICT	CITY OF POUGHKEEPSIE
PROPSD DENSITY	58 UNITS

SITE AREA CALCULATIONS

PARKING REQUIREMENTS	
1.5 SPACES PER 1-BEDROOM UNIT @ 29 UNITS	= 43.5 SPACES
2.0 SPACES PER 2-BEDROOM UNIT @ 29 UNITS	= 58.0 SPACES
TOTAL PARKING REQUIRED	= 102 SPACES
TOTAL PARKING PROVIDED	= 102 SPACES
MINIMUM LOT AREA	
2,200 SF PER 1-BEDROOM UNIT @ 29 UNITS	= 63,800 SF
2,800 SF PER 2-BEDROOM UNIT @ 29 UNITS	= 81,200 SF
TOTAL LOT AREA REQUIRED	= 145,000 SF
TOTAL LOT AREA PROVIDED	= 147,145.7 SF
USABLE OPEN SPACE	
300 SF PER 1-BEDROOM UNIT @ 29 UNITS	= 8,700 SF
400 SF PER 2-BEDROOM UNIT @ 29 UNITS	= 11,600 SF
TOTAL OPEN SPACE REQUIRED	= 20,300 SF
TOTAL OPEN SPACE PROVIDED	= 32,850 SF



OVERALL SITE LOCATION

SCALE: 1"= 200'

INDEX TO DRAWINGS

SHEET No.	DESCRIPTION
1.	SITE LAYOUT PLAN
2.	WATER AND SEWER UTILITY PLAN
3.	GRADING AND DRAINAGE PLAN
4.	EROSION CONTROL PLAN
5.	STANDARD CITY DETAILS
6.	STANDARD CITY DETAILS
7.	STANDARD CITY DETAILS
8.	STANDARD CITY DETAILS
9.	EXISTING CONDITIONS SURVEY (BY OTHERS)
10.	LANDSCAPING PLAN (BY OTHERS)
11.	LIGHTING PLAN (BY OTHERS)

GENERAL NOTES:

1. THE APPLICANT SHALL COMPLY WITH ALL APPLICABLE FEDERAL, STATE, AND LOCAL RULES AND REGULATIONS, INCLUDING BUT NOT LIMITED TO THE SEGR, FRESHWATER WETLAND PERMIT REGULATIONS, THE TOWN GRADING LAW AND THE TOWN FLOOD PLAIN MANAGEMENT.
2. THE APPLICANT SHALL BEAR THE SOLE RESPONSIBILITY FOR ENSURING THAT ALL IMPROVEMENTS ARE COMPLETED AND MAINTAINED IN ACCORDANCE WITH APPROVED PLANS, SPECIFICATIONS AND STANDARDS.
3. NO CERTIFICATE OF OCCUPANCY SHALL BE ISSUED UNTIL ALL REQUIRED IMPROVEMENTS ARE SATISFACTORILY COMPLETED, AND THE PLANNING BOARD HAS PROVIDED WRITTEN AUTHORIZATION TO THE BUILDING DEPARTMENT.
4. THE APPLICANT SHALL BE RESPONSIBLE FOR KEEPING EXISTING PUBLIC HIGHWAYS AND ADJACENT LANDS FREE OF DEBRIS, SOIL AND OTHER MATTER WHICH MAY ACCUMULATE DUE TO CONSTRUCTION RELATED TO THE SITE.
5. ALL PLANT MATERIALS INSTALLED PURSUANT TO THIS SITE DEVELOPMENT PLAN SHALL CONFORM TO THE AMERICAN STANDARD NURSERY STOCK (ANSI Z60.1-1986) OF THE AMERICAN ASSOCIATION OF NURSERYMEN OR EQUIVALENT RECOGNIZED STANDARD, AND SHALL BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH ACCEPTED INDUSTRY PRACTICE.
6. NO PORTION OF THIS SITE SHALL BE USED FOR STORAGE OR DISPLAY OF ANY PRODUCT OR MATERIAL, OR FOR PARKING OF ANY VEHICLES, OR FOR THE CONDUCT OF ANY OTHER BUSINESS OPERATIONS, UNLESS SPECIFICALLY DESIGNATED FOR SUCH USE ON THIS SITE DEVELOPMENT PLAN.
7. ALL REQUIRED EROSION & SEDIMENT CONTROL MEASURES SHALL BE INSTALLED AND THE BUILDING DEPARTMENT SHALL BE NOTIFIED PRIOR TO ANY SOIL DISTURBANCE OR ISSUANCE OF ANY GRADING PERMIT.
8. THE CONTRACTOR SHALL COORDINATE ALL WORK WITH ANY AND ALL OTHER CONTRACTORS PERFORMING WORK ON-SITE.
9. THE CONTRACTOR AND/OR HIS REPRESENTATIVE(S) ARE ENTIRELY RESPONSIBLE FOR EXECUTING THE WORK AND MAINTAINING THE WORK SITE IN ACCORDANCE WITH ALL APPLICABLE SAFETY CODES AND STANDARDS.
10. THE CONTRACTOR SHALL CLEAN-UP AT THE END OF EACH WORK DAY AND LEAVE THE WORK SITE CLEAN, WITH ALL ITEMS ATTENDED TO; NO FASTENERS, TOOLS AND/OR ANY OTHER ITEM MAY NOT BE LEFT UNATTENDED.
11. THE CONTRACTOR IS RESPONSIBLE FOR THE REMOVAL AND DISPOSAL OF ALL CONSTRUCTION MATERIALS AND DEBRIS FROM THE WORK SITE ON A DAILY BASIS.
12. ON-SITE VERIFICATION OF ALL DIMENSIONS AND CONDITIONS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
13. THE ENGINEER SHALL BE NOTIFIED, IN WRITING, OF ANY ANY DISCREPANCIES OF CONFLICTS THAT MAY ARISE PERTAINING TO THESE DRAWINGS. NO DECISION SHALL BE MADE WITHOUT APPROVAL OR CONSENT FROM THE ENGINEER.
14. ALL CONSTRUCTION SHALL COMPLY WITH NYS FIRE PREVENTION AND BUILDING CODES, LOCAL BUILDING CODES AND REQUIREMENTS, AS WELL AS FEDERAL PLUMBING AND ELECTRICAL CODES.
15. THE CONTRACTOR SHALL PERFORM ALL WORK IN A PROFESSIONAL AND SKILLED MANNER.
16. ANY EXISTING CONDITIONS DAMAGED OR DESTROYED DURING THE COURSE OF WORK SHALL BE RESTORED AT THE CONTRACTORS EXPENSE TO "AS GOOD AS CONDITION" AS DETERMINED BY THE ENGINEER.
17. ACCESS TO ALL PUBLIC UTILITIES INCLUDING FIRE HYDRANTS AND ALARMS, POLICE CALL BOXES, STREET LIGHTS, MANHOLES, ETC. SHALL BE KEPT UNOBSTRUCTED DURING CONSTRUCTION.
18. ALL MATERIALS AND EQUIPMENT USED FOR THIS PROJECT AND/OR KEPT ON-SITE SHALL COMPLY WITH NYS ENERGY CONSERVATION CONSTRUCTION CODE.
19. THESE PLANS ARE CERTIFIED TO BE IN ACCORDANCE WITH NYS BUILDING CODES; THE ENGINEER ASSUMES NO RESPONSIBILITY FOR THE FINISHED PRODUCT.
20. THE CONTRACTOR SHALL PROVIDE PROTECTIVE BARRICADES, PROPER SIGNAGE AND LIGHTING TO PREVENT UNAUTHORIZED ENTRY TO THE WORK-SITE.
21. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL OSHA REGULATIONS AND NYS LABOR COMPLIANCE.
22. THE CONTRACTOR SHALL COMPLY WITH ALL DRAWING NOTES.
23. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING HIGHWAY WORK PERMIT BEFORE PERFORMING ANY WORK WITHIN RIGHT-OF-WAY.
24. APPROPRIATE TRAFFIC CONTROL DEVICES & PERSONEL SHALL BE PROVIDED IN ACCORDANCE WITH FEDERAL, STATE OR LOCAL REGULATIONS TO REGULATE, WARN & GUIDE TRAFFIC THROUGH THE WORK SITE.

OWNER / APPLICANT:

O'NEILL GROUP HAMILTON, LLC
241 HUDSON STREET
HACKENSACK, NEW JERSEY 07601

PREPARED BY:

ARICO ASSOCIATES
ENGINEERS
LAND PLANNERS
& CONSULTANTS

NOTE

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SECTION 7209, SUBDIVISION 2 OF THE
NEW YORK STATE EDUCATION LAW.

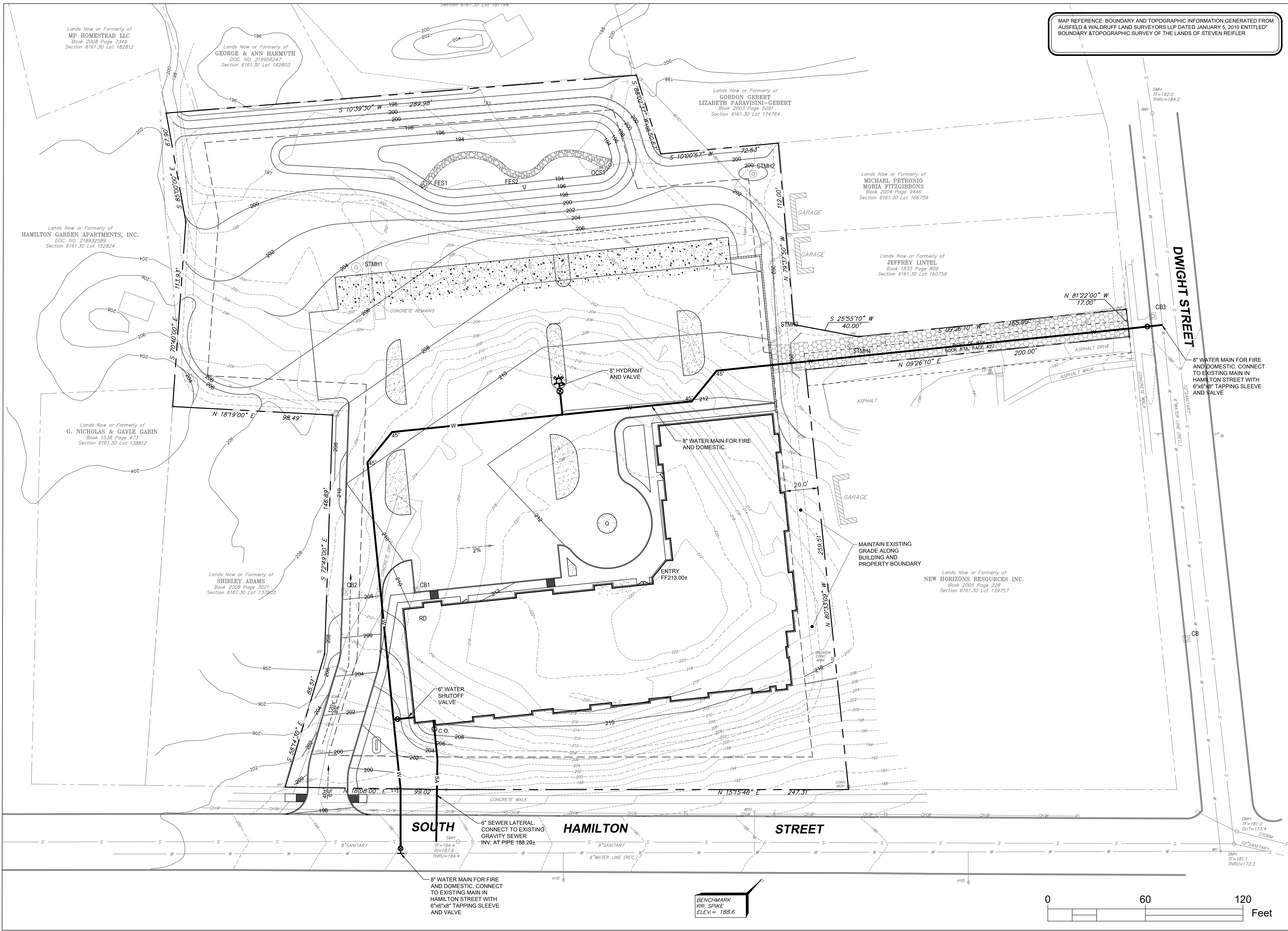
Fire Chief
Police Chief
Building Inspector
City Engineer
Commissioner of Public Works

CITY PLANNING BOARD
CITY OF POUGHKEEPSIE, NEW YORK

SITE PLAN APPROVAL

FILE NO.
PROJECT NAME:
PROJECT LOCATION:
APPLICANT:

OFFICIALLY APPROVED at the Planning Board
meeting held the ____ day of ____ 2____
PLANNING CHAIR:
DESIGNED:
DATE SIGNED:
CONDITION(S):



UTILITY PLAN
SCALE: 1"= 30'

Fire Chief	CITY PLANNING BOARD CITY OF Poughkeepsie, New York SITE PLAN APPROVAL FILE NO.: PROJECT NAME: PROJECT LOCATION: APPLICANT: OFFICIALLY APPROVED at the Planning Board meeting held the _____ day of _____, 2023. PLANNING CHAIR: DESIGNED: DATE SIGNED: CONDITION(S):
Police Chief	
Building Inspector	
City Engineer	
Commissioner of Public Works	

ISSUED FOR REVIEW

NOT FOR CONSTRUCTION

RECORD PLANS

DATE SUBMITTED:

NO.	DATE	BY	DATE
1	05.11.23	DFA	05.11.23
2	08.01.23	DFA	08.01.23
3	10.03.23	DFA	10.03.23

NO.	PER CITY COMMENTS	DESCRIPTION
1	REVISED: PARKING, LANDSCAPING	
2	REVISED: PER CITY COMMENTS	

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LICENSE NO. 00000000000000000000

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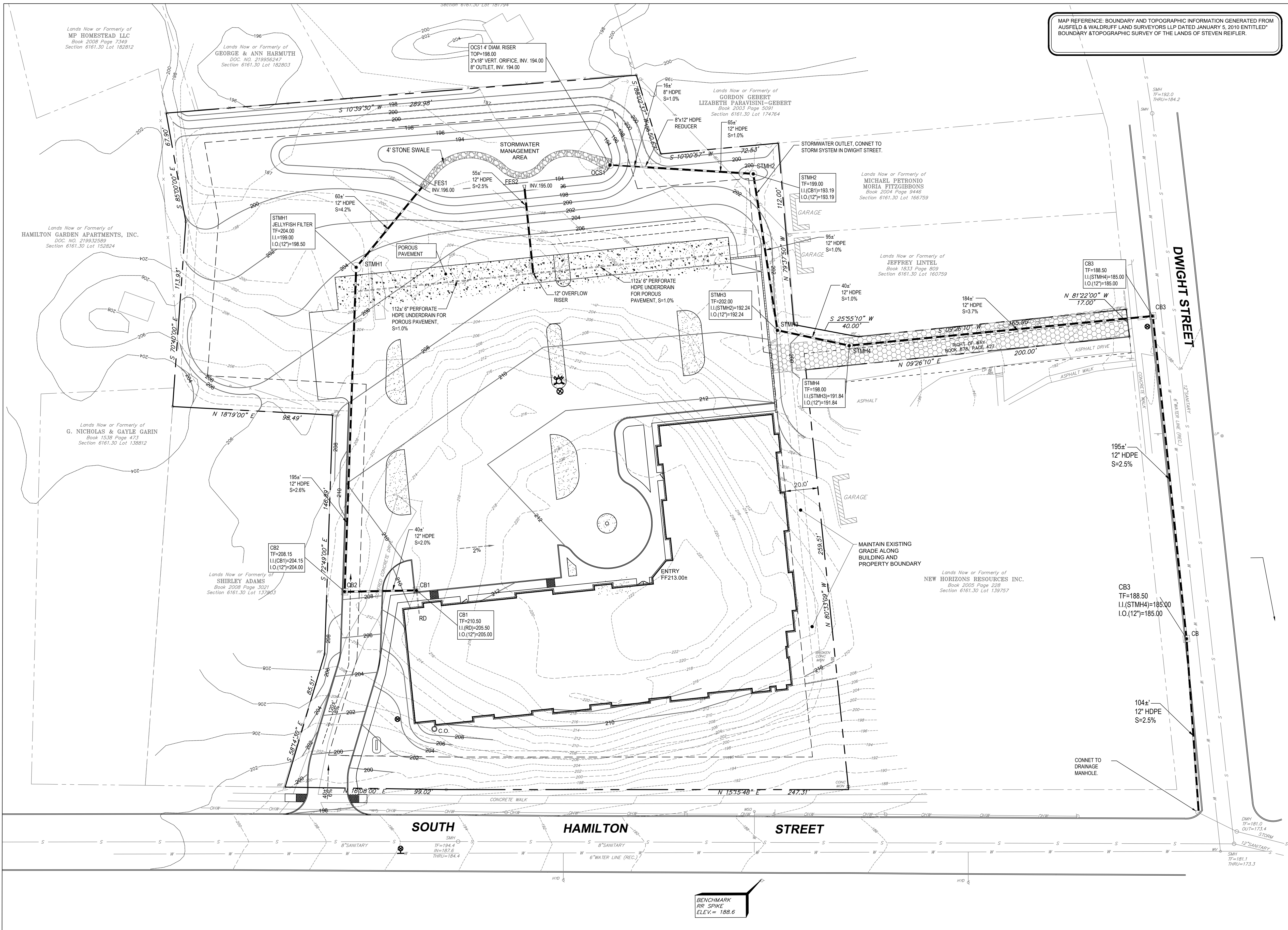
1407 Route 9, Bldg 2, Suite 6, Clifton Park, New York 12065
518.573.0989
aricoassociates@gmail.com

CITY OF Poughkeepsie - COUNTY OF Dutchess
WATER & SEWER UTILITY PLAN
SOUTH HAMILTON

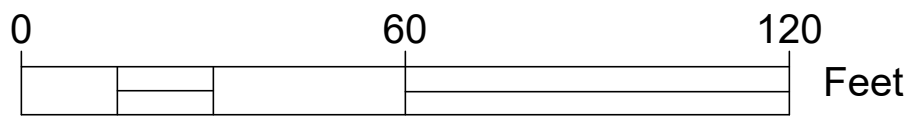
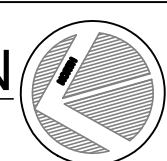
DATE: JANUARY 2023
JOB NO. 22.10
SCALE: AS NOTED
DRAWN BY: DFA
CHECKED BY: DFA
DRAWING NO.
S2
SHEET 2 OF 8

TAX MAP : 6161-30-151783

Page 195 of 752



GRADING & STORM PLAN
SCALE: 1" = 30'



Fire Chief	_____
Police Chief	_____
Building Inspector	_____
City Engineer	_____
Commissioner of Public Works	_____

CITY PLANNING BOARD CITY OF Poughkeepsie, New York	
SITE PLAN APPROVAL	
FILE NO.	_____
PROJECT NAME:	_____
PROJECT LOCATION:	_____
APPLICANT:	_____
OFFICIALLY APPROVED at the Planning Board meeting held the _____ day of _____ 2023.	
PLANNING CHAIR:	_____
DESIGNED:	_____
DATE SIGNED:	_____
CONDITION(S):	_____

ISSUED FOR REVIEW

NOT FOR CONSTRUCTION

RECORD PLANS

DATE SUBMITTED: _____

NO.	DATE	DESCRIPTION
1	05.11.23	DEA
2	08.01.23	DEA
3	10.03.23	DEA

NO. 1

PER CITY COMMENTS

REVISED PER CITY COMMENTS

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ATTORNEY AT LAW
BY A LICENSED PROFESSIONAL ENGINEER
EDUCATION LAW ARTICLE 145 SEC. 7209
DOMINICK, E. ARICO, P.E.
ARICO ASSOCIATES, INC.

ARICO ASSOCIATES

ENGINEERS
LAND PLANNERS
& CONSULTANTS

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CITY OF Poughkeepsie

COUNTY OF Dutchess

GRADING & DRAINAGE PLAN

SOUTH HAMILTON

DATE: JANUARY 2023

JOB NO. 22.10

SCALE: AS NOTED

DRAWN BY: DFA

CHECKED BY: DFA

DRAWING NO.

SHEET 3 OF 6

BEFORE UNDERTAKING ANY CONSTRUCTION ACTIVITY, ALL CONTRACTORS AND SUBCONTRACTORS INVOLVED WITH SITE WORK THAT INVOLVES PHYSICAL GROUND DISTURBANCE ON THE PROJECT SITE SHALL SIGN AND DATE A COPY OF THE CERTIFICATION STATEMENT, WHICH IS LOCATED IN THE STORMWATER POLLUTION PREVENTION PLAN (SWPPP), PREPARED FOR THIS PROJECT.

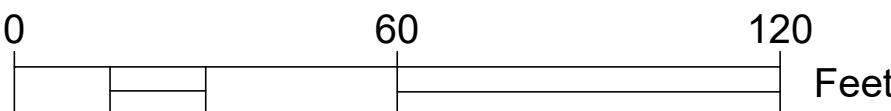
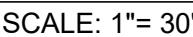
- SEEING:
- A. SOW GRASS SEED BETWEEN APRIL 1 & MAY 15TH OR BETWEEN AUGUST 15TH & OCTOBER 15TH, EXCEPT AS OTHERWISE APPROVED IN WRITING BY THE ENGINEER. NO SEEDING SHALL OCCUR ON FROZEN GROUND, HIGHLY MUDDY SOILS, OR DURING EXTREMELY HOT OR DROUGHT CONDITIONS.
- B. SEED MIXTURE: 40% FENWAY RED FESCUE, 30% ABBEY KENTUCKY BLUEGRASS BLEND, 20% DEVILS PERENNIAL RYE, & 10% ENCHANTED PERENNIAL RYE. PROVIDE SEED MIXTURE EQUAL TO SCOTT'S PURE PREMIUM SUN & SHADE NORTH GRASS SEED MIXTURE, OR APPROVED EQUIVALENT. VARIETY MAY BE ALTERED DEPENDING ON AVAILABILITY OF SEED FROM MANUFACTURER.
14. TOPSOIL:
- A. PROVIDE TOPSOIL FROM EXISTING STOCKPILES STRIPPED FROM THE PROJECT SITE & APPROVED BY THE ENGINEER. IF REQUIRED, PROVIDE ADDITIONAL TOPSOIL CONFORMING TO THE FOLLOWING:
- B. ORIGINAL LOAM TOPSOIL, WELL DRAINED HOMOGENEOUS TEXTURE & OF UNIFORM GRADE, WITHOUT THE ADMIXTURE OF SUBSOIL MATERIAL & ENTIRELY FREE OF DENSE MATERIAL, HARPOAN, SOD, OR ANY OTHER OBJECTIONABLE FOREIGN MATERIAL.
- C. CONTAINING NOT LESS THAN 4% NOR MORE THAN 20% ORGANIC MATTER IN THAT PORTION OF A SAMPLE PASSING A 1/4 INCH SIEVE DETERMINED BY THE NET COMBUSTION METHOD ON A SAMPLE DRIED AT 105 DEGREES C.
- D. CONTAINING A PH VALUE WITHIN THE RANGE OF 4.5 TO 7.0 ON THAT PORTION OF THE SAMPLE THAT PASSES A 1/4 INCH SIEVE.
15. MIXED CROPPED FERTILIZERS:
- A. MIXED CROPPED FERTILIZERS SHALL CONTAIN TOTAL NITROGEN, AVAILABLE PHOSPHORIC ACID & SOLUBLE POTASH IN THE RATIO OF 10-6-4 (50% N/U/P). 50% OF TOTAL NITROGEN SHALL BE DERIVED FROM UREAFORM FURNISHING A MINIMUM OF 3.5% WATER INSOLUBLE NITROGEN (3.5% WN). THE BALANCE OF THE NITROGEN SHALL BE PRESENT AS METHYLENE UREA, WATER-SOLUBLE UREA, NITRATE & AMMONIACAL COMPOUNDS. OTHER FERTILIZERS MEETING NYSDOT SPECIFICATION SECTION 713-03 FERTILIZER CAN BE USED.
16. MULCH:
- A. FURY APPLICATION, STRAW: STALKS OF OATS, WHEAT, RYE OR OTHER APPROVED CROPS THAT ARE FREE OF NOXIOUS WEED SEEDS. WEIGHT SHALL BE BASED ON A 15% MOISTURE CONTENT.
- B. HYDRO APPLICATION: COLORED WOOD CELLULOSE FIBER PRODUCT SPECIFICALLY DESIGNED FOR USE AS A HYDRO-MECHANICAL APPLIED MULCH, BY CONVED HYDRO MULCH, CONVED FIBER, OR APPROVED EQUIVALENT.
17. JUTE MESH:
- A. HEAVY JUTE EROSION CLOTH WITH 12 INCH LENGTHS OF NO. 11 GAGE WIRE BENT TO FORM A "U" WIRE STAPLE, BY JUTE PRODUCTS, INC., OR APPROVED EQUIVALENT.
18. EROSION CONTROL BLANKETS:
- A. FOR USE IN REPLACE OF MULCH ON SLOPES GREATER THAN 5% AS SHOWN ON THE PLANS. CONSISTING OF LIGHTWEIGHT PHOTODEGRADABLE POLYPROPYLENE NETS WITH AN APPROXIMATE WEIGHT OF 1.64 LBS/1,000 SQUARE FEET, DEGRADABLE THREAD, STRAW FIBER WITH AN APPROXIMATE WEIGHT OF 0.5 LBS/SQ YARD, WITH MANUFACTURER'S DOT STAPLE SYSTEM & 6 INCH ROSTAKBANK BINDER. BINDER SHALL BE PLANTING IN 12 INCHES. USE DEGRADABLE DOUBLE NET STRAW BLANKET, PRODUCT NO. S150 BY NORTH AMERICAN GREEN, OR APPROVED EQUIVALENT.
19. DUST SUPPRESSION MEASURES MAY INCLUDE: 1) USING A DEDICATED ON-SITE WATER TRUCK FOR ROAD AND SOILS WETTING, CAPABLE OF SPRAYING WATER DIRECTLY ONTO OFF-ROAD AREAS, INCLUDING EXCAVATIONS AND STOCKPILES; 2) CLEARING AND GRADING TO REDUCE DUST GENERATING SITES THAT WILL BE IN PLACE LONG ENOUGH TO LIMIT THE AREA OF EXPOSED, UNVEGETATED SOILS VULNERABLE TO DUST PRODUCTION; AND 3) GRAVEL WILL BE USED ON ROADWAYS TO PROVIDE A CLEAN AND DUST-FREE ROAD SURFACE.

1. SILT FENCE SHALL BE PLACED ON THE DOWNSLOPE SIDE OF DISTURBED AREAS AND AROUND THE PERIMETER OF SOIL STOCKPILES.
2. SILT FENCE SHALL BE PLACED AROUND THE BOUNDARY OF WETLANDS ADJACENT TO THE WORK AREA, AND AT THE EDGE OF WETLANDS AFTER CONSTRUCTION IS COMPLETED.
3. SILT FENCE SHALL BE REPAIRED OR REPLACED WHEN THE ENDS ARE FRAYED OR WORN, AND WHEN THE FENCE IS NOT ANCHORED 6" INTO THE GROUND, WHEN THE ACCUMULATED SEDIMENT REACHES 30% OF THE SILT FENCE HEIGHT, THE SEDIMENT SHALL BE REMOVED AND DISPOSED OF IN AN APPROPRIATE UPLAND AREA.

1. DUST SHALL BE CONTROLLED ON THIS PROJECT BY USE OF A WATER TRUCK.
2. THE QUALIFIED INSPECTOR WILL DETERMINE THE FREQUENCY OF WATER APPLICATION IN ORDER TO CONTROL DUST.
3. CHEMICALS OR OTHER METHODS OF DUST CONTROL ARE PROHIBITED TO BE USED ON THIS PROJECT, UNLESS APPROVED BY NYSDEC REGION 3.

1. STABILIZED CONSTRUCTION ACCESS SHALL BE INSTALLED WHERE NECESSARY TO PREVENT SEDIMENT FROM BEING TRACKED ONTO ROADWAYS.
2. PERMANENT TRAFFIC CORRIDORS SHALL BE ESTABLISHED AND "ROUTES OF CONVENIENCE" SHALL BE AVOIDED. CONSTRUCTION TRAFFIC SHALL NOT CROSS STREAMS OR DITCHES EXCEPT AT SUITABLE CROSSING FACILITIES, AND SHALL NOT OPERATE UNNECESSARILY WITHIN DRAINAGE DITCHES.
3. IF INTERNAL CONSTRUCTION ROADS ARE DETERMINED TO BE A SOURCE OF SEDIMENT-LADEN RUNOFF TO SENSITIVE AREAS, THEY SHALL BE STABILIZED AS SOON AS PRACTICABLE.

1. AREAS TO RECEIVE TEMPORARY SEEDING AND MULCHING SHALL RECEIVE BOTH GRASS SEED AND MULCH.
2. A JUTE MESH SHALL BE PLACED OVER THE MULCH IN AREAS WHERE WIND OR WATER EROSION PREVENTS ESTABLISHMENT OF GRASS COVER.



Fire Chief
Police Chief
Building Inspector
City Engineer
Commissioner of Public Works

CITY PLANNING BOARD
CITY OF POUGHKEEPSIE, NEW YORK

FILE NO:
PROJECT NAME:
PROJECT LOCATION:
APPLICANT:

OFFICIALLY APPROVED at the Planning Board meeting held the ____ day of _____ 2____

DESIGNEE:
DATE SIGNED:
CONDITION(S):

☐ ISSUED FOR REVIEW
☐ NOT FOR CONSTRUCTION
☐ RECORD PLANS

DATE SUBMITTED:

REVISIONS			
NO.	PER CITY COMMENTS	DESCRIPTION	BY DATE
1			DFA 05.13.23
2		REMOVED PARKING, LANDSCAPING	DFA 08.01.23
3		REVISED PER CITY COMMENTS	DFA 10.05.23

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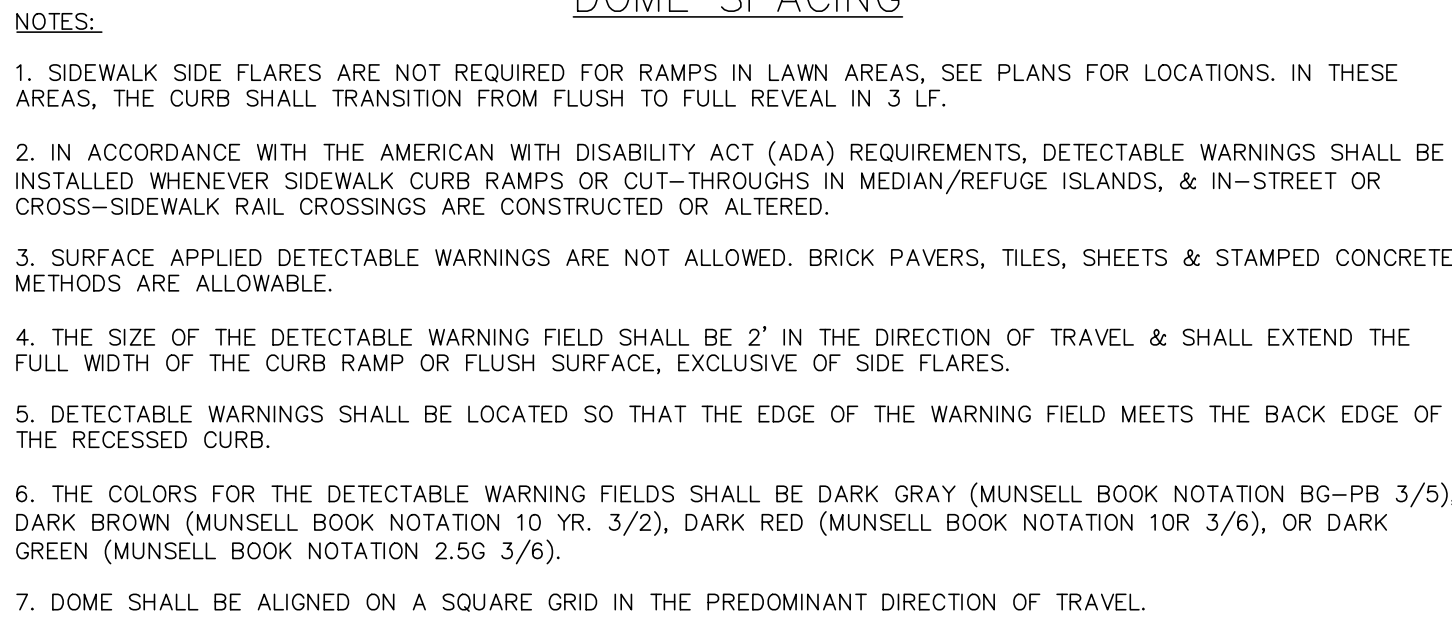
DOMINICK F. ARICO, P.E.
LIC. NO. 086515



ARICO ASSOCIATES
ENGINEERS
LAND PLANNERS
& CONSULTANTS
1407 Route 9, Bldg 2 Suite 6, Clifton Park, New York 12065
518.573.6989
aricoassociates@gmail.com

CITY OF POUGHKEEPSIE COUNTY OF DUTCHESS
EROSION CONTROL PLAN
SOUTH HAMILTON

TAY MAP · 6161-30-151783



REINFORCED CONCRETE SIDEWALK RAMP DETAILS

DATE SUBMITTED

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DOMINICK F. ARICO, P.E.



CITY OF POUGHKEEPSIE COUNTY OF DUTCHESS
STANDARD DETAILS
SOUTH HAMILTON

TAYMAD · 6161-30-151783

DATE: JANUARY 2023

JOB NO. 22.10

SCALE: AS NOTED
DRAWN BY: DFA
CHECKED BY: DFA

DRAWING NO.
D4

D1
SHEET 5 OF 8



JELLYFISH JF6 ONLINE CONFIGURATION STANDARD DETAIL

Fire Chief
Police Chief
Building Inspector
City Engineer
Commissioner of Public Works

CITY PLANNING BOARD
CITY OF POUGHKEEPSIE, NEW YORK

SITE PLAN APPROVAL
FILE NO:
PROJECT NAME:
PROJECT LOCATION:
APPLICANT:

OFFICIALLY APPROVED at the Planning Board
meeting held the _____ day of _____ 2_____
PLANNING CHAIR:
DESIGNEE:
DATE SIGNED:
CONDITION(S):



A. DISINFECTION METHODS IN ACCORDANCE WITH AWWA C651.

B. CHLORINATION SHALL PROVIDE A MINIMUM OF 25 PPM RESIDUAL AFTER 24-HOURS CONTACT IN PIPELINE. IN GENERAL, THIS RESIDUAL MAY BE EXPECTED WHEN AN APPLICATION OF 50 PPM ALTHOUGH SOME CONDITIONS MAY REQUIRE MORE.

C. CHLORINE, IN A FORM OF A 1% SLURRY OF HIGH TEST CALCIUM HYPOCHLORITE (WH. PERCHLORINE, PITTCO, ETC.) SHALL BE FED INTO THE PIPELINE IN SUCH A MANNER AS TO MIX WITH THE WATER FLOWING IN THE PIPELINE. (A 1% SLURRY RESULTS FROM MIXING 1 POUND OF CALCIUM HYPOCHLORITE WITH 7.5 GALLONS OF WATER).

D. THE FOLLOWING TABLE PROVIDES INFORMATION AS TO THE REQUIRED QUANTITY OF SLURRY TO BE USED PER 100 FEET OF PIPE TO PROVIDE A CHLORINE CONCENTRATION OF 50 PPM:

A. DURING THE PROCESS OF CHLORINATING THE PIPELINE, ALL VALVES AND OTHER PIPELINE APPURTENANCES SHALL BE OPERATED SEVERAL TIMES TO PROVIDE SUFFICIENT CONTACT WITH THE CHLORINATING AGENT.

HYDROSTATIC WATER MAIN TEST:

A. THE HYDROSTATIC TEST METHOD SHALL BE IN ACCORDANCE WITH ANSI/AWWA C600.

B. ALL PIPELINES, SERVICES, AIR RELEASE VALVES AND APPURTENANCES SHALL BE HYDROSTATIC TESTED.

C. WATER MAINS SHALL BE GIVEN A HYDROSTATIC TEST OF AT LEAST 1.5 TIMES THE WATER SYSTEM OPERATING PRESSURE OR 150 POUNDS PER SQUARE INCH (PSI), WHICHEVER IS GREATER. THE PRESSURE MEASUREMENT SHALL BE MADE FROM THE LOWEST ELEVATION OF THE FORCE MAIN SECTION BEING TESTED. THE TEST PRESSURE SHALL EXCEED THE PIPE OR VALVE RATED PRESSURES.

D. THE HYDROSTATIC TEST DURATION OF EACH WATER/FIRE LINE TEST SECTION IS 2 HOURS (UNINTERRUPTED).

E. THE MAXIMUM ALLOWABLE LEAKAGE OF FORCE MAIN SHALL BE BASED UPON THE FOLLOWING TABLE:

- A. THE HYDROSTATIC TEST METHOD SHALL BE IN ACCORDANCE WITH ANSI/AWWA C600.
- B. ALL PIPELINES, SERVICES, AIR RELEASE VALVES AND APPURTENANCES SHALL BE HYDROSTATIC TESTED.
- C. ALL WATER MAINS SHALL BE GIVEN A HYDROSTATIC TEST OF AT LEAST 1.5 TIMES THE WATER SYSTEM OPERATING PRESSURE OR 150 POUNDS PER SQUARE INCH (PSI), WHICHEVER IS GREATER. THE PRESSURE MEASUREMENT SHALL BE MADE FROM THE LOWEST ELEVATION OF THE FORCE MAIN SECTION BEING TESTED. THE TEST PRESSURE SHALL NOT EXCEED THE PIPE OR VALVE RATED PRESSURES.
- D. THE HYDROSTATIC TEST DURATION OF EACH WATER/FIRE LINE TEST SECTION IS 2 HOURS (UNINTERRUPTED).
- E. THE MAXIMUM ALLOWABLE LEAKAGE OF FORCE MAIN SHALL BE BASED UPON THE FOLLOWING TABLE:

The diagram illustrates a trench cross-section with the following details:

- Top Layer:** A horizontal layer at the top, labeled "SUBGRADE, SEE NOTES".
- Trench Backfill:** The main body of the trench is filled with material labeled "TRENCH BACKFILL: NYSOT SELECT GRANULAR FILL, ITEM NO. 203.07, OR NATIVE SOIL FREE OF FOREIGN MATERIALS, FROZEN EARTH & ROCKS LARGER THAN 6\".
- Pipe Location:** A circular pipe is shown in the center, labeled "PIPE O.D. ± 2.0\", TYP.". It has a hatched pattern and is surrounded by a "6\" SDR-11 WATER MAIN, 5.0\" MIN. COVER FROM FINISHED GRADE."
- Bedding:** Below the pipe is a layer labeled "BEDDING BELOW BELLS".
- Excavation:** An arrow points to the area around the pipe labeled "OVEREXCAVATE AT PIPE BELLS TO PROVIDE 6\" MIN. BEDDING BELOW BELLS".
- Bottom:** The bottom of the trench is labeled "TRENCH BOTTOM".
- Location Tape:** A dashed line above the pipe indicates the "BURIED PIPELINE LOCATION TAPE, METALLIC BLUE COLOR, 6\" WIDE, WITH CAUTION, BURIED WATER LINE BELOW IMPRINTED IN LARGE BLACK LETTERS IN CONTINUOUSLY REPETITIVE ELEMENTS ALONG THE TAPE, INSTALLED 2' BELOW FINISHED GRADE."
- Dimensions:** On the left side, there are two vertical dimension lines: one labeled "5' ±" and another labeled "2' ±".
- Labels:** Various labels point to different parts of the trench, including "UNDISTURBED UNEXCAVATED EARTH" on the left and "PROVIDE REQUIRED TRENCH WIDTH FOR STABILITY IN ACCORDANCE WITH OSHA STANDARDS" at the top.

SCALE: NO SCALE



NOTES:

1. REQUIRED BEARING AREAS ARE DUE TO THRUSTS CAUSED BY 225 PSF TOTAL INTERNAL PRESSURE (MAX. 150 PSIG WORKING PRESSURE PLUS 75 PSIG SURGE ALLOWANCE).
2. CONCRETE THRU-ROCK BOLT SHALL BE PLACED SO THAT THE PIPE & FITTING SHALL BE ACCESSIBLE FOR FUTURE REMOVAL. ONLY BY MULTILAYERING THE AREA GIVEN IN THE TABLE FOR THE APPROPRIATE PIPE SIZE AND FITTING BY PLACING CONCRETE THRU-ROCK BOLTS. THRU-ROCK BOLTS ARE REQUIRED AT ALL HORIZONTAL & VERTICAL BENDS.
3. REQUIRED BEARING AREAS ARE BASED ON ALLOWABLE SOIL BEARING CAPACITY OF 2,000 POUNDS PER SQUARE FOOT FOR SAND. DUE TO OTHER SOIL CONDITIONS THAT MAY BE ENCOUNTERED, BEARING AREAS MAY BE MODIFIED (BY THE ENGINEER) ONLY BY MULTILAYERING THE AREA GIVEN IN THE TABLE FOR THE APPROPRIATE PIPE SIZE AND FITTING BY THE CORRECTION FACTORS LISTED BELOW. IN THE EVENT THAT ENGENDERED SOIL CONDITIONS ARE OTHER THAN SAND, THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY. THE ENGINEER SHALL BE RESPONSIBLE FOR PROVIDING THE CORRECTION FACTORS TO THE MODIFIED CONCRETE THRU-ROCK BOLT(S) IN ACCORDANCE WITH THE ENGINEER'S WRITTEN INSTRUCTIONS. THE ENGINEER SHALL BE RESPONSIBLE FOR NOTIFYING THE ANCHORING CONTRACTOR OF ANY MODIFICATIONS TO THRU-ROCK BOLTS FROM THOSE DEPICTED ABOVE WITHOUT RECEIVING THE ENGINEER'S WRITTEN PERMISSION TO DO SO.

SOIL	ALLOWABLE SOIL PRESSURE (PSF)	CORRECTION FACTOR
SOFT CLAY	1,000	2.00
SAND	2,000	1.00
SAND & GRAVEL	3,000	0.67
SAND & GRAVEL	4,000	0.50
CEMENTED W/CLAY	4,000	0.50
HARD SHALE	10,000	0.20

4. IN MUCK, PILE, OR RECENTLY PLACED FILL, ALL THURSTLS SHALL BE RESISTED BY PILES, OR BY INSTALLING THE RODS TO SOLID FOUNDATIONS, OR BY INSTALLATION OF SUCH UNSTABLE MATERIAL AND REPLACEMENT WITH BALLAST OF SUFFICIENT STABILITY TO RESIST THE THURSTLS, ALL AS REQUIRED BY THE ENGINEER.

Fire Chief
Police Chief
Building Inspector
City Engineer
Commissioner of Public Works

☐ ISSUED FOR REVIEW
☐ NOT FOR CONSTRUCTION
☐ RECORD PLANS

DATE SUBMITTED:

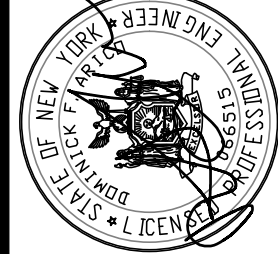
REVISIONS			
NO.	PER CITY COMMENTS	DESCRIPTION	BY DATE
1			DFA 05.13.23
2	REVISED: PARKING, LANDSCAPING		DFA 09.01.23
3	REVISED PER CITY COMMENTS		DFA 10.05.23

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CITY OF POUGHKEEPSIE COUNTY OF DUTCHESS
STANDARD DETAILS
SOUTH HAMILTON

TAX MAP : 6161-30-151783

DATE: JANUARY 2023

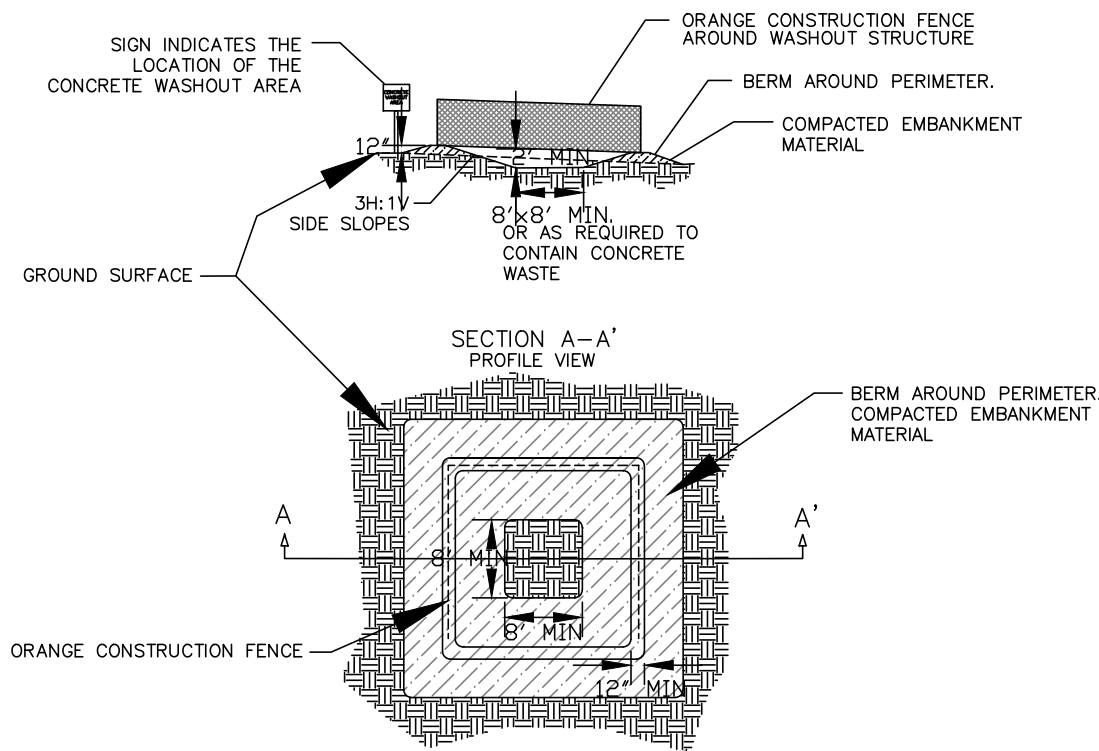
JOB NO. 22.10

SCALE: AS NOTED
DRAWN BY: DFA
CHECKED BY: DFA

OFFICIALLY APPROVED at the Planning Board meeting held the ____ day of _____ 2____
PLANNING CHAIR:
DESIGNEE:
DATE SIGNED:
CONDITION(S):

DRAWING NO

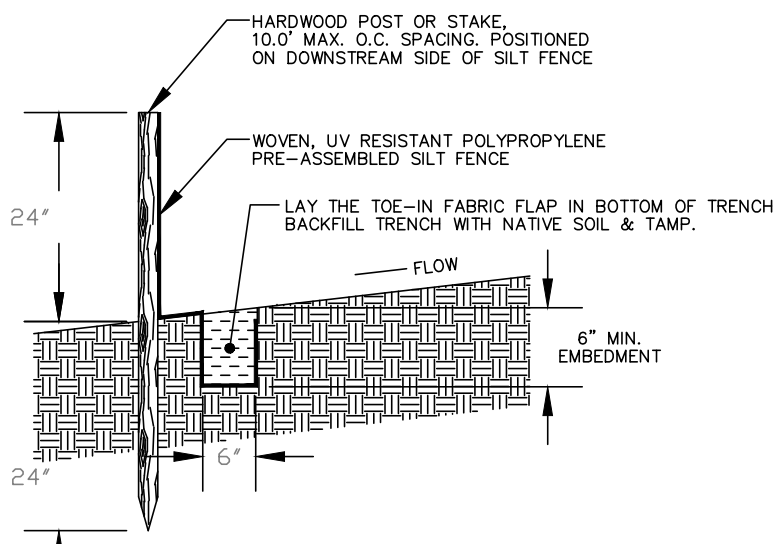
D3
SHEET 7 OF 8



- NOTES:
1. CONCRETE WASHOUT AREA SHALL BE INSTALLED PRIOR TO ANY CONCRETE PLACEMENT ON SITE.
 2. VEHICLE TRACKING CONTROL IS REQUIRED AT CONCRETE WASHOUT ENTRANCE IF ACCESS TO CONCRETE WASHOUT AREA IS OFF PAVEMENT.
 3. THE CONCRETE WASHOUT AREA SHALL BE REPAIRED AND/OR ENLARGED AS NECESSARY TO MAINTAIN CAPACITY FOR WASTE CONCRETE.
 4. WASTE MATERIAL FROM CONCRETE WASHOUT OPERATIONS MUST BE REMOVED AND LEGALLY DISPOSED OF WHEN IT HAS ACCUMULATED TWO-THIRDS OF THE NET STORAGE CAPACITY OF THE STRUCTURE.
 5. AT THE END OF CONSTRUCTION, ALL CONCRETE SHALL BE REMOVED FROM THE SITE AND LEGALLY DISPOSED OF AT AN APPROVED WASTE SITE.
 6. WHEN THE CONCRETE WASHOUT AREA IS REMOVED, THE DISTURBED AREA SHALL BE SEEDD AND MULCHED OR OTHERWISE STABILIZED IN A MANNER ACCEPTED BY THE TOWN.

CONCRETE WASHOUT AREA DETAIL

SCALE: NO SCALE



- NOTES:
1. THE LENGTH OF SLOPE ABOVE THE SILT FENCE SHALL NOT EXCEED THE FOLLOWING LIMITS:

SLOPE	MAX. SLOPE LENGTH
1V:2H (50%)	50 FEET
1V:3H (33%)	75 FEET
1V:4H (25%)	125 FEET
1V:5H (20%)	175 FEET
FLATTER THAN 1V:5H (20%)	200 FEET

2. DRAINAGE AREA SHALL NOT BE GREATER THAN 0.25 ACRES PER 100 L.F. OF SILT FENCE.

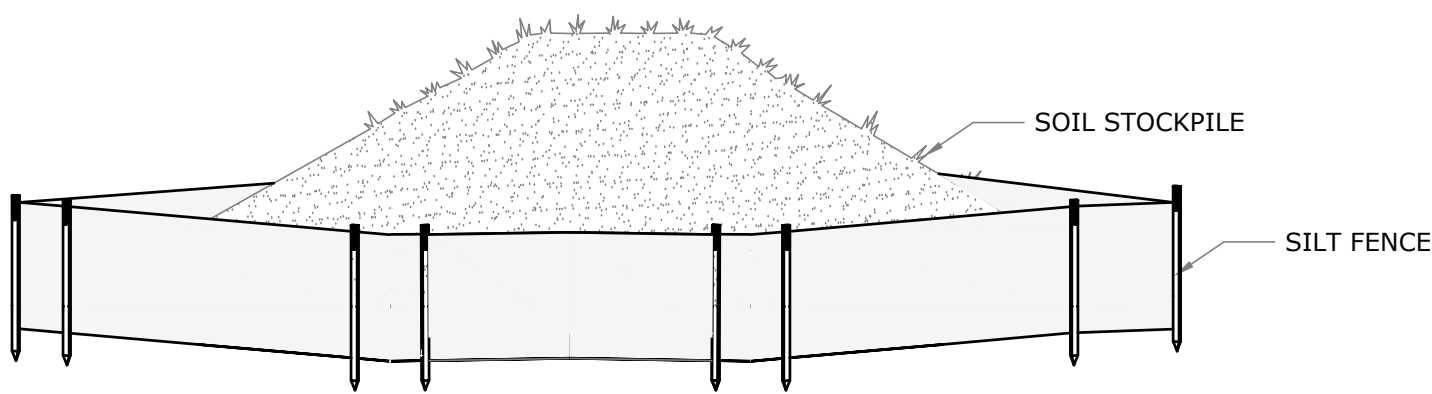
3. ALL SILT FENCES SHALL BE PLACED AS CLOSE TO THE WORK AREA AS POSSIBLE, AND THE AREA BELOW THE SILT FENCE MUST BE UNDISTURBED OR STABILIZED.

4. WHERE ENDS OF FILTER CLOTH COME TOGETHER, THEY SHALL BE OVERLAPPED, FOLDED AND STAPLED TO PREVENT SEDIMENT BYPASS.

5. SILT FENCES SHALL BE REMOVED WHEN THEY HAVE SERVED THEIR USEFULNESS TO PREVENT BLOCKING OR IMPEDING STORMWATER RUNOFF.

TEMPORARY SILT FENCE DETAIL

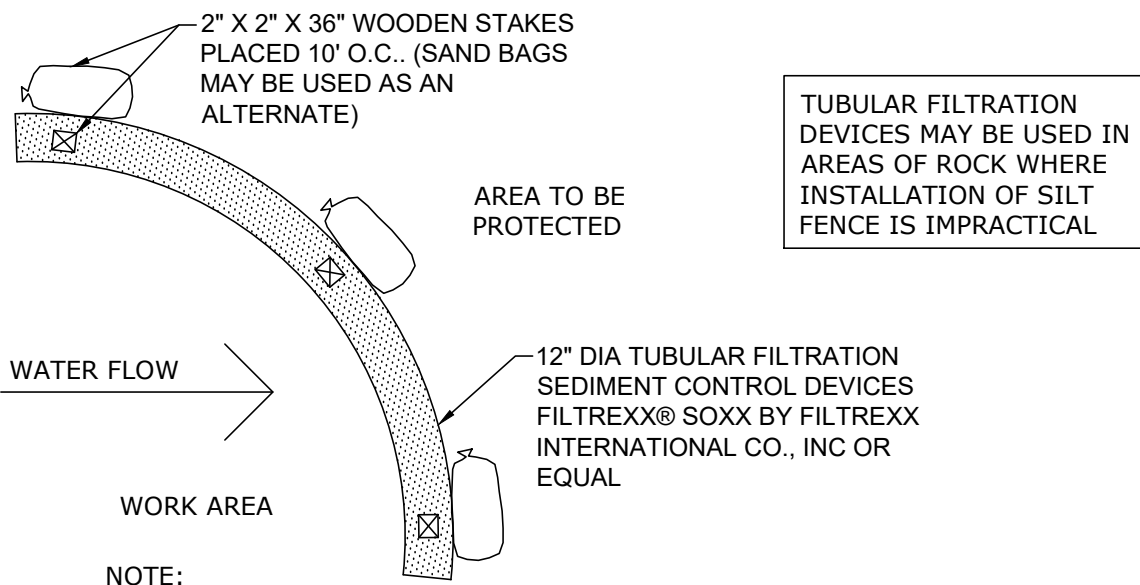
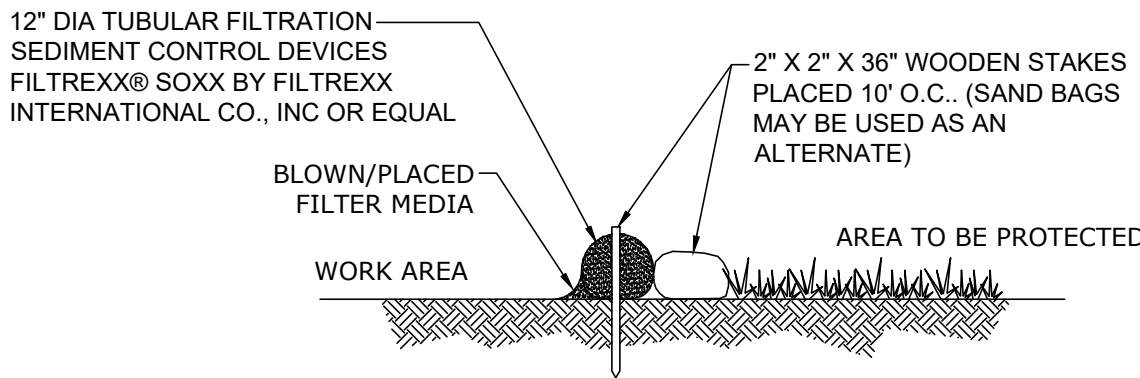
SCALE: NO SCALE



1. TOPSOIL AND FILL THAT IS ANTICIPATED TO REMAIN STOCKPILED ON-SITE FOR PERIODS GREATER THAN 30 DAYS SHALL BE STABILIZED BY SEEDING. PRIOR TO THE SEEDING OPERATION, THE STOCKPILED MATERIAL SHALL BE GRADED TO PERMIT THE USE OF CONVENTIONAL EQUIPMENT FOR SEEDBED PREPARATION, SEEDING, MULCH APPLICATION, AND MULCH ANCHORING. IN NO CASE SHALL ERODIBLE MATERIALS BE STOCKPILED WITHIN 25 FT OF ANY DITCH, STREAM, OR OTHER SURFACE WATERBODY.
2. SILT FENCE SHALL BE CONSTRUCTED AROUND ALL STOCKPILES OF FILL, TOPSOIL AND EXCAVATED OVERBURDEN THAT ARE TO REMAIN EXPOSED FOR PERIODS LESS THAN 30 DAYS. SILT FENCE SHALL BE ANCHORED AND MAINTAINED IN GOOD CONDITION UNTIL SUCH TIME AS SAID STOCKPILES ARE REMOVED AND STOCKPILING AREAS ARE BROUGHT TO FINAL GRADE AND PERMANENTLY STABILIZED.
3. TEMPORARY STABILIZATION MEASURES SHALL START AS SOON AS PRACTICAL ON PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITIES HAVE TEMPORARILY OR PERMANENTLY CEASED, BUT NOT MORE THAN 14 DAYS AFTER WORK HAS CEASED.

SOIL STOCKPILE DETAIL

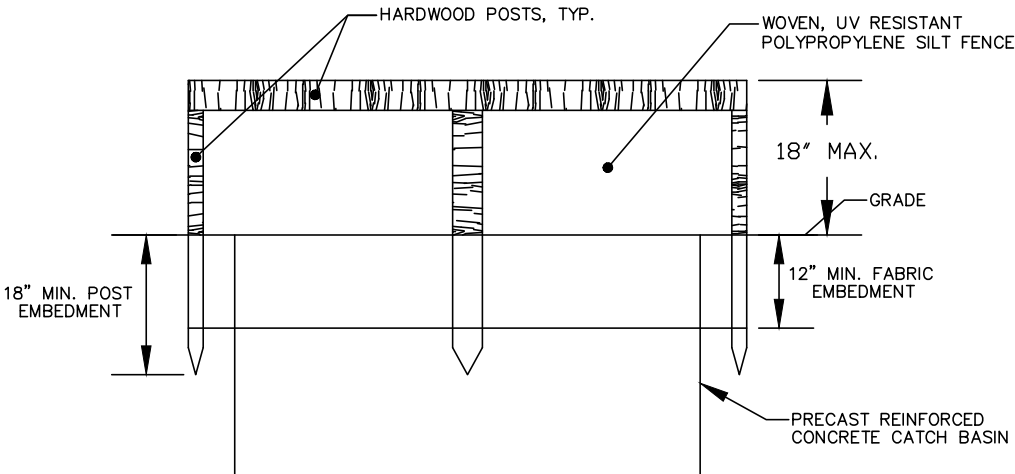
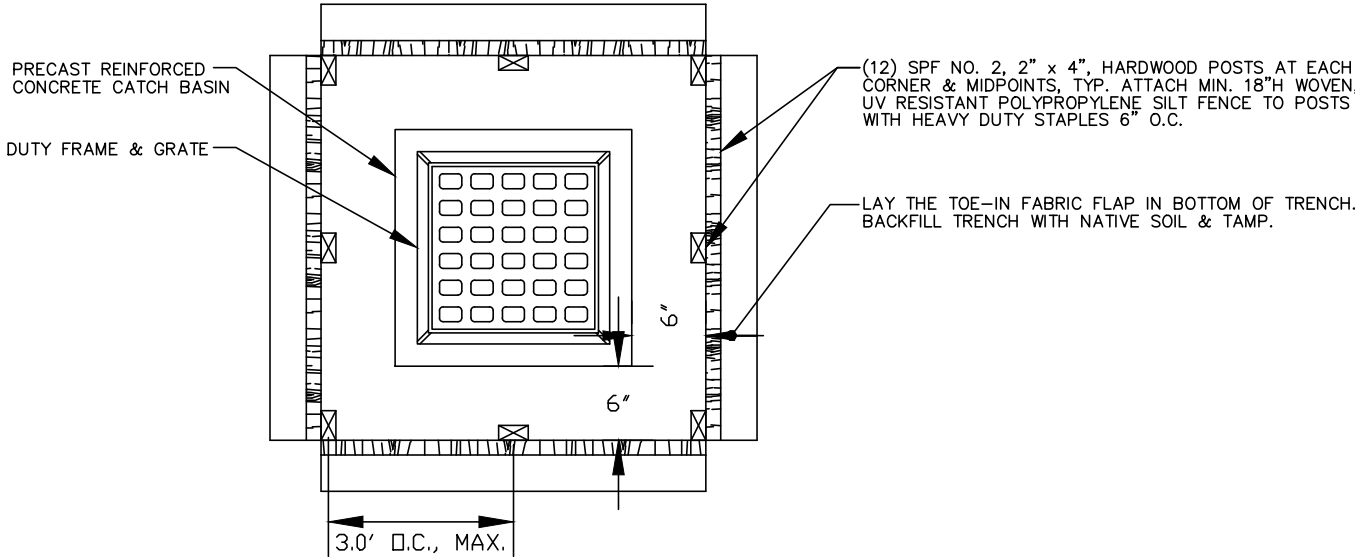
SCALE: NONE



- NOTE:
1. FILL TUBULAR FILTRATION SEDIMENT CONTROL DEVICE WITH FILTER MEDIA APPROVED BY NYSDEC FOR THIS APPLICATION.

TUBULAR FILTRATION SEDIMENT CONTROL DEVICE

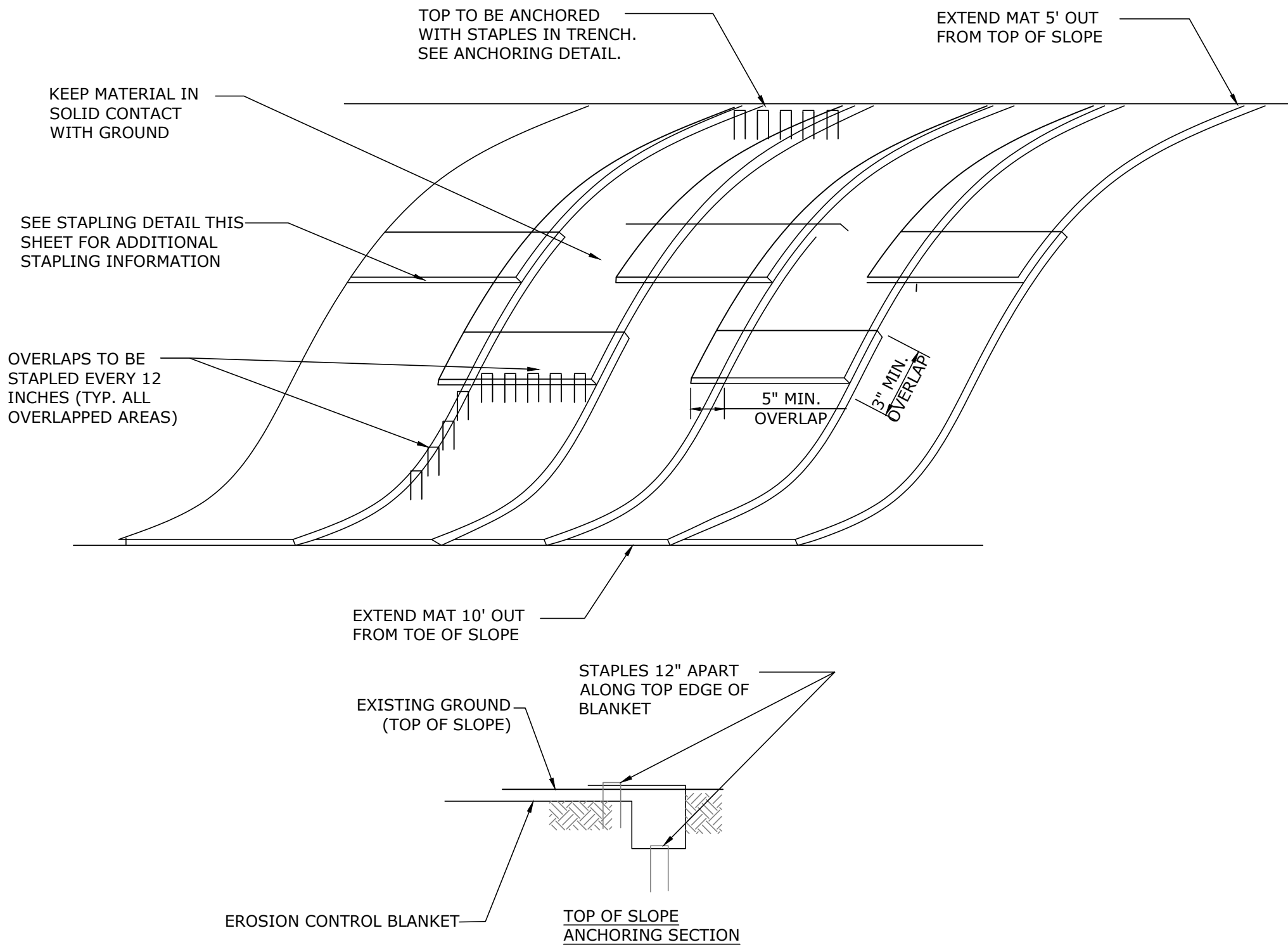
SCALE: NO SCALE



- NOTES:
1. CUT FABRIC FROM A CONTINUOUS ROLL TO ELIMINATE JOINTS. IF JOINTS ARE NEEDED THEY MUST BE OVERLAPPED TO THE NEXT STAKE.
 2. 1 ACRE MAXIMUM DRAINAGE AREA PER PROTECTION DEVICE.
 3. LAND AREA SLOPE IMMEDIATELY SURROUNDING THE PROTECTION DEVICE SHOULD NOT EXCEED 1%.
 4. REMOVE SEDIMENT FROM THE POOL AREA AS NECESSARY WITH CARE TO PREVENT UNDERCUTTING OR DAMAGING THE FILTER FABRIC.
 5. PROTECTION DEVICE SHALL BE KEPT IN PLACE UNTIL ASPHALT CONCRETE PLACEMENT IS COMMENCED.

TEMPORARY FILTER FABRIC CATCH BASIN PROTECTION DETAIL

SCALE: NO SCALE



NOTES:

- 1) APPLY APPLICATION OF ANY LIME, FERTILIZER, AND SEED BEFORE INSTALLING TURF REINFORCEMENT MAT.
- 2) INSTALL USING ALL MANUFACTURERS INSTALLATION RECOMMENDATIONS AND GUIDELINES.
- 3) BLANKET FOR 5:1 SLOPES - S75 BY NORTH AMERICAN GREEN
BLANKET FOR 2:1 TO 3:1 SLOPES - HYDRAMAX STEEP SLOPE MATRIX - NORTH AMERICAN GREEN

EROSION CONTROL BLANKET DETAIL

SCALE: NONE

Fire Chief	
Police Chief	
Building Inspector	
City Engineer	
Commissioner of Public Works	

CITY PLANNING BOARD CITY OF Poughkeepsie, NEW YORK
SITE PLAN APPROVAL
FILE NO:
PROJECT NAME:
PROJECT LOCATION:
APPLICANT:
OFFICIALLY APPROVED at the Planning Board meeting held the _____ day of _____, 2023.
PLANNING CHAIR:
DESIGNEE:
DATE SIGNED:
CONDITION(S):

☐ ISSUED FOR REVIEW
☐ NOT FOR CONSTRUCTION
☐ RECORD PLANS

DATE SUBMITTED:

NO.	DATE	DESCRIPTION
1	05/11/23	PER CITY COMMENTS
2	08/01/23	REVISED: PARKING LANDSCAPING
3	10/03/23	REVISED PER CITY COMMENTS

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LICENSE NO. 14572



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518.573.0959
aricoassociates@gmail.com

CITY OF Poughkeepsie - COUNTY OF Dutchess
STANDARD DETAILS
SOUTH HAMILTON

DATE: JANUARY 2023
JOB NO. 22-10

SCALE: AS NOTED
DRAWN BY: DFA
CHECKED BY: DFA

DRAWING NO.

D4
SHEET 8 OF 8

APPENDIX G

Is this project subject to Chapter 10 of the NYS Design Manual (i.e. WQv is equal to post-development 1 year runoff volume)?..... **No**

Design Point: DA1.1
P= 1.00 inch *Manually enter P, Total Area and Impervious Cover.*

Breakdown of Subcatchments						
Catchment Number	Total Area (Acres)	Impervious Area (Acres)	Percent Impervious %	Rv	WQv (ft ³)	Description
1	0.74	0.73	99%	0.94	2,519	Porous Pavement
2	0.94	0.81	86%	0.83	2,817	Filter-JellyFish
3						
4						
5						
6						
7						
8						
9						
10						
Subtotal (1-30)	1.68	1.54	92%	0.88	5,336	Subtotal 1
Total	1.68	1.54	92%	0.88	5,336	Initial WQv

0.12 af

Identify Runoff Reduction Techniques By Area			
Technique	Total Contributing Area	Contributing Impervious Area	Notes
	(Acre)	(Acre)	
Conservation of Natural Areas	0.00	0.00	minimum 10,000 sf
Riparian Buffers	0.00	0.00	maximum contributing length 75 feet to 150 feet
Filter Strips	0.00	0.00	
Tree Planting	0.00	0.00	Up to 100 sf directly connected impervious area may be subtracted per tree
Total	0.00	0.00	

Recalculate WQv after application of Area Reduction Techniques					
	Total Area (Acres)	Impervious Area (Acres)	Percent Impervious %	Runoff Coefficient Rv	WQv (ft ³)
"<<Initial WQv"	1.68	1.54	92%	0.88	5,336
Subtract Area	0.00	0.00			
WQv adjusted after Area Reductions	1.68	1.54	92%	0.88	5,336
Disconnection of Rooftops		0.00			
Adjusted WQv after Area Reduction and Rooftop Disconnect	1.68	1.54	92%	0.88	5,336
WQv reduced by Area Reduction techniques					0

0.12 af

0.00 af

Runoff Reduction Volume and Treated volumes						
	Runoff Reduction Techniques/Standard SMPs		Total Contributing Area	Total Contributing Impervious Area	WQv Reduced (RRv)	WQv Treated
			(acres)	(acres)	cf	cf
Area/Volume Reduction	Conservation of Natural Areas	RR-1	0.00	0.00		
	Sheetflow to Riparian Buffers/Filter Strips	RR-2	0.00	0.00		
	Tree Planting/Tree Pit	RR-3	0.00	0.00		
	Disconnection of Rooftop Runoff	RR-4		0.00		
	Vegetated Swale	RR-5	0.00	0.00	0	
	Rain Garden	RR-6	0.00	0.00	0	
	Stormwater Planter	RR-7	0.00	0.00	0	
	Rain Barrel/Cistern	RR-8	0.00	0.00	0	
	Porous Pavement	RR-9	0.74	0.73	2519	
	Green Roof (Intensive & Extensive)	RR-10	0.00	0.00	0	
Standard SMPs w/RRv Capacity	Infiltration Trench	I-1	0.00	0.00	0	0
	Infiltration Basin	I-2	0.00	0.00	0	0
	Dry Well	I-3	0.00	0.00	0	0
	Underground Infiltration System	I-4				
	Bioretention & Infiltration Bioretention	F-5	0.00	0.00	0	0
	Dry swale	O-1	0.00	0.00	0	0
Standard SMPs	Micropool Extended Detention (P-1)	P-1				
	Wet Pond (P-2)	P-2				
	Wet Extended Detention (P-3)	P-3				
	Multiple Pond system (P-4)	P-4				
	Pocket Pond (p-5)	P-5				
	Surface Sand filter (F-1)	F-1				
	Underground Sand filter (F-2)	F-2				
	Perimeter Sand Filter (F-3)	F-3				
	Organic Filter (F-4)	F-4	0.94	0.81		2817.000
	Shallow Wetland (W-1)	W-1				
	Extended Detention Wetland (W-2)	W-2				
	Pond/Wetland System (W-3)	W-3				
	Pocket Wetland (W-4)	W-4				
	Wet Swale (O-2)	O-2				
Totals by Area Reduction →			0.00	0.00	0	
Totals by Volume Reduction →			0.74	0.73	2519	
Totals by Standard SMP w/RRV →			0.00	0.00	0	0
Totals by Standard SMP →			0.94	0.81		2817
Totals (Area + Volume + all SMPs) →			1.68	1.54	2,519	2,817
	Impervious Cover √	okay				
	Total Area √	okay				

Minimum RRv

Enter the Soils Data for the site		
Soil Group	Acres	S
A		55%
B		40%
C	100.00	30%
D		20%
Total Area	100	
Calculate the Minimum RRv		
S =	0.30	
Impervious =	1.54	acre
Precipitation	1	in
Rv	0.95	
Minimum RRv	1,593	ft ³
	0.04	af

NOI QUESTIONS

#	NOI Question	Reported Value	
		cf	af
28	Total Water Quality Volume (WQv) Required	5336	0.123
30	Total RRV Provided	2519	0.058
31	Is RRV Provided ≥WQv Required?	No	
32	Minimum RRV	1593	0.037
32a	Is RRV Provided ≥ Minimum RRV Required?	Yes	
33a	Total WQv Treated	2817	0.065
34	Sum of Volume Reduced & Treated	5336	0.123
34	Sum of Volume Reduced and Treated	5336	0.123
35	Is Sum RRV Provided and WQv Provided ≥WQv Required?	Yes	

47.21%

Apply Peak Flow Attenuation			
36	Channel Protection	C_{pv}	
37	Overbank	Q_p	
37	Extreme Flood Control	Q_f	
	Are Quantity Control requirements met?	Yes	Plan Completed

Planning

Practice	Description	Application
Preservation of Undisturbed Areas	Delineate and place into permanent conservation undisturbed forests, native vegetated areas, riparian corridors, wetlands, and natural terrain.	Considered & Not Applied
Preservation of Buffers	Define, delineate and preserve naturally vegetated buffers along perennial streams, rivers, shorelines and wetlands.	Considered & Applied
Reduction of Clearing and Grading	Limit clearing and grading to the minimum amount needed for roads, driveways, foundations, utilities and stormwater management facilities.	Considered & Not Applied
Locating Development in Less Sensitive Areas	Avoid sensitive resource areas such as floodplains, steep slopes, erodible soils, wetlands, mature forests and critical habitats by locating development to fit the terrain in areas that will create the least impact.	Considered & Not Applied
Open Space Design	Use clustering, conservation design or open space design to reduce impervious cover, preserve more open space and protect water resources.	Considered & Applied
Soil Restoration	Restore the original properties and porosity of the soil by deep till and amendment with compost to reduce the generation of runoff and enhance the runoff reduction performance of post construction practices.	Considered & Applied
Roadway Reduction	Minimize roadway widths and lengths to reduce site impervious area	N/A
Sidewalk Reduction	Minimize sidewalk lengths and widths to reduce site impervious area	Considered & Applied
Driveway Reduction	Minimize driveway lengths and widths to reduce site impervious area	N/A
Cul-de-sac Reduction	Minimize the number of cul-de-sacs and incorporate landscaped areas to reduce their impervious cover.	N/A
Building Footprint Reduction	Reduce the impervious footprint of residences and commercial buildings by using alternate or taller buildings while maintaining the same floor to area ratio.	Considered & Applied
Parking Reduction	Reduce imperviousness on parking lots by eliminating unneeded spaces, providing compact car spaces and efficient parking lanes, minimizing stall dimensions, using porous pavement surfaces in overflow parking areas, and using multi-storied parking decks where appropriate.	Considered & Applied

Porous Pavement Worksheet

$$A_p = V_w / (n \times dt)$$

A_p Required porous pavement surface area ft^2

V_w Design Volume ft^3

n porosity of gravel bed/resevoir

Assume .4 for gravel

dt depth of gravel bed/resevoir

Design Point:	DA1.1						
Enter Site Data For Drainage Area to be Treated by Practice							
Catchment Number	Total Area (Acres)	Impervious Area (Acres)	Percent Impervious %	Rv	WQv (ft ³)	Precipitation (in)	Description
1	0.74	0.73	0.99	0.94	2519.22	1.00	Porous Pavement
Enter Soil Infiltration Rate							
Soil Infiltration Rate		0.00	in/hour	<i>Infiltration rate should be a minimum of 0.5 in/hr.</i>			
Calculate Required Surface Area							
Design Volume		Vw	2,519	ft ³			
Are underdrains being used?			Yes	-	<i>Only Gravel Bed Depth below underdrain can be considered.</i>		
Porosity of Gravel Bed		n	0.40	-			
Gravel Bed Depth		dt	2.00	ft	<i>Must be the depth below the underdrain.</i>		
Required Surface Area		Ap	3,149	sf			
Surface Area Provided			4,860	sf	<i>Dimensions of pavement can be provided here</i>		
Storage Volume Provided			3,888	ft ³			
Determine the Runoff Reduction							
RRv	2,519	ft³					

Porous Pavement Worksheet

$$A_p = V_w / (n \times d_t)$$

A_p Required porous pavement surface area ft^2

V_w Design Volume ft^3

n porosity of gravel bed/resevoir

Assume .4 for gravel

d_t depth of gravel bed/resevoir

Design Point:	DA1.1						
Enter Site Data For Drainage Area to be Treated by Practice							
Catchment Number	Total Area (Acres)	Impervious Area	Percent Impervious	Rv	WQV (ft^3)	Precipitation (in)	Description
Enter Soil Infiltration Rate							
Soil Infiltration Rate		in/hour	Infiltration rate should be a minimum of 0.5 in/hr.				
Calculate Required Surface Area							
Design Volume	Vw		ft^3				
Are underdrains being used?			-				
Porosity of Gravel Bed	n		-				
Gravel Bed Depth	dt		ft				
Required Surface Area	Ap	#VALUE!	sf				
Surface Area Provided			sf	Dimensions of pavement can be provided here			
Storage Volume Provided		0	ft^3	Practice too small			
Determine the Runoff Reduction							
RRv	Error	ft^3					

Porous Pavement Worksheet

$$A_p = V_w / (n \times d_t)$$

A_p Required porous pavement surface area ft^2
 V_w Design Volume ft^3
 n porosity of gravel bed/reservoir
 d_t depth of gravel bed/reservoir

Assume .4 for gravel

Design Point:	DA1.1						
Enter Site Data For Drainage Area to be Treated by Practice							
Catchment Number	Total Area (Acres)	Impervious Area	Percent Impervious	Rv	WQV (cu ft)	Precipitation (in)	Description
Enter Soil Infiltration Rate							
Soil Infiltration Rate		in/hour	Infiltration rate should be a minimum of 0.5 in/hr.				
Calculate Required Surface Area							
Design Volume	Vw		ft^3				
Are underdrains being used?			-				
Porosity of Gravel Bed	n		-				
Gravel Bed Depth	dt		ft				
Required Surface Area	Ap	#VALUE!	sf				
Surface Area Provided			sf	Dimensions of pavement can be provided here			
Storage Volume Provided		0	ft^3	Practice too small			
Determine the Runoff Reduction							
RRv	Error	ft^3					

Porous Pavement Worksheet

$$A_p = V_w / (n \times d_t)$$

A_p Required porous pavement surface area ft^2

V_w Design Volume ft^3

n porosity of gravel bed/resevoir

Assume .4 for gravel

d_t depth of gravel bed/resevoir

Design Point:	DA1.1						
Enter Site Data For Drainage Area to be Treated by Practice							
Catchment Number	Total Area (Acres)	Impervious Area	Percent Impervious	Rv	WQV (ft^3)	Precipitation (in)	Description
Enter Soil Infiltration Rate							
Soil Infiltration Rate			<i>in/hour</i>	<i>Infiltration rate should be a minimum of 0.5 in/hr.</i>			
Calculate Required Surface Area							
Design Volume		V_w		ft^3			
Are underdrains being used?				-			
Porosity of Gravel Bed		n		-			
Gravel Bed Depth		d_t		ft			
Required Surface Area		A_p	#VALUE!	sf			
Surface Area Provided				sf	<i>Dimensions of pavement can be provided here</i>		
Storage Volume Provided			0	ft^3	<i>Practice too small</i>		
Determine the Runoff Reduction							
RRv	Error	ft^3					

HAMILTON ST_POSTvsPRE_10.03.23_OUTLET CONTROL

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Project Notes

Rainfall events imported from "NRCS-Rain.txt" for 7040 NY Dutchess S

Rainfall events imported from "NRCS-Rain.txt" for 7040 NY Dutchess S

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Rainfall Events Listing (selected events)

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	1-Year	NRCC 24-hr	C	Default	24.00	1	2.64	2
2	10-Year	NRCC 24-hr	C	Default	24.00	1	4.71	2
3	50-Year	NRCC 24-hr	C	Default	24.00	1	7.01	2
4	100-Year	NRCC 24-hr	C	Default	24.00	1	8.32	2

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Area Listing (all nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
236,749	74	>75% Grass cover, Good, HSG C (POA1-POST, POA1-PRE, POA1.1-POST, POA2-POST, POA2-PRE, POA3-POST, POA3.1-POST, POA4-POST, POA4-PRE)
43,560	98	Paved parking, HSG C (POA1.1-POST)
63,815	98	Paved roads w/curbs & sewers, HSG C (POA2-POST, POA2-PRE)
23,958	98	Roofs, HSG C (POA1.1-POST)
143,312	72	Woods/grass comb., Good, HSG C (POA1-PRE, POA2-PRE, POA3-PRE, POA4-PRE)
511,394	80	TOTAL AREA

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Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
0	HSG A	
0	HSG B	
511,394	HSG C	POA1-POST, POA1-PRE, POA1.1-POST, POA2-POST, POA2-PRE, POA3-POST, POA3-PRE, POA3.1-POST, POA4-POST, POA4-PRE
0	HSG D	
0	Other	
511,394		TOTAL AREA

HAMILTON ST_POSTvsPRE_10.03.23_OUTLET CONTROL

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Ground Covers (all nodes)

HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover
0	0	236,749	0	0	236,749	>75% Grass cover, Good
0	0	43,560	0	0	43,560	Paved parking
0	0	63,815	0	0	63,815	Paved roads w/curbs & sewers
0	0	23,958	0	0	23,958	Roofs
0	0	143,312	0	0	143,312	Woods/grass comb., Good
0	0	511,394	0	0	511,394	TOTAL AREA

HAMILTON ST_POSTvsPRE_10.03.23_OUTLET CONTROL

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Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Width (inches)	Diam/Height (inches)	Inside-Fill (inches)
1	POA1.1-POST	0.00	0.00	45.0	0.0100	0.010	0.0	12.0	0.0
2	POA1.1-POST	0.00	0.00	190.0	0.0150	0.010	0.0	12.0	0.0
3	POA1.1-POST	0.00	0.00	70.0	0.0500	0.010	0.0	18.0	0.0
4	SMA1	194.00	193.75	25.0	0.0100	0.010	0.0	8.0	0.0

Time span=5.00-20.00 hrs, dt=0.03 hrs, 501 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentPOA1-POST:DA1 Runoff Area=0.080 ac 0.00% Impervious Runoff Depth>0.61"
 Flow Length=20' Slope=0.0200 '/' Tc=5.0 min CN=74 Runoff=0.07 cfs 176 cf

SubcatchmentPOA1-PRE:DA1 Runoff Area=1.780 ac 0.00% Impervious Runoff Depth>0.52"
 Flow Length=557' Tc=32.9 min CN=72 Runoff=0.56 cfs 3,349 cf

SubcatchmentPOA1.1-POST:DA1.1 Runoff Area=2.490 ac 62.25% Impervious Runoff Depth>1.45"
 Flow Length=615' Tc=6.5 min CN=89 Runoff=4.82 cfs 13,125 cf

SubcatchmentPOA2-POST:DA2 Runoff Area=2.760 ac 27.72% Impervious Runoff Depth>0.94"
 Flow Length=533' Tc=16.4 min CN=81 Runoff=2.48 cfs 9,375 cf

SubcatchmentPOA2-PRE:DA2 Runoff Area=3.310 ac 21.15% Impervious Runoff Depth>0.78"
 Flow Length=734' Tc=14.3 min CN=78 Runoff=2.61 cfs 9,405 cf

SubcatchmentPOA3-POST:DA3 Runoff Area=0.030 ac 0.00% Impervious Runoff Depth>0.61"
 Flow Length=20' Slope=0.0500 '/' Tc=5.0 min CN=74 Runoff=0.03 cfs 66 cf

SubcatchmentPOA3-PRE:DA3 Runoff Area=0.050 ac 0.00% Impervious Runoff Depth>0.53"
 Flow Length=20' Slope=0.0500 '/' Tc=5.0 min CN=72 Runoff=0.04 cfs 96 cf

SubcatchmentPOA3.1-POST:DA3.1 Runoff Area=0.140 ac 0.00% Impervious Runoff Depth>0.61"
 Flow Length=85' Slope=0.1000 '/' Tc=6.6 min CN=74 Runoff=0.11 cfs 308 cf

SubcatchmentPOA4-POST:DA4 Runoff Area=0.350 ac 0.00% Impervious Runoff Depth>0.61"
 Flow Length=178' Tc=10.8 min CN=74 Runoff=0.23 cfs 769 cf

SubcatchmentPOA4-PRE:DA4 Runoff Area=0.750 ac 0.00% Impervious Runoff Depth>0.56"
 Flow Length=390' Tc=26.9 min CN=73 Runoff=0.29 cfs 1,522 cf

Pond SMA1: SMA1 Peak Elev=196.53' Storage=13,120 cf Inflow=4.82 cfs 13,125 cf
 Outflow=0.00 cfs 0 cf

Link TOTAL POA2: TOTAL DA2 Inflow=2.48 cfs 9,375 cf
 Primary=2.48 cfs 9,375 cf

Total Runoff Area = 511,394 sf Runoff Volume = 38,191 cf Average Runoff Depth = 0.90"
74.32% Pervious = 380,061 sf 25.68% Impervious = 131,333 sf

Time span=5.00-20.00 hrs, dt=0.03 hrs, 501 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentPOA1-POST:DA1 Runoff Area=0.080 ac 0.00% Impervious Runoff Depth>1.94"
 Flow Length=20' Slope=0.0200 '/' Tc=5.0 min CN=74 Runoff=0.22 cfs 563 cf

SubcatchmentPOA1-PRE:DA1 Runoff Area=1.780 ac 0.00% Impervious Runoff Depth>1.76"
 Flow Length=557' Tc=32.9 min CN=72 Runoff=2.14 cfs 11,403 cf

SubcatchmentPOA1.1-POST:DA1.1 Runoff Area=2.490 ac 62.25% Impervious Runoff Depth>3.25"
 Flow Length=615' Tc=6.5 min CN=89 Runoff=10.30 cfs 29,365 cf

SubcatchmentPOA2-POST:DA2 Runoff Area=2.760 ac 27.72% Impervious Runoff Depth>2.50"
 Flow Length=533' Tc=16.4 min CN=81 Runoff=6.61 cfs 25,041 cf

SubcatchmentPOA2-PRE:DA2 Runoff Area=3.310 ac 21.15% Impervious Runoff Depth>2.25"
 Flow Length=734' Tc=14.3 min CN=78 Runoff=7.60 cfs 27,014 cf

SubcatchmentPOA3-POST:DA3 Runoff Area=0.030 ac 0.00% Impervious Runoff Depth>1.94"
 Flow Length=20' Slope=0.0500 '/' Tc=5.0 min CN=74 Runoff=0.08 cfs 211 cf

SubcatchmentPOA3-PRE:DA3 Runoff Area=0.050 ac 0.00% Impervious Runoff Depth>1.79"
 Flow Length=20' Slope=0.0500 '/' Tc=5.0 min CN=72 Runoff=0.13 cfs 325 cf

SubcatchmentPOA3.1-POST:DA3.1 Runoff Area=0.140 ac 0.00% Impervious Runoff Depth>1.94"
 Flow Length=85' Slope=0.1000 '/' Tc=6.6 min CN=74 Runoff=0.37 cfs 985 cf

SubcatchmentPOA4-POST:DA4 Runoff Area=0.350 ac 0.00% Impervious Runoff Depth>1.93"
 Flow Length=178' Tc=10.8 min CN=74 Runoff=0.78 cfs 2,457 cf

SubcatchmentPOA4-PRE:DA4 Runoff Area=0.750 ac 0.00% Impervious Runoff Depth>1.84"
 Flow Length=390' Tc=26.9 min CN=73 Runoff=1.05 cfs 5,021 cf

Pond SMA1: SMA1 Peak Elev=198.04' Storage=24,657 cf Inflow=10.30 cfs 29,365 cf
 Outflow=0.35 cfs 4,819 cf

Link TOTAL POA2: TOTAL DA2 Inflow=6.61 cfs 29,860 cf
 Primary=6.61 cfs 29,860 cf

Total Runoff Area = 511,394 sf Runoff Volume = 102,385 cf Average Runoff Depth = 2.40"
74.32% Pervious = 380,061 sf 25.68% Impervious = 131,333 sf

Time span=5.00-20.00 hrs, dt=0.03 hrs, 501 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentPOA1-POST:DA1 Runoff Area=0.080 ac 0.00% Impervious Runoff Depth>3.72"
 Flow Length=20' Slope=0.0200 '/' Tc=5.0 min CN=74 Runoff=0.42 cfs 1,081 cf

SubcatchmentPOA1-PRE:DA1 Runoff Area=1.780 ac 0.00% Impervious Runoff Depth>3.48"
 Flow Length=557' Tc=32.9 min CN=72 Runoff=4.22 cfs 22,474 cf

SubcatchmentPOA1.1-POST:DA1.1 Runoff Area=2.490 ac 62.25% Impervious Runoff Depth>5.31"
 Flow Length=615' Tc=6.5 min CN=89 Runoff=16.35 cfs 48,031 cf

SubcatchmentPOA2-POST:DA2 Runoff Area=2.760 ac 27.72% Impervious Runoff Depth>4.45"
 Flow Length=533' Tc=16.4 min CN=81 Runoff=11.52 cfs 44,552 cf

SubcatchmentPOA2-PRE:DA2 Runoff Area=3.310 ac 21.15% Impervious Runoff Depth>4.13"
 Flow Length=734' Tc=14.3 min CN=78 Runoff=13.70 cfs 49,611 cf

SubcatchmentPOA3-POST:DA3 Runoff Area=0.030 ac 0.00% Impervious Runoff Depth>3.72"
 Flow Length=20' Slope=0.0500 '/' Tc=5.0 min CN=74 Runoff=0.16 cfs 406 cf

SubcatchmentPOA3-PRE:DA3 Runoff Area=0.050 ac 0.00% Impervious Runoff Depth>3.52"
 Flow Length=20' Slope=0.0500 '/' Tc=5.0 min CN=72 Runoff=0.25 cfs 639 cf

SubcatchmentPOA3.1-POST:DA3.1 Runoff Area=0.140 ac 0.00% Impervious Runoff Depth>3.72"
 Flow Length=85' Slope=0.1000 '/' Tc=6.6 min CN=74 Runoff=0.70 cfs 1,891 cf

SubcatchmentPOA4-POST:DA4 Runoff Area=0.350 ac 0.00% Impervious Runoff Depth>3.72"
 Flow Length=178' Tc=10.8 min CN=74 Runoff=1.48 cfs 4,720 cf

SubcatchmentPOA4-PRE:DA4 Runoff Area=0.750 ac 0.00% Impervious Runoff Depth>3.59"
 Flow Length=390' Tc=26.9 min CN=73 Runoff=2.03 cfs 9,771 cf

Pond SMA1: SMA1 Peak Elev=198.44' Storage=28,254 cf Inflow=16.35 cfs 48,031 cf
 Outflow=2.69 cfs 23,426 cf

Link TOTAL POA2: TOTAL DA2 Inflow=14.24 cfs 67,978 cf
 Primary=14.24 cfs 67,978 cf

Total Runoff Area = 511,394 sf Runoff Volume = 183,176 cf Average Runoff Depth = 4.30"
74.32% Pervious = 380,061 sf 25.68% Impervious = 131,333 sf

Time span=5.00-20.00 hrs, dt=0.03 hrs, 501 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentPOA1-POST:DA1 Runoff Area=0.080 ac 0.00% Impervious Runoff Depth>4.81"
 Flow Length=20' Slope=0.0200 '/' Tc=5.0 min CN=74 Runoff=0.54 cfs 1,397 cf

SubcatchmentPOA1-PRE:DA1 Runoff Area=1.780 ac 0.00% Impervious Runoff Depth>4.53"
 Flow Length=557' Tc=32.9 min CN=72 Runoff=5.47 cfs 29,291 cf

SubcatchmentPOA1.1-POST:DA1.1 Runoff Area=2.490 ac 62.25% Impervious Runoff Depth>6.50"
 Flow Length=615' Tc=6.5 min CN=89 Runoff=19.77 cfs 58,722 cf

SubcatchmentPOA2-POST:DA2 Runoff Area=2.760 ac 27.72% Impervious Runoff Depth>5.60"
 Flow Length=533' Tc=16.4 min CN=81 Runoff=14.33 cfs 56,089 cf

SubcatchmentPOA2-PRE:DA2 Runoff Area=3.310 ac 21.15% Impervious Runoff Depth>5.26"
 Flow Length=734' Tc=14.3 min CN=78 Runoff=17.24 cfs 63,152 cf

SubcatchmentPOA3-POST:DA3 Runoff Area=0.030 ac 0.00% Impervious Runoff Depth>4.81"
 Flow Length=20' Slope=0.0500 '/' Tc=5.0 min CN=74 Runoff=0.20 cfs 524 cf

SubcatchmentPOA3-PRE:DA3 Runoff Area=0.050 ac 0.00% Impervious Runoff Depth>4.58"
 Flow Length=20' Slope=0.0500 '/' Tc=5.0 min CN=72 Runoff=0.32 cfs 832 cf

SubcatchmentPOA3.1-POST:DA3.1 Runoff Area=0.140 ac 0.00% Impervious Runoff Depth>4.81"
 Flow Length=85' Slope=0.1000 '/' Tc=6.6 min CN=74 Runoff=0.89 cfs 2,444 cf

SubcatchmentPOA4-POST:DA4 Runoff Area=0.350 ac 0.00% Impervious Runoff Depth>4.80"
 Flow Length=178' Tc=10.8 min CN=74 Runoff=1.89 cfs 6,100 cf

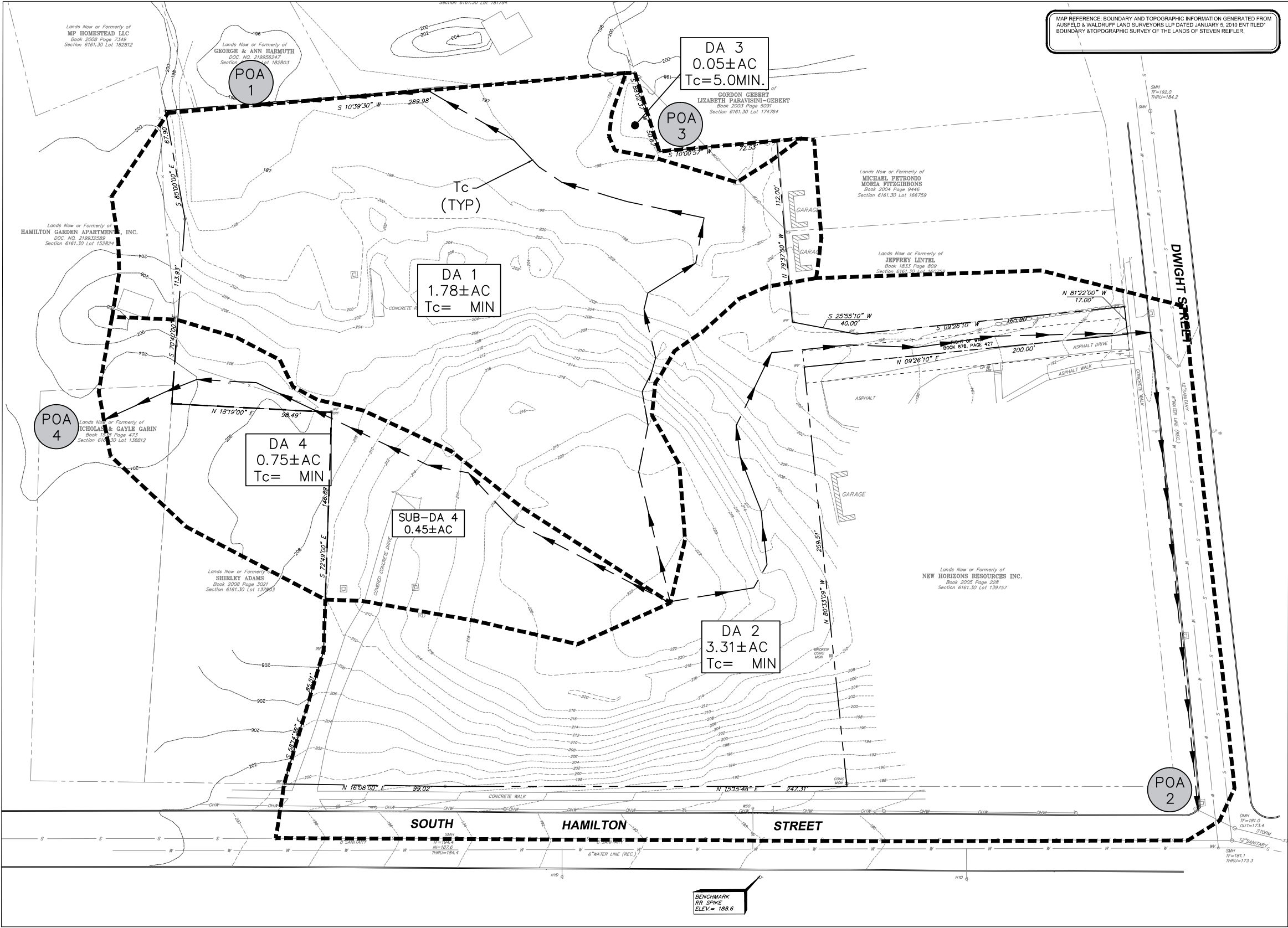
SubcatchmentPOA4-PRE:DA4 Runoff Area=0.750 ac 0.00% Impervious Runoff Depth>4.66"
 Flow Length=390' Tc=26.9 min CN=73 Runoff=2.62 cfs 12,681 cf

Pond SMA1: SMA1 Peak Elev=199.10' Storage=34,574 cf Inflow=19.77 cfs 58,722 cf
 Outflow=2.90 cfs 34,084 cf

Link TOTAL POA2: TOTAL DA2 Inflow=17.15 cfs 90,173 cf
 Primary=17.15 cfs 90,173 cf

Total Runoff Area = 511,394 sf Runoff Volume = 231,231 cf Average Runoff Depth = 5.43"
74.32% Pervious = 380,061 sf 25.68% Impervious = 131,333 sf

APPENDIX H



ISSUED FOR REVIEW
NOT FOR CONSTRUCTION
RECORD PLANS

DATE SUBMITTED:

NO.	REVISIONS	DESCRIPTION	BY	DATE

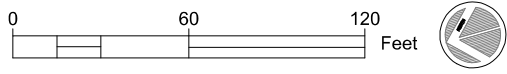
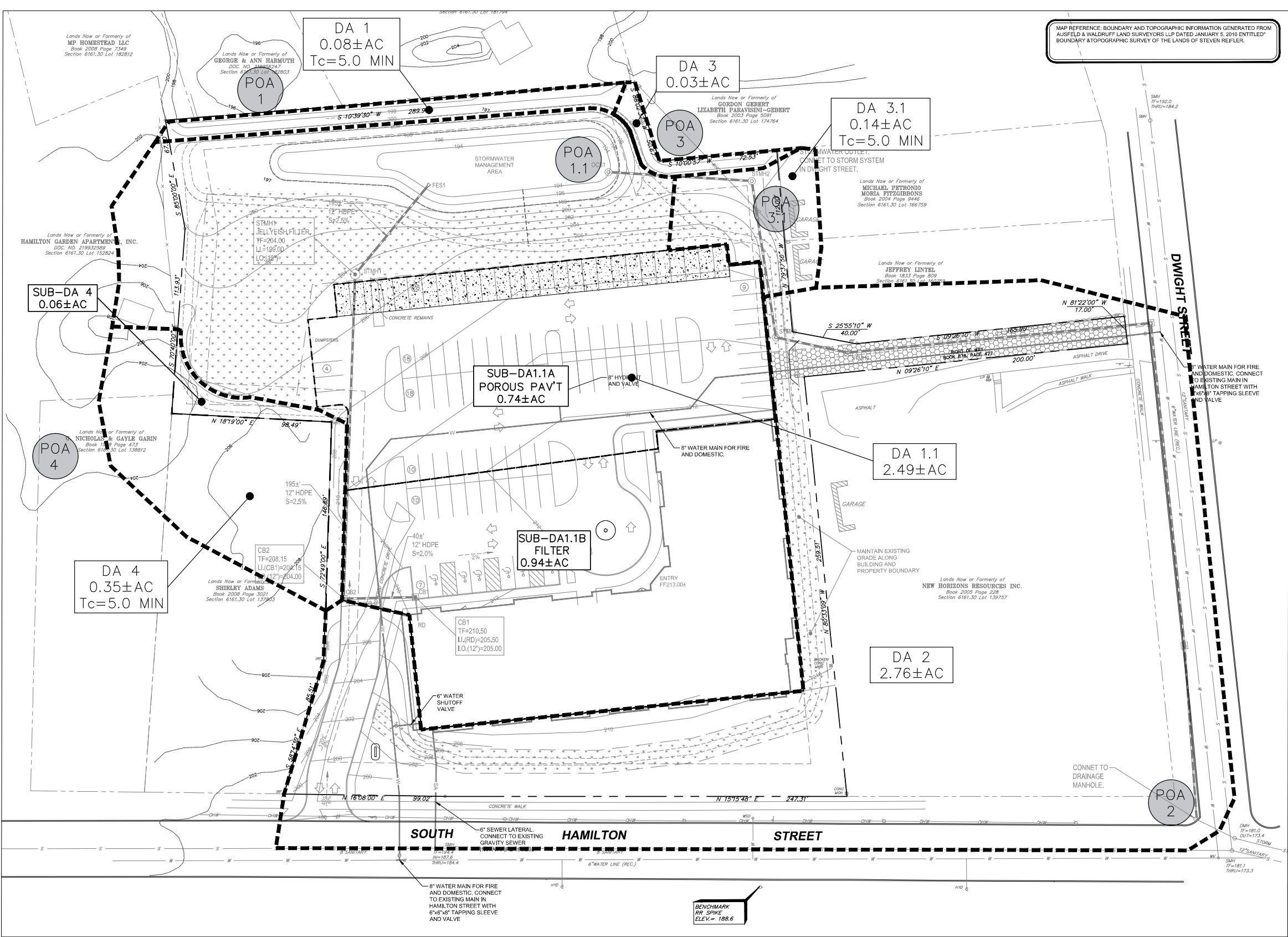
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QTY OF POUGHKEEPSIE COUNTY OF DUTCHESS
PRE-DEVELOPMENT DRAINAGE
SOUTH HAMILTON
LOCATED AT
100 SOUTH HAMILTON STREET
TAX MAP: 6161-30-151763

DATE: AUGUST 2023
JOB NO: 22.10
SCALE: AS NOTED
DRAWN BY: DFA
CHECKED BY: DFA
DRAWING NO:
SWPPP
1.0
SHEET 2 OF 3



ISSUED FOR REVIEW
NOT FOR CONSTRUCTION
RECORD PLANS

DATE SUBMITTED:

NO.	REVISIONS	DESCRIPTION	DATE	BY
1	1	PER CITY COMMENTS	05.13.23	DFA

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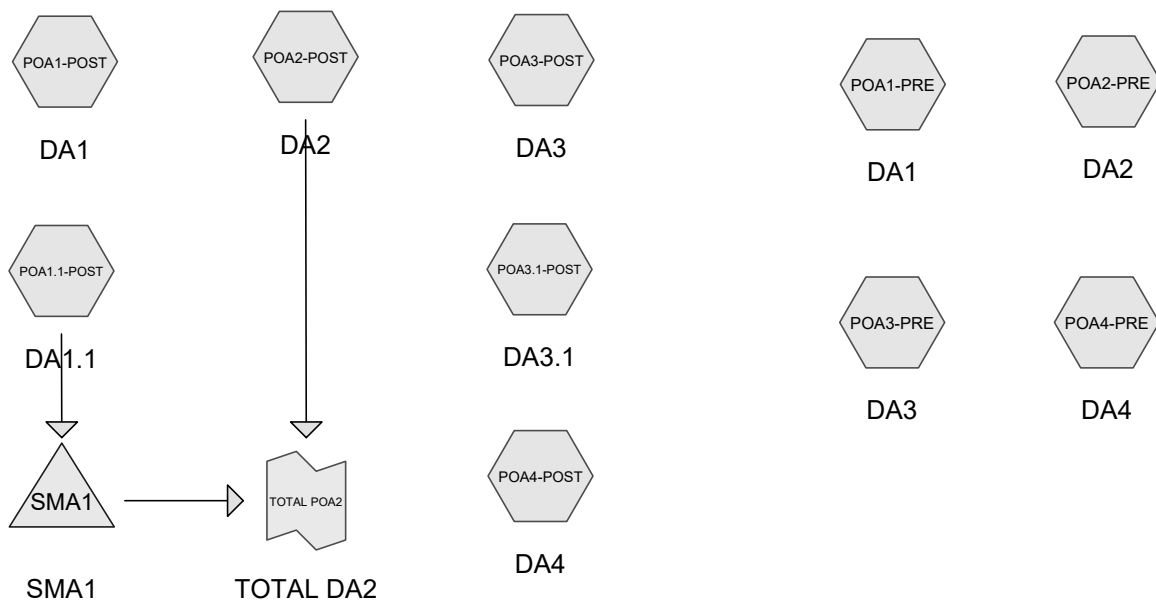


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CITY OF POUGHKEEPSIE COUNTY OF DUTCHESS
POST-DEVELOPMENT DRAINAGE SOUTH HAMILTON STREET
LOCATED AT
100 SOUTH HAMILTON STREET
TAX MAP: 6161-30-151763

DATE: AUGUST 2023
JOB NO. 22.10
SCALE: AS NOTED
DRAWN BY: DFA
CHECKED BY: DFA
DRAWING NO.
SWPPP 2.0

SHEET 1 OF 1



Routing Diagram for HAMILTON ST_POSTvsPRE_10.03.23_OUTLET CONTROL
 Prepared by ARICO ASSOCIATES, Printed 9/25/2023
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Time span=5.00-20.00 hrs, dt=0.03 hrs, 501 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentPOA1-POST:DA1 Runoff Area=0.080 ac 0.00% Impervious Runoff Depth>0.61"
Flow Length=20' Slope=0.0200 '/ Tc=5.0 min CN=74 Runoff=0.07 cfs 176 cf

SubcatchmentPOA1-PRE:DA1 Runoff Area=1.780 ac 0.00% Impervious Runoff Depth>0.52"
Flow Length=557' Tc=32.9 min CN=72 Runoff=0.56 cfs 3,349 cf

SubcatchmentPOA1.1-POST:DA1.1 Runoff Area=2.490 ac 62.25% Impervious Runoff Depth>1.45"
Flow Length=615' Tc=6.5 min CN=89 Runoff=4.82 cfs 13,125 cf

SubcatchmentPOA2-POST:DA2 Runoff Area=2.760 ac 27.72% Impervious Runoff Depth>0.94"
Flow Length=533' Tc=16.4 min CN=81 Runoff=2.48 cfs 9,375 cf

SubcatchmentPOA2-PRE:DA2 Runoff Area=3.310 ac 21.15% Impervious Runoff Depth>0.78"
Flow Length=734' Tc=14.3 min CN=78 Runoff=2.61 cfs 9,405 cf

SubcatchmentPOA3-POST:DA3 Runoff Area=0.030 ac 0.00% Impervious Runoff Depth>0.61"
Flow Length=20' Slope=0.0500 '/ Tc=5.0 min CN=74 Runoff=0.03 cfs 66 cf

SubcatchmentPOA3-PRE:DA3 Runoff Area=0.050 ac 0.00% Impervious Runoff Depth>0.53"
Flow Length=20' Slope=0.0500 '/ Tc=5.0 min CN=72 Runoff=0.04 cfs 96 cf

SubcatchmentPOA3.1-POST:DA3.1 Runoff Area=0.140 ac 0.00% Impervious Runoff Depth>0.61"
Flow Length=85' Slope=0.1000 '/ Tc=6.6 min CN=74 Runoff=0.11 cfs 308 cf

SubcatchmentPOA4-POST:DA4 Runoff Area=0.350 ac 0.00% Impervious Runoff Depth>0.61"
Flow Length=178' Tc=10.8 min CN=74 Runoff=0.23 cfs 769 cf

SubcatchmentPOA4-PRE:DA4 Runoff Area=0.750 ac 0.00% Impervious Runoff Depth>0.56"
Flow Length=390' Tc=26.9 min CN=73 Runoff=0.29 cfs 1,522 cf

Pond SMA1: SMA1 Peak Elev=196.53' Storage=13,120 cf Inflow=4.82 cfs 13,125 cf
Outflow=0.00 cfs 0 cf

Link TOTAL POA2: TOTAL DA2 Inflow=2.48 cfs 9,375 cf
Primary=2.48 cfs 9,375 cf

Total Runoff Area = 511,394 sf Runoff Volume = 38,191 cf Average Runoff Depth = 0.90"
74.32% Pervious = 380,061 sf 25.68% Impervious = 131,333 sf

Summary for Subcatchment POA1-POST: DA1

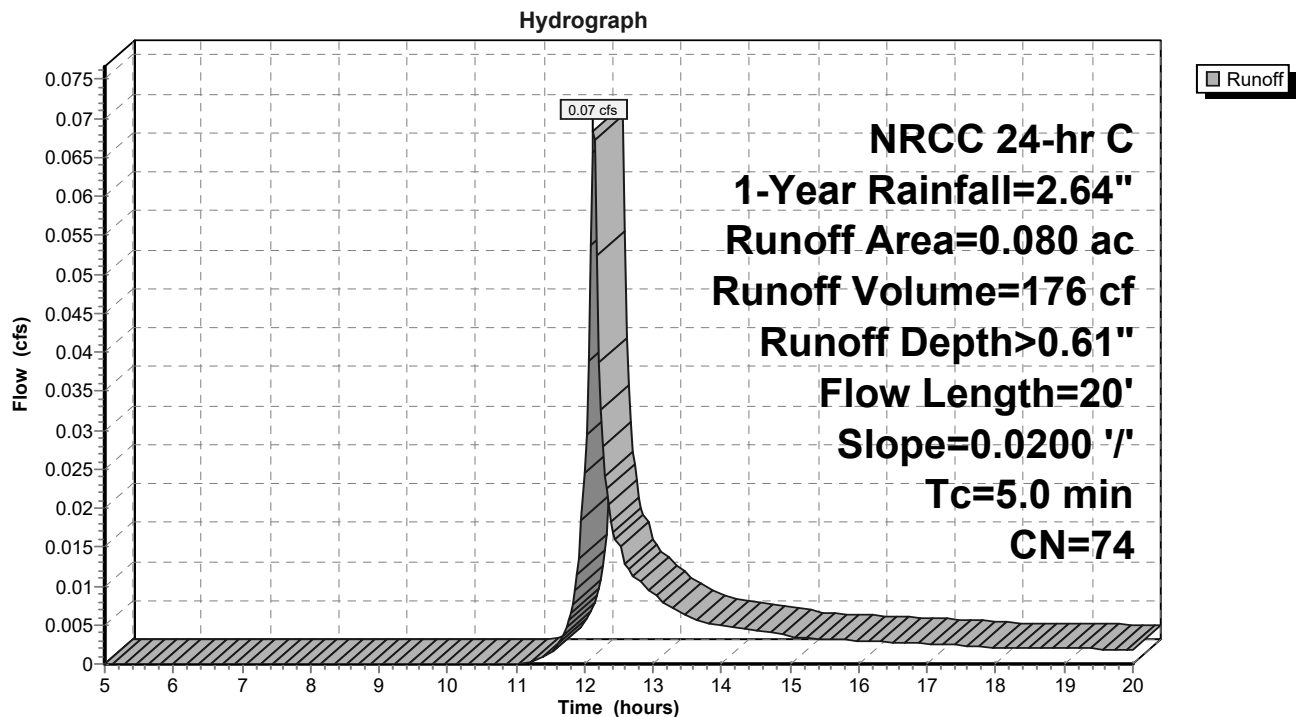
Runoff = 0.07 cfs @ 12.13 hrs, Volume= 176 cf, Depth> 0.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 1-Year Rainfall=2.64"

Area (ac)	CN	Description
0.080	74	>75% Grass cover, Good, HSG C
0.080		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.0	20	0.0200	0.08		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"
4.0	20	Total, Increased to minimum Tc = 5.0 min			

Subcatchment POA1-POST: DA1



Summary for Subcatchment POA1-PRE: DA1

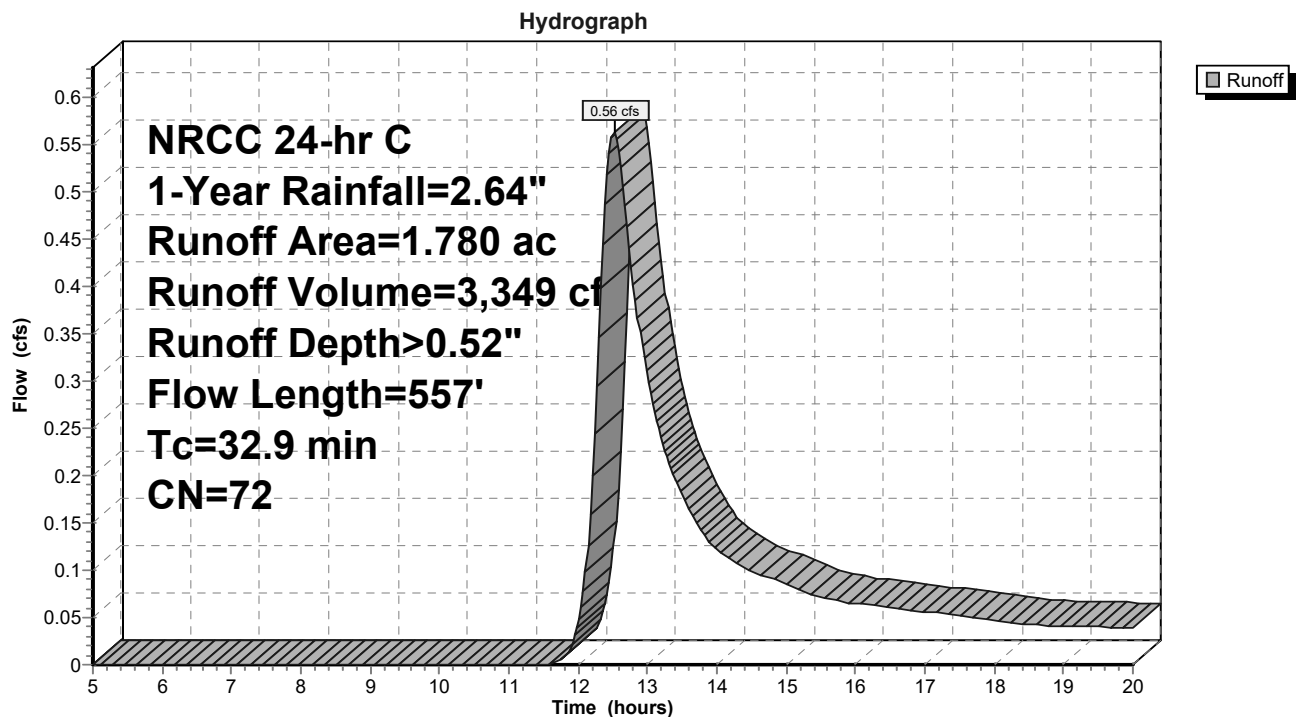
Runoff = 0.56 cfs @ 12.52 hrs, Volume= 3,349 cf, Depth> 0.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 1-Year Rainfall=2.64"

Area (ac)	CN	Description
1.640	72	Woods/grass comb., Good, HSG C
0.140	74	>75% Grass cover, Good, HSG C
1.780	72	Weighted Average
1.780		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
21.5	100	0.0200	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.18"
0.6	86	0.2100	2.29		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
10.8	371	0.0130	0.57		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
32.9	557	Total			

Subcatchment POA1-PRE: DA1



Summary for Subcatchment POA1.1-POST: DA1.1

[47] Hint: Peak is 104% of capacity of segment #3

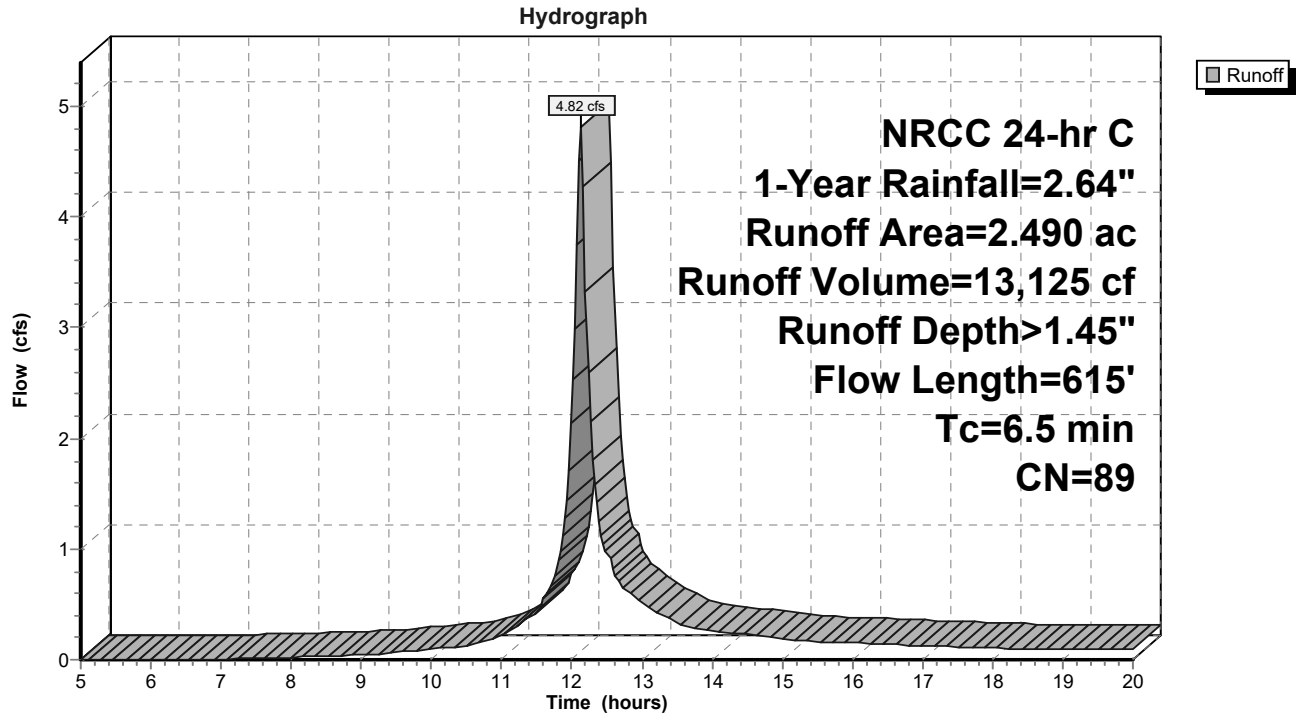
Runoff = 4.82 cfs @ 12.14 hrs, Volume= 13,125 cf, Depth> 1.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 1-Year Rainfall=2.64"

Area (ac)	CN	Description
1.000	98	Paved parking, HSG C
0.550	98	Roofs, HSG C
0.940	74	>75% Grass cover, Good, HSG C
2.490	89	Weighted Average
0.940		37.75% Pervious Area
1.550		62.25% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	30	0.0200	0.13		Sheet Flow, Grass: Short n= 0.150 P2= 3.18"
0.8	150	0.0260	3.27		Shallow Concentrated Flow, Paved Kv= 20.3 fps
0.1	45	0.0100	5.90	4.63	Pipe Channel, 12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25' n= 0.010 PVC, smooth interior
0.4	190	0.0150	7.22	5.67	Pipe Channel, 12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25' n= 0.010 PVC, smooth interior
0.1	70	0.0500	17.28	30.53	Pipe Channel, 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.010 PVC, smooth interior
1.3	130	0.0050	1.63	16.34	Channel Flow, Area= 10.0 sf Perim= 9.0' r= 1.11' n= 0.069 Riprap, 6-inch
6.5	615	Total			

Subcatchment POA1.1-POST: DA1.1



Summary for Subcatchment POA2-POST: DA2

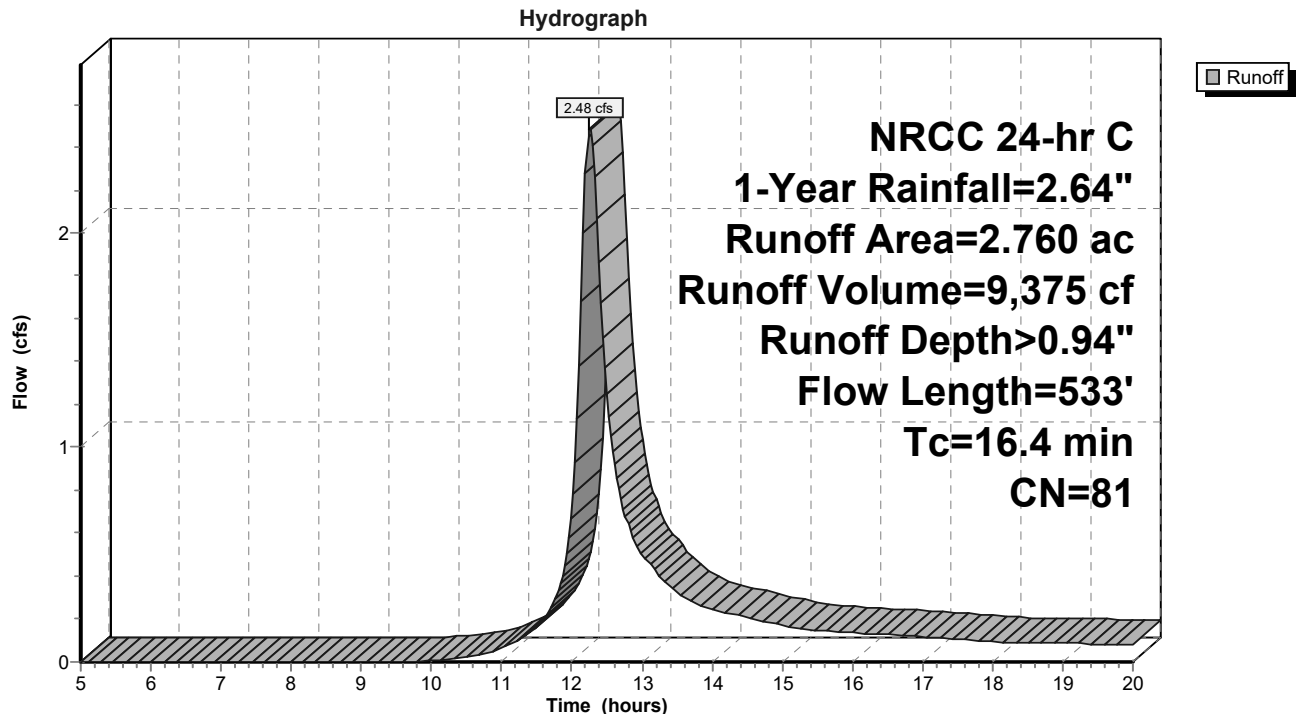
Runoff = 2.48 cfs @ 12.26 hrs, Volume= 9,375 cf, Depth> 0.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 1-Year Rainfall=2.64"

Area (ac)	CN	Description
1.995	74	>75% Grass cover, Good, HSG C
0.765	98	Paved roads w/curbs & sewers, HSG C
2.760	81	Weighted Average
1.995		72.28% Pervious Area
0.765		27.72% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.3	100	0.0200	0.12		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"
0.6	137	0.0510	3.64		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
1.5	296	0.0270	3.34		Shallow Concentrated Flow, Paved Kv= 20.3 fps
16.4	533	Total			

Subcatchment POA2-POST: DA2



Summary for Subcatchment POA2-PRE: DA2

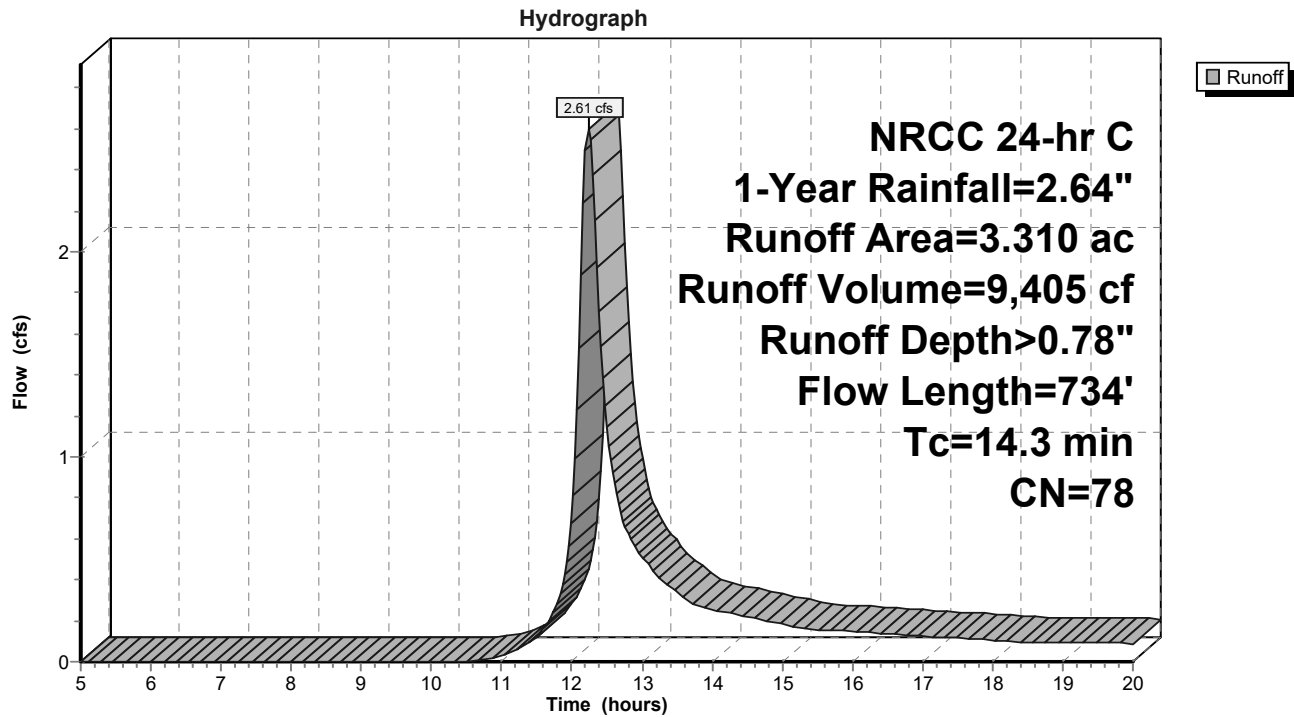
Runoff = 2.61 cfs @ 12.23 hrs, Volume= 9,405 cf, Depth> 0.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 1-Year Rainfall=2.64"

Area (ac)	CN	Description
1.150	72	Woods/grass comb., Good, HSG C
1.460	74	>75% Grass cover, Good, HSG C
0.700	98	Paved roads w/curbs & sewers, HSG C
3.310	78	Weighted Average
2.610		78.85% Pervious Area
0.700		21.15% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	100	0.1200	0.16		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.18"
1.3	123	0.1000	1.58		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.0	215	0.0510	3.64		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
1.5	296	0.0270	3.34		Shallow Concentrated Flow, Paved Kv= 20.3 fps
14.3	734	Total			

Subcatchment POA2-PRE: DA2



Summary for Subcatchment POA3-POST: DA3

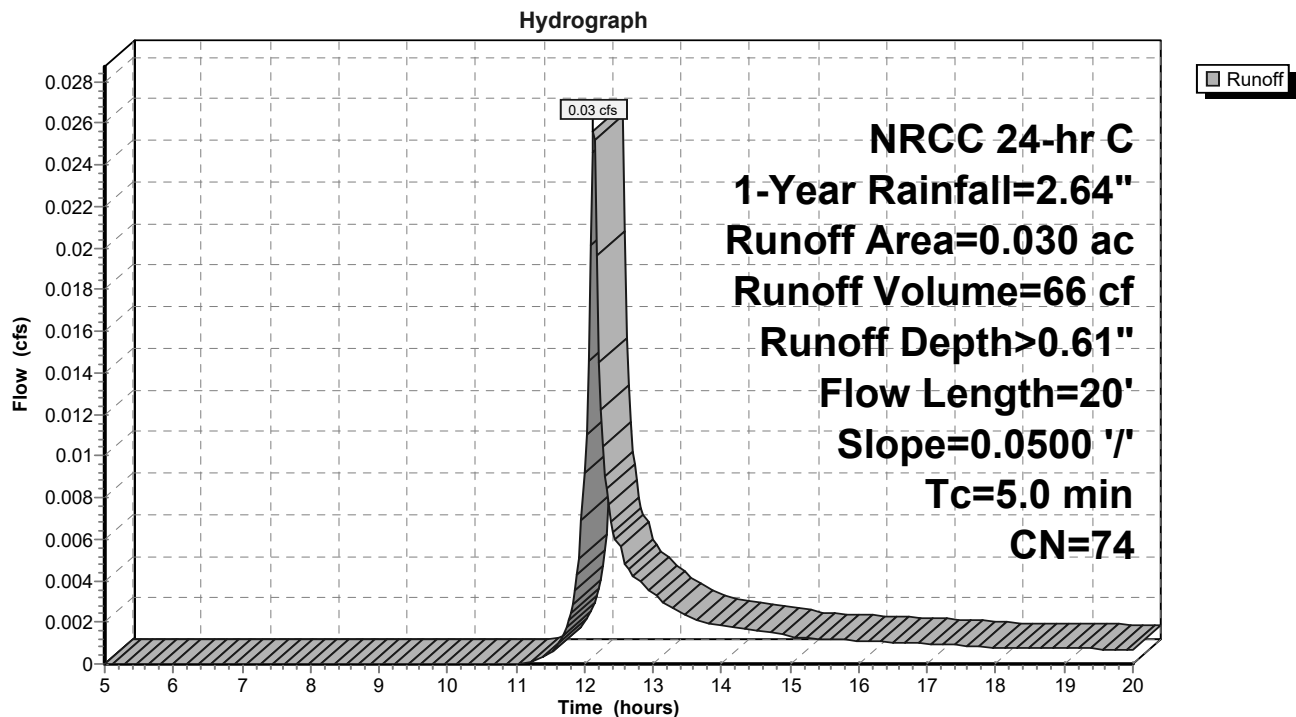
Runoff = 0.03 cfs @ 12.13 hrs, Volume= 66 cf, Depth> 0.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 1-Year Rainfall=2.64"

Area (ac)	CN	Description
0.030	74	>75% Grass cover, Good, HSG C
0.030		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.7	20	0.0500	0.12		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"
2.7	20	Total, Increased to minimum Tc = 5.0 min			

Subcatchment POA3-POST: DA3



Summary for Subcatchment POA3-PRE: DA3

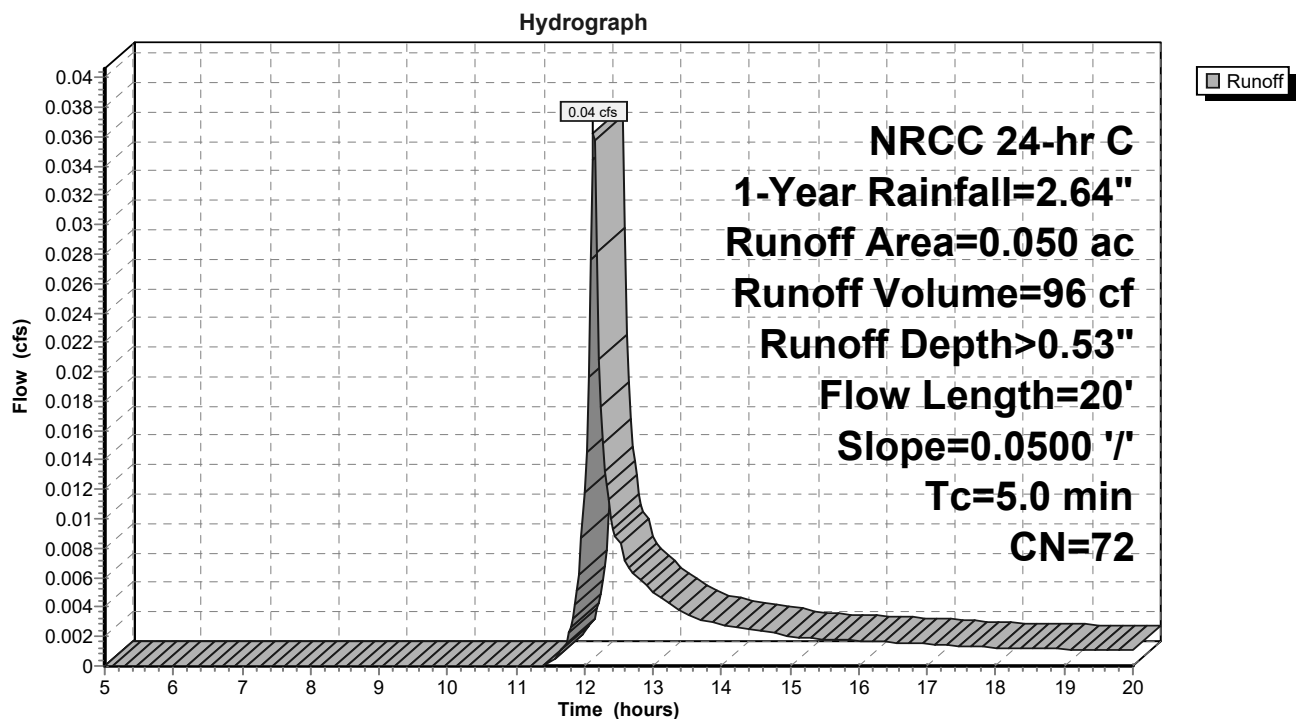
Runoff = 0.04 cfs @ 12.13 hrs, Volume= 96 cf, Depth> 0.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 1-Year Rainfall=2.64"

Area (ac)	CN	Description
0.050	72	Woods/grass comb., Good, HSG C
0.050		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.1	20	0.0500	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.18"
4.1	20	Total, Increased to minimum Tc = 5.0 min			

Subcatchment POA3-PRE: DA3



Summary for Subcatchment POA3.1-POST: DA3.1

TO CB1

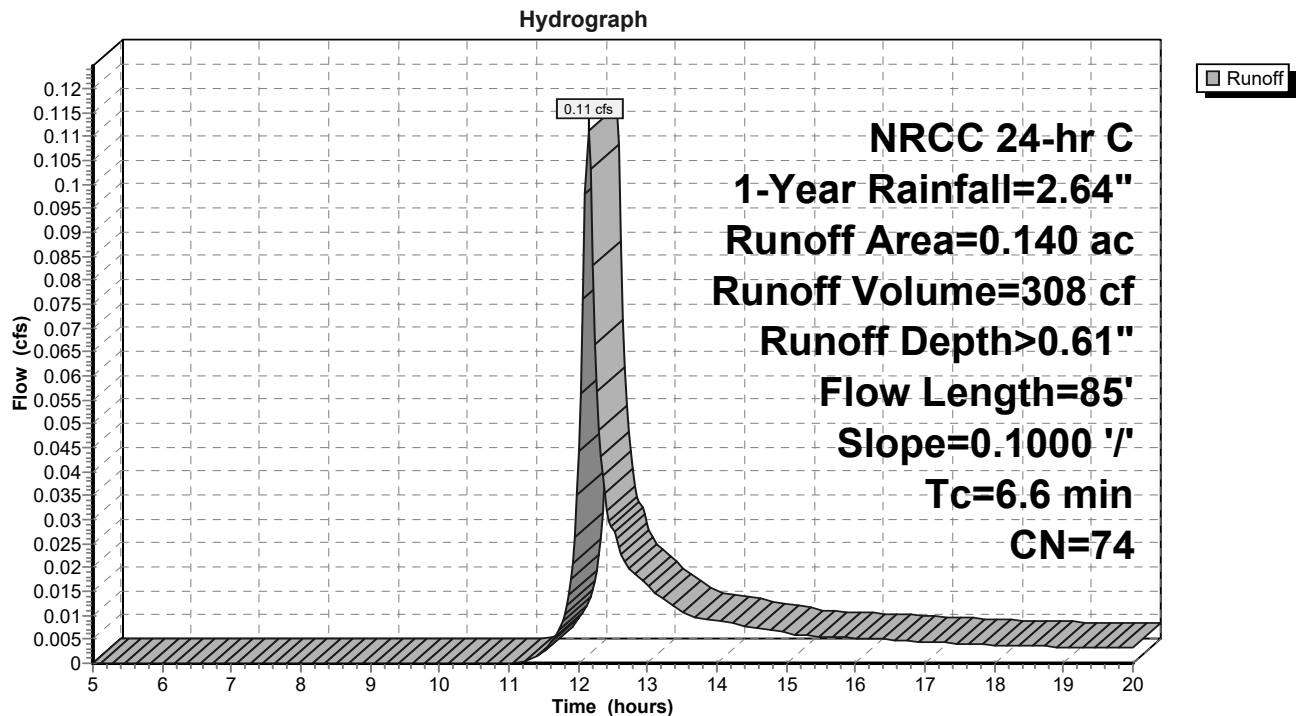
Runoff = 0.11 cfs @ 12.14 hrs, Volume= 308 cf, Depth> 0.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 1-Year Rainfall=2.64"

Area (ac)	CN	Description
0.041	74	>75% Grass cover, Good, HSG C
0.099	74	>75% Grass cover, Good, HSG C
0.140	74	Weighted Average
0.140		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.6	85	0.1000	0.21		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"

Subcatchment POA3.1-POST: DA3.1



Summary for Subcatchment POA4-POST: DA4

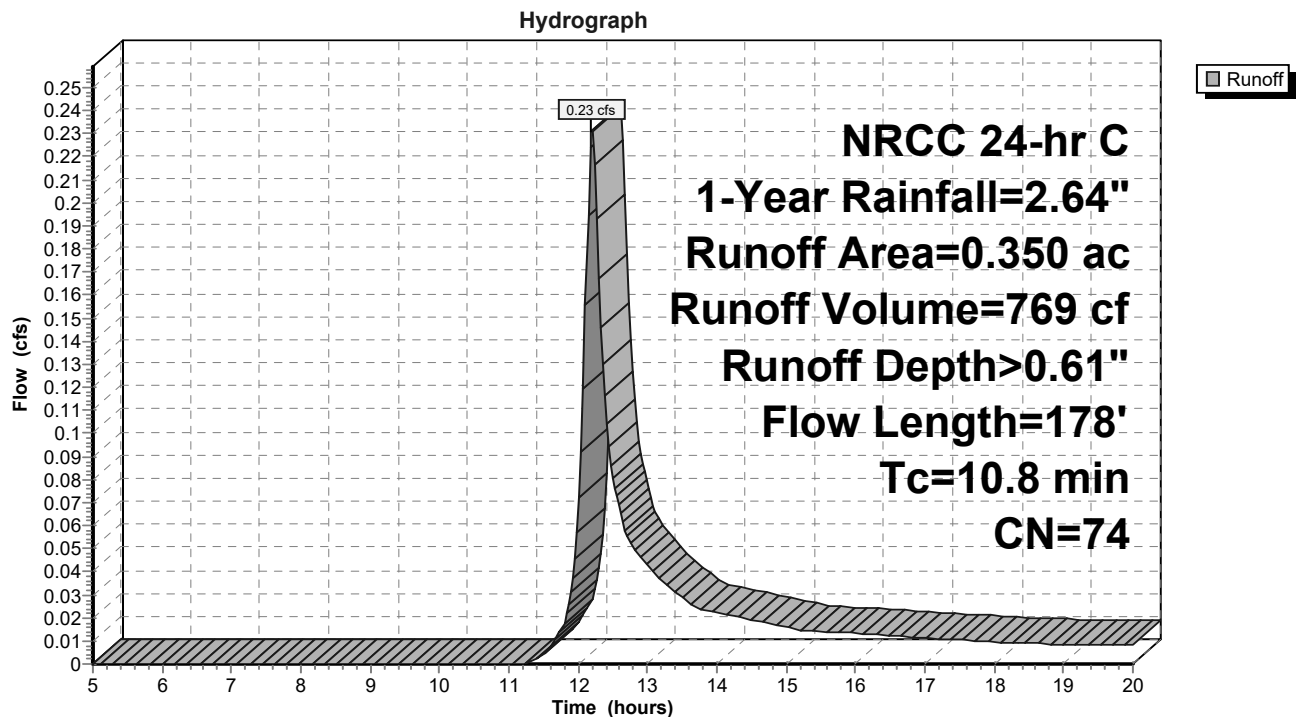
Runoff = 0.23 cfs @ 12.19 hrs, Volume= 769 cf, Depth> 0.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 1-Year Rainfall=2.64"

Area (ac)	CN	Description
0.350	74	>75% Grass cover, Good, HSG C
0.350		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.3	100	0.0450	0.16		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"
0.5	78	0.0250	2.55		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
10.8	178	Total			

Subcatchment POA4-POST: DA4



Summary for Subcatchment POA4-PRE: DA4

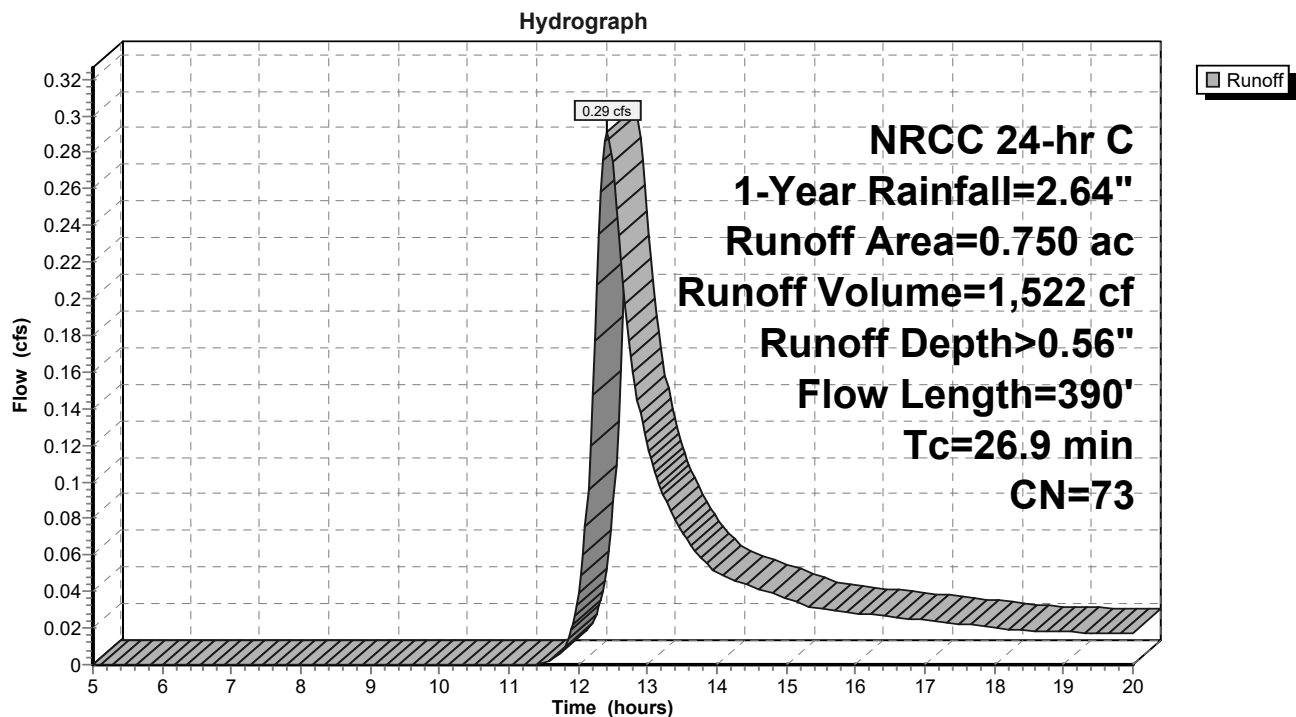
Runoff = 0.29 cfs @ 12.42 hrs, Volume= 1,522 cf, Depth> 0.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 1-Year Rainfall=2.64"

Area (ac)	CN	Description
0.450	72	Woods/grass comb., Good, HSG C
0.300	74	>75% Grass cover, Good, HSG C
0.750	73	Weighted Average
0.750		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.2	100	0.0150	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.18"
1.5	140	0.0920	1.52		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.2	150	0.0160	2.04		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
26.9	390	Total			

Subcatchment POA4-PRE: DA4



Summary for Pond SMA1: SMA1

Inflow Area = 108,464 sf, 62.25% Impervious, Inflow Depth > 1.45" for 1-Year event
 Inflow = 4.82 cfs @ 12.14 hrs, Volume= 13,125 cf
 Outflow = 0.00 cfs @ 5.00 hrs, Volume= 0 cf, Atten= 100%, Lag= 0.0 min
 Primary = 0.00 cfs @ 5.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
 Peak Elev= 196.53' @ 20.00 hrs Surf.Area= 6,753 sf Storage= 13,120 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= (not calculated: no outflow)

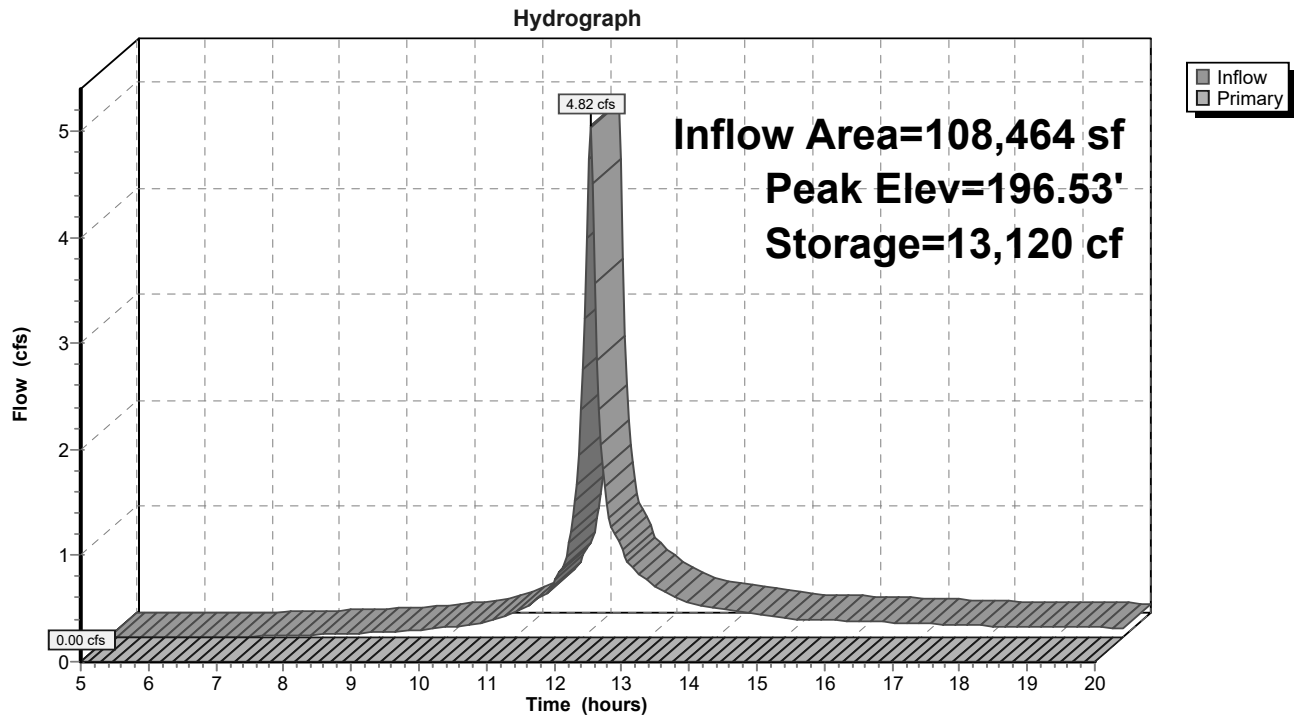
Volume	Invert	Avail.Storage	Storage Description	
#1	194.00'	44,331 cf	Custom Stage Data (Conic) Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
194.00	3,650	0	0	3,650
196.00	6,170	9,710	9,710	6,218
198.00	8,520	14,627	24,337	8,644
200.00	11,550	19,993	44,331	11,756

Device	Routing	Invert	Outlet Devices
#1	Primary	194.00'	8.0" Round Culvert - RISER OUTLET L= 25.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 194.00' / 193.75' S= 0.0100 '/' Cc= 0.900 n= 0.010, Flow Area= 0.35 sf
#2	Device 1	194.00'	3.0" W x 18.0" H Vert. Orifice - SIDE OF RISER C= 0.600 Limited to weir flow at low heads
#3	Device 2	198.00'	48.0" Horiz. Orifice - TOP OF RISER C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=194.00' (Free Discharge)

↑ **1=Culvert - RISER OUTLET** (Controls 0.00 cfs)
 ↑ **2=Orifice - SIDE OF RISER** (Controls 0.00 cfs)
 ↑ **3=Orifice - TOP OF RISER** (Controls 0.00 cfs)

Pond SMA1: SMA1

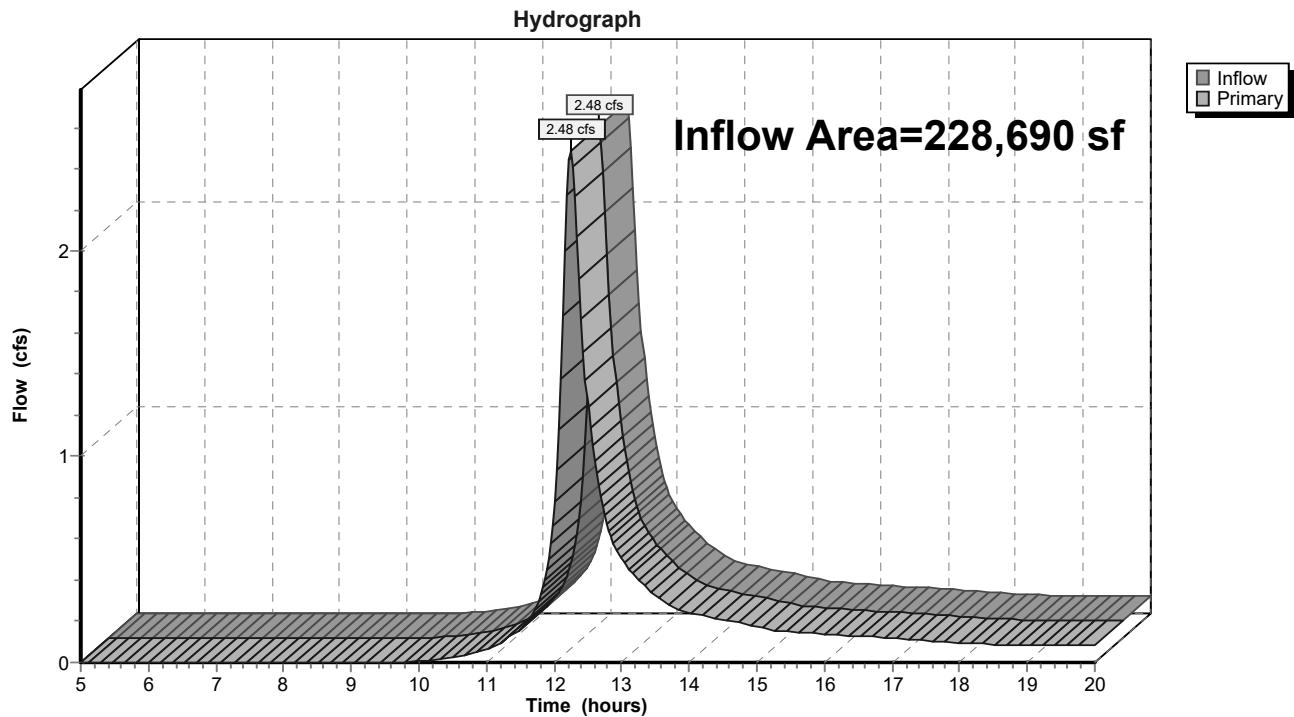


Summary for Link TOTAL POA2: TOTAL DA2

Inflow Area = 228,690 sf, 44.10% Impervious, Inflow Depth > 0.49" for 1-Year event
 Inflow = 2.48 cfs @ 12.26 hrs, Volume= 9,375 cf
 Primary = 2.48 cfs @ 12.26 hrs, Volume= 9,375 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs

Link TOTAL POA2: TOTAL DA2



Time span=5.00-20.00 hrs, dt=0.03 hrs, 501 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentPOA1-POST:DA1	Runoff Area=0.080 ac 0.00% Impervious Runoff Depth>1.94"
	Flow Length=20' Slope=0.0200 '/' Tc=5.0 min CN=74 Runoff=0.22 cfs 563 cf
SubcatchmentPOA1-PRE:DA1	Runoff Area=1.780 ac 0.00% Impervious Runoff Depth>1.76"
	Flow Length=557' Tc=32.9 min CN=72 Runoff=2.14 cfs 11,403 cf
SubcatchmentPOA1.1-POST:DA1.1	Runoff Area=2.490 ac 62.25% Impervious Runoff Depth>3.25"
	Flow Length=615' Tc=6.5 min CN=89 Runoff=10.30 cfs 29,365 cf
SubcatchmentPOA2-POST:DA2	Runoff Area=2.760 ac 27.72% Impervious Runoff Depth>2.50"
	Flow Length=533' Tc=16.4 min CN=81 Runoff=6.61 cfs 25,041 cf
SubcatchmentPOA2-PRE:DA2	Runoff Area=3.310 ac 21.15% Impervious Runoff Depth>2.25"
	Flow Length=734' Tc=14.3 min CN=78 Runoff=7.60 cfs 27,014 cf
SubcatchmentPOA3-POST:DA3	Runoff Area=0.030 ac 0.00% Impervious Runoff Depth>1.94"
	Flow Length=20' Slope=0.0500 '/' Tc=5.0 min CN=74 Runoff=0.08 cfs 211 cf
SubcatchmentPOA3-PRE:DA3	Runoff Area=0.050 ac 0.00% Impervious Runoff Depth>1.79"
	Flow Length=20' Slope=0.0500 '/' Tc=5.0 min CN=72 Runoff=0.13 cfs 325 cf
SubcatchmentPOA3.1-POST:DA3.1	Runoff Area=0.140 ac 0.00% Impervious Runoff Depth>1.94"
	Flow Length=85' Slope=0.1000 '/' Tc=6.6 min CN=74 Runoff=0.37 cfs 985 cf
SubcatchmentPOA4-POST:DA4	Runoff Area=0.350 ac 0.00% Impervious Runoff Depth>1.93"
	Flow Length=178' Tc=10.8 min CN=74 Runoff=0.78 cfs 2,457 cf
SubcatchmentPOA4-PRE:DA4	Runoff Area=0.750 ac 0.00% Impervious Runoff Depth>1.84"
	Flow Length=390' Tc=26.9 min CN=73 Runoff=1.05 cfs 5,021 cf
Pond SMA1: SMA1	Peak Elev=198.04' Storage=24,657 cf Inflow=10.30 cfs 29,365 cf
	Outflow=0.35 cfs 4,819 cf
Link TOTAL POA2: TOTAL DA2	Inflow=6.61 cfs 29,860 cf
	Primary=6.61 cfs 29,860 cf

Total Runoff Area = 511,394 sf Runoff Volume = 102,385 cf Average Runoff Depth = 2.40"
74.32% Pervious = 380,061 sf 25.68% Impervious = 131,333 sf

Summary for Subcatchment POA1-POST: DA1

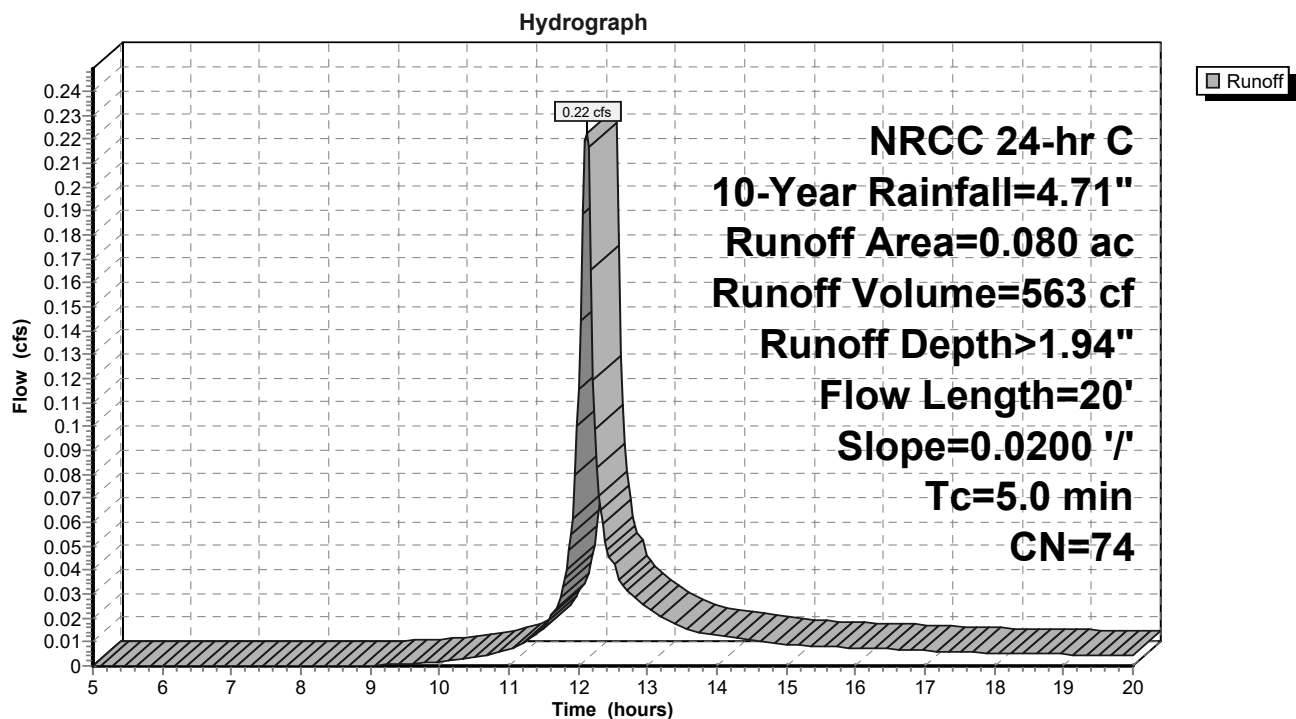
Runoff = 0.22 cfs @ 12.12 hrs, Volume= 563 cf, Depth> 1.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 10-Year Rainfall=4.71"

Area (ac)	CN	Description
0.080	74	>75% Grass cover, Good, HSG C
0.080		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.0	20	0.0200	0.08		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"
4.0	20	Total, Increased to minimum Tc = 5.0 min			

Subcatchment POA1-POST: DA1



Summary for Subcatchment POA1-PRE: DA1

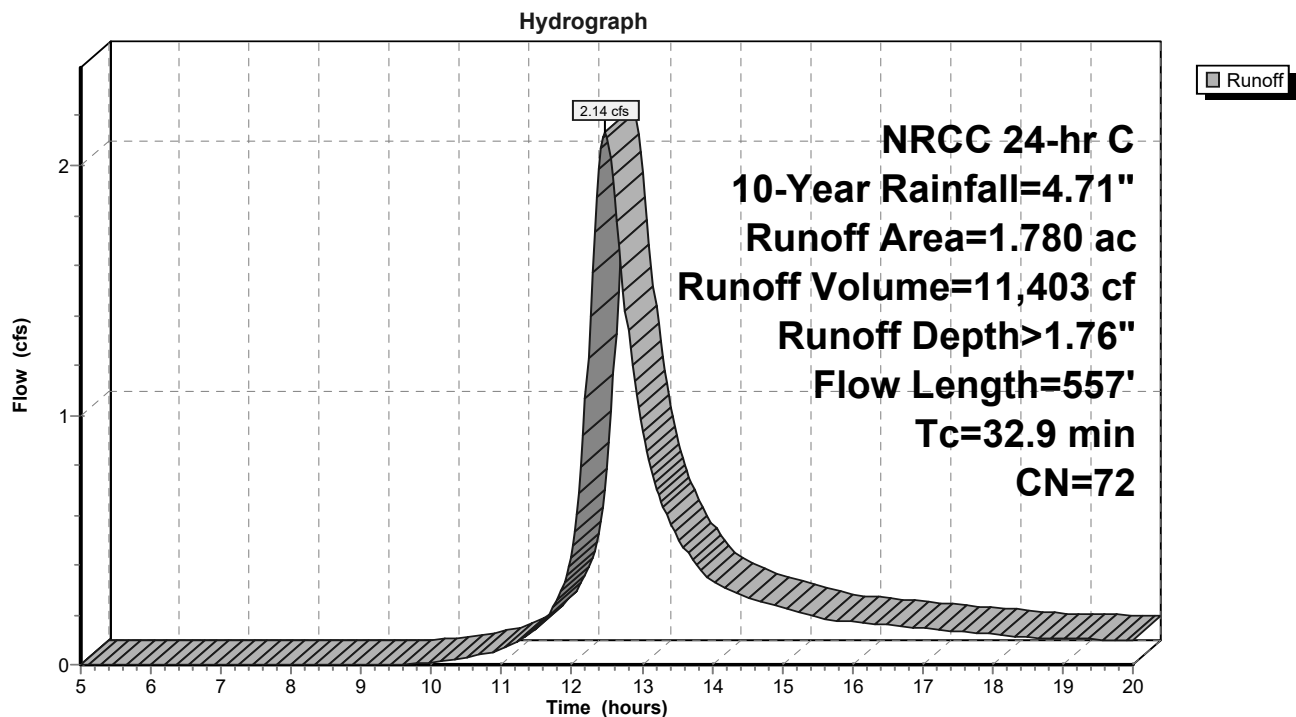
Runoff = 2.14 cfs @ 12.47 hrs, Volume= 11,403 cf, Depth> 1.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 10-Year Rainfall=4.71"

Area (ac)	CN	Description
1.640	72	Woods/grass comb., Good, HSG C
0.140	74	>75% Grass cover, Good, HSG C
1.780	72	Weighted Average
1.780		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
21.5	100	0.0200	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.18"
0.6	86	0.2100	2.29		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
10.8	371	0.0130	0.57		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
32.9	557	Total			

Subcatchment POA1-PRE: DA1



Summary for Subcatchment POA1.1-POST: DA1.1

[47] Hint: Peak is 222% of capacity of segment #3

[47] Hint: Peak is 182% of capacity of segment #4

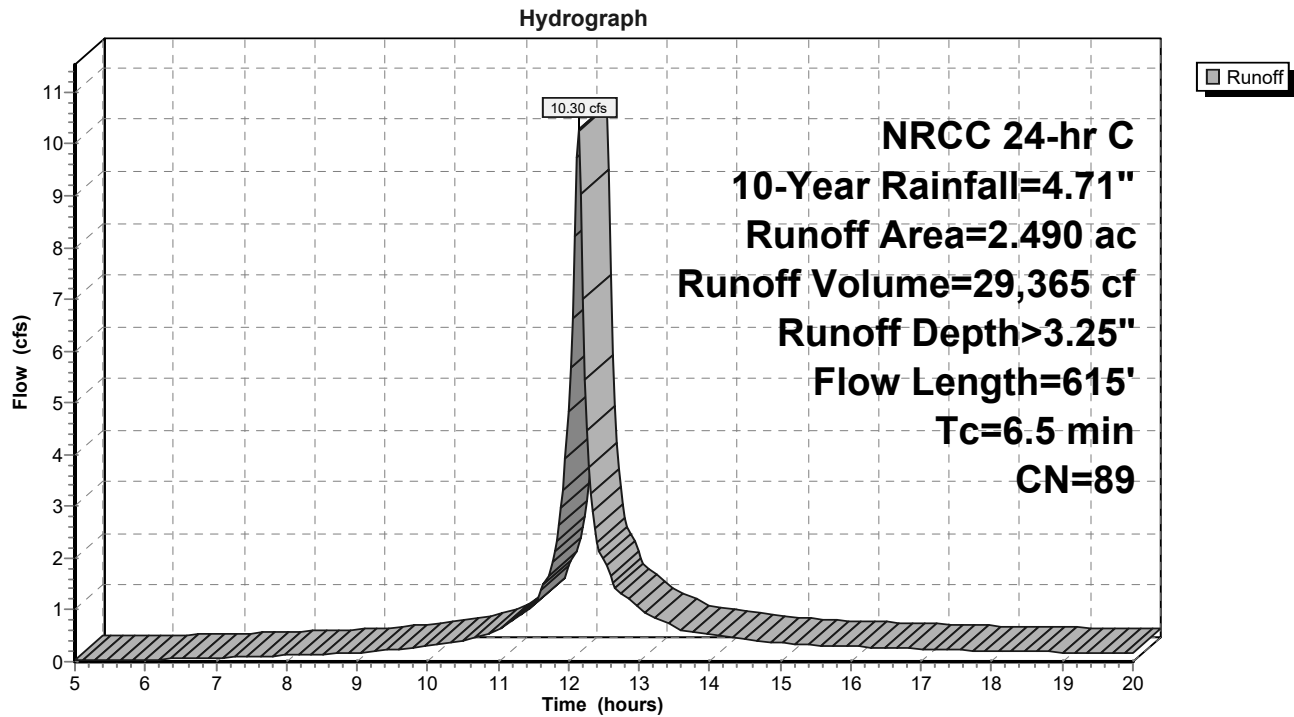
Runoff = 10.30 cfs @ 12.13 hrs, Volume= 29,365 cf, Depth> 3.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 10-Year Rainfall=4.71"

Area (ac)	CN	Description
1.000	98	Paved parking, HSG C
0.550	98	Roofs, HSG C
0.940	74	>75% Grass cover, Good, HSG C
2.490	89	Weighted Average
0.940		37.75% Pervious Area
1.550		62.25% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	30	0.0200	0.13		Sheet Flow, Grass: Short n= 0.150 P2= 3.18"
0.8	150	0.0260	3.27		Shallow Concentrated Flow, Paved Kv= 20.3 fps
0.1	45	0.0100	5.90	4.63	Pipe Channel, 12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25' n= 0.010 PVC, smooth interior
0.4	190	0.0150	7.22	5.67	Pipe Channel, 12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25' n= 0.010 PVC, smooth interior
0.1	70	0.0500	17.28	30.53	Pipe Channel, 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.010 PVC, smooth interior
1.3	130	0.0050	1.63	16.34	Channel Flow, Area= 10.0 sf Perim= 9.0' r= 1.11' n= 0.069 Riprap, 6-inch
6.5	615	Total			

Subcatchment POA1.1-POST: DA1.1



Summary for Subcatchment POA2-POST: DA2

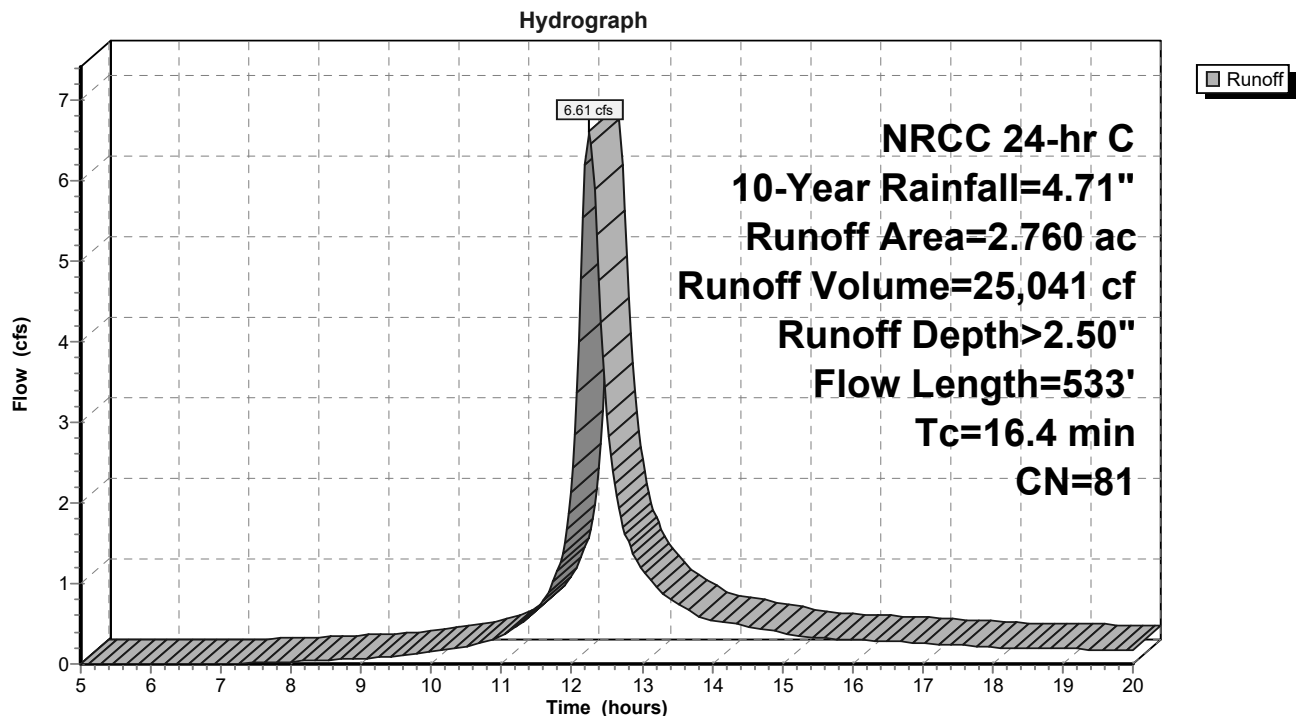
Runoff = 6.61 cfs @ 12.25 hrs, Volume= 25,041 cf, Depth> 2.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 10-Year Rainfall=4.71"

Area (ac)	CN	Description
1.995	74	>75% Grass cover, Good, HSG C
0.765	98	Paved roads w/curbs & sewers, HSG C
2.760	81	Weighted Average
1.995		72.28% Pervious Area
0.765		27.72% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.3	100	0.0200	0.12		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"
0.6	137	0.0510	3.64		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
1.5	296	0.0270	3.34		Shallow Concentrated Flow, Paved Kv= 20.3 fps
16.4	533	Total			

Subcatchment POA2-POST: DA2



Summary for Subcatchment POA2-PRE: DA2

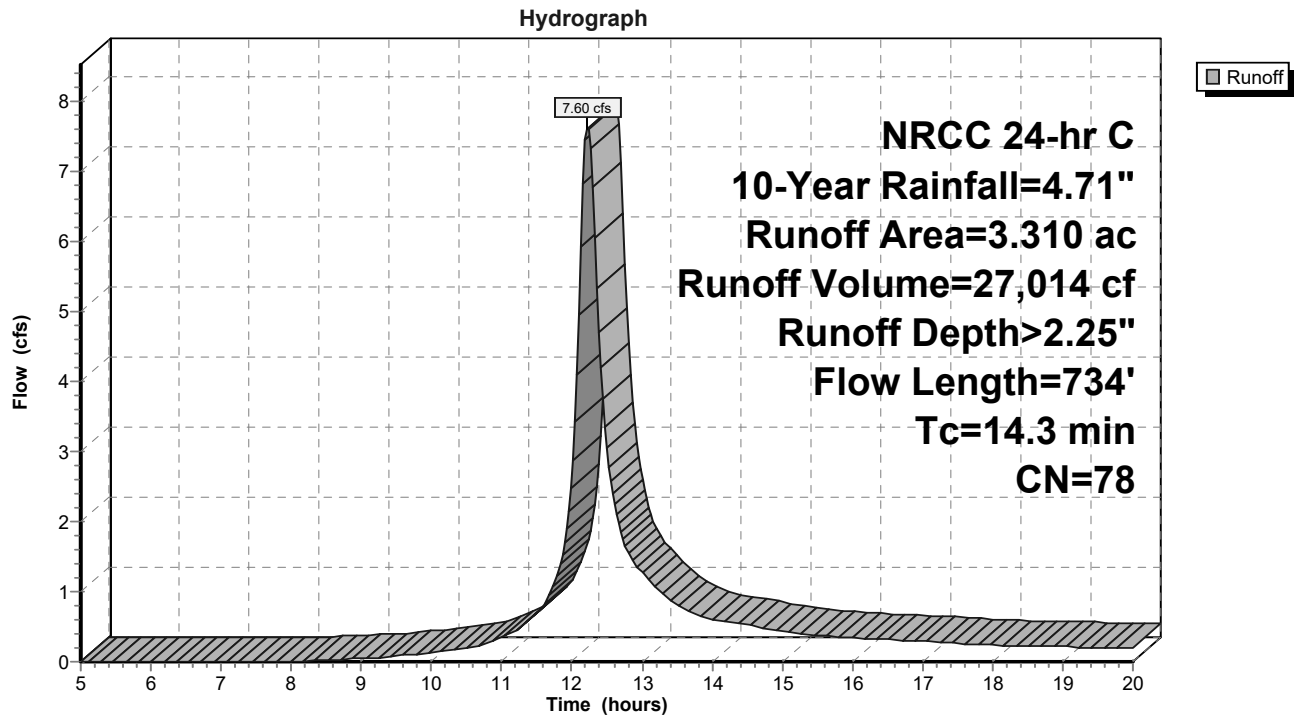
Runoff = 7.60 cfs @ 12.23 hrs, Volume= 27,014 cf, Depth> 2.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 10-Year Rainfall=4.71"

Area (ac)	CN	Description
1.150	72	Woods/grass comb., Good, HSG C
1.460	74	>75% Grass cover, Good, HSG C
0.700	98	Paved roads w/curbs & sewers, HSG C
3.310	78	Weighted Average
2.610		78.85% Pervious Area
0.700		21.15% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	100	0.1200	0.16		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.18"
1.3	123	0.1000	1.58		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.0	215	0.0510	3.64		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
1.5	296	0.0270	3.34		Shallow Concentrated Flow, Paved Kv= 20.3 fps
14.3	734	Total			

Subcatchment POA2-PRE: DA2



Summary for Subcatchment POA3-POST: DA3

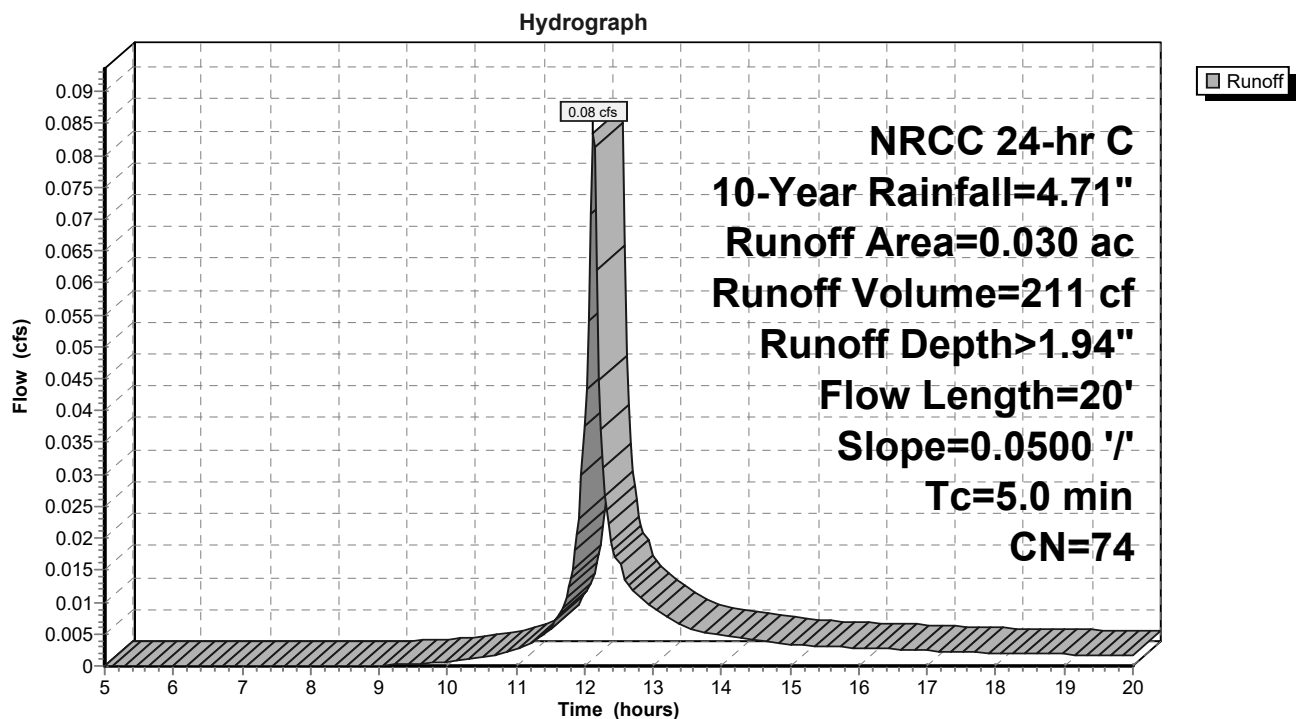
Runoff = 0.08 cfs @ 12.12 hrs, Volume= 211 cf, Depth> 1.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 10-Year Rainfall=4.71"

Area (ac)	CN	Description
0.030	74	>75% Grass cover, Good, HSG C
0.030		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.7	20	0.0500	0.12		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"
2.7	20	Total, Increased to minimum Tc = 5.0 min			

Subcatchment POA3-POST: DA3



Summary for Subcatchment POA3-PRE: DA3

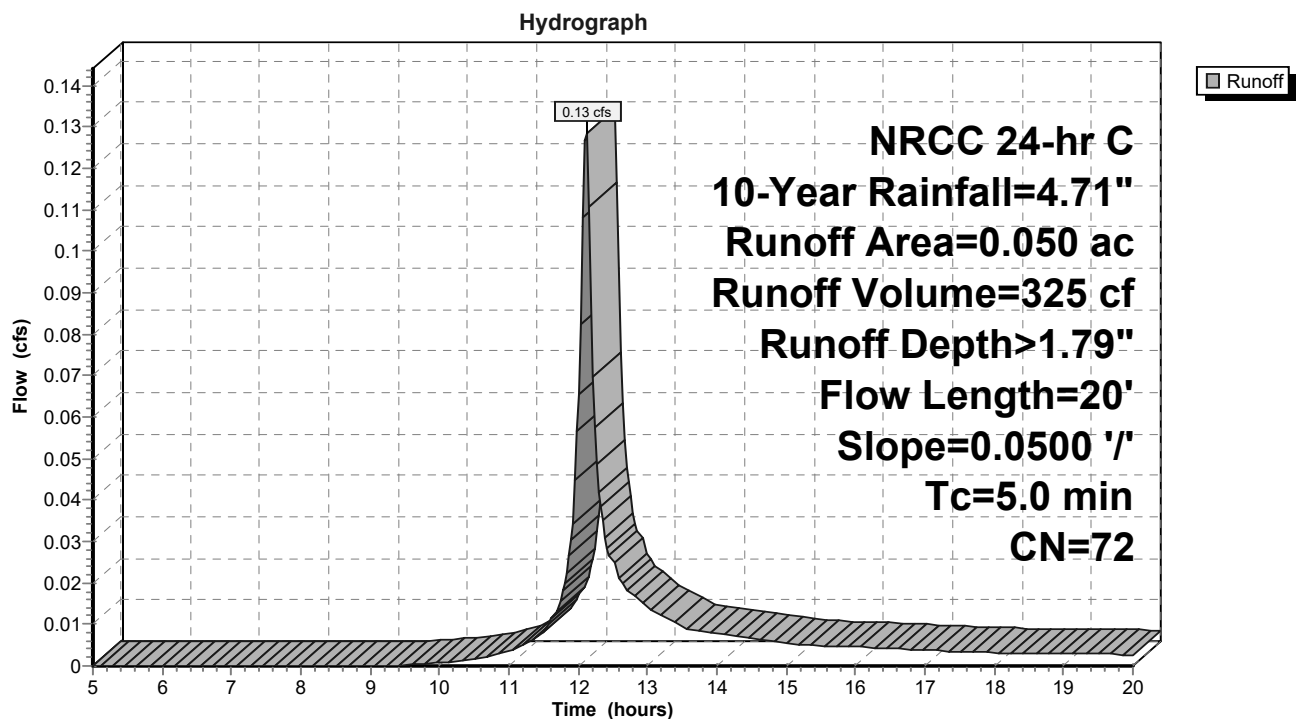
Runoff = 0.13 cfs @ 12.12 hrs, Volume= 325 cf, Depth> 1.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 10-Year Rainfall=4.71"

Area (ac)	CN	Description
0.050	72	Woods/grass comb., Good, HSG C
0.050		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.1	20	0.0500	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.18"
4.1	20	Total, Increased to minimum Tc = 5.0 min			

Subcatchment POA3-PRE: DA3



Summary for Subcatchment POA3.1-POST: DA3.1

TO CB1

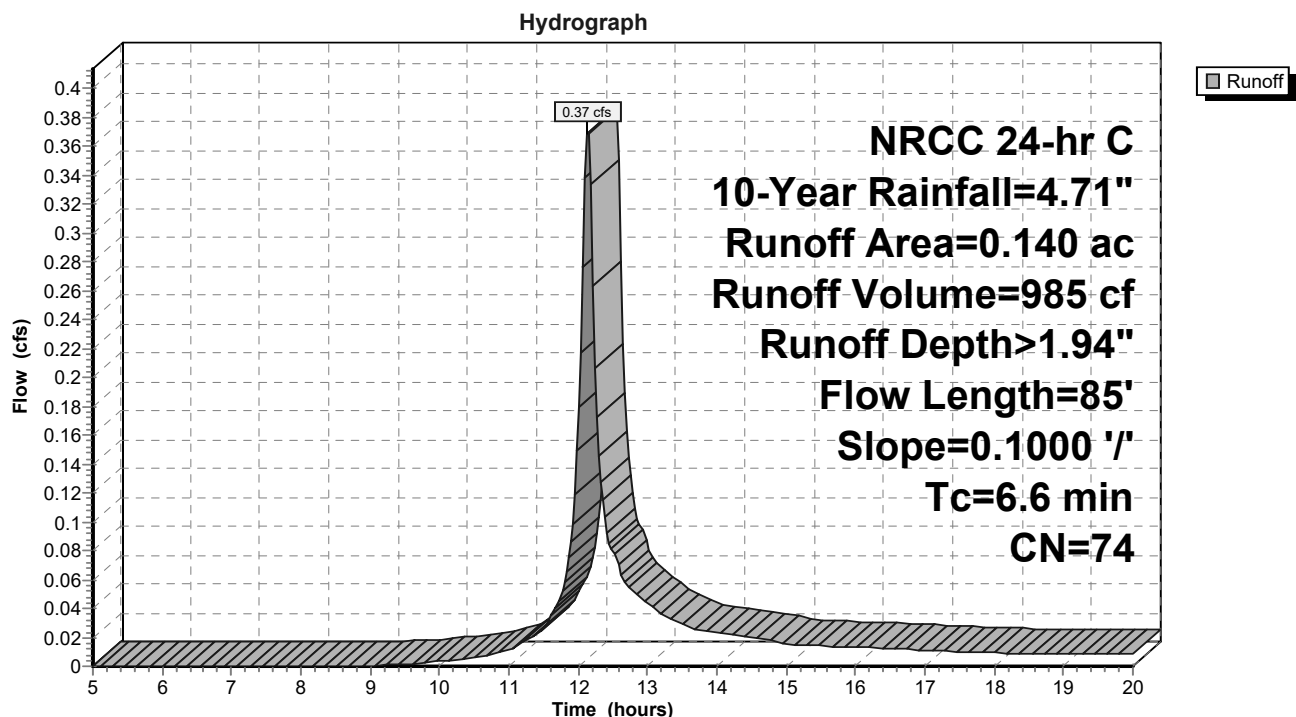
Runoff = 0.37 cfs @ 12.14 hrs, Volume= 985 cf, Depth> 1.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 10-Year Rainfall=4.71"

Area (ac)	CN	Description
0.041	74	>75% Grass cover, Good, HSG C
0.099	74	>75% Grass cover, Good, HSG C
0.140	74	Weighted Average
0.140		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.6	85	0.1000	0.21		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"

Subcatchment POA3.1-POST: DA3.1



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Summary for Subcatchment POA4-POST: DA4

Runoff = 0.78 cfs @ 12.19 hrs, Volume= 2,457 cf, Depth> 1.93"

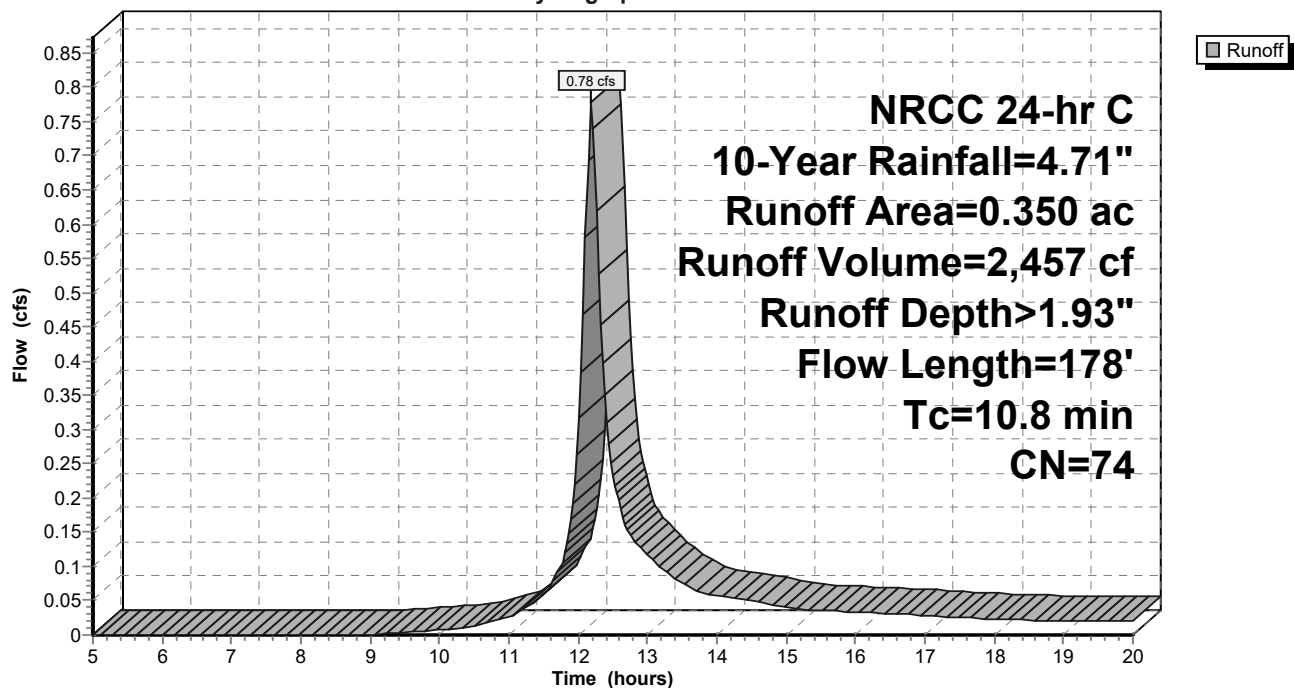
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 10-Year Rainfall=4.71"

Area (ac)	CN	Description
0.350	74	>75% Grass cover, Good, HSG C
0.350		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.3	100	0.0450	0.16		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"
0.5	78	0.0250	2.55		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
10.8	178	Total			

Subcatchment POA4-POST: DA4

Hydrograph



Summary for Subcatchment POA4-PRE: DA4

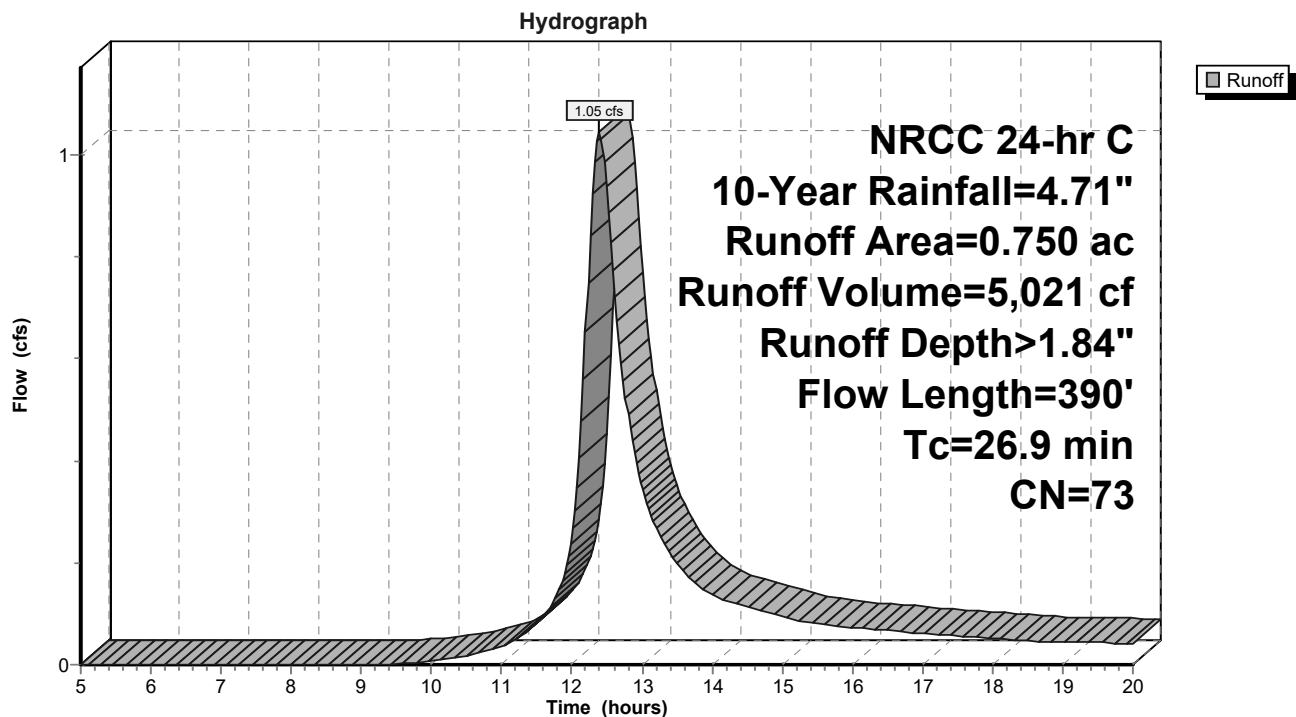
Runoff = 1.05 cfs @ 12.39 hrs, Volume= 5,021 cf, Depth> 1.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 10-Year Rainfall=4.71"

Area (ac)	CN	Description
0.450	72	Woods/grass comb., Good, HSG C
0.300	74	>75% Grass cover, Good, HSG C
0.750	73	Weighted Average
0.750		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.2	100	0.0150	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.18"
1.5	140	0.0920	1.52		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.2	150	0.0160	2.04		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
26.9	390	Total			

Subcatchment POA4-PRE: DA4



Summary for Pond SMA1: SMA1

[82] Warning: Early inflow requires earlier time span

Inflow Area = 108,464 sf, 62.25% Impervious, Inflow Depth > 3.25" for 10-Year event
 Inflow = 10.30 cfs @ 12.13 hrs, Volume= 29,365 cf
 Outflow = 0.35 cfs @ 15.07 hrs, Volume= 4,819 cf, Atten= 97%, Lag= 176.2 min
 Primary = 0.35 cfs @ 15.07 hrs, Volume= 4,819 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
 Peak Elev= 198.04' @ 15.07 hrs Surf.Area= 8,572 sf Storage= 24,657 cf

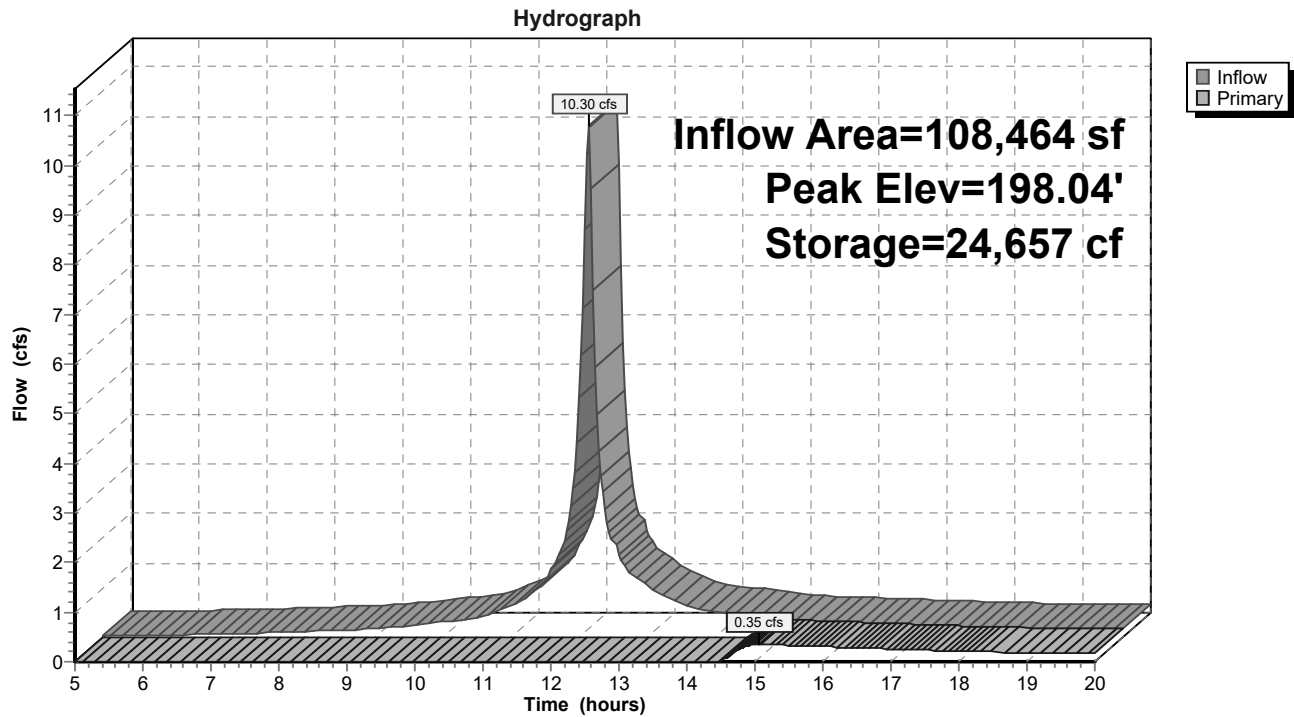
Plug-Flow detention time= 430.2 min calculated for 4,818 cf (16% of inflow)
 Center-of-Mass det. time= 253.9 min (1,019.4 - 765.5)

Volume	Invert	Avail.Storage	Storage Description	
#1	194.00'	44,331 cf	Custom Stage Data (Conic) Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
194.00	3,650	0	0	3,650
196.00	6,170	9,710	9,710	6,218
198.00	8,520	14,627	24,337	8,644
200.00	11,550	19,993	44,331	11,756

Device	Routing	Invert	Outlet Devices
#1	Primary	194.00'	8.0" Round Culvert - RISER OUTLET L= 25.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 194.00' / 193.75' S= 0.0100 '/' Cc= 0.900 n= 0.010, Flow Area= 0.35 sf
#2	Device 1	194.00'	3.0" W x 18.0" H Vert. Orifice - SIDE OF RISER C= 0.600 Limited to weir flow at low heads
#3	Device 2	198.00'	48.0" Horiz. Orifice - TOP OF RISER C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.30 cfs @ 15.07 hrs HW=198.04' (Free Discharge)
 ↑ **1=Culvert - RISER OUTLET** (Passes 0.30 cfs of 2.55 cfs potential flow)
 ↑ **2=Orifice - SIDE OF RISER** (Passes 0.30 cfs of 3.27 cfs potential flow)
 ↑ **3=Orifice - TOP OF RISER** (Weir Controls 0.30 cfs @ 0.63 fps)

Pond SMA1: SMA1

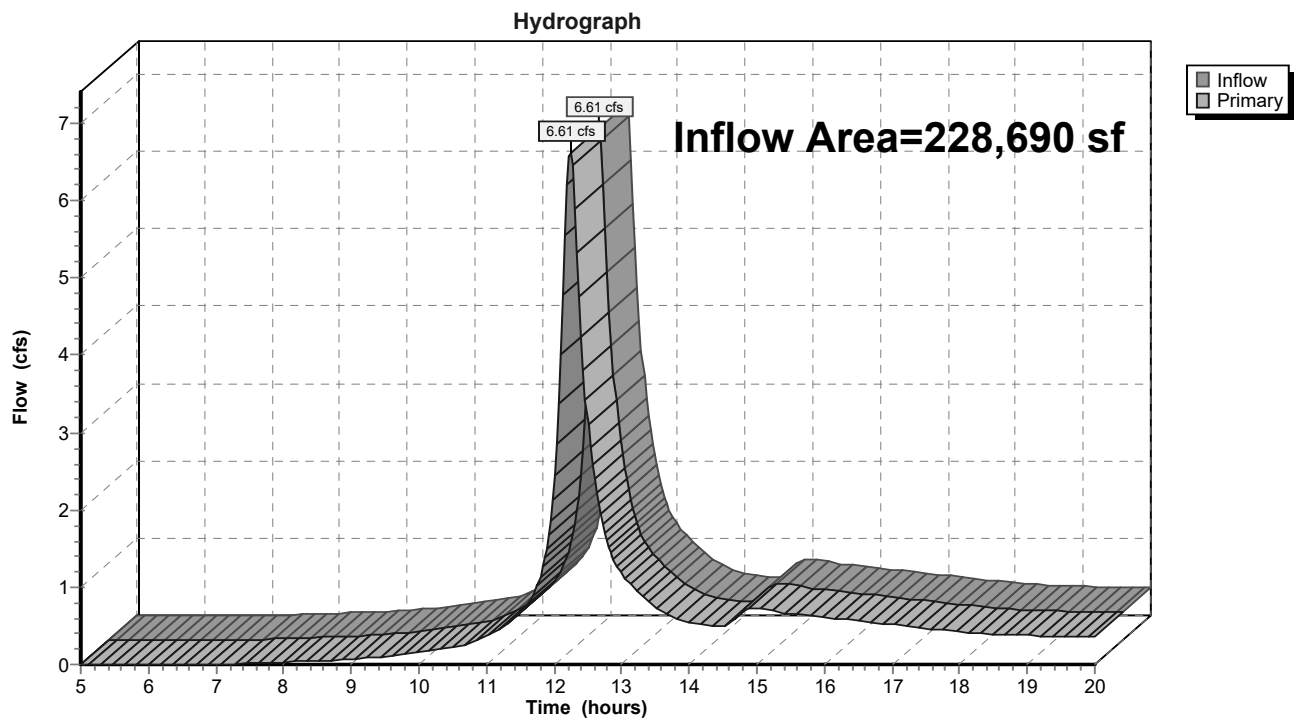


Summary for Link TOTAL POA2: TOTAL DA2

Inflow Area = 228,690 sf, 44.10% Impervious, Inflow Depth > 1.57" for 10-Year event
 Inflow = 6.61 cfs @ 12.25 hrs, Volume= 29,860 cf
 Primary = 6.61 cfs @ 12.25 hrs, Volume= 29,860 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs

Link TOTAL POA2: TOTAL DA2



Time span=5.00-20.00 hrs, dt=0.03 hrs, 501 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentPOA1-POST:DA1 Runoff Area=0.080 ac 0.00% Impervious Runoff Depth>3.72"
 Flow Length=20' Slope=0.0200 '/' Tc=5.0 min CN=74 Runoff=0.42 cfs 1,081 cf

SubcatchmentPOA1-PRE:DA1 Runoff Area=1.780 ac 0.00% Impervious Runoff Depth>3.48"
 Flow Length=557' Tc=32.9 min CN=72 Runoff=4.22 cfs 22,474 cf

SubcatchmentPOA1.1-POST:DA1.1 Runoff Area=2.490 ac 62.25% Impervious Runoff Depth>5.31"
 Flow Length=615' Tc=6.5 min CN=89 Runoff=16.35 cfs 48,031 cf

SubcatchmentPOA2-POST:DA2 Runoff Area=2.760 ac 27.72% Impervious Runoff Depth>4.45"
 Flow Length=533' Tc=16.4 min CN=81 Runoff=11.52 cfs 44,552 cf

SubcatchmentPOA2-PRE:DA2 Runoff Area=3.310 ac 21.15% Impervious Runoff Depth>4.13"
 Flow Length=734' Tc=14.3 min CN=78 Runoff=13.70 cfs 49,611 cf

SubcatchmentPOA3-POST:DA3 Runoff Area=0.030 ac 0.00% Impervious Runoff Depth>3.72"
 Flow Length=20' Slope=0.0500 '/' Tc=5.0 min CN=74 Runoff=0.16 cfs 406 cf

SubcatchmentPOA3-PRE:DA3 Runoff Area=0.050 ac 0.00% Impervious Runoff Depth>3.52"
 Flow Length=20' Slope=0.0500 '/' Tc=5.0 min CN=72 Runoff=0.25 cfs 639 cf

SubcatchmentPOA3.1-POST:DA3.1 Runoff Area=0.140 ac 0.00% Impervious Runoff Depth>3.72"
 Flow Length=85' Slope=0.1000 '/' Tc=6.6 min CN=74 Runoff=0.70 cfs 1,891 cf

SubcatchmentPOA4-POST:DA4 Runoff Area=0.350 ac 0.00% Impervious Runoff Depth>3.72"
 Flow Length=178' Tc=10.8 min CN=74 Runoff=1.48 cfs 4,720 cf

SubcatchmentPOA4-PRE:DA4 Runoff Area=0.750 ac 0.00% Impervious Runoff Depth>3.59"
 Flow Length=390' Tc=26.9 min CN=73 Runoff=2.03 cfs 9,771 cf

Pond SMA1: SMA1 Peak Elev=198.44' Storage=28,254 cf Inflow=16.35 cfs 48,031 cf
 Outflow=2.69 cfs 23,426 cf

Link TOTAL POA2: TOTAL DA2 Inflow=14.24 cfs 67,978 cf
 Primary=14.24 cfs 67,978 cf

Total Runoff Area = 511,394 sf Runoff Volume = 183,176 cf Average Runoff Depth = 4.30"
74.32% Pervious = 380,061 sf 25.68% Impervious = 131,333 sf

Summary for Subcatchment POA1-POST: DA1

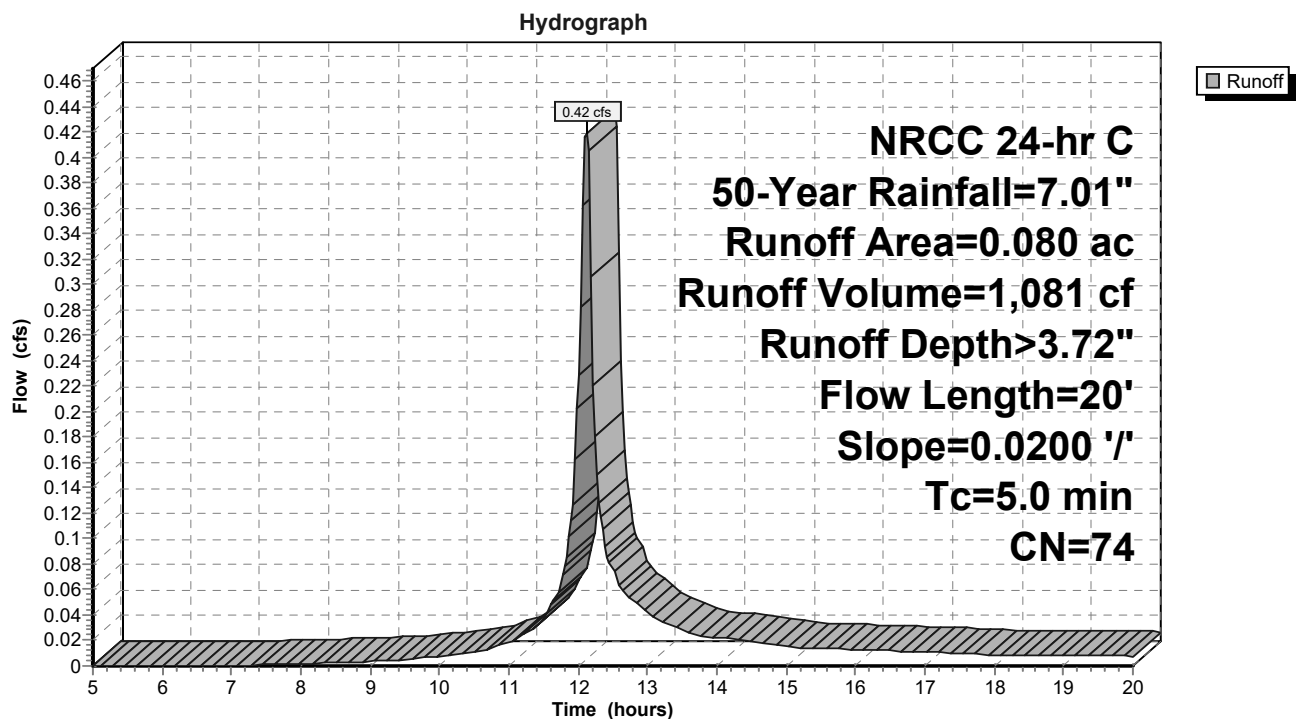
Runoff = 0.42 cfs @ 12.12 hrs, Volume= 1,081 cf, Depth> 3.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 50-Year Rainfall=7.01"

Area (ac)	CN	Description
0.080	74	>75% Grass cover, Good, HSG C
0.080		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.0	20	0.0200	0.08		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"
4.0	20	Total, Increased to minimum Tc = 5.0 min			

Subcatchment POA1-POST: DA1



Summary for Subcatchment POA1-PRE: DA1

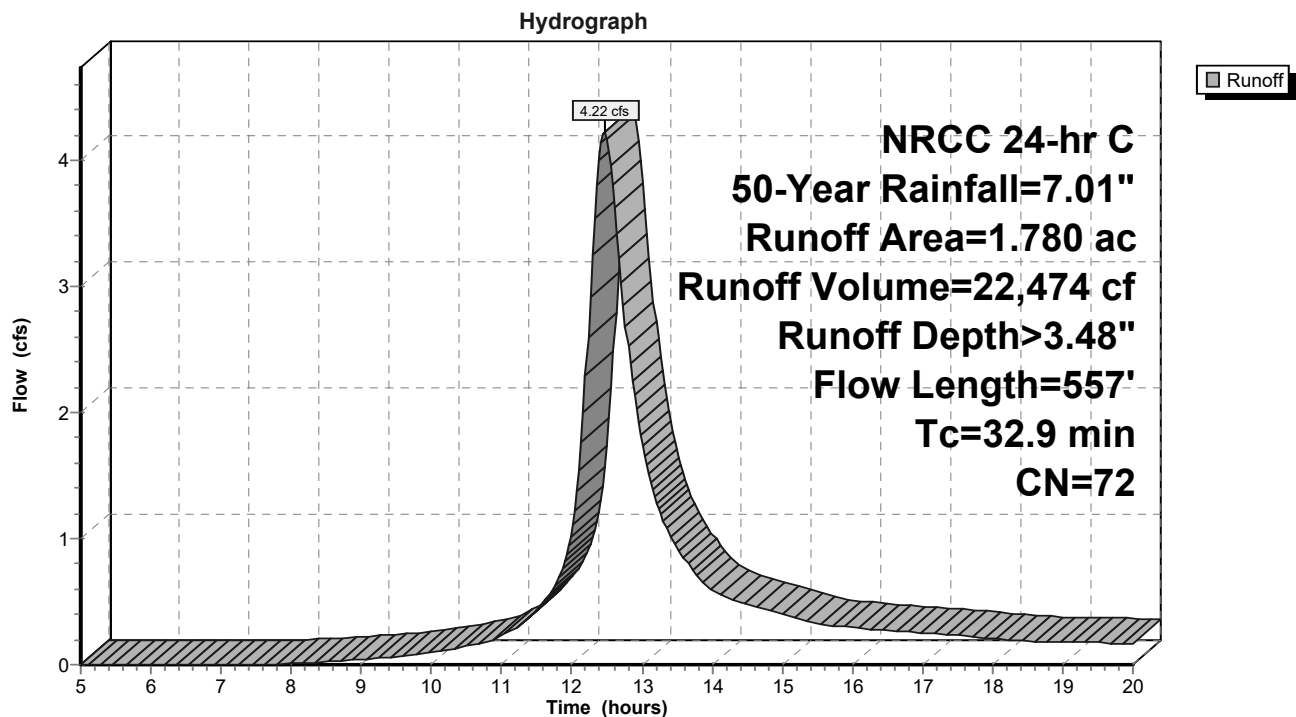
Runoff = 4.22 cfs @ 12.46 hrs, Volume= 22,474 cf, Depth> 3.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 50-Year Rainfall=7.01"

Area (ac)	CN	Description
1.640	72	Woods/grass comb., Good, HSG C
0.140	74	>75% Grass cover, Good, HSG C
1.780	72	Weighted Average
1.780		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
21.5	100	0.0200	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.18"
0.6	86	0.2100	2.29		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
10.8	371	0.0130	0.57		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
32.9	557	Total			

Subcatchment POA1-PRE: DA1



Summary for Subcatchment POA1.1-POST: DA1.1

[47] Hint: Peak is 353% of capacity of segment #3

[47] Hint: Peak is 288% of capacity of segment #4

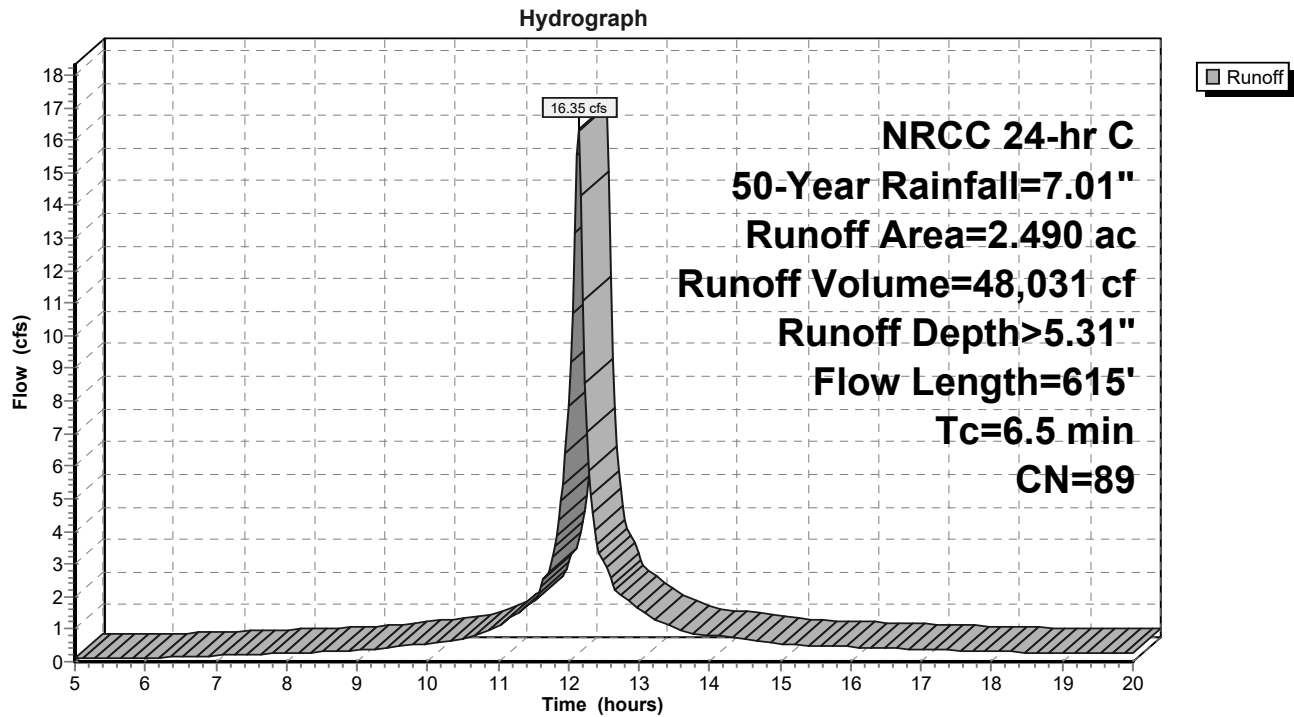
Runoff = 16.35 cfs @ 12.13 hrs, Volume= 48,031 cf, Depth> 5.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 50-Year Rainfall=7.01"

Area (ac)	CN	Description
1.000	98	Paved parking, HSG C
0.550	98	Roofs, HSG C
0.940	74	>75% Grass cover, Good, HSG C
2.490	89	Weighted Average
0.940		37.75% Pervious Area
1.550		62.25% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	30	0.0200	0.13		Sheet Flow, Grass: Short n= 0.150 P2= 3.18"
0.8	150	0.0260	3.27		Shallow Concentrated Flow, Paved Kv= 20.3 fps
0.1	45	0.0100	5.90	4.63	Pipe Channel, 12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25' n= 0.010 PVC, smooth interior
0.4	190	0.0150	7.22	5.67	Pipe Channel, 12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25' n= 0.010 PVC, smooth interior
0.1	70	0.0500	17.28	30.53	Pipe Channel, 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.010 PVC, smooth interior
1.3	130	0.0050	1.63	16.34	Channel Flow, Area= 10.0 sf Perim= 9.0' r= 1.11' n= 0.069 Riprap, 6-inch
6.5	615	Total			

Subcatchment POA1.1-POST: DA1.1



Summary for Subcatchment POA2-POST: DA2

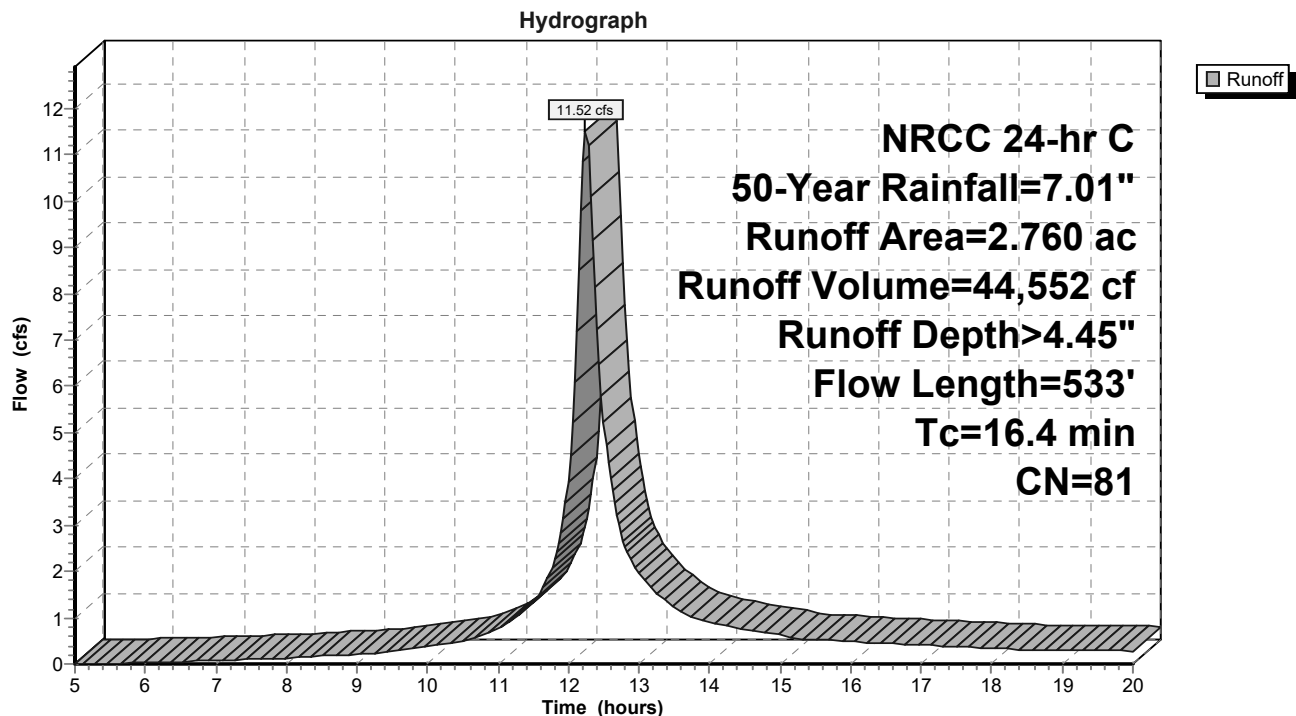
Runoff = 11.52 cfs @ 12.24 hrs, Volume= 44,552 cf, Depth> 4.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 50-Year Rainfall=7.01"

Area (ac)	CN	Description
1.995	74	>75% Grass cover, Good, HSG C
0.765	98	Paved roads w/curbs & sewers, HSG C
2.760	81	Weighted Average
1.995		72.28% Pervious Area
0.765		27.72% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.3	100	0.0200	0.12		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"
0.6	137	0.0510	3.64		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
1.5	296	0.0270	3.34		Shallow Concentrated Flow, Paved Kv= 20.3 fps
16.4	533	Total			

Subcatchment POA2-POST: DA2



Summary for Subcatchment POA2-PRE: DA2

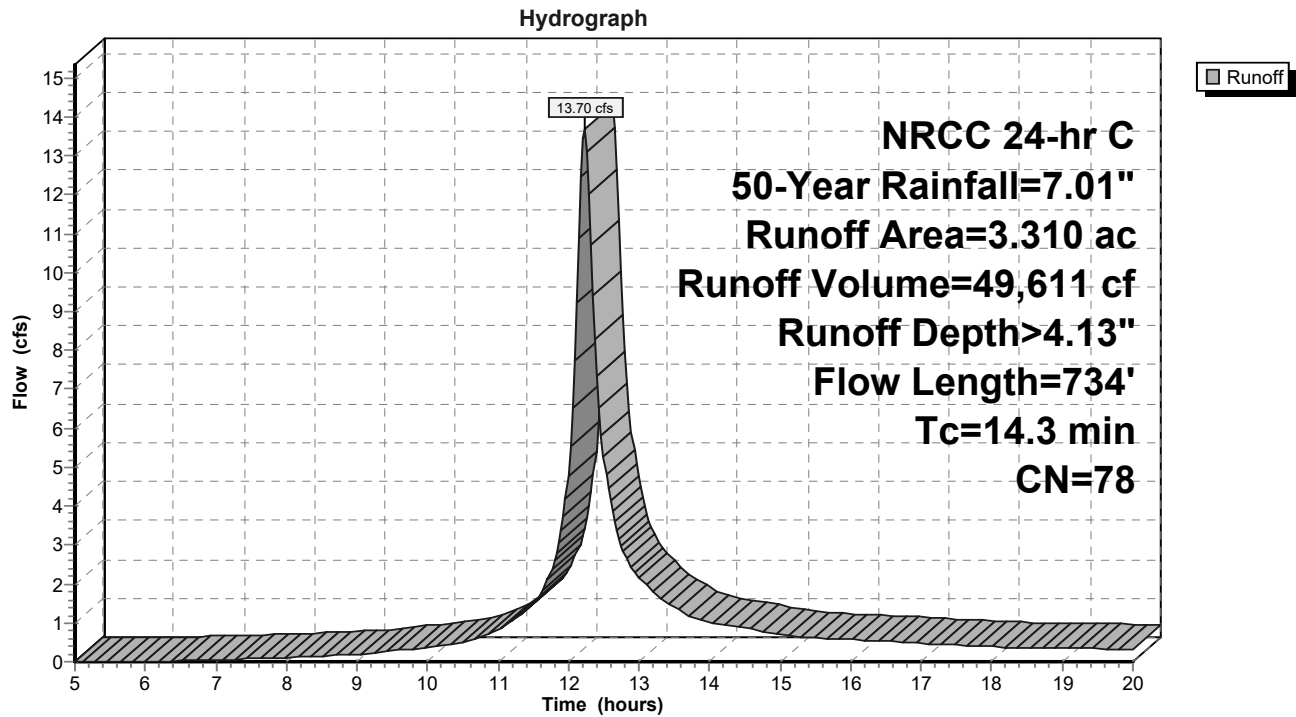
Runoff = 13.70 cfs @ 12.22 hrs, Volume= 49,611 cf, Depth> 4.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 50-Year Rainfall=7.01"

Area (ac)	CN	Description
1.150	72	Woods/grass comb., Good, HSG C
1.460	74	>75% Grass cover, Good, HSG C
0.700	98	Paved roads w/curbs & sewers, HSG C
3.310	78	Weighted Average
2.610		78.85% Pervious Area
0.700		21.15% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	100	0.1200	0.16		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.18"
1.3	123	0.1000	1.58		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.0	215	0.0510	3.64		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
1.5	296	0.0270	3.34		Shallow Concentrated Flow, Paved Kv= 20.3 fps
14.3	734	Total			

Subcatchment POA2-PRE: DA2



Summary for Subcatchment POA3-POST: DA3

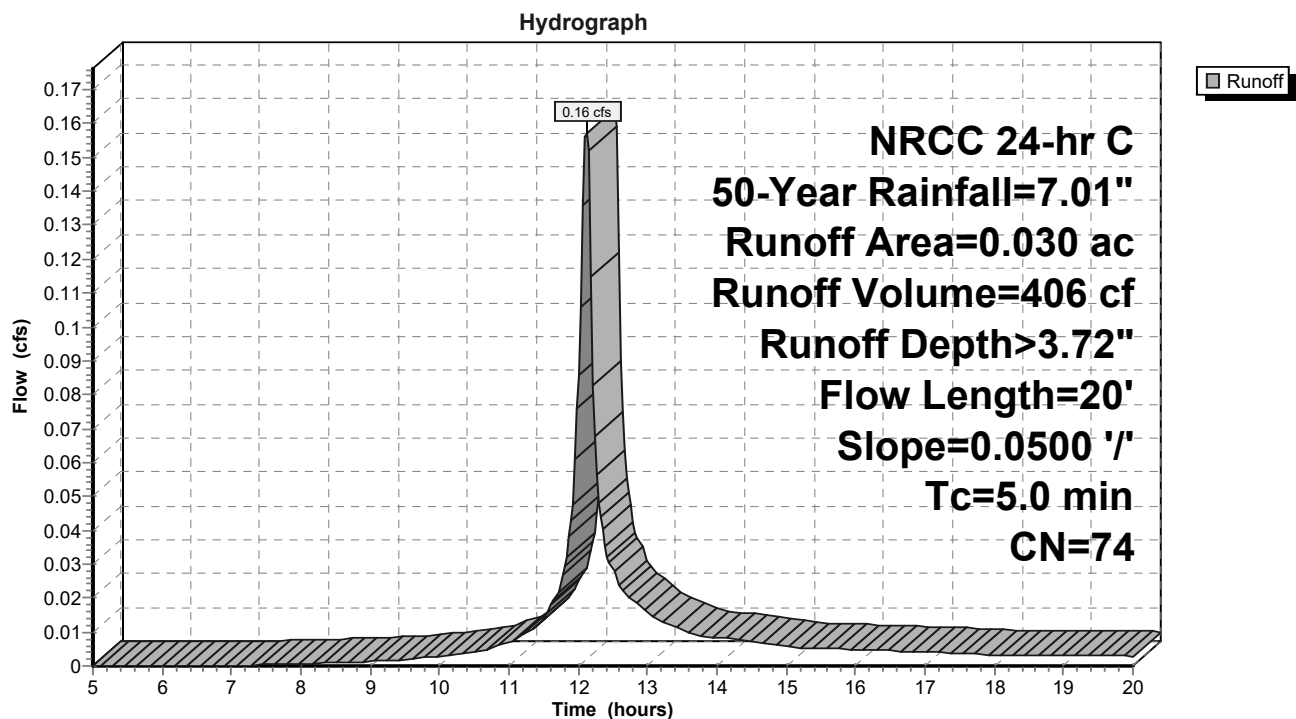
Runoff = 0.16 cfs @ 12.12 hrs, Volume= 406 cf, Depth> 3.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 50-Year Rainfall=7.01"

Area (ac)	CN	Description
0.030	74	>75% Grass cover, Good, HSG C
0.030		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.7	20	0.0500	0.12		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"
2.7	20	Total, Increased to minimum Tc = 5.0 min			

Subcatchment POA3-POST: DA3



Summary for Subcatchment POA3-PRE: DA3

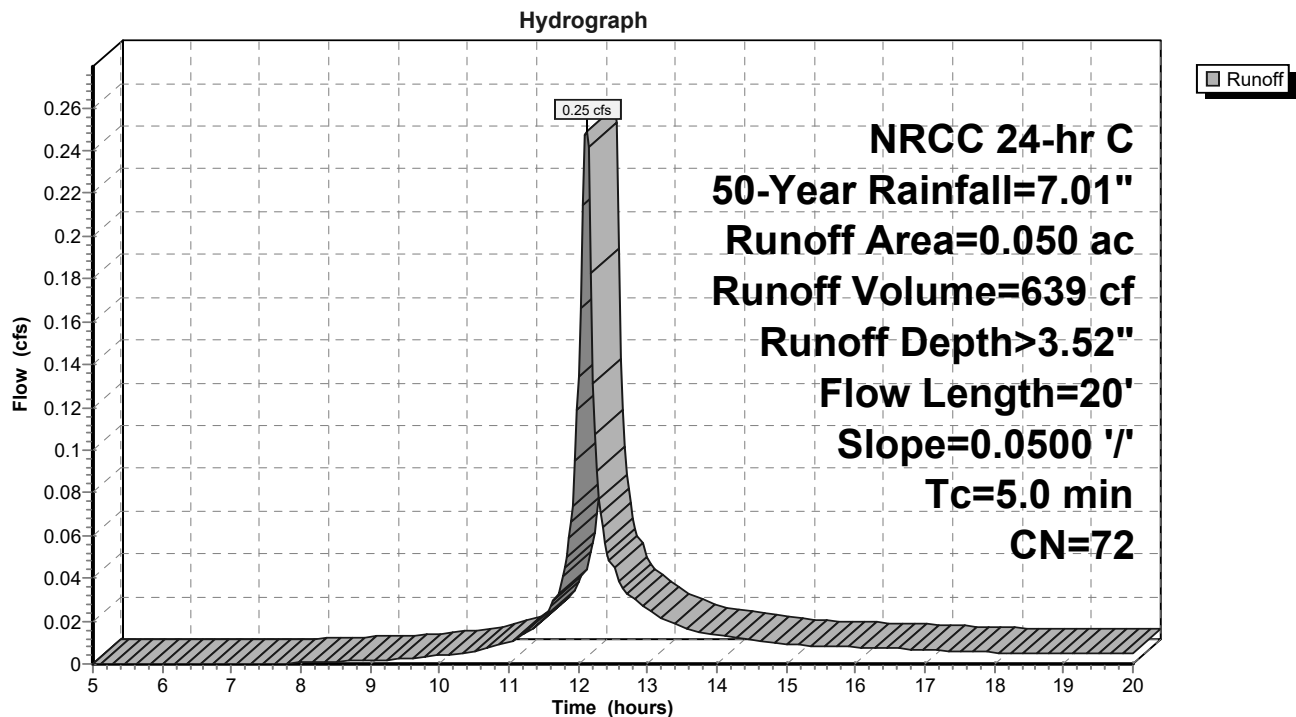
Runoff = 0.25 cfs @ 12.12 hrs, Volume= 639 cf, Depth> 3.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 50-Year Rainfall=7.01"

Area (ac)	CN	Description
0.050	72	Woods/grass comb., Good, HSG C
0.050		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.1	20	0.0500	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.18"
4.1	20	Total, Increased to minimum Tc = 5.0 min			

Subcatchment POA3-PRE: DA3



Summary for Subcatchment POA3.1-POST: DA3.1

TO CB1

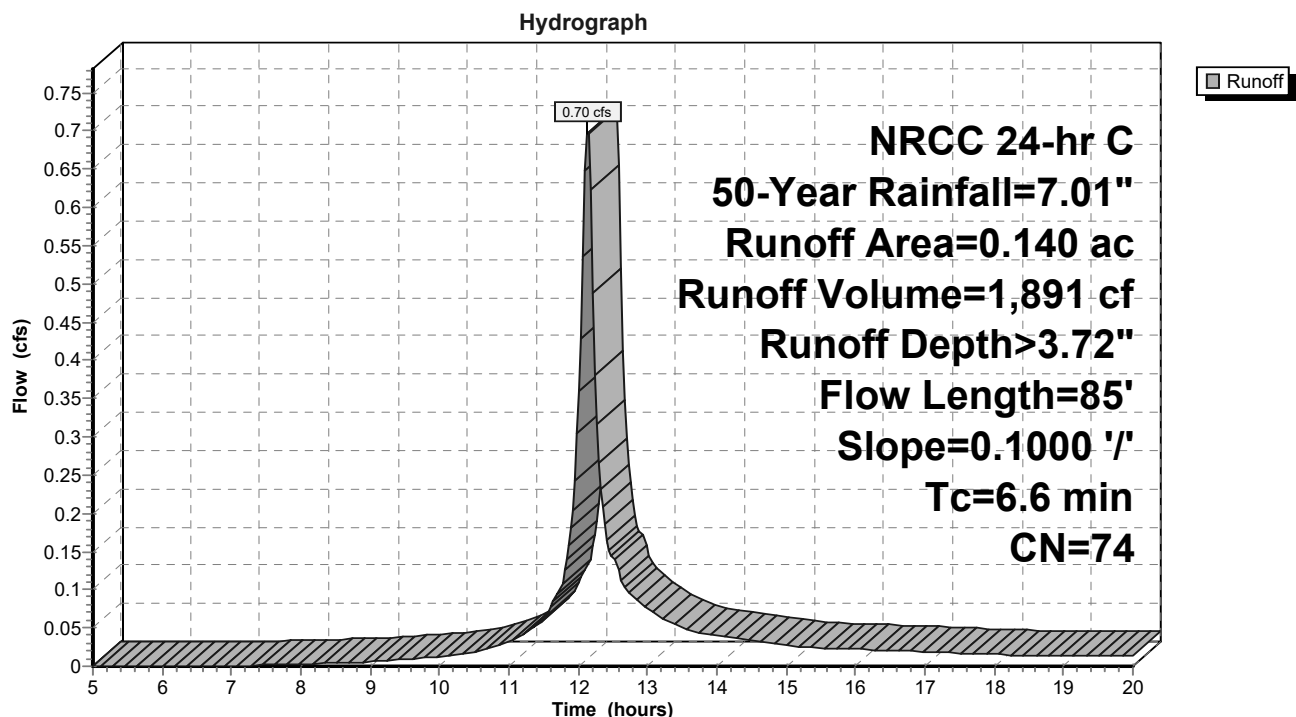
Runoff = 0.70 cfs @ 12.14 hrs, Volume= 1,891 cf, Depth> 3.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 50-Year Rainfall=7.01"

Area (ac)	CN	Description
0.041	74	>75% Grass cover, Good, HSG C
0.099	74	>75% Grass cover, Good, HSG C
0.140	74	Weighted Average
0.140		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.6	85	0.1000	0.21		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"

Subcatchment POA3.1-POST: DA3.1



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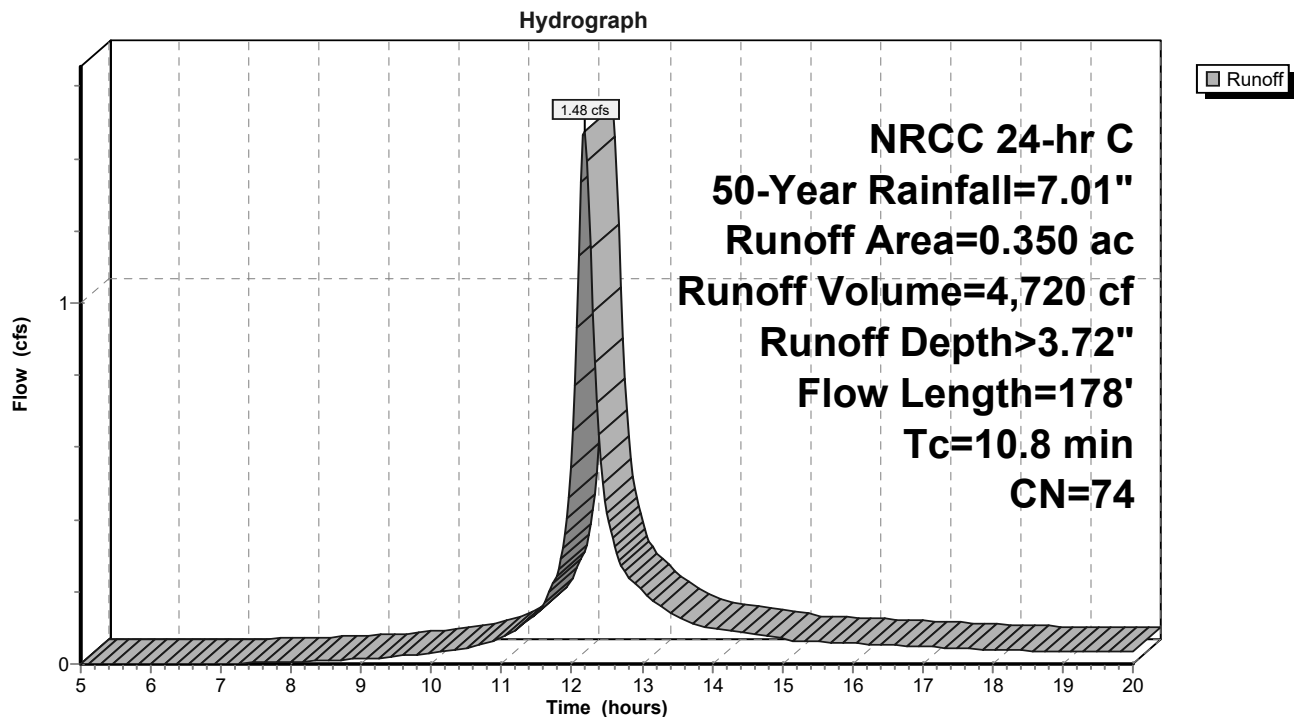
Summary for Subcatchment POA4-POST: DA4

Runoff = 1.48 cfs @ 12.18 hrs, Volume= 4,720 cf, Depth> 3.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 50-Year Rainfall=7.01"

Area (ac)	CN	Description
0.350	74	>75% Grass cover, Good, HSG C
0.350		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.3	100	0.0450	0.16		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"
0.5	78	0.0250	2.55		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
10.8	178	Total			

Subcatchment POA4-POST: DA4

Summary for Subcatchment POA4-PRE: DA4

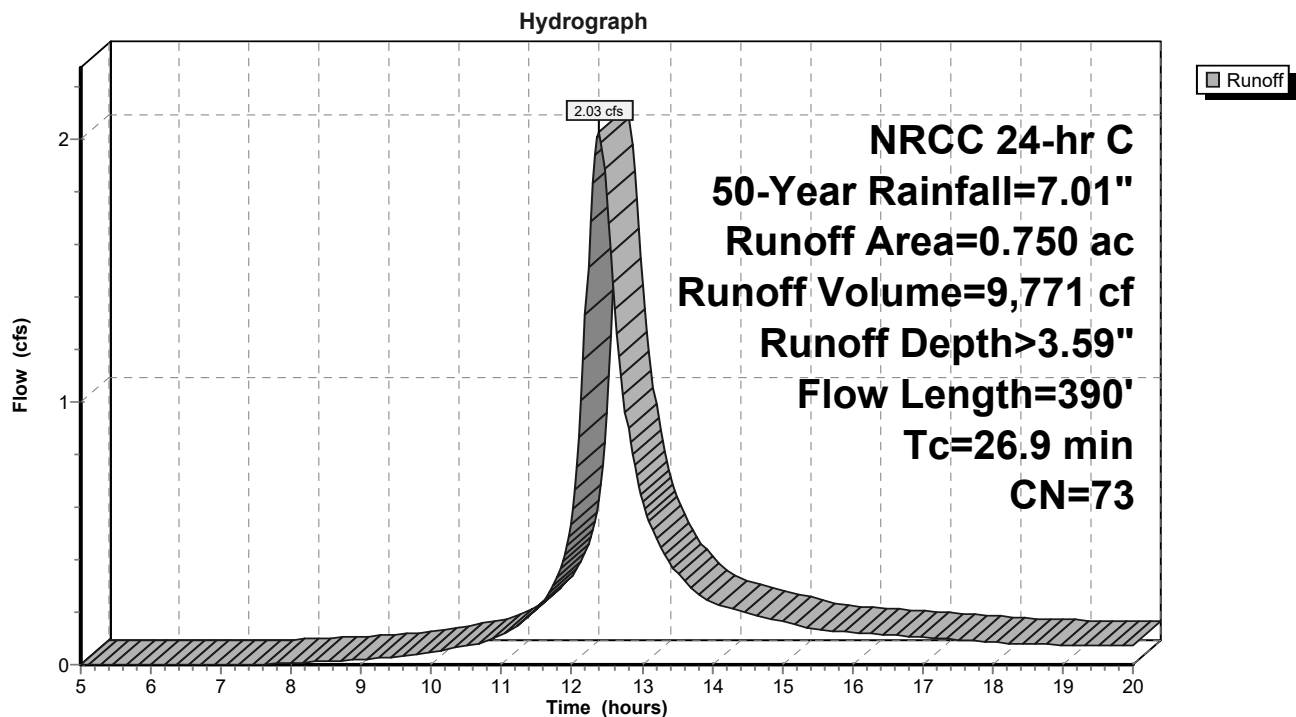
Runoff = 2.03 cfs @ 12.38 hrs, Volume= 9,771 cf, Depth> 3.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 50-Year Rainfall=7.01"

Area (ac)	CN	Description
0.450	72	Woods/grass comb., Good, HSG C
0.300	74	>75% Grass cover, Good, HSG C
0.750	73	Weighted Average
0.750		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.2	100	0.0150	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.18"
1.5	140	0.0920	1.52		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.2	150	0.0160	2.04		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
26.9	390	Total			

Subcatchment POA4-PRE: DA4



Summary for Pond SMA1: SMA1

[82] Warning: Early inflow requires earlier time span

Inflow Area = 108,464 sf, 62.25% Impervious, Inflow Depth > 5.31" for 50-Year event
 Inflow = 16.35 cfs @ 12.13 hrs, Volume= 48,031 cf
 Outflow = 2.69 cfs @ 12.58 hrs, Volume= 23,426 cf, Atten= 84%, Lag= 26.5 min
 Primary = 2.69 cfs @ 12.58 hrs, Volume= 23,426 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
 Peak Elev= 198.44' @ 12.58 hrs Surf.Area= 9,152 sf Storage= 28,254 cf

Plug-Flow detention time= 200.2 min calculated for 23,422 cf (49% of inflow)
 Center-of-Mass det. time= 106.0 min (861.0 - 754.9)

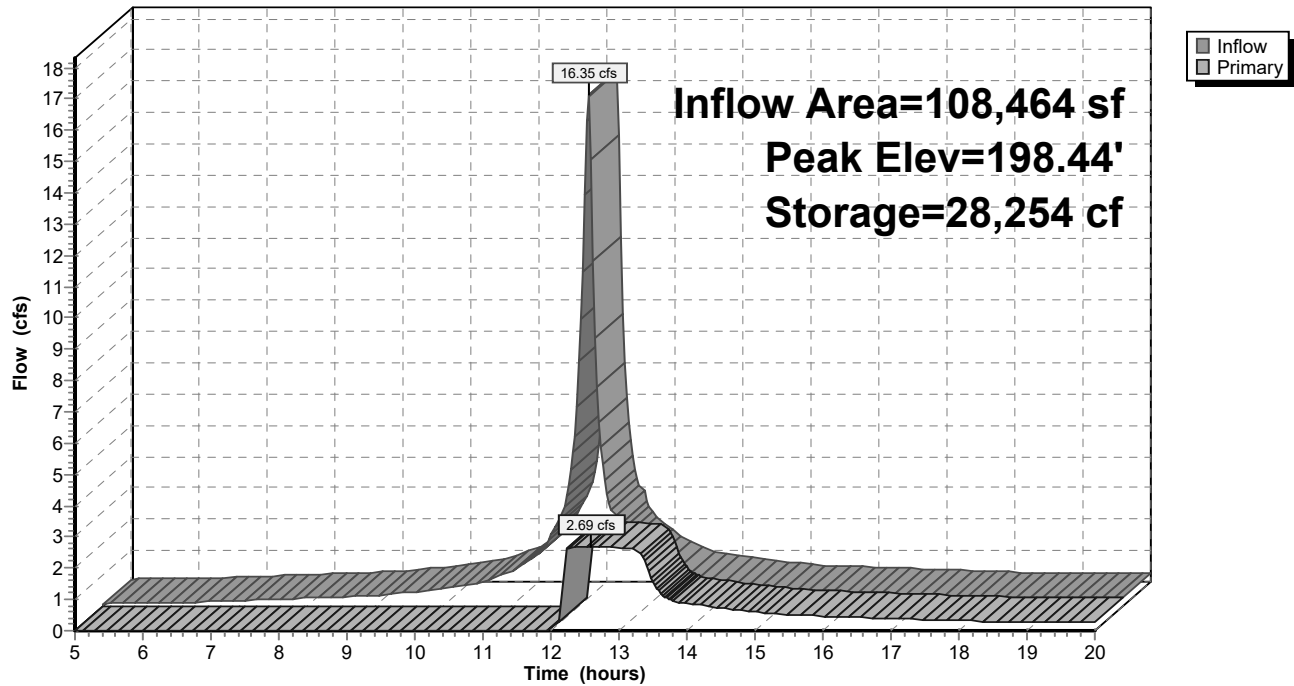
Volume	Invert	Avail.Storage	Storage Description	
#1	194.00'	44,331 cf	Custom Stage Data (Conic) Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
194.00	3,650	0	0	3,650
196.00	6,170	9,710	9,710	6,218
198.00	8,520	14,627	24,337	8,644
200.00	11,550	19,993	44,331	11,756

Device	Routing	Invert	Outlet Devices
#1	Primary	194.00'	8.0" Round Culvert - RISER OUTLET L= 25.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 194.00' / 193.75' S= 0.0100 '/' Cc= 0.900 n= 0.010, Flow Area= 0.35 sf
#2	Device 1	194.00'	3.0" W x 18.0" H Vert. Orifice - SIDE OF RISER C= 0.600 Limited to weir flow at low heads
#3	Device 2	198.00'	48.0" Horiz. Orifice - TOP OF RISER C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.69 cfs @ 12.58 hrs HW=198.44' (Free Discharge)
 1=Culvert - RISER OUTLET (Inlet Controls 2.69 cfs @ 7.71 fps)
 2=Orifice - SIDE OF RISER (Passes 2.69 cfs of 3.46 cfs potential flow)
 3=Orifice - TOP OF RISER (Passes 2.69 cfs of 12.12 cfs potential flow)

Pond SMA1: SMA1

Hydrograph

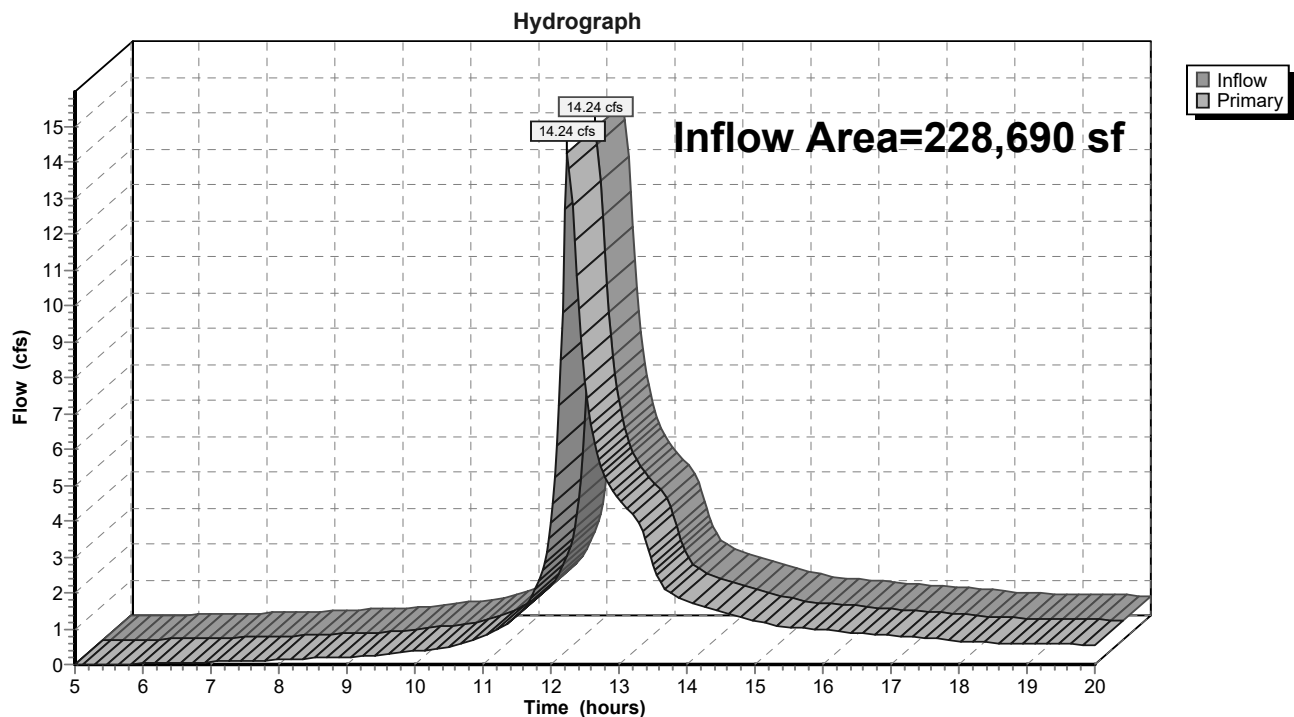


Summary for Link TOTAL POA2: TOTAL DA2

Inflow Area = 228,690 sf, 44.10% Impervious, Inflow Depth > 3.57" for 50-Year event
 Inflow = 14.24 cfs @ 12.24 hrs, Volume= 67,978 cf
 Primary = 14.24 cfs @ 12.24 hrs, Volume= 67,978 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs

Link TOTAL POA2: TOTAL DA2



Time span=5.00-20.00 hrs, dt=0.03 hrs, 501 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentPOA1-POST:DA1 Runoff Area=0.080 ac 0.00% Impervious Runoff Depth>4.81"
 Flow Length=20' Slope=0.0200 '/' Tc=5.0 min CN=74 Runoff=0.54 cfs 1,397 cf

SubcatchmentPOA1-PRE:DA1 Runoff Area=1.780 ac 0.00% Impervious Runoff Depth>4.53"
 Flow Length=557' Tc=32.9 min CN=72 Runoff=5.47 cfs 29,291 cf

SubcatchmentPOA1.1-POST:DA1.1 Runoff Area=2.490 ac 62.25% Impervious Runoff Depth>6.50"
 Flow Length=615' Tc=6.5 min CN=89 Runoff=19.77 cfs 58,722 cf

SubcatchmentPOA2-POST:DA2 Runoff Area=2.760 ac 27.72% Impervious Runoff Depth>5.60"
 Flow Length=533' Tc=16.4 min CN=81 Runoff=14.33 cfs 56,089 cf

SubcatchmentPOA2-PRE:DA2 Runoff Area=3.310 ac 21.15% Impervious Runoff Depth>5.26"
 Flow Length=734' Tc=14.3 min CN=78 Runoff=17.24 cfs 63,152 cf

SubcatchmentPOA3-POST:DA3 Runoff Area=0.030 ac 0.00% Impervious Runoff Depth>4.81"
 Flow Length=20' Slope=0.0500 '/' Tc=5.0 min CN=74 Runoff=0.20 cfs 524 cf

SubcatchmentPOA3-PRE:DA3 Runoff Area=0.050 ac 0.00% Impervious Runoff Depth>4.58"
 Flow Length=20' Slope=0.0500 '/' Tc=5.0 min CN=72 Runoff=0.32 cfs 832 cf

SubcatchmentPOA3.1-POST:DA3.1 Runoff Area=0.140 ac 0.00% Impervious Runoff Depth>4.81"
 Flow Length=85' Slope=0.1000 '/' Tc=6.6 min CN=74 Runoff=0.89 cfs 2,444 cf

SubcatchmentPOA4-POST:DA4 Runoff Area=0.350 ac 0.00% Impervious Runoff Depth>4.80"
 Flow Length=178' Tc=10.8 min CN=74 Runoff=1.89 cfs 6,100 cf

SubcatchmentPOA4-PRE:DA4 Runoff Area=0.750 ac 0.00% Impervious Runoff Depth>4.66"
 Flow Length=390' Tc=26.9 min CN=73 Runoff=2.62 cfs 12,681 cf

Pond SMA1: SMA1 Peak Elev=199.10' Storage=34,574 cf Inflow=19.77 cfs 58,722 cf
 Outflow=2.90 cfs 34,084 cf

Link TOTAL POA2: TOTAL DA2 Inflow=17.15 cfs 90,173 cf
 Primary=17.15 cfs 90,173 cf

Total Runoff Area = 511,394 sf Runoff Volume = 231,231 cf Average Runoff Depth = 5.43"
74.32% Pervious = 380,061 sf 25.68% Impervious = 131,333 sf

Summary for Subcatchment POA1-POST: DA1

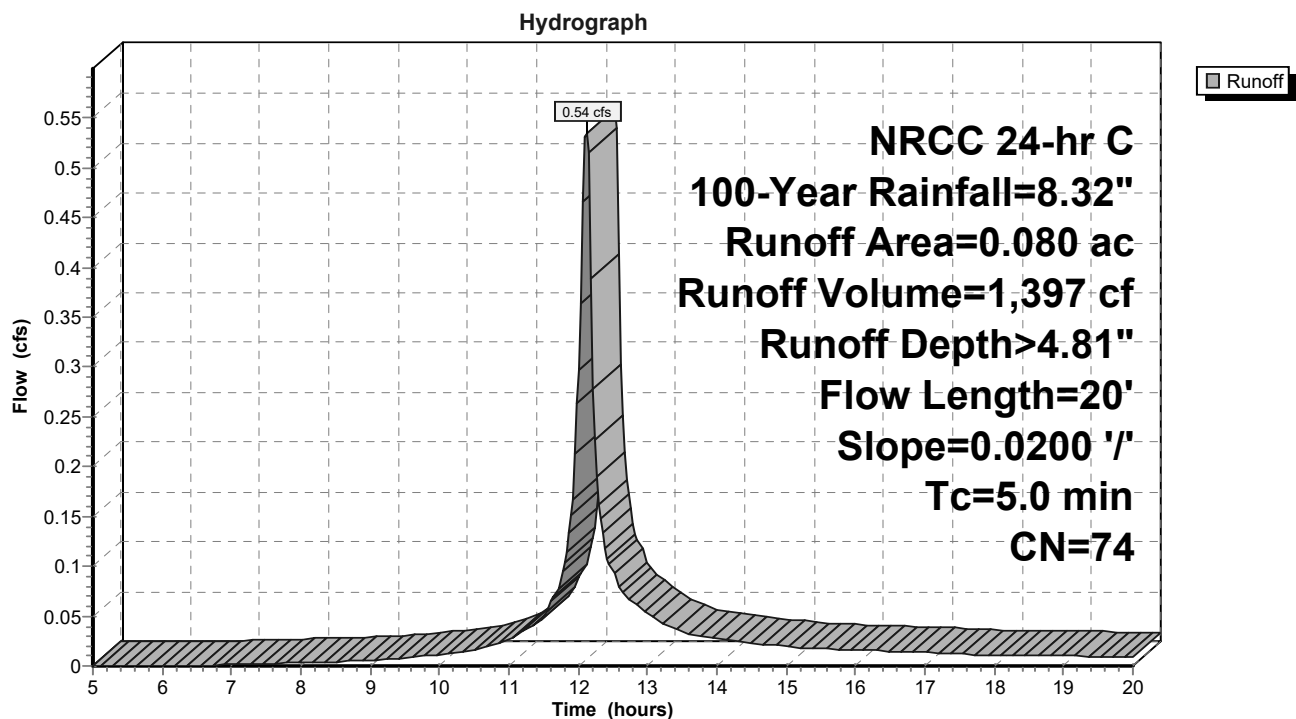
Runoff = 0.54 cfs @ 12.12 hrs, Volume= 1,397 cf, Depth> 4.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 100-Year Rainfall=8.32"

Area (ac)	CN	Description
0.080	74	>75% Grass cover, Good, HSG C
0.080		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.0	20	0.0200	0.08		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"
4.0	20	Total, Increased to minimum Tc = 5.0 min			

Subcatchment POA1-POST: DA1



Summary for Subcatchment POA1-PRE: DA1

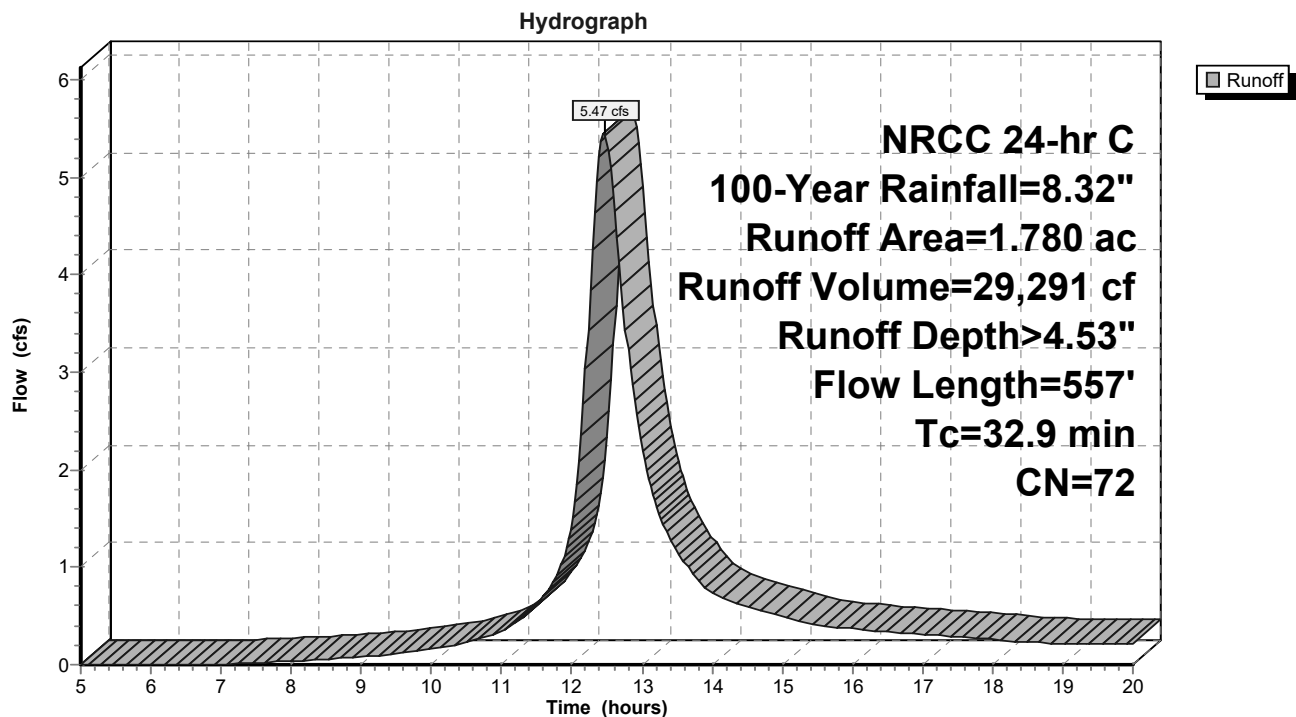
Runoff = 5.47 cfs @ 12.46 hrs, Volume= 29,291 cf, Depth> 4.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 100-Year Rainfall=8.32"

Area (ac)	CN	Description
1.640	72	Woods/grass comb., Good, HSG C
0.140	74	>75% Grass cover, Good, HSG C
1.780	72	Weighted Average
1.780		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
21.5	100	0.0200	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.18"
0.6	86	0.2100	2.29		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
10.8	371	0.0130	0.57		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
32.9	557	Total			

Subcatchment POA1-PRE: DA1



Summary for Subcatchment POA1.1-POST: DA1.1

[47] Hint: Peak is 427% of capacity of segment #3
 [47] Hint: Peak is 348% of capacity of segment #4
 [47] Hint: Peak is 121% of capacity of segment #6

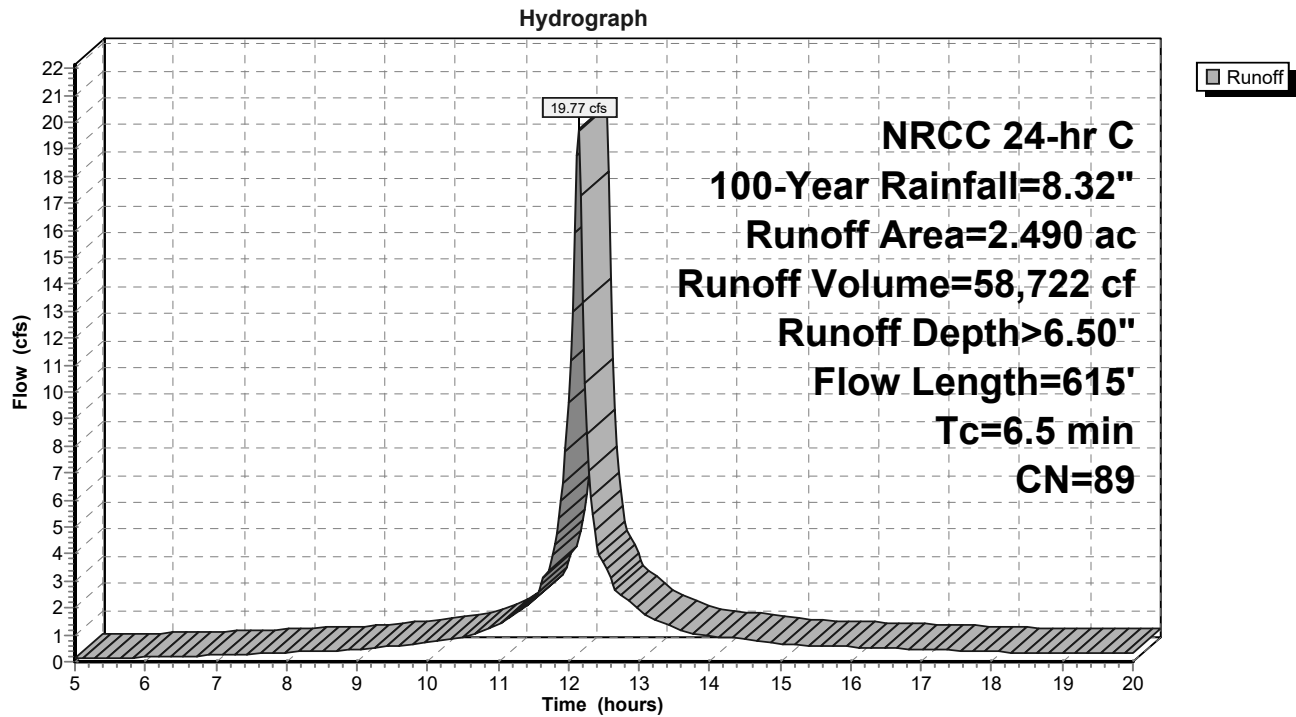
Runoff = 19.77 cfs @ 12.13 hrs, Volume= 58,722 cf, Depth> 6.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
 NRCC 24-hr C 100-Year Rainfall=8.32"

Area (ac)	CN	Description
1.000	98	Paved parking, HSG C
0.550	98	Roofs, HSG C
0.940	74	>75% Grass cover, Good, HSG C
2.490	89	Weighted Average
0.940		37.75% Pervious Area
1.550		62.25% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	30	0.0200	0.13		Sheet Flow, Grass: Short n= 0.150 P2= 3.18"
0.8	150	0.0260	3.27		Shallow Concentrated Flow, Paved Kv= 20.3 fps
0.1	45	0.0100	5.90	4.63	Pipe Channel, 12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25' n= 0.010 PVC, smooth interior
0.4	190	0.0150	7.22	5.67	Pipe Channel, 12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25' n= 0.010 PVC, smooth interior
0.1	70	0.0500	17.28	30.53	Pipe Channel, 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.010 PVC, smooth interior
1.3	130	0.0050	1.63	16.34	Channel Flow, Area= 10.0 sf Perim= 9.0' r= 1.11' n= 0.069 Riprap, 6-inch
6.5	615	Total			

Subcatchment POA1.1-POST: DA1.1



HAMILTON ST_POSTvsPRE_10.03.23_OUTLET CONRCC 24-hr C 100-Year Rainfall=8.32"

Prepared by ARICO ASSOCIATES

Printed 9/25/2023

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Summary for Subcatchment POA2-POST: DA2

Runoff = 14.33 cfs @ 12.24 hrs, Volume= 56,089 cf, Depth> 5.60"

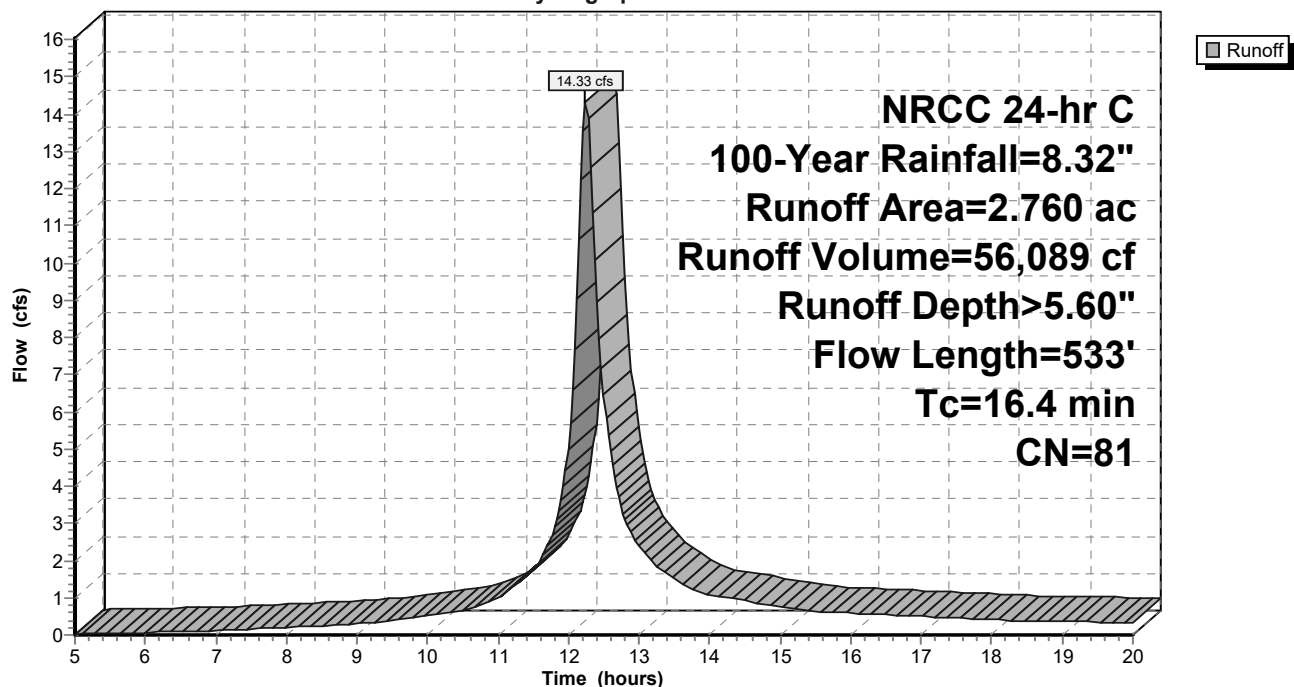
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 100-Year Rainfall=8.32"

Area (ac)	CN	Description
1.995	74	>75% Grass cover, Good, HSG C
0.765	98	Paved roads w/curbs & sewers, HSG C
2.760	81	Weighted Average
1.995		72.28% Pervious Area
0.765		27.72% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.3	100	0.0200	0.12		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"
0.6	137	0.0510	3.64		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
1.5	296	0.0270	3.34		Shallow Concentrated Flow, Paved Kv= 20.3 fps
16.4	533	Total			

Subcatchment POA2-POST: DA2

Hydrograph



Summary for Subcatchment POA2-PRE: DA2

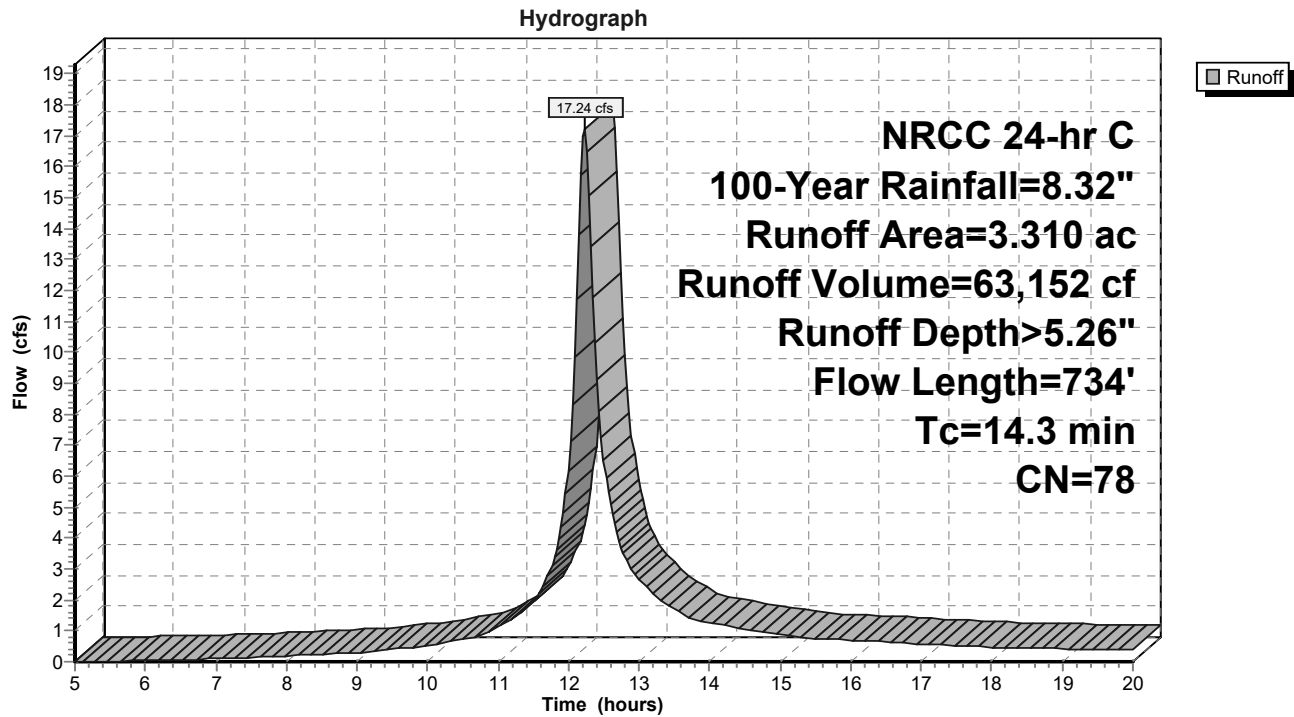
Runoff = 17.24 cfs @ 12.22 hrs, Volume= 63,152 cf, Depth> 5.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
 NRCC 24-hr C 100-Year Rainfall=8.32"

Area (ac)	CN	Description
1.150	72	Woods/grass comb., Good, HSG C
1.460	74	>75% Grass cover, Good, HSG C
0.700	98	Paved roads w/curbs & sewers, HSG C
3.310	78	Weighted Average
2.610		78.85% Pervious Area
0.700		21.15% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	100	0.1200	0.16		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.18"
1.3	123	0.1000	1.58		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.0	215	0.0510	3.64		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
1.5	296	0.0270	3.34		Shallow Concentrated Flow, Paved Kv= 20.3 fps
14.3	734	Total			

Subcatchment POA2-PRE: DA2



Summary for Subcatchment POA3-POST: DA3

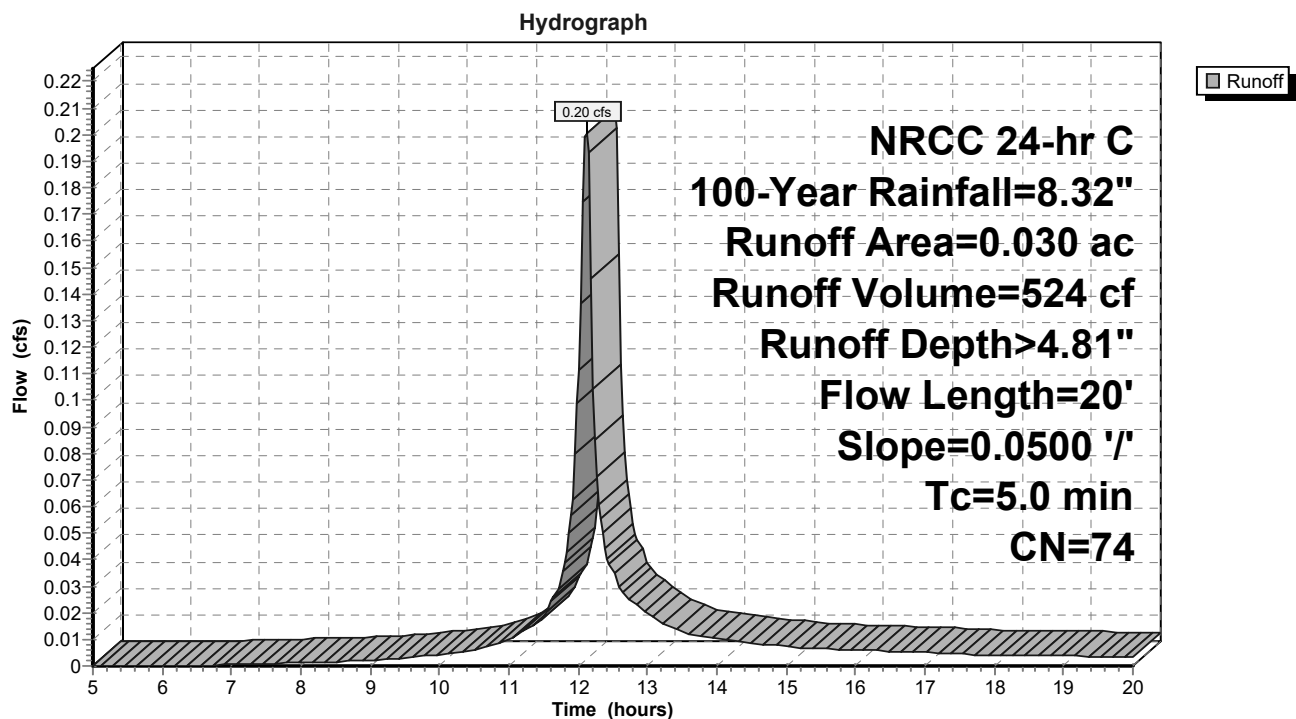
Runoff = 0.20 cfs @ 12.12 hrs, Volume= 524 cf, Depth> 4.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
 NRCC 24-hr C 100-Year Rainfall=8.32"

Area (ac)	CN	Description
0.030	74	>75% Grass cover, Good, HSG C
0.030		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.7	20	0.0500	0.12		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"
2.7	20	Total, Increased to minimum Tc = 5.0 min			

Subcatchment POA3-POST: DA3



HAMILTON ST_POSTvsPRE_10.03.23_OUTLET CONRCC 24-hr C 100-Year Rainfall=8.32"

Prepared by ARICO ASSOCIATES

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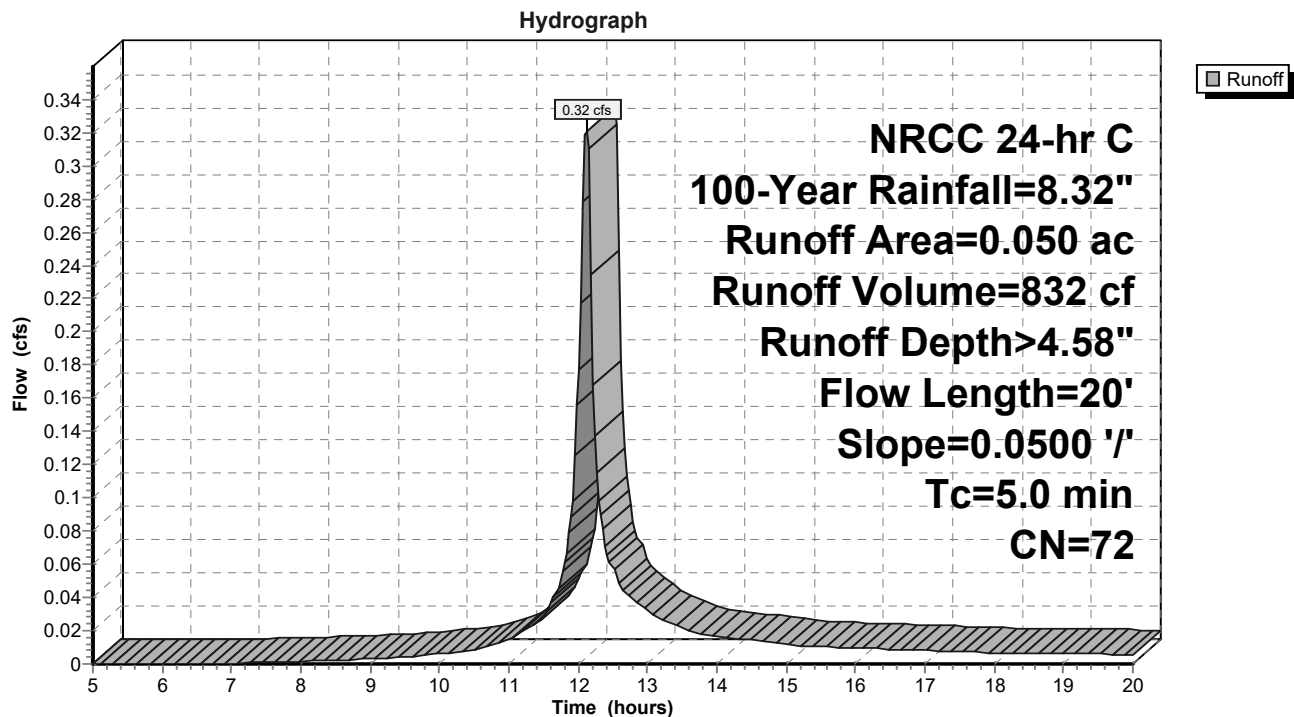
Summary for Subcatchment POA3-PRE: DA3

Runoff = 0.32 cfs @ 12.12 hrs, Volume= 832 cf, Depth> 4.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 100-Year Rainfall=8.32"

Area (ac)	CN	Description
0.050	72	Woods/grass comb., Good, HSG C
0.050		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.1	20	0.0500	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.18"
4.1	20	Total, Increased to minimum Tc = 5.0 min			

Subcatchment POA3-PRE: DA3

Summary for Subcatchment POA3.1-POST: DA3.1

TO CB1

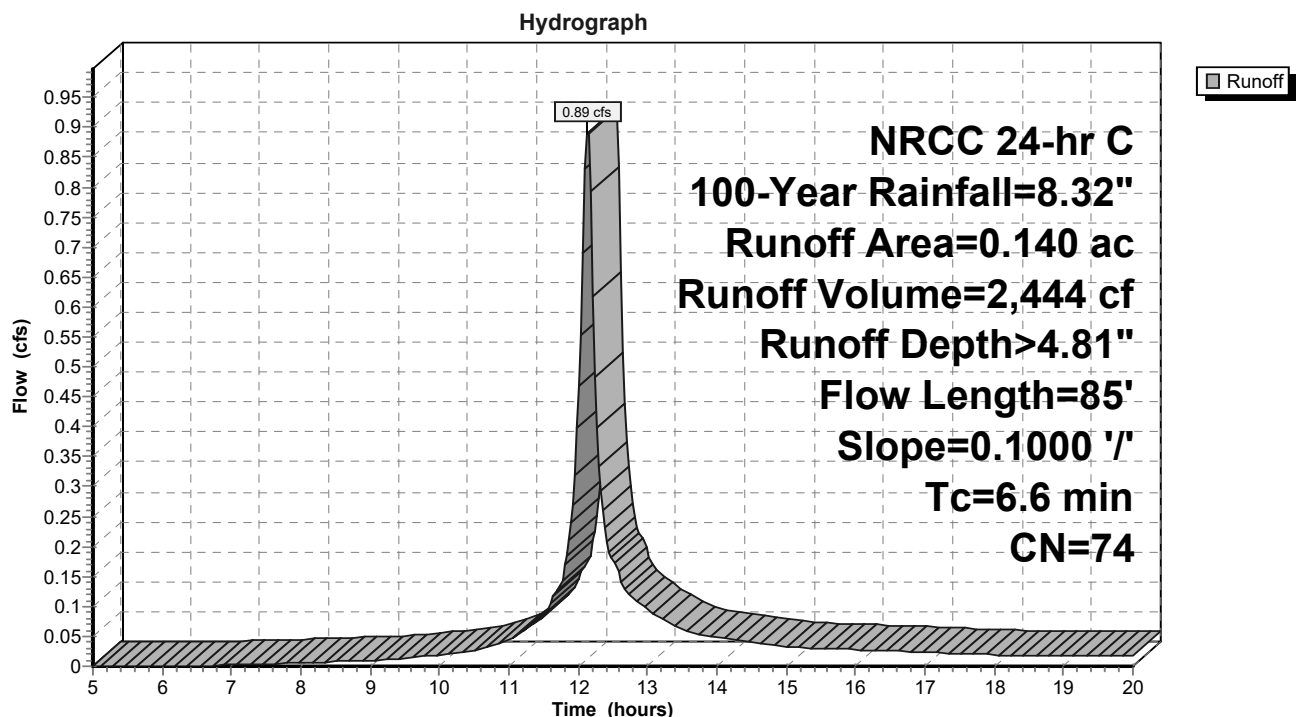
Runoff = 0.89 cfs @ 12.14 hrs, Volume= 2,444 cf, Depth> 4.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 100-Year Rainfall=8.32"

Area (ac)	CN	Description
0.041	74	>75% Grass cover, Good, HSG C
0.099	74	>75% Grass cover, Good, HSG C
0.140	74	Weighted Average
0.140		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.6	85	0.1000	0.21		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"

Subcatchment POA3.1-POST: DA3.1



HAMILTON ST_POSTvsPRE_10.03.23_OUTLET CONRCC 24-hr C 100-Year Rainfall=8.32"

Prepared by ARICO ASSOCIATES

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Summary for Subcatchment POA4-POST: DA4

Runoff = 1.89 cfs @ 12.18 hrs, Volume= 6,100 cf, Depth> 4.80"

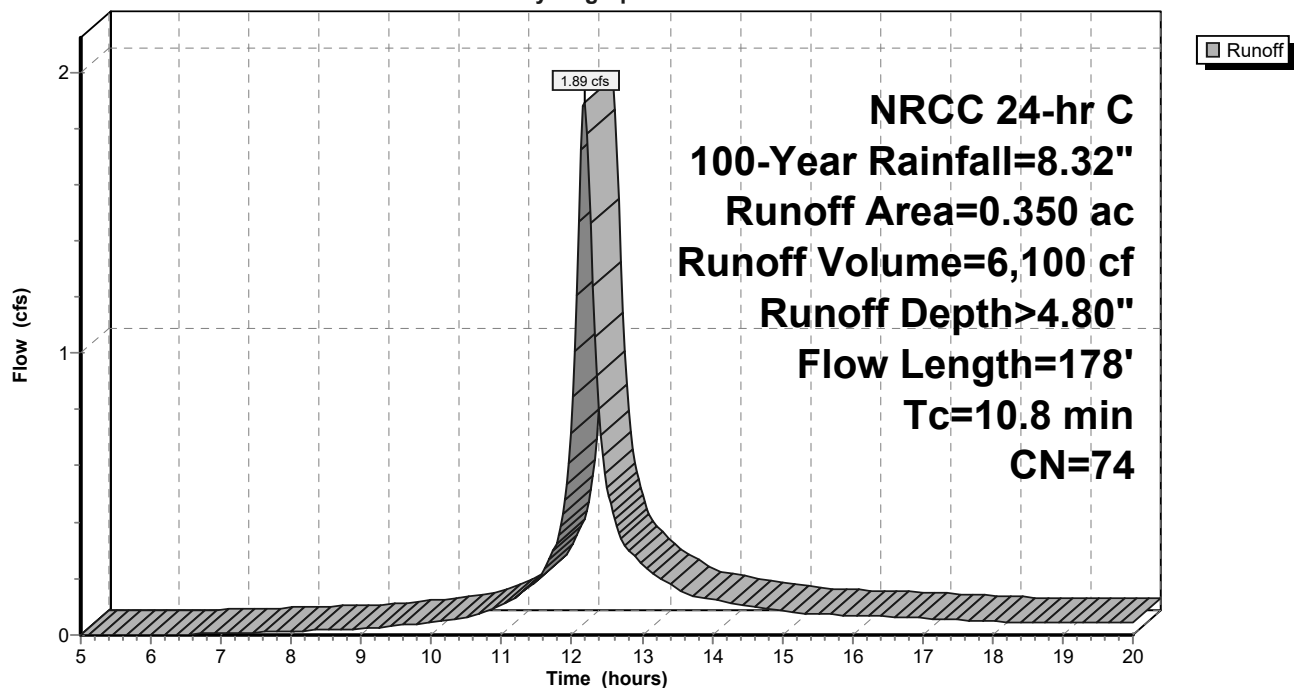
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 100-Year Rainfall=8.32"

Area (ac)	CN	Description
0.350	74	>75% Grass cover, Good, HSG C
0.350		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.3	100	0.0450	0.16		Sheet Flow, Grass: Dense n= 0.240 P2= 3.18"
0.5	78	0.0250	2.55		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
10.8	178	Total			

Subcatchment POA4-POST: DA4

Hydrograph



Summary for Subcatchment POA4-PRE: DA4

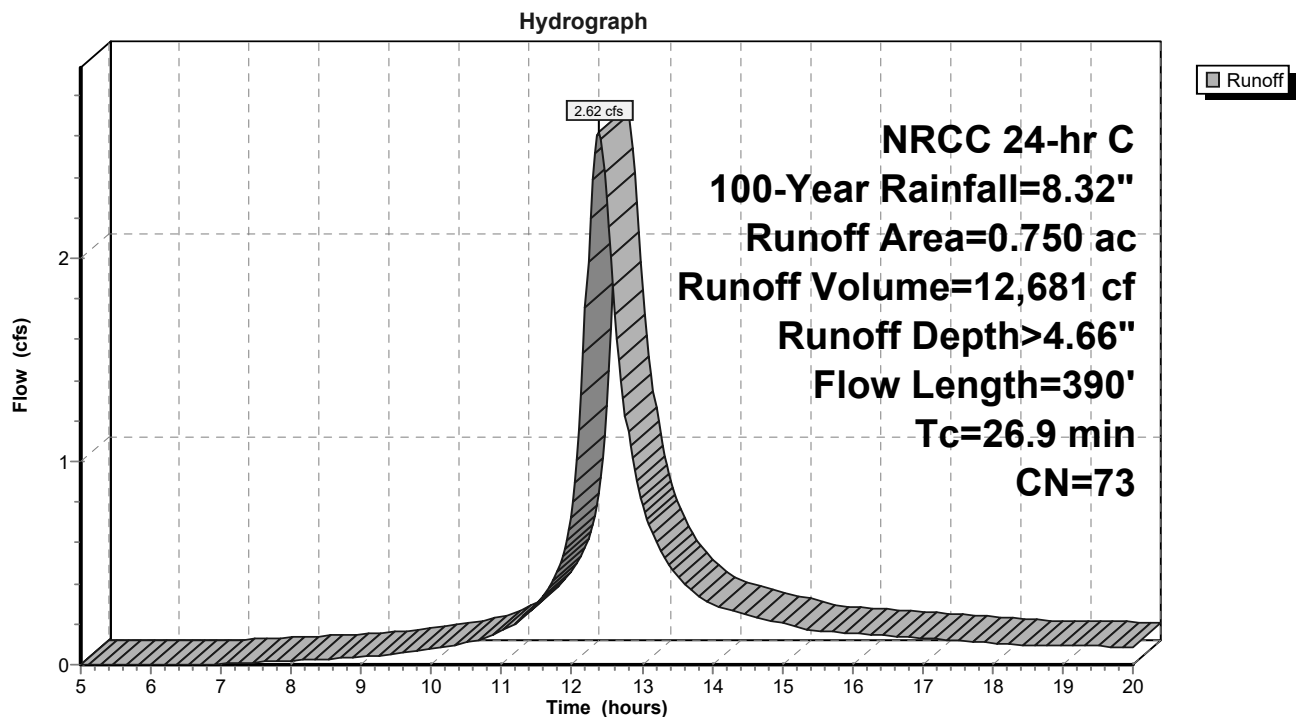
Runoff = 2.62 cfs @ 12.38 hrs, Volume= 12,681 cf, Depth> 4.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
NRCC 24-hr C 100-Year Rainfall=8.32"

Area (ac)	CN	Description
0.450	72	Woods/grass comb., Good, HSG C
0.300	74	>75% Grass cover, Good, HSG C
0.750	73	Weighted Average
0.750		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.2	100	0.0150	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.18"
1.5	140	0.0920	1.52		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.2	150	0.0160	2.04		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
26.9	390	Total			

Subcatchment POA4-PRE: DA4



Summary for Pond SMA1: SMA1

[82] Warning: Early inflow requires earlier time span

Inflow Area = 108,464 sf, 62.25% Impervious, Inflow Depth > 6.50" for 100-Year event
 Inflow = 19.77 cfs @ 12.13 hrs, Volume= 58,722 cf
 Outflow = 2.90 cfs @ 12.62 hrs, Volume= 34,084 cf, Atten= 85%, Lag= 29.1 min
 Primary = 2.90 cfs @ 12.62 hrs, Volume= 34,084 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs
 Peak Elev= 199.10' @ 12.62 hrs Surf.Area= 10,128 sf Storage= 34,574 cf

Plug-Flow detention time= 189.4 min calculated for 34,077 cf (58% of inflow)
 Center-of-Mass det. time= 106.1 min (857.3 - 751.3)

Volume	Invert	Avail.Storage	Storage Description	
#1	194.00'	44,331 cf	Custom Stage Data (Conic) Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
194.00	3,650	0	0	3,650
196.00	6,170	9,710	9,710	6,218
198.00	8,520	14,627	24,337	8,644
200.00	11,550	19,993	44,331	11,756

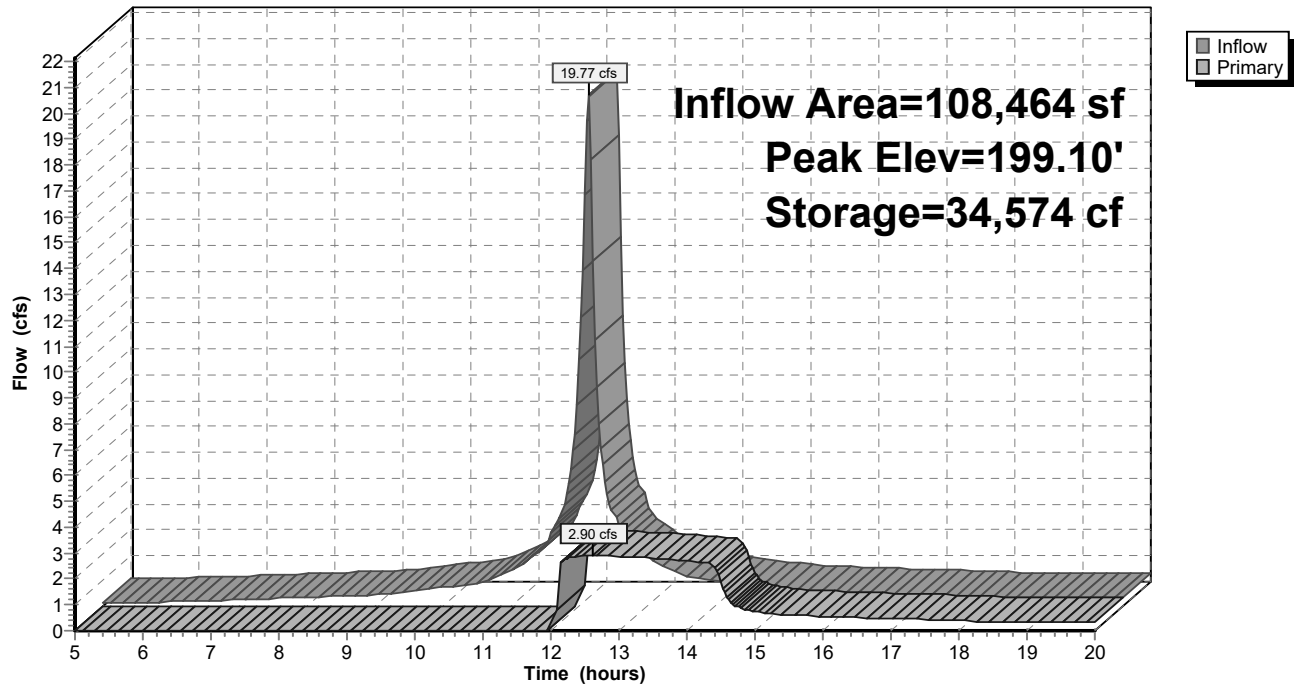
Device	Routing	Invert	Outlet Devices
#1	Primary	194.00'	8.0" Round Culvert - RISER OUTLET L= 25.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 194.00' / 193.75' S= 0.0100 '/' Cc= 0.900 n= 0.010, Flow Area= 0.35 sf
#2	Device 1	194.00'	3.0" W x 18.0" H Vert. Orifice - SIDE OF RISER C= 0.600 Limited to weir flow at low heads
#3	Device 2	198.00'	48.0" Horiz. Orifice - TOP OF RISER C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.90 cfs @ 12.62 hrs HW=199.10' (Free Discharge)

- ↑ **1=Culvert - RISER OUTLET** (Inlet Controls 2.90 cfs @ 8.30 fps)
- ↑ **2=Orifice - SIDE OF RISER** (Passes 2.90 cfs of 3.76 cfs potential flow)
- ↑ **3=Orifice - TOP OF RISER** (Passes 2.90 cfs of 47.35 cfs potential flow)

Pond SMA1: SMA1

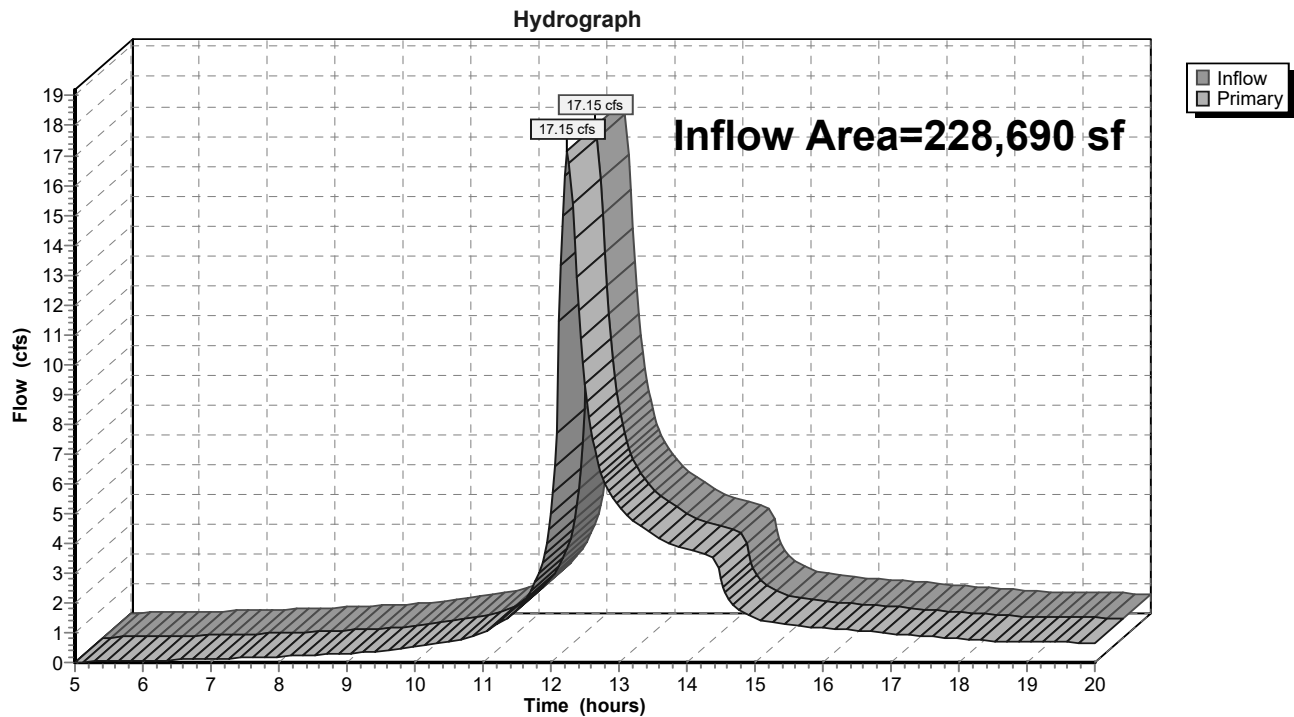
Hydrograph



Summary for Link TOTAL POA2: TOTAL DA2

Inflow Area = 228,690 sf, 44.10% Impervious, Inflow Depth > 4.73" for 100-Year event
 Inflow = 17.15 cfs @ 12.24 hrs, Volume= 90,173 cf
 Primary = 17.15 cfs @ 12.24 hrs, Volume= 90,173 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.03 hrs

Link TOTAL POA2: TOTAL DA2

APPENDIX I

KEVIN L. PATTON, P.E.
36 PATTON ROAD
NEWBURGH, N.Y. 12550
845 275-7732 PATTONGEOTECH.COM

CLIENT:	One Key Development 241 Hudson Street Hackensack, NJ 07601	PROJECT:	Harrington Heights Poughkeepsie, N.Y.
		Project No.	23216
		Date:	July 6, 2023

SUBSURFACE INVESTIGATION REPORT

PROJECT DESCRIPTION

This geotechnical investigation report was prepared for use in the design and construction of a new three-story multi-family apartment building on a previously-developed lot at 100 South Hamilton Street, as shown on the attached plan. The building will have an L-shaped footprint of approximately 22,800 square feet, with the main section aligned generally north-south, along South Hamilton Street, with a width of approximately seventy feet and a length of about 235 feet. A wing seventy feet in width will extend approximately one hundred feet east from the south part of the building. A stormwater control area is proposed along the east side of the site.

The proposed building is expected to have a first floor elevation of 213 feet, with no basement level. It will straddle the top of a small hill which peaks at about 223 feet in the middle of the building area, most of which is at elevations of 210 to 223 feet, with low points of 206 feet at the northwest corner, 208 feet at the southwest corner and 202 feet at the northeast corner. The existing elevations in the proposed stormwater control area are approximately 197 to 198 feet. Aerial photos indicate that a large residence on the site was demolished approximately forty to fifty years ago, and that since then the site has been idle, and was covered with trees prior to starting the project. Remnants of the driveway are still present, near the northeast corner of the site, minor demolition debris was encountered in the former house area, and some fill materials were noted in the proposed stormwater control area.

The Surficial Geologic Map of New York State (N.Y. State Museum, 1989,) indicates that the site is in a large area of glacial till over shallow bedrock. The USDA Soil Survey indicates that the native topsoil type in the project area is Dutchess-Cardigan-Urban land complex. Dutchess soils typically form over deposits of glacial till composed of sandy silty gravel with few flat (flaggy) cobbles, and are typically more than seven feet thick over bedrock. Cardigan soils typically form in till deposits composed of silt and sand with traces gravel and trace flaggy cobbles, with bedrock at about thirty to 34 inches depth. The soil survey reports that the water table is typically at a depth of six feet or more year-round where these soils are present. The local bedrock is thinly-layered gray siltstone, shale and sandstone of the Ordovician-age Austin Glen formation. Bedrock was encountered in several of the test pits. The soils and rock encountered in the test pits were generally consistent with the reference data. Some minor areas of fill were also noted.

SOIL INVESTIGATION AND TEST RESULTS

Eight test pits were excavated on June 14, 2023, six in the proposed building area and two in the proposed stormwater control area. The two test pits at the north end of the building were offset to the north of the building due to the site conditions. Only the north half of the stormwater area was accessible. The test pits were excavated using a tracked mini-excavator by One Key LLC. The subsurface investigation was supervised and witnessed by Kevin Patton, P.E.

Soils and bedrock were observed and sampled, and laboratory testing was performed on representative soil samples, for moisture content, particle size distribution and Atterberg Limits. USCS classifications of the soil, per ASTM D2487 and D2488, are provided on the logs and on the subsurface profile drawing.

TEST PIT SUMMARY								
	Building Area						Stormwater	
Test Pit No.	TP1	TP2	TP3	TP4	TP5	TP6	TP7	TP8
Elevation, approx.	212	205	214	222	222	209	197.5	197
Depth, feet	12	6	7.5	8	5	4.9	6	8.5
Depth to Rock, feet	-	5.2	6.5	7.5	4.2	4.9	-	-



PHOTO 1. Excavating test pit TP4. All photos were taken on June 14, 2023.

Laboratory Data

Natural Moisture Content, Percent						
Depth, feet	TP1	TP2	TP3	TP4	TP7	TP8
2					16.5	
3		14.1	13.7			
4				13.6		20.6
5					13.5	
8	13.6					
11	14.4					

SOIL TEXTURE					
Particle Size Analysis					
Sample		TP1	TP3	TP7	TP8
Depth		8 feet	3 feet	2 feet	4 feet
Type					
USCS Class		SC-SM	ML	CL	ML
Sieve Size	mm	Percent Passing by Weight			
¾"	19.0	86	100	100	100
#4	4.75	74	92	98	100
#10	2.00	66	88	97	99
#40	0.425	53	82	94	96
#200	0.075	35	54	67	53
Hydrometer	0.050	28	42	54	33
	0.005	5	11	17	1
	0.002	1	5	9	0
Atterberg Limits					
Liquid Limit		22	-	28	-
Plastic Limit		16	-	17	-
Plasticity Index		6	Non-plastic	11	Non-plastic

The moisture content test results were consistent with the observed moist to very moist or wet conditions in well-consolidated soils with negligible organic content. The particle size analyses and Atterberg Limits tests represent some of the typical native soils in the project area. The first sample was composed of 26% gravel, 39% sand and 35% silty clay. The second sample consisted of 8% gravel, 38% sand and 54% silt, the third was 2% gravel, 31% sand and 67% clay, and the fourth sample contained 47% sand and 53% silt, with no gravel. Occasional cobbles and few boulders were present in the soil, as noted in the logs, but were not included in the tests. The Atterberg Limits tests indicate that the fine fraction of the more highly-plastic soils is silty clay or lean clay, making them somewhat moisture-sensitive, but indicating a low potential for detrimental shrink-swell (expansive) behavior. The Atterberg Limits were not tested for the two silty samples; the sample from TP3 had low cohesion; it might have a plastic limit but does not have a liquid limit. The sample from TP8 was fine-sandy cohesionless silt, without an LL or a PL.

SUBSURFACE PROFILE AND SUMMARY OF SOIL CONDITIONS

The observed subsurface conditions are described in the test pit logs and are summarized in the drawing attached to this report. Most of the native soils and most of the existing fill consisted of fine-sandy silt with traces gravel, few cobbles and boulders. Some of the native soils in test pits TP3 and TP4 were identified as glacial till, but most of the native material appears to be till that has been significantly reworked by frost, soil creep and similar factors. Well-graded granular soil (gravelly silty clayey sand) was only encountered in test pit TP1, below elevation 205; this was also the only soil in the building area that was noted as being 'very moist,' with moist conditions observed elsewhere.

Bedrock was encountered in five of the six building area test pits. It was not found in test pit TP1, near the northeast building corner, which stopped at twelve feet depth, approximately elevation 200. Near the top of the hill, bedrock was encountered at 7.5 feet depth, approximately elevation 214.5 in test pit TP4, and in TP5 it was at 50 inches depth, elevation 218. Test pit TP2, near the proposed northwest building corner, encountered rock at five feet depth, roughly elevation 200. Test pit TP6, in the low southeast corner area, stopped on rock at about five feet depth, at an approximate elevation of 204 feet. Near the southwest corner, TP3 encountered bedrock at 6.5 feet depth, approximately 207 feet elevation. The bedrock was thin-bedded shale, with near-vertical beds striking northeast observed in test pit TP2. The first six to twelve inches of the bedrock could be ripped easily with the small excavator. The other three test pits in the building area ended in soil at elevation of 200 to 207 feet. The other test pits indicate rock near elevation 200 at the northwest corner (TP2,) becoming shallower toward the south and east (± 207 ft southwest, TP3; 214 ft north-center, TP4; 218 feet center, TP5,) then dropping off at the southeast corner (TP6, ± 204 feet.)

The soils in test pit TP8, near the north end of the proposed stormwater control area, consisted of approximately thirty inches of moist fill (gravelly silt and sand) over silt and fine sand, which was very moist to sixty inches depth, where it became wet. Test pit TP7, near the center of the proposed practice, encountered two inches of organics over eight inches of silty sandy fill over clay, which was moist from ten inches to 32 inches depth, where it became wet. A rock outcrop projects above grade off-site, just east of the property line in this area.

Fill materials were observed in the two stormwater area test pits as noted above, and in three of the six building area test pits. The fill materials in the stormwater control area were mostly soil, but included minor amounts of debris such as steel, concrete, cinders and glass. In the building area, twelve to eighteen inches of sandy silt fill was noted in test pits TP1 and TP4, with a couple small pipes at the latter location. Test pit TP5, near the location of the former residence, encountered forty inches of sandy silty fill over a ten-inch layer of bricks with soil, over bedrock.

SUBGRADE PREPARATION

The investigation indicates that the subgrade conditions throughout the proposed building area are suitable for the use of shallow spread footing foundations and slabs-on-grade, subject to performing the required subgrade preparation operations, as described below.

The test pits indicate that the top of rock is typically about five feet below existing grade in the building area, deeper near the northeast corner. Using five feet as an average, approximately five-eighths of the slab is expected to be supported on a bedrock subgrade at elevation 212 feet. At the north end and at the southwest and southeast corners of the building the subgrade will transition from rock to native soil, then to fill. Most of the footings will also bear on rock, transitioning to native soil at the north end and at the two south corners, however at the south end the footings may be made deeper as the grade drops off; with close proximity to the property line, this appears to be more practical than building up a fill pad to the same elevation as the rest of the foundation.

Remove all existing pavement, topsoil, soft subsoil, stumps and large roots from the subgrade surface, in all foundation and slab areas. Remove existing slabs and foundations, if encountered, in their entirety from below new foundation areas, and to at least twenty-four inches below bottom-of-slab in slab-on-grade areas, and as needed to prevent interference with the new construction. Remove uncontrolled fill in its entirety from below new foundation areas and from below slab-on-grade areas unless otherwise approved by the Engineer.

Excavate to the design subgrade elevation, or deeper if required to reach suitable subgrade, which shall be bedrock (hard or weathered) or undisturbed native soils, free from topsoil, fine organics and root masses. Where the subgrade material is soil it shall have a stiff/ medium-dense or harder consistency. Dewatering and draining of the excavation may be required to some extent during excavation, with storms expected to be the main source of water.

Trim soil to the required subgrade elevation using excavation methods that minimize disturbance of the final soil surface. Compact the surface as needed to consolidate any soil that was loosened during excavation. Remove any pockets or small zones of unsuitable materials that are encountered, and replace them with controlled compacted fill. Contact the Engineer prior to performing any significant extra excavation. Where old foundations, stumps or boulders are removed, or where other over-excavation work is performed to prepare subgrade areas, the sides of the excavation shall be trimmed back to stable soil as each lift of backfill is placed. Where deficient soil is removed from below footing locations, the remediated area shall extend at least one foot out from the footing per foot of depth (1 to 1 splay.)

Where bedrock is present and excavation is performed by ripping, hammering or blasting, remove the rock to an approximately level and uniform elevation, with a slope of ten percent or less in areas below footings and slabs. If the rock subgrade surface has open fractures, level and seat the surface by tracking back-and-forth over it with a bulldozer or excavator and/or spade it with the excavator bucket, then compact the surface with several passes of a vibratory trench roller or a single-drum soil roller.

Footings may bear directly on the prepared soil or rock subgrade, or on controlled compacted fill placed over the subgrade. Where bedrock is present, over-excavation of the rock by six to twelve inches is

recommended, with the rock replaced by compacted granular site-borrow soil or Structural Fill, to act as a cushioning layer and as a leveling course for the concrete forms. Where fine-grained native soil is present at the bearing elevation, a layer up to four inches thick of Structural Fill or equivalent site-borrow soil may be placed in the footing bottom to protect the soil surface, after properly preparing the surface to a level and stable condition. This layer shall be thoroughly compacted with a vibratory plate tamper or roller, and its surface shall not extend above the design bearing elevation. Footing bearing surfaces shall be free from frost, mud and loose soil or standing water, when concrete is placed. Rock surfaces should be thoroughly moistened prior to placing concrete.

Protect the prepared subgrade surface from erosion, from frost and from construction damage. Traffic from dump trucks and similar heavy vehicles should be minimized on the exposed surface of the subgrade and on compacted fine-grained fills.

EXCAVATION

The existing fill and native soils may be excavated using conventional heavy equipment, such as tracked excavators and bulldozers. Backhoes and mini-excavators should be suitable for the excavation of shallow trenches in soil, but may require assistance if boulders are encountered. The investigation indicates that the soils which will be encountered in the building excavations are likely to predominately be OSHA Type B, requiring a minimum slope of 1-to-1 in shallow excavations, with benching permitted. Soil types and excavation requirements must be confirmed by a qualified representative of the Contractor during construction.

Rock excavation is expected to be needed in most of the building area. A large bulldozer with a ripper tooth should be able to excavate the shallow rock, provided it can 'bite' into the bedrock surface, which may be smooth on top of the hill. A large excavator with a hydraulic hoe-ram will likely be required to excavate to depths of more than two or three feet into the bedrock. Excavation work in the building area should be practical using conventional methods, without blasting. Blasting could, however, be used to facilitate the removal of bedrock. The project area is sufficiently distant from existing structures, roads and other public areas to perform shallow blasting without causing adverse impacts to them. There is sufficient overburden present to use small, lightly-loaded blast holes to fracture the bedrock, so that it can be more easily removed by conventional excavation, although some ripping and hammering could still be required.

Little or no shoring of excavations should be required for building construction, as there appears to be sufficient distance from the property line to the estimated limits of the foundation work area to allow the use of conventional excavation slopes. Trench boxes or other temporary shoring will be required for work in deep trenches, other than trenches in rock, and may be required when excavating trenches in wet areas, regardless of depth. The design of any necessary shoring or other support-of-excavation is the responsibility of the Contractor and is not included in this report.

Groundwater seepage rates in the building excavations are expected to be slow, but may be persistent during the wet season. Groundwater seepage and stormwater should be removed promptly from the excavations, and the groundwater elevation should be maintained at a depth of at least one foot below the soil surface in active construction areas. When dewatering excavations, the water level should be drawn

down at a controlled rate to minimize sloughing, allowing the water to drain from the soil in the sides of the excavation. Gravity drainage and conventional shallow sump pits installed in the excavation should be adequate for the control of groundwater seepage and stormwater.

FILL MATERIALS

All fill placed below foundations and slabs shall consist of Structural Fill or Structural Fill HD, as described below, or shall be suitable site-borrow soil or rock, as described below, or shall be other imported fill of a quality at least equal to that of the site-borrow fill.

Soils excavated from the site are expected to be of suitable quality for re-use as fill and backfill for foundations, slabs and pavement areas. Exclude topsoil and wet, organic otherwise deficient soils from the excavated soil, which should be blended as much as practical, to produce a uniformly compactible fill material. The site-borrow soil will likely require some require drying prior to compaction. Boulders and large cobbles must be removed from the borrow fill. Clumps of clayey soil should be excluded from the fill, but if included they must be thoroughly broken up and mixed in.

Rock excavated from the site may also be used as fill in building and pavement areas. If the rock is excavated by ripping or hammering, it should be processed by crushing and/or screening to approximately the grading of Structural Fill, below. Blasted rock will likely be suitable for use as fill after removing the oversize pieces, without further processing.

Open-graded gravel or crushed stone should not be placed directly against the silty native soils for most applications, as the soil may erode into the stone. The gravel should be separated from the soil by a geotextile layer or by a layer of well-graded soil. Pipe bedding in utility trenches may also act as a drainage path for groundwater seepage. If coarse granular bedding is used in sloping trenches, it should be interrupted periodically by sections of well-graded bedding, to regulate the flow of this seepage.

Structural Fill, if imported for use below foundations and slabs, shall be good-quality bank-run sand and gravel or crushed stone, and should be a well-graded product complying with the gradation limits below. Structural Fill may also be used as foundation backfill. Structural Fill HD (Heavy Duty) should be used in areas to be protected from heavy construction traffic and where subgrade stabilization and/or enhanced drainage is needed and is suitable for use as fill during winter conditions.

Sieve size		Structural Fill	Structural Fill HD
Inch	mm	Percent Passing by Weight	
4"	100	100	100
1½"	37.5	60-100	50-95
#4	4.75	20-85	20-50
#40	0.425	5-50	5-25
#200	0.075	0-30	0-10
Plasticity Index		12 max.	4 max.

All fill materials shall be composed of sound, durable particles, shall be free from frost or snow, garbage, construction debris or other deleterious material, and shall be substantially free from organic matter and roots. Recycled crushed concrete and masonry from a registered source may be acceptable for some applications above the water table, subject to approval by the Designer of Record. Fill shall not be placed over frozen or unstable soil, unless approved by the Engineer.

CLSM (Controlled Low-Strength Material, aka flowable fill) may be used under footings and foundations when specifically approved by the Engineer, and may also be used to backfill trenches or other excavations, typically where rapid fill placement is required, fill areas are narrow, or the use of conventional compaction methods is not practical. For support of footings, a CLSM mix consisting of sand, cement and water, with a 56-day compressive strength of 75 to 200 psi, is appropriate. CLSM may produce high fluid pressures during placement, and caution must be used for placements against foundation walls, near unbraced cuts, etc. Pipes or tanks can also float if not properly restrained during placement. CLSM should not be placed against unprotected aluminum; CLSM containing flyash should not be used in contact with cast iron or ductile iron. Hardened CLSM masses may also adversely affect groundwater flow, possibly causing erosion under or along the CLSM, particularly in sloping trenches.

Other Fill Materials:

- Crushed stone base course for slabs-on-grade should consist of ASTM C33 #56 or #57 stone ($\frac{3}{8}$ - to $\frac{3}{4}$ -inch size,) or as required by the slab system design.
- Crushed stone or gravel for footing drains should consist of ASTM C33 #5, #56 or #57 stone ($\frac{3}{4}$ -inch or $\frac{3}{8}$ - $\frac{3}{4}$ -inch size.)
- Well-graded granular subbase material (NYSDOT Item 733-04 'Item 4' or similar) should be used under sidewalks and exterior slabs.

FILL PLACEMENT AND COMPACTION

Soil surfaces, including the surface of the subgrade and of previously-placed fill materials, shall be prepared to a stiff and essentially unyielding condition prior to placing each new lift of fill. Bedrock surfaces to receive fill shall be free from voids or loose areas and the rock surface shall be free from large loose pieces of stone. Use mid-size equipment to compact the site-borrow fill or similar materials. Vibratory trench rollers, and single-drum soil rollers with a nominal size of three to five tons, should be appropriate for the observed soil conditions. Larger rollers may be used when compacting well-graded granular fill over essentially unyielding surfaces. In areas with limited access, vibratory plate tampers or jumping-jack tampers may be used. Avoid over-compacting the shallow soils in landscaped areas.

Soil fill and processed rock fill shall be placed in controlled lifts, with each lift compacted to the required density at a moisture content close to optimum moisture, as determined by the ASTM D1557 Modified Proctor Method. When the moisture content of fill which will support footings or slabs is within two percent of optimum, fill may be placed in lifts with compacted thicknesses of up to eight inches. If the moisture content is two to 2.5 percent from optimum, reduce the maximum thickness to six inches, and if it is more than 2.5 percent from optimum, discontinue compaction. In roadway, embankment and other non-structural locations, fill may be placed in lifts with a compacted thickness of up to twelve inches when the moisture content of the fill is within two percent of optimum, and up to eight inches thick when it is

two to 2.5 percent from optimum. Use a reduced lift thickness if required to obtain the specified percent compaction and when using small compaction equipment.

When placing fill during freezing temperatures, use Structural Fill HD or similar fill material containing less than ten percent non-plastic fines. Fill produced from natural sand and gravel is preferable to crushed stone or concrete fill for winter work, for ease of compaction. Compact the fill with a maximum lift thickness of six inches and do not compact fill whose moisture content is more than two percent above or below optimum. Do not place or compact fill when the air temperature is less than 25°F. Do not place frozen fill materials.

Mass rock fills may be constructed using well-graded shot-rock from blasting. The design lift thicknesses should be approximately twelve to eighteen inches, depending on the dimensions of the fill, the fill placement method and the compaction method, and may require adjustment based on the actual size and grading of the rock after it is blasted. Rock fill should be placed in a manner that minimizes segregation and which creates an initially-dense state. Allowing the rock fill to free-fall into place from a belt or bucket can be an effective method, but dust, noise and flying particles are of concern. Lifts of rock fill may be acceptably placed with a bulldozer, but a moving pile of fill should be maintained in front of the blade, rather than running the pile out completely. Re-work any boulder pockets to a dense, well-graded condition, and level the surface of the fill prior to compaction. The rock fill should be compacted in a wet condition when practical, but may also be compacted when dry or moist. Rock fills should be placed in conformance with an engineered design which includes any necessary requirements for drainage; for geotextiles and/or aggregate filter zones between the rock fill and the subgrade, adjacent and/or overlying soils; the clearance depth for utilities, etc.

Where fill will be placed against slopes, bench the fill into the slope to create a stair-step interface, for improved stability and groundwater control. Lightly scarify the surface of the existing soil prior to placing the fill, and key the fill into the subgrade at the toe of the slope. When the fill is more than five feet high against a slope of twenty percent or more, the key should be at least two feet deep and ten feet wide.

Backfill placed against foundation walls should be compacted with trench rollers or with similar equipment which will not produce damaging stresses on the wall. Place backfill equally on opposite sides of the foundation unless otherwise indicated by the specifications or drawings. Foundation walls acting as retaining walls should be braced (i.e. by installation of the floor framing) before placing backfill, unless otherwise indicated on the drawings or approved by the Engineer.

Open-graded stone base course material for slabs-on-grade should be graded level and seated with one or more compaction passes, to help resist displacement during slab area preparation and concrete placement.

Compaction Requirements

Compact each lift of soil fill supporting slabs or foundations with at least six one-way compaction passes, even if the required compaction percentage is obtained with fewer passes. Each compaction pass shall be made at a slow walking speed (less than four feet per second,) with the equipment passing completely over all areas of the fill. Fill materials shall be compacted to at least the following percentage of the ASTM D1557

maximum dry density. For coarse-graded fill materials with more than thirty percent retained on the 3/4-inch sieve, the ASTM D4253 Maximum Index Density test may be substituted for the D1557 test.

Location	Percent Compaction
Below Footings, Foundations, Slabs and Pavements	95% minimum
Exterior Foundation Backfill in Landscaped Areas	90% minimum

Mass rock fills must be compacted with the largest vibratory single-drum soil roller that is available and appropriate for the site, typically in the ten-ton to sixteen-ton size range. Compact each lift with at least six vibratory roller passes, with the passes applied in two perpendicular directions, or in multiple directions, when possible. Vary the vibration frequency to maximize compaction, using the high-frequency mode for the final passes, for increased compaction depth with less surface disturbance. Continue compaction until the fill surface is in an unyielding condition.

TESTING

The prepared subgrade shall be inspected to verify that it has been prepared in conformance with the requirements of this report, prior to placing fill or foundations. Recommended test procedures and frequencies are provided below.

PROOF-ROLLING: Proof-rolling of the prepared subgrade soil is not required, but may be performed to determine the limits of a soft area. Use an appropriately-sized vehicle, to avoid damaging wet and/or fine-grained, but otherwise acceptable soils. Observe the effects of the moving vehicle; if the soil exhibits excessive deflection, rutting or cracking, additional excavation or drying of the subgrade may be required.

BEARING CAPACITY: The prepared subgrade surface throughout the foundation and slab areas area shall be probed thoroughly to check for soft spots. The foundation subgrade shall be in a dense and unyielding condition, substantially free from soft areas and/or loose material. If these conditions are not encountered at the bearing elevation, testing shall be performed with a Static Cone Penetrometer or equivalent device in foundation bearing areas. If the design bearing capacity is not indicated within three inches of the bearing surface, the soil conditions shall be corrected and/or, if approved by the Engineer, the footing depth may be increased to reach acceptable soil. The slab subgrade areas shall be densely consolidated and sufficiently stiff to prevent rutting or displacement during slab base course and concrete placement operations. Where fractured rock or bedrock is present at the subgrade surface, it shall be visually inspected to verify that it is generally level and free from accumulations of loose material, and that in areas where it is fractured, the fractured mass is in a dense and stable condition, free from large open voids.

COMPACTION TESTING: Compaction testing shall be performed for each lift of soil fill supporting foundations, with testing performed while the work is in progress. Testing is also recommended for the fill supporting slabs or pavement, exterior backfill and for fill placed in embankments. Compaction testing is recommended for all retaining wall construction, with requirements to be determined by the wall designer. Compaction tests of fill and backfill supporting foundations and slabs should be performed in at least one location per 50 linear feet or per 1000 square feet of fill surface, per lift. At least one test per 100 linear feet

or per 2500 square feet should be performed for each lift of fill in embankment, roadway and other non-structural areas. Compaction tests should be performed with a nuclear moisture-density gauge, per ASTM Test Method D6938, unless otherwise approved. Required percent compaction values are provided above. Rock fill placement shall be visually monitored for fill quality and grading and for conformance with the requirements for lift thickness and placement methods and for application of the specified compaction effort.

CLSM: When flowable fill is used to support footings or foundations, at least one set of three 6x12-inch test cylinders shall be cast from each day's placement, per ASTM D4832. Test the cylinders for unit weight and for compliance with the specified strength requirements. Cast additional cylinders if early tests are needed.

GEOSYNTHETIC MATERIALS

Geosynthetic materials are expected to be used for reinforcement and drainage applications at the site on an as-needed basis, or where required by Code, such as for footing drains. Geosynthetic materials shall be installed over a smooth and evenly shaped subgrade, to avoid 'tenting' of the material over voids or high points. The geosynthetic shall be installed substantially free from wrinkles, and fill material shall be placed and spread in a manner which pushes the wrinkles out but which does not otherwise displace the geosynthetic material. Vehicles shall not drive on the exposed geosynthetics. The following material types are recommended.

Drainage Separation: For footing drains and similar applications, a woven drainage geotextile with at least 4% open area, with an apparent opening size of 0.21mm (#70) or smaller, should be installed around the open-graded drainage stone. Non-woven geotextiles are not recommended for use in this application, due to the presence of fine particles in the native soil that will tend to clog the fabric. If stone drains are installed in areas of fine sand or cohesionless silt, provide a zone of clean sand at least three inches thick between the geotextile and the soil.

Subgrade Reinforcement: Typically, a woven reinforcing geotextile with a grab tensile strength of at least 200 pounds should be used where needed to improve the stability of soft subgrade soils. Geogrids may be used instead of woven geotextiles, especially if free drainage is desired. A minimum of twelve inches of granular fill cover is typically required to fully mobilize the strength of the reinforcing geosynthetic.

Subgrade Separation: Where fines from the subgrade may infiltrate into an overlying granular layer, and strengthening of the subgrade and some vertical drainage is desirable, a non-woven geotextile should be used. A four-ounce nominal weight fabric is suitable for light- to moderate-duty applications; heavier fabric should be used where it will be subjected to heavy traffic, especially when it is placed in contact with sharp or very coarse-graded materials.

Woven reinforcement geotextiles will usually act as an infiltration barrier when installed in a continuous horizontal layer, and non-woven separation geotextile may also work as a barrier; this may or may not be desirable, depending on the installation location and conditions, and should be considered when specifying or using these products.

ENGINEERING DESIGN VALUES AND RECOMMENDATIONS

Soil engineering properties and recommendations for design are provided in this section; additional important design considerations are also discussed in the other sections of this report. The design values assume that the building will be supported by a conventional spread footing foundation with slab-on-grade floor, as described in the previous sections, and will be provided with proper drainage.

Bearing Capacity and Soil Pressure

Allowable Bearing Capacity, q_a	
Footings on Soil, bearing at least 42 inches below finished grade, with a minimum width of 12 inches	4000 psf
Footings on Soil, bearing at least 24 inches below finished grade, with a minimum width of 36 inches	4000 psf
Minor Footings on Soil, bearing at least 12 inches below finished grade, with a minimum width of 12 inches	2000 psf
Footings on Fractured and/or Weathered Rock, bearing at least 12 inches below finished grade, with a minimum width of 12 inches	4000 psf

Soil Properties	Native Soils	Rock, fractured
Moist Density, γ , lbs/cu ft	130	160
Effective Internal Angle of Friction, ϕ	32°	42°
Coefficient of Active Earth Pressure, k_a	0.31	0.20
Coefficient of Passive Earth Pressure, k_p	3.25	5.0
Coefficient of At-Rest Earth Pressure, k_o	0.47	0.33
Lateral Bearing Capacity (psf per ft below grade)	210	400
Coefficient of Friction (vs. concrete)	0.35	0.60
Modulus of Subgrade Reaction, k , psi per inch	175	550

Footings subject to frost shall bear at least 42 inches below finished grade, or shall be otherwise protected from frost. Footings not exposed to frost, or bearing on non-frost susceptible subgrade material (e.g. fractured bedrock) shall bear at least one foot below finished grade. Bearing elevations of footings shall be established such that a line drawn between the bottoms of two adjacent footings is not steeper than 30 degrees between the closest points on the footings. (Slope of 1 vertical to 1.75 horizontal.)

Settlement of the foundation elements over the course of construction, due to normal elastic compression of the underlying soils, is expected to be one half inch or less in the well-consolidated soils at this site. Up to one inch of settlement and $\frac{3}{4}$ -inch of differential settlement should be assumed for design purposes. No measurable settlement is expected where foundations bear on rock. If fill is placed below footings, an additional quarter inch of settlement should be expected per five feet of fill thickness.

If encountered above the design bearing depth, undisturbed bedrock and rock that has been fractured by excavation are non-frost-susceptible subgrade materials, and frost-protected shallow footings, as specified

in ASCE 32, may bear on rock at depths as shallow as twelve inches when the required vertical foundation wall insulation is also provided. Where footing depths are reduced, pinning to rock may be required if the footing will be subjected to significant uplift or lateral forces. These installation conditions must be reviewed and approved on a case-by-case basis.

Control of Groundwater and Soil Gases

Minor groundwater seepage should be expected in excavations and below-grade areas during and after construction. It is understood that the building will not have a basement and the first floor slab will be entirely above grade. Adequate drainage must be provided to maintain the groundwater at an elevation below the slab base course; footing drains may be helpful, but are not required by Code for the above-grade slab condition. Construction dewatering is discussed elsewhere in this report. For the completed construction, the elevations shown on the plans indicate that a foundation drain, if used, should be able to discharge by gravity.

Soil gases that could normally be expected to impact the structure are water vapor and radon. The interior spaces should be adequately protected from these gases by providing drainage and moisture protection measures as described above, and by sealing all wall-to-slab joints, concrete cracks, pipe penetrations, drainage sumps, etc. If an open-graded base course is used under the slab, a passive or powered vapor mitigation system using small-diameter PVC pipes can be included or can be added retroactively. The potential for these gases to adversely impact the use of the building is estimated to be low, if the above recommended practices are used, and normal interior ventilation is provided.

Seismic and Expansive Properties

Seismic Design Values: The Seismic Site Class and Seismic Design Category for the proposed construction were determined per section 1613 of the New York State Building Code and ASCE 7-16. Seismic values for the site were obtained from the current database maintained by the Applied Technology Council, Redwood City, Cal., and are consistent with the published maps in the Building Code. Values were as follow.

Occupancy Category	I/II/III	
Seismic Site Class	C- Very Dense Soil and Soft Rock D - Stiff Soil	
IBC Seismic Design Category	SDC - B	
Maximum Acceleration	0.2 sec S_S 1.0 sec S_1	0.195 g 0.055 g
Maximum Spectral Response Acceleration	0.2 sec S_{MS} 1.0 sec S_{M1}	0.254 g 0.082 g
5% Damped Spectral Response	0.2 sec S_{DS} 1.0 sec S_{D1}	0.169 g 0.055 g

The seismic design values are based on the “risk adjusted maximum probable earthquake.” These are not the maximum values that *could* occur, they are values that are not likely to be exceeded during the service life of a typical structure.

Liquefaction Potential: The soils encountered in the investigation have very low liquefaction susceptibility. Liquefaction is typically associated with thick deposits of loose poorly-graded sand situated below the water table, and may also occur in loose, poorly-graded cohesionless silt. A similar phenomenon, cyclic softening, can occur in saturated soft clays. Those soils are also prone to shifting or spreading during seismic events when they are situated near a slope, even if the slope is gentle. The soils encountered in the test pits had textures, drainage conditions, locations and/or sufficient consolidation (density) to resist liquefaction or unusual movement during the design seismic event. No special mitigation measures are required.

Expansive Soils and Frost Heave: The soils encountered in the investigation have a very low potential for expansion due to shrinking and swelling resulting from moisture changes. This behavior is typically associated with high-plasticity silt and clay soils. Physical testing and qualitative examination indicate that the soil properties do not meet the criteria for potentially expansive soils as defined in section 1803.5.3 of the Code. No mitigation measures are required. The on-site soils are moderately to highly susceptible to frost heave. Frost heave can be minimized by providing good drainage and by thoroughly compacting the soil. Well-graded granular fill should be used in areas where frost heave could result in damage.



PHOTO 2. The slope along the west side of the site, overlooking South Hamilton Street.



PHOTO 3. The proposed stormwater control area on the east side of the site. A rock outcrop forms a long knob on the far side of the fence.

NOTES AND LIMITATIONS

Please see the attached pages for additional information. Subsurface conditions encountered during construction shall be compared to the test pit logs and to this report; any significant variations from anticipated conditions must be evaluated for their effect on the design. This report summarizes the results of a limited investigation and does not purport to predict every variation in subsurface conditions. Elevations, slopes, contours, project layout and similar or related data provided in this report were interpreted from the drawings, from field data or from other information which was provided, unless otherwise noted.

This geotechnical investigation was conducted to evaluate the engineering properties of the soils at the site, to aid in the design and construction of the proposed work. The investigation did not include evaluation of the potential effects of the proposed construction on other properties, nor did it include inspection of, or sampling for, items of environmental concern such as the presence of soil contaminants or of regulated wetlands, and did not include review of local zoning regulations, codes, floodplain boundaries or similar matters, unless specifically referenced in the report. This investigation was conducted solely for the use of the Client, the Client's Project Designers and Agents and the Authorities Having Jurisdiction; this report should not be used by others, nor for any use other than its stated purpose, without contacting the Engineer. Any such use is solely at the user's risk.



Prepared by Kevin L. Patton, P.E.

Attachments:

- Soil Technical Notes
- Test Pit Location Plan
- Test Pit Logs
- Laboratory Test Reports
- Bedrock Geologic Map
- Surficial Geologic Map
- USDA Soil Survey Data

Soil Technical Notes:

Soil Classifications, Descriptions and Properties

The USCS (Unified Soil Classification System) was used to classify the soils in this report. The USCS is described in ASTM D2487 (laboratory test method) and D2488 (visual-manual method.) The USCS classification gives a 'Group Symbol' and 'Group Name' based on particle size distribution (gradation,) clay properties (Atterberg Limits) and basic composition (mineral or organic.)

USCS Soil Classes

Soils with less than 5% passing the #200 sieve:

GW, GP, SW, SP – Well-graded gravel, Poorly-graded gravel, Well-graded sand, Poorly-graded sand.

Soils with 12% to 50% passing the #200 sieve:

GC, GM, GC-GM, SC, SM, SC-SM – Clayey gravel, Silty gravel, Silty clayey gravel, Clayey sand, Silty sand, Silty clayey sand.

Soils with 5% to 12% passing the #200 sieve use a dual symbol, such as SW-SC (Clayey well-graded sand.)

Soils with more than 50% passing the #200 sieve:

CL-ML, ML, CL, MH, CH, OL, OH – Silty clay, Silt, Lean clay, Elastic silt, Fat clay, Organic silt, Organic clay.

Highly organic soils:

PT – Peat.

The soil group name is modified with the term 'with sand' or 'with gravel' added if the soil contains more than 15% of these materials; clays and silts with 30% or more plus-#200 material are described as 'sandy' or 'gravelly' (whichever is predominate.) Examples – GM, Silty gravel with sand; CL, Gravelly lean clay.

Particle size	Fine- and Coarse-grained Soils	Atterberg Limits
>12" (300mm) Boulders 12" to 3" (300-75mm) Cobbles 3" to #4 (75-4.75mm) Gravel #4 to #200 (4.75-0.075mm) Sand <#200 (0.075mm) Silt & Clay	The USCS classification applies to the material smaller than the 3-inch sieve. 'Fine-Grained Soils' (silts and clays) have more than 50% passing the #200 sieve and are classified by their Atterberg Limits. 'Coarse-Grained Soils' (sands and gravels) have less than 50% passing the #200 sieve. When more than 50% of the plus-200 material is retained on the #4 sieve the general soil type is gravel, and if more than 50% is finer than the #4 sieve, it is sand. Clean coarse-grained soils are classified as well-graded (Classes GW, SW) or poorly-graded (GP, SP.) Well-graded soils have a wider range of sizes and are typically more stable. Poorly-graded soils are usually more permeable.	Test is performed on the clay, silt and fine sand fraction of the soil: Liquid Limit (LL) – moisture content (%) at which soil becomes very soft. Plastic Limit (PL) – moisture content at which soil crumbles. Plasticity Index (PI) = LL minus PL Higher PI values may indicate reduced permeability and increased drying shrinkage. LL > 50 indicates soil with a higher potential to shrink and swell due to changing moisture content. Silts have lower PI values, and behave like very fine sand; most silts also contain some clay. Behavior of clays is partly controlled by electrochemical forces and varies among the several clay minerals.
Organic Soils		
Highly organic soils such as peat are visually classified. Partly organic soils, with a mix of organic and mineral matter, are classified visually and by Atterberg Limits tests.		
Moisture Content		
Moisture is visually estimated and samples are usually tested. Soil moisture capacity varies with texture and compaction. Typical examples: GW, moist at 3%, saturated at 9% SP, moist at 6%, saturated at 20%. CL, moist at 12%, saturated at 33%.		
Color	Relative Quantities	USDA Soil Classification
Soil color sometimes indicates groundwater conditions, with subdued colors below the water table and mottled (mixed) colors in the zone of seasonal water table fluctuation. Color changes tend to be more prominent in fine-grained soils.	Estimated percentages in descriptions: <5% - Trace 5-10% - Traces 10-25% - Little 25-35% - Some 'And' - Approx. equal amounts 'Few' - <10% (cobbles and boulders)	USDA classifications are based on the relative amounts of sand, silt and clay in the soil fraction passing the #10 (2mm) sieve. 'Gravelly' indicates more than 15% of #10 to 3" size. 'Channery' indicates 15 to 35% thin flat pieces up to 6" long.

KEVIN L. PATTON, P.E. 36 PATTON ROAD NEWBURGH, NY 12550 PATTONGEOTECH.COM 845 275-7732	CLIENT:	One Key LLC		
	PROJECT:	Harrington Heights, Poughkeepsie, N.Y.		
	DATE:	6/14/2023	Project No.:	23216
	WEATHER:	Cloudy, 65F		

TEST PIT LOG					
EXCAVATING COMPANY:	One Key LLC	LOCATION:	50 ft NE from NE corner of proposed building	TEST PIT NUMBER	TP1
OPERATOR:	Garrett	ELEVATION:	212+/-		
EQUIPMENT:	Takeuchi TB180FR Mini-Excavator	WATER DEPTH:			
INSPECTOR:	Kevin Patton				

Depth	USCS Class	Moisture	Description	Notes
0-12"	ML	Slightly moist	Fill. Loose. Grayish brown sandy silt with trace brick, glass, few small roots.	
12-24"	ML	Moist	Native soil. Loose to medium-dense. Mottled brown silt loam.	
24-42"	ML	Moist	Dense. Slightly mottled brown silt with little sand, trace gravel and cobbles.	
42" to 7ft	ML	Moist	Dense. Mottled pale brown silt with little sand, trace gravel and cobbles, few boulders to 24" size.	
7ft - 12ft	SC-SM	Very Moist	Dense. Mottled pale brown sand and silty clay with little gravel, few cobbles and boulders to 24" size.	
12 ft	-	-	Stopped at 12 feet in soil.	



KEVIN L. PATTON, P.E. 36 PATTON ROAD NEWBURGH, NY 12550 PATTONGEOTECH.COM 845 275-7732	CLIENT:	One Key LLC		
	PROJECT:	Harrington Heights, Poughkeepsie, N.Y.		
	DATE:	6/14/2023	Project No.:	23216
	WEATHER:	Cloudy, 65F		

TEST PIT LOG					
EXCAVATING COMPANY:	One Key LLC	LOCATION:	30 ft N from NW corner of proposed building	TEST PIT NUMBER	TP2
OPERATOR:	Garrett	ELEVATION:	205+/-		
EQUIPMENT:	Takeuchi TB180FR Mini-Excavator	WATER DEPTH:			
INSPECTOR:	Kevin Patton				

Depth	USCS Class	Moisture	Description	Notes
0-6"	ML	Slightly moist.	Loose. Grayish brown silt loam with some small roots.	
6-24"	ML	Moist	Medium-dense. Pale orangeish brown silt with some sand, traces gravel, few cobbles.	
24-62"	ML	Moist	Dense. Slightly mottled olive brown silt with some sand, little gravel and few cobbles.	
62"-72"	Rock	-	Top of shale bedrock. Beds nearly vertical, strike northeast. Ripped 12 inches into rock, which was still somewhat soft at this depth.	
72"	-	-	Stopped in rock at 6 feet.	



KEVIN L. PATTON, P.E. 36 PATTON ROAD NEWBURGH, NY 12550 PATTONGEOTECH.COM 845 275-7732	CLIENT:	One Key LLC		
	PROJECT:	Harrington Heights, Poughkeepsie, N.Y.		
	DATE:	6/14/2023	Project No.:	23216
	WEATHER:	Cloudy, 65F		

TEST PIT LOG				
EXCAVATING COMPANY:	One Key LLC	LOCATION:	50 ft N from SW corner of proposed building	TEST PIT NUMBER TP3
OPERATOR:	Garrett	ELEVATION:	214+/-	
EQUIPMENT:	Takeuchi TB180FR Mini-Excavator	WATER DEPTH:		
INSPECTOR:	Kevin Patton			

Depth	USCS Class	Moisture	Description	Notes
0-4"	ML	Moist	Loose. Topsoil. Brown silt loam with fine roots.	
4"-22"	ML	Moist	Medium-dense. Pale orangeish brown silt with some fine-graded sand, trace gravel and cobbles.	
22"-48"	ML	Moist	Dense. Pale orangeish brown silt and fine-graded sand, trace gravel and cobbles, with few small boulders.	
48"-6.5ft	ML	Moist	Very dense. Till. Mottled olive brown silt with little sand, little gravel, few cobbles and boulders.	
6.5-7.5 ft	Rock	-	Top of shale bedrock. Ripped 12 inches into rock with ease.	
7.5 ft	-	-	Stopped in rock.	



KEVIN L. PATTON, P.E. 36 PATTON ROAD NEWBURGH, NY 12550 PATTONGEOTECH.COM 845 275-7732	CLIENT:	One Key LLC		
	PROJECT:	Harrington Heights, Poughkeepsie, N.Y.		
	DATE:	6/14/2023	Project No.:	23216
	WEATHER:	Cloudy, 65F		

TEST PIT LOG				
EXCAVATING COMPANY:	One Key LLC	LOCATION:	North wing of proposed building, S from center	TEST PIT NUMBER TP4
OPERATOR:	Garrett	ELEVATION:	222+/-	
EQUIPMENT:	Takeuchi TB180FR Mini-Excavator	WATER DEPTH:		
INSPECTOR:	Kevin Patton			

Depth	USCS Class	Moisture	Description	Notes
0-18"	ML	Moist	Loose. Fill. Silt with some sand, trace gravel and cobbles, few small boulders. Buried pipes (+/- 1.5" diameter steel, 2" lead.)	
18"-48"	ML	Moist	Dense. Till. Olive brown silt with little sand, little gravel, few cobbles and small boulders.	
48"-7.5ft	ML	Moist	Same, Very dense.	
7.5ft - 8ft	Rock	-	Shale bedrock. Ripped six inches into the surface, with effort.	
8 ft	-	-	Stopped in rock.	



KEVIN L. PATTON, P.E. 36 PATTON ROAD NEWBURGH, NY 12550 PATTONGEOTECH.COM 845 275-7732	CLIENT:	One Key LLC		
	PROJECT:	Harrington Heights, Poughkeepsie, N.Y.		
	DATE:	6/14/2023	Project No.:	23216
	WEATHER:	Cloudy, 65F		

TEST PIT LOG				
EXCAVATING COMPANY:	One Key LLC	LOCATION:	Inside corner of proposed building	TEST PIT NUMBER TP5
OPERATOR:	Garrett	ELEVATION:	222+/-	
EQUIPMENT:	Takeuchi TB180FR Mini-Excavator	WATER DEPTH:		
INSPECTOR:	Kevin Patton			

Depth	USCS Class	Moisture	Description	Notes
0-40"	ML	Moist	Loose. Fill. Grayish brown silt with some sand, little gravel, few cobbles, bricks and shale fragments.	
40-50"	Bricks	Moist	Loose. Fill. Mostly bricks with little soil mixed in.	
50-60"	Rock	-	Shale bedrock. Ripped to 12", becoming hard.	
60"	-	-	Stopped in rock.	



KEVIN L. PATTON, P.E. 36 PATTON ROAD NEWBURGH, NY 12550 PATTONGEOTECH.COM 845 275-7732	CLIENT:	One Key LLC		
	PROJECT:	Harrington Heights, Poughkeepsie, N.Y.		
	DATE:	6/14/2023	Project No.:	23216
	WEATHER:	Cloudy, 65F		

TEST PIT LOG				
EXCAVATING COMPANY:	One Key LLC	LOCATION:	East wing of proposed building, near east end	TEST PIT NUMBER TP6
OPERATOR:	Garrett	ELEVATION:	209+/-	
EQUIPMENT:	Takeuchi TB180FR Mini-Excavator	WATER DEPTH:		
INSPECTOR:	Kevin Patton			

Depth	USCS Class	Moisture	Description	Notes
0-10"	ML	Moist	Loose. Grayish brown silt with little sand, trace gravel, few cobbles. Few roots.	
10"-36"	ML	Moist	Medium-dense. Pale orangeish brown silt with little sand, traces gravel, few cobbles, few small boulders.	
36"-58"	ML	Moist	Dense. Pale yellowish brown silt with little sand, trace gravel, few cobbles.	
58"	Rock	-	Top of bedrock. Flat, hard.	
58"	-	-	Stopped on top of rock.	



KEVIN L. PATTON, P.E. 36 PATTON ROAD NEWBURGH, NY 12550 PATTONGEOTECH.COM 845 275-7732	CLIENT: One Key LLC	
	PROJECT: Harrington Heights, Poughkeepsie, N.Y.	
	DATE: 6/14/2023	Project No.: 23216
	WEATHER: Cloudy, 65F	

TEST PIT LOG				
EXCAVATING COMPANY:	One Key LLC	LOCATION:	Proposed stormwater control area, center	TEST PIT NUMBER TP7
OPERATOR:	Garrett			
EQUIPMENT:	Takeuchi TB180FR Mini-Excavator	ELEVATION:	197.5+/-	
INSPECTOR:	Kevin Patton	WATER DEPTH:		

Depth	USCS Class	Moisture	Description	Notes
0-2"	PT	Moist	Decomposed organics and leaf-litter. Dusky brown.	
2"-10"	SM	Moist	Loose. Fill. Brownish gray sand with some silt, little gravel, trace glass, small pocket of coal cinders.	
10"-32"	CL	Moist	Native Soil. Medium-dense. Pale yellowish brown, mottled orange. Lean Clay with some fine-graded sand, trace gravel. Massive structure, no significant joints or pores.	
32"-6ft	CL	Wet	Dense. Mottled olive brown-orange-gray Lean Clay with trace gravel, traces sand, trace roots. Dense.	
6 ft	-	-	Stopped in soil.	

No photo available.

KEVIN L. PATTON, P.E. 36 PATTON ROAD NEWBURGH, NY 12550 PATTONGEOTECH.COM 845 275-7732	CLIENT:	One Key LLC		
	PROJECT:	Harrington Heights, Poughkeepsie, N.Y.		
	DATE:	6/14/2023	Project No.:	23216
	WEATHER:	Cloudy, 65F		

TEST PIT LOG				
EXCAVATING COMPANY:	One Key LLC	LOCATION:	Proposed stormwater control area, north end	TEST PIT NUMBER TP8
OPERATOR:	Garrett	ELEVATION:	197+/-	
EQUIPMENT:	Takeuchi TB180FR Mini-Excavator	WATER DEPTH:		
INSPECTOR:	Kevin Patton			

Depth	USCS Class	Moisture	Description	Notes
0-30"	ML	Moist	Loose. Fill. Dusky brown silt and sand with little gravel, trace concrete, two streetcar rails.	
30"-60"	ML	Very Moist	Native soil. Loose to Medium-dense. Mottled olive brown and gray silt and fine sand. Massive structure. Trace concretions (cemented sand,) 3/8" size, noted in lab sample.	
60"-8.5ft	ML	Wet	Same, wet.	
8.5 ft	-	-	Stopped in soil.	



GENERALIZED SUBSURFACE PROFILE

NO HORIZONTAL SCALE.
USCS SOIL CLASSIFICATIONS ARE IN BRACKETS.

IN GENERAL, RED PATTERNS INDICATE RELATIVELY CLEAN SANDY OR GRAVELLY SOILS,
PURPLE PATTERNS INDICATE SOILS WITH SIGNIFICANT CLAY CONTENT AND ORANGE
PATTERNS INDICATE SOILS WITH A SIGNIFICANT SILT CONTENT.

THESE SECTIONS ARE GENERALIZED REPRESENTATIONS OF THE SUBSURFACE PROFILE,
BASED ON THE SUBSURFACE EXPLORATION DATA, OBSERVATIONS, RESEARCH, AND OTHER
RELEVANT INFORMATION. THE SOILS INFORMATION PRESENTED HEREIN SHOULD BE
INTERPRETED IN CONJUNCTION WITH THE INFORMATION FROM THE TEST PIT LOGS AND
THE GEOTECHNICAL INVESTIGATION REPORT. SITE CONDITIONS MAY DIFFER FROM THOSE
ENCOUNTERED AT THE TEST PIT LOCATIONS.



			HARRINGTON HEIGHTS	KEVIN L. PATTON, P.E. 36 PATTON ROAD NEWBURGH, NY 12550 845 275-7732 PATTONGEOTECH.COM
			100 SOUTH HAMILTON ST., POUGHKEEPSIE, N.Y.	
			SUBSURFACE PROFILE	
0	7/5/2023	KLP		
REV.	DATE	BY		

KEVIN L. PATTON, P.E.
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CLIENT:	One Key LLC		
PROJECT:	Harrington Heights, South Hamilton St., Poughkeepsie		
PROJECT No.:	23216	SAMPLE LOT No.:	230614-1
DATE SAMPLED:	6/14/2023	DATE TESTED:	6/20/2023
SAMPLED BY:	Kevin Patton	TESTED BY:	Wyeth Patton

MOISTURE CONTENT OF SOIL
TEST METHOD: ASTM D2216

Location	DEPTH, FT.	% MOISTURE
TP1	8'	13.6
TP1	11'	14.4
TP2	3'	14.1
TP3	3'	13.7
TP4	4'	13.6
TP7	2'	16.5
TP7	5'	13.5
TP8	4'	20.6

Moisture content is expressed as a percent of the dry mass of the soil.

Reviewed by: *Kevin Patton*

Form NMC

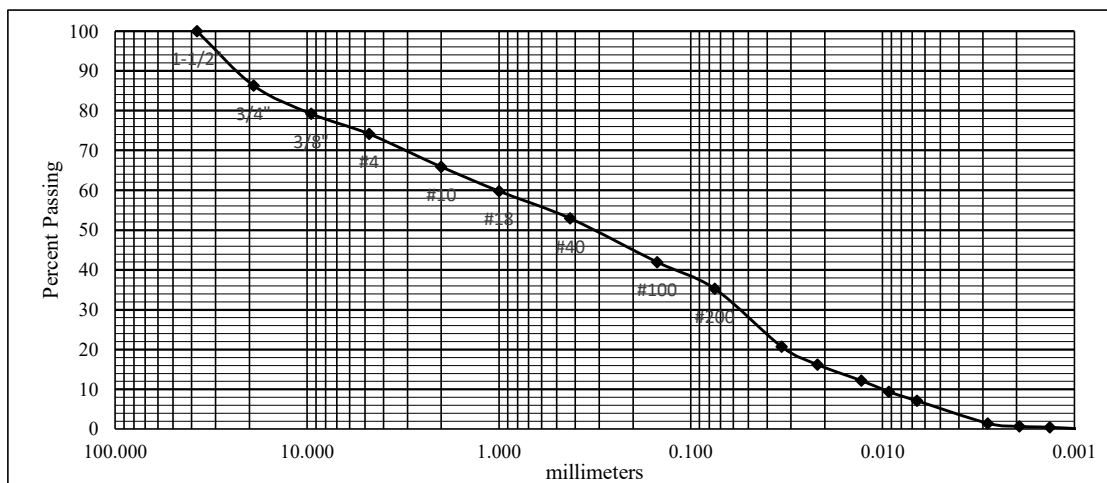
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CLIENT:	One Key LLC		
PROJECT:	Harrington Heights, South Hamilton St., Poughkeepsie		
PROJECT No.:	23216	SAMPLE LOT No.:	230614-1
DATE SAMPLED:	6/14/2023	DATE TESTED:	6/20/2023
SAMPLED BY:	Kevin Patton	TESTED BY:	Wyeth Patton

SIEVE-AND-HYDROMETER ANALYSIS TEST REPORT
TEST METHOD(S): ASTM D422, AASHTO T88

Sample Location	TP1
Depth	8 feet

Sieve Size		Percent Retained	Percent Passing	Specification
inches	mm			
1-1/2"	37.5	0	100	
3/4"	19.0	14	86	
3/8"	9.5	7	79	
#4	4.75	5	74	
#10	2.00	8	66	
#18	1.00	6	60	
#40	0.425	7	53	
#100	0.150	11	42	
#200	0.075	7	35	
Hydrometer Analysis	0.050	7	28	
	0.020	13	15	
	0.010	5	10	
	0.005	5	5	
	0.002	4	1	
	0.001	1	0	



USDA Particle Size Classification:	USDA Textural Class: Gravelly Sandy Loam
Gravel, 3" to 2.00mm: 34	USCS Classification (ASTM D2487/D2488): SC-SM, Silty Clayey Sand with Gravel
Sand, 2.00 to 0.050mm: 38	
Silt, 0.050 to 0.002mm: 27	Atterberg Limits were determined by: Test
Clay, <0.002mm: 1	
Total: 100	

Reviewed by: Kevin Patton

Form HYD

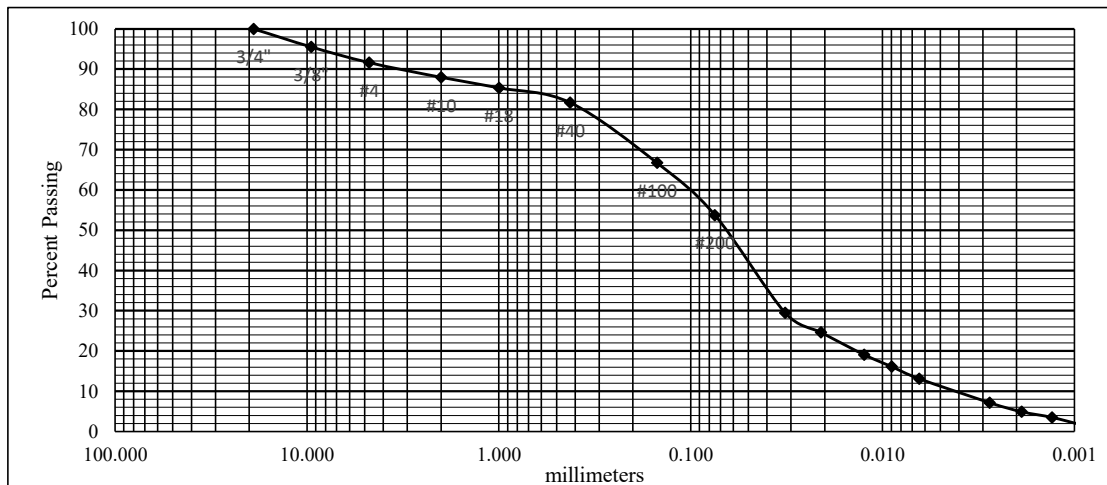
KEVIN L. PATTON, P.E.
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CLIENT:	One Key LLC		
PROJECT:	Harrington Heights, South Hamilton St., Poughkeepsie		
PROJECT No.:	23216	SAMPLE LOT No.:	230614-1
DATE SAMPLED:	6/14/2023	DATE TESTED:	6/20/2023
SAMPLED BY:	Kevin Patton	TESTED BY:	Wyeth Patton

SIEVE-AND-HYDROMETER ANALYSIS TEST REPORT
TEST METHOD(S): ASTM D422, AASHTO T88

Sample Location	TP3
Depth	3 feet

Sieve Size		Percent Retained	Percent Passing	Specification
inches	mm			
3/4"	19.0	0	100	
3/8"	9.5	5	95	
#4	4.75	3	92	
#10	2.00	4	88	
#18	1.00	3	85	
#40	0.425	3	82	
#100	0.150	15	67	
#200	0.075	13	54	
Hydrometer Analysis	0.050	12	42	
	0.020	18	24	
	0.010	7	17	
	0.005	6	11	
	0.002	6	5	
	0.001	3	2	



USDA Particle Size Classification:		USDA Textural Class:	Sandy Loam
Gravel, 3" to 2.00mm	12	USCS Classification (ASTM D2487/D2488): ML, Sandy Silt	
Sand, 2.00 to 0.050mm:	46		
Silt, 0.050 to 0.002mm:	37		
Clay, <0.002mm	5		
Total	100		
		Atterberg Limits were determined by: Estimated (ASTM D2488)	

Reviewed by: Kevin Patton

Form HYD

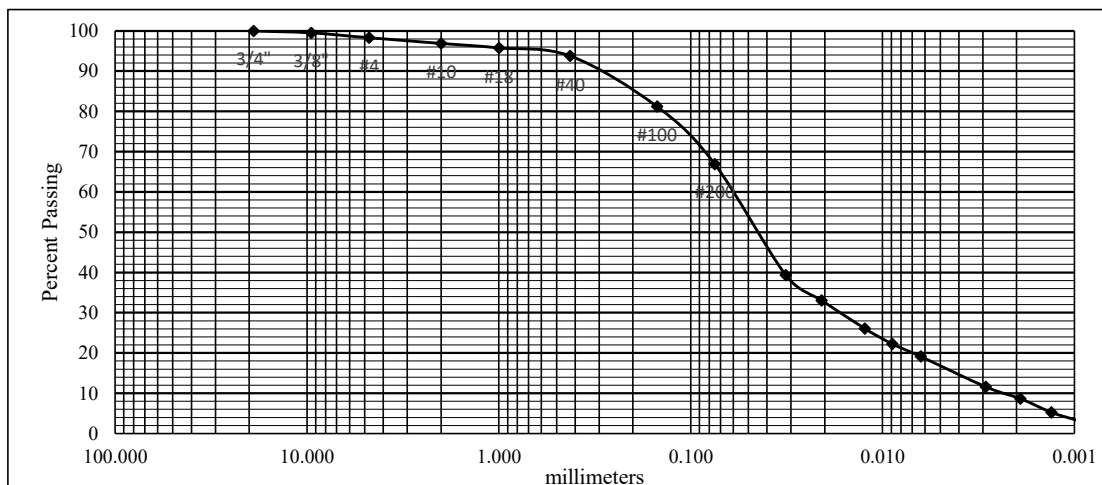
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CLIENT:	One Key LLC		
PROJECT:	Harrington Heights, South Hamilton St., Poughkeepsie		
PROJECT No.:	23216	SAMPLE LOT No.:	230614-1
DATE SAMPLED:	6/14/2023	DATE TESTED:	6/20/2023
SAMPLED BY:	Kevin Patton	TESTED BY:	Wyeth Patton

SIEVE-AND-HYDROMETER ANALYSIS TEST REPORT
TEST METHOD(S): ASTM D422, AASHTO T88

Sample Location	TP7
Depth	2 feet

Sieve Size		Percent Retained	Percent Passing	Specification
inches	mm			
3/4"	19.0	0	100	
3/8"	9.5	1	99	
#4	4.75	1	98	
#10	2.00	1	97	
#18	1.00	1	96	
#40	0.425	2	94	
#100	0.150	13	81	
#200	0.075	14	67	
Hydrometer Analysis	0.050	13	54	
	0.020	22	32	
	0.010	9	23	
	0.005	6	17	
	0.002	8	9	
	0.001	6	3	



USDA Particle Size Classification:	USDA Textural Class: Loam
Gravel, 3" to 2.00mm: 3	USCS Classification (ASTM D2487/D2488): CL, Sandy Clay
Sand, 2.00 to 0.050mm: 43	
Silt, 0.050 to 0.002mm: 45	
Clay, <0.002mm: 9	
Total: 100	Atterberg Limits were determined by: Test

Reviewed by: Kevin Patton

Form HYD

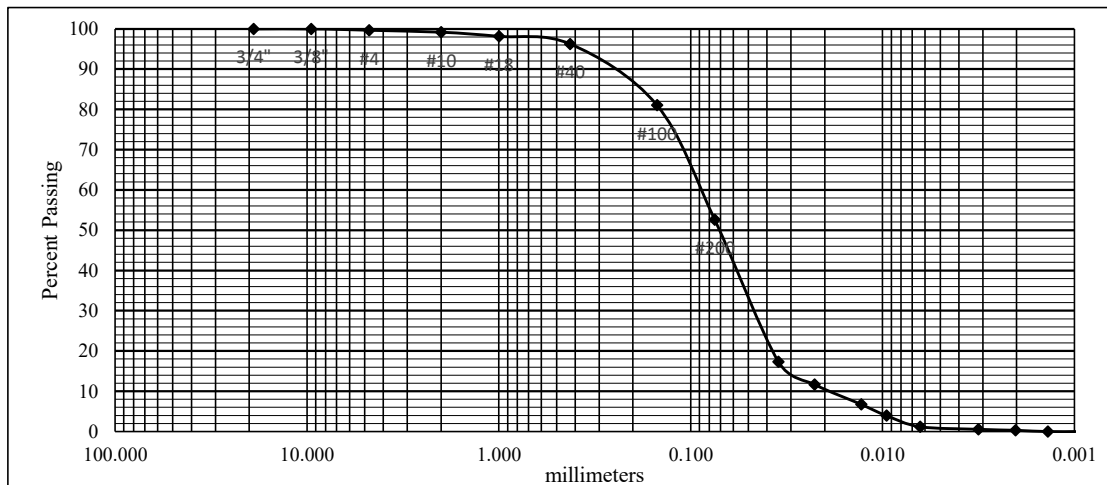
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CLIENT:	One Key LLC		
PROJECT:	Harrington Heights, South Hamilton St., Poughkeepsie		
PROJECT No.:	23216	SAMPLE LOT No.:	230614-1
DATE SAMPLED:	6/14/2023	DATE TESTED:	6/20/2023
SAMPLED BY:	Kevin Patton	TESTED BY:	Wyeth Patton

SIEVE-AND-HYDROMETER ANALYSIS TEST REPORT
TEST METHOD(s): ASTM D422, AASHTO T88

Sample Location	TP8
Depth	4 feet

Sieve Size		Percent Retained	Percent Passing	Specification
inches	mm			
3/4"	19.0	0	100	
3/8"	9.5	0	100	
#4	4.75	0	100	
#10	2.00	1	99	
#18	1.00	1	98	
#40	0.425	2	96	
#100	0.150	15	81	
#200	0.075	28	53	
Hydrometer Analysis	0.050	20	33	
	0.020	23	10	
	0.010	6	4	
	0.005	3	1	
	0.002	1	0	
	0.001	0	0	



USDA Particle Size Classification:	USDA Textural Class: Sandy Loam
Gravel, 3" to 2.00mm: 1	USCS Classification (ASTM D2487/D2488): ML, Sandy Silt
Sand, 2.00 to 0.050mm: 66	
Silt, 0.050 to 0.002mm: 33	
Clay, <0.002mm: 0	
Total: 100	Atterberg Limits were determined by: Estimated (ASTM D2488)

Reviewed by: Kevin Patton

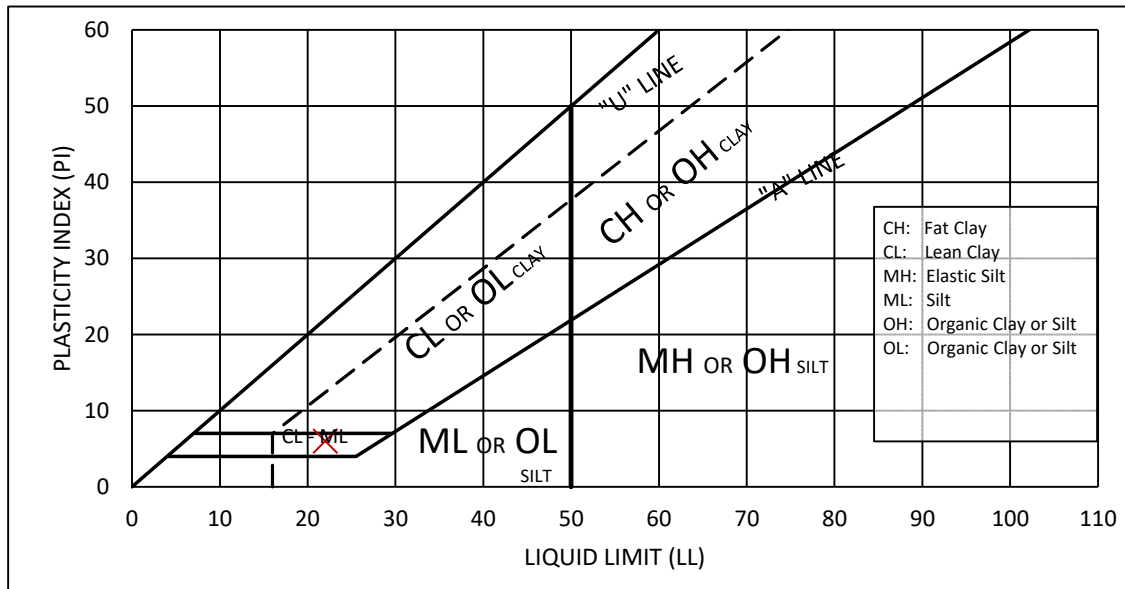
Form HYD

KEVIN L. PATTON, P.E.
36 PATTON ROAD
NEWBURGH, NY 12550
845 275-7732 PATTONGEOTECH.COM

CLIENT:	One Key LLC		
PROJECT:	Harrington Heights, South Hamilton St., Poughkeepsie		
PROJECT No.:	23216	SAMPLE LOT No.:	230614-1
DATE SAMPLED:	6/14/2023	DATE TESTED:	6/20/2023
SAMPLED BY:	Kevin Patton	TESTED BY:	Wyeth Patton

ATTERBERG LIMITS TEST
TEST METHODS: ASTM D4318/ AASHTO T89, T90

Sample Location	TP1
Depth	8 feet
Percent Passing #40	53
Liquid Limit (LL)	22
Plastic Limit (PL)	16
Plasticity Index (PI)	6
USCS Class of -#40	CL-ML, Silty Clay



LL, PL and PI values are percent moisture of the soil by dry mass.

Test is performed on the 'matrix' fraction of the soil, finer than the #40 (0.425mm) sieve.

The Liquid Limit is the moisture content at which the matrix fraction of the soil changes from a stiff to a flowing consistency. The plastic limit is the moisture content at which it changes from cohesive to crumbly. The Plasticity Index is the Liquid Limit minus the Plastic Limit.

Reviewed by: *Kevin Patton*

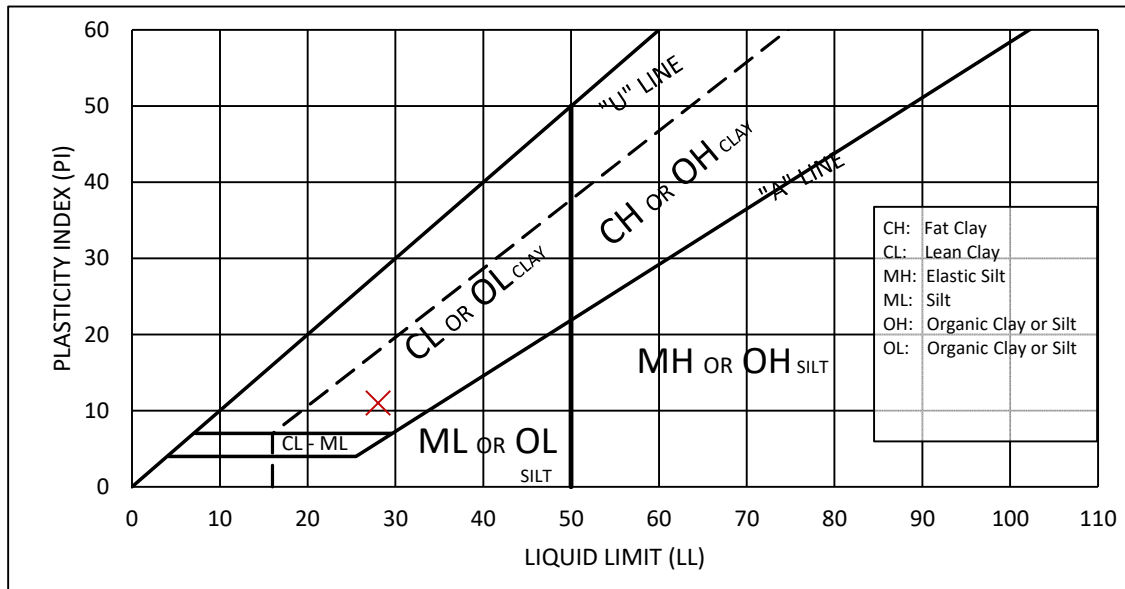
Form ABL

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CLIENT:	One Key LLC		
PROJECT:	Harrington Heights, South Hamilton St., Poughkeepsie		
PROJECT No.:	23216	SAMPLE LOT No.:	230614-1
DATE SAMPLED:	6/14/2023	DATE TESTED:	6/20/2023
SAMPLED BY:	Kevin Patton	TESTED BY:	Wyeth Patton

ATTERBERG LIMITS TEST
TEST METHODS: ASTM D4318/ AASHTO T89, T90

Sample Location	TP7
Depth	2 feet
Percent Passing #40	94
Liquid Limit (LL)	28
Plastic Limit (PL)	17
Plasticity Index (PI)	11
USCS Class of -#40	CL, Lean Clay



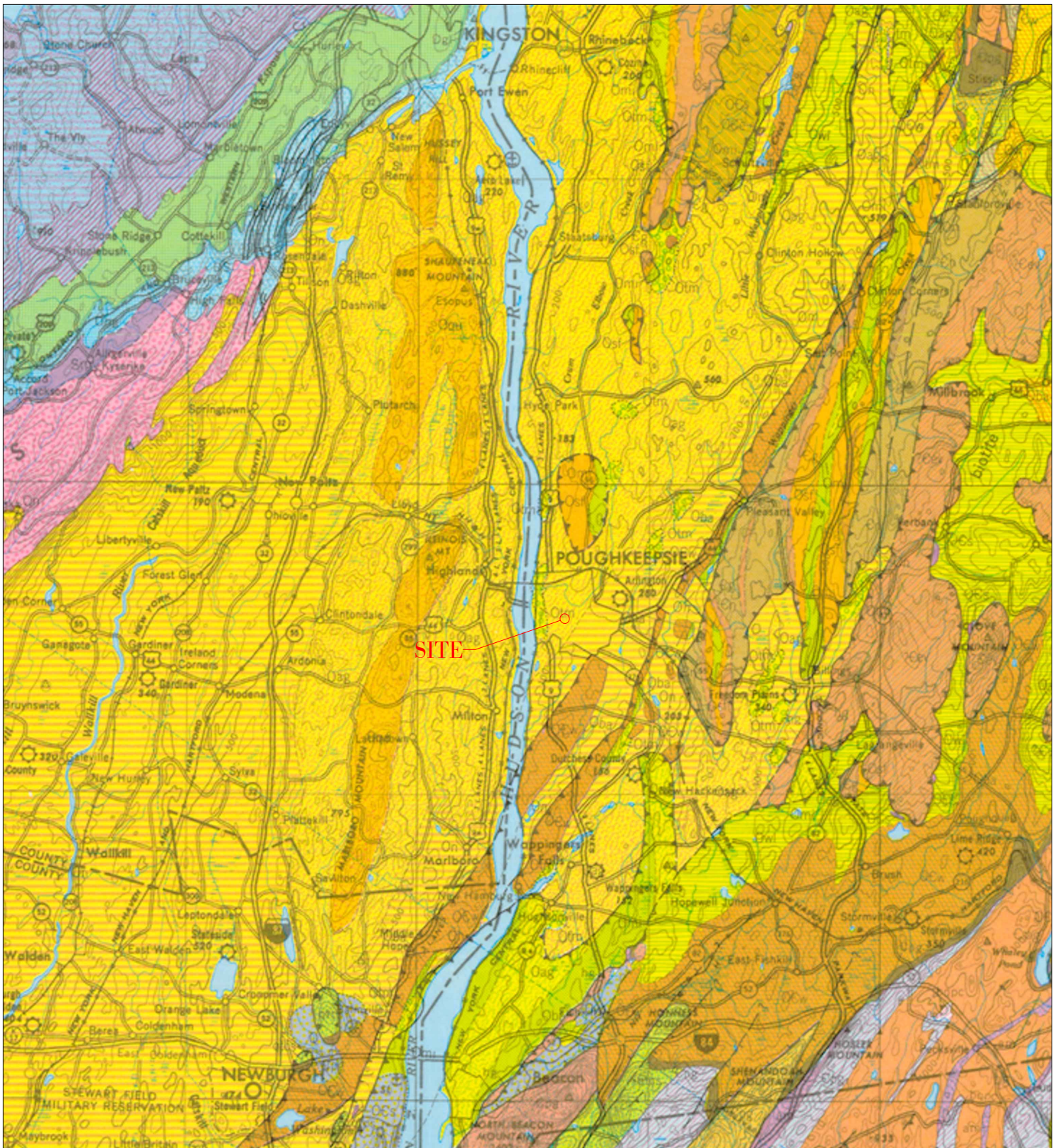
LL, PL and PI values are percent moisture of the soil by dry mass.

Test is performed on the 'matrix' fraction of the soil, finer than the #40 (0.425mm) sieve.

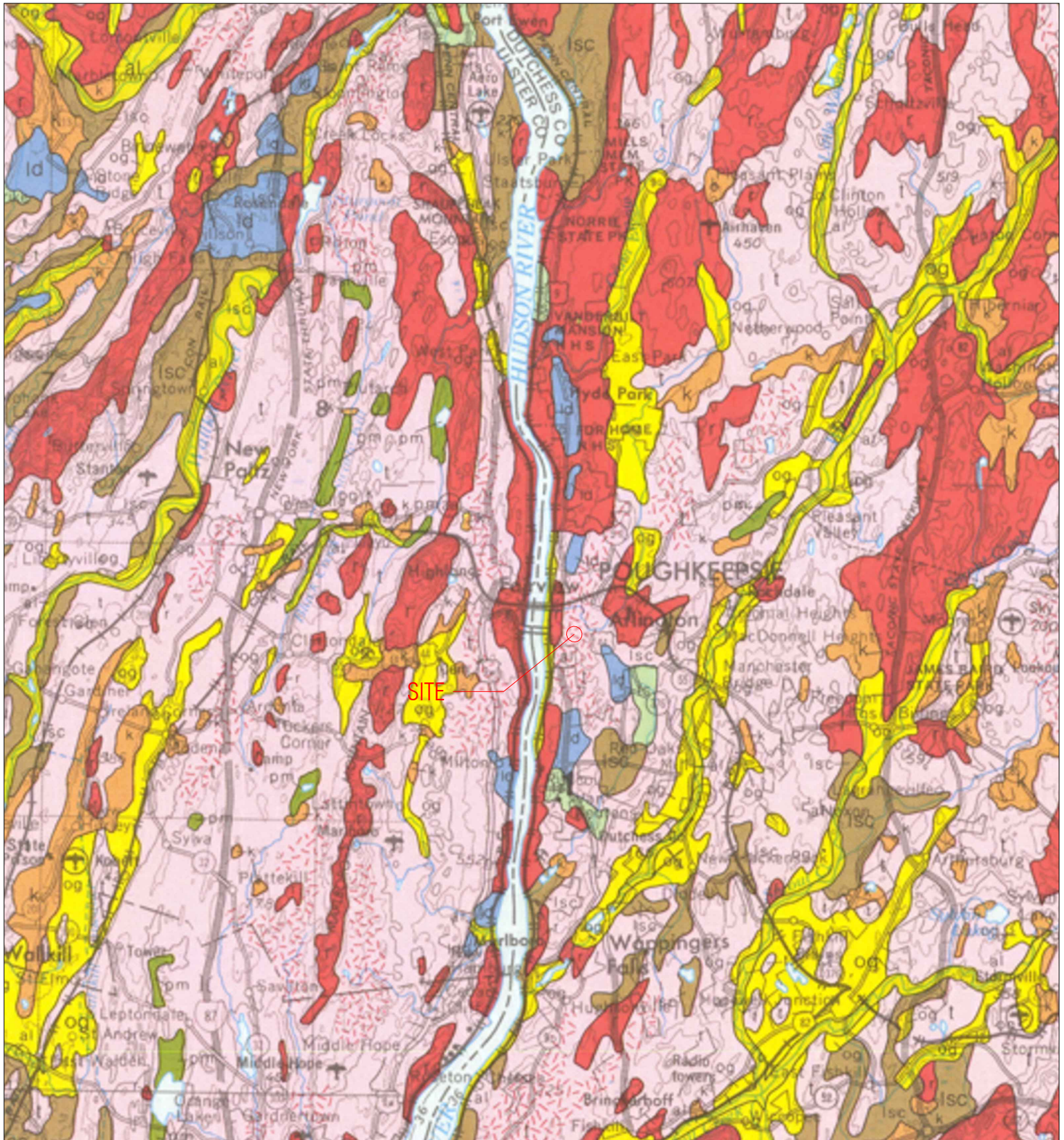
The Liquid Limit is the moisture content at which the matrix fraction of the soil changes from a stiff to a flowing consistency. The plastic limit is the moisture content at which it changes from cohesive to crumbly. The Plasticity Index is the Liquid Limit minus the Plastic Limit.

Reviewed by: *Kevin Patton*

Form ABL



Approximate location of the project site, shown on part of the Bedrock Geologic Map of New York (1970.) The site is in an area of Ordovician-age sedimentary bedrock consisting mostly of gray siltstone, shale and greywacke sandstone. The bedrock at the site is shown as Austin Glen Formation (yellow, symbol Oag,) which is generally bounded to the east and west by the somewhat older Normanskill shale (yellow with red stripes, On,) and is overlain by the younger Quassaic quartzite in the high areas (orange, Oqu.) Taconic melange (green, Otm) is indicated along the Hudson River in Poughkeepsie, to the west of the site; it is composed of slaty, sheared Austin Glen bedrock. To the southeast, the older Wappinger dolostone (brown, OCw,) is down-faulted against the Normanskill Formation bedrock.



Approximate location of the project site, shown on a partial copy of the Surficial Geologic Map of New York (N.Y. State Museum, 1989.) The map indicates that the soils at and near the site are glacial till, shown in pink with the symbol 't.' A red stipple pattern is also shown, indicating that bedrock is generally within three to ten feet of the surface. Areas where bedrock is mostly very shallow or is exposed are indicated by red. The map units are generalized; alluvium, outwash and lacustrine deposits have all been encountered in parts of the City of Poughkeepsie where till is indicated by the map.

EXPLANATION

	al — Recent deposits Generally confined to floodplains within a valley, oxidized, non-calcareous, fine sand to gravel, in larger valleys may be overlain by silt, subject to frequent flooding, thickness 1-10 meters.		fg — Fluvial sand and gravel Deposits of sand and gravel, occasional laterally continuous lenses of silt, deposition farther from glacier, age uncertain.
	alf — Alluvial fan Fan shaped accumulations, poorly stratified silt, sand and boulders, at the foot of steep slopes, generally permeable.		k — Kame deposits Includes kames, eskers, kame terraces, kame deltas, coarse to fine gravel and/or sand, deposition adjacent to ice, lateral variability in sorting, coarseness and thickness, locally firmly cemented with calcareous cement, thickness variable (10-30 meters).
	b — Beach Sand and gravel deposit at marine shoreline, thickness variable.		km — Kame moraine Variable texture (size and sorting) from boulders to sand, deposition at an ice margin during deglaciation, positive constructional relief, locally cemented with calcareous cement, thickness variable (10-30 meters).
	bi — Barrier island Sand and gravel deposit as barrier island, south shore of Long Island, may have associated dunes, thickness variable.		tm — Till moraine More variably sorted than till, generally more permeable than till, deposition adjacent to ice, more variably drained, may include ablation till, thickness variable (10-30 meters).
	pm — Swamp deposits Peat-muck, organic silt and sand in poorly drained areas, un-oxidized, may be overlying marl and lake silts, potential land instability, thickness generally 2-20 meters.		t — Till Variable texture (e.g. clay, silt-clay, boulder clay), usually poorly sorted diamict, deposition beneath glacier ice, relatively impermeable (loamy matrix), variable clast content — ranging from abundant well-rounded diverse lithologies in valley tills to relatively angular, more limited lithologies in upland tills, tends to be sandy in areas underlain by gneiss or sandstone, potential land instability on steep slopes, thickness variable (1-50 meters).
	ld — Lacustrine delta Coarse to fine gravel and sand, stratified, generally well sorted, deposited at a lake shoreline, thickness variable (3-15 meters).		af — Artificial fill
	lsc — Lacustrine silt and clay Generally laminated silt and clay, deposited in proglacial lakes, generally calcareous, potential land instability, thickness variable (up to 100 meters).		r — Bedrock Exposed or generally within 1 meter of surface.
	ls — Lacustrine sand Sand deposits associated with large bodies of water, generally a near-shore deposit or near a sand source, well sorted, stratified, generally quartz sand, thickness variable (2-20 meters).		Bedrock stipple overprint Bedrock may be within 1-3 meters of surface, may sporadically crop out, variable mantle of rock debris and glacial till.
	og — Outwash sand and gravel Coarse to fine gravel with sand, proglacial fluvial deposition, well rounded and stratified, generally finer texture away from ice border, thickness variable (2-20 meters).		

MAP SYMBOLS



Contact

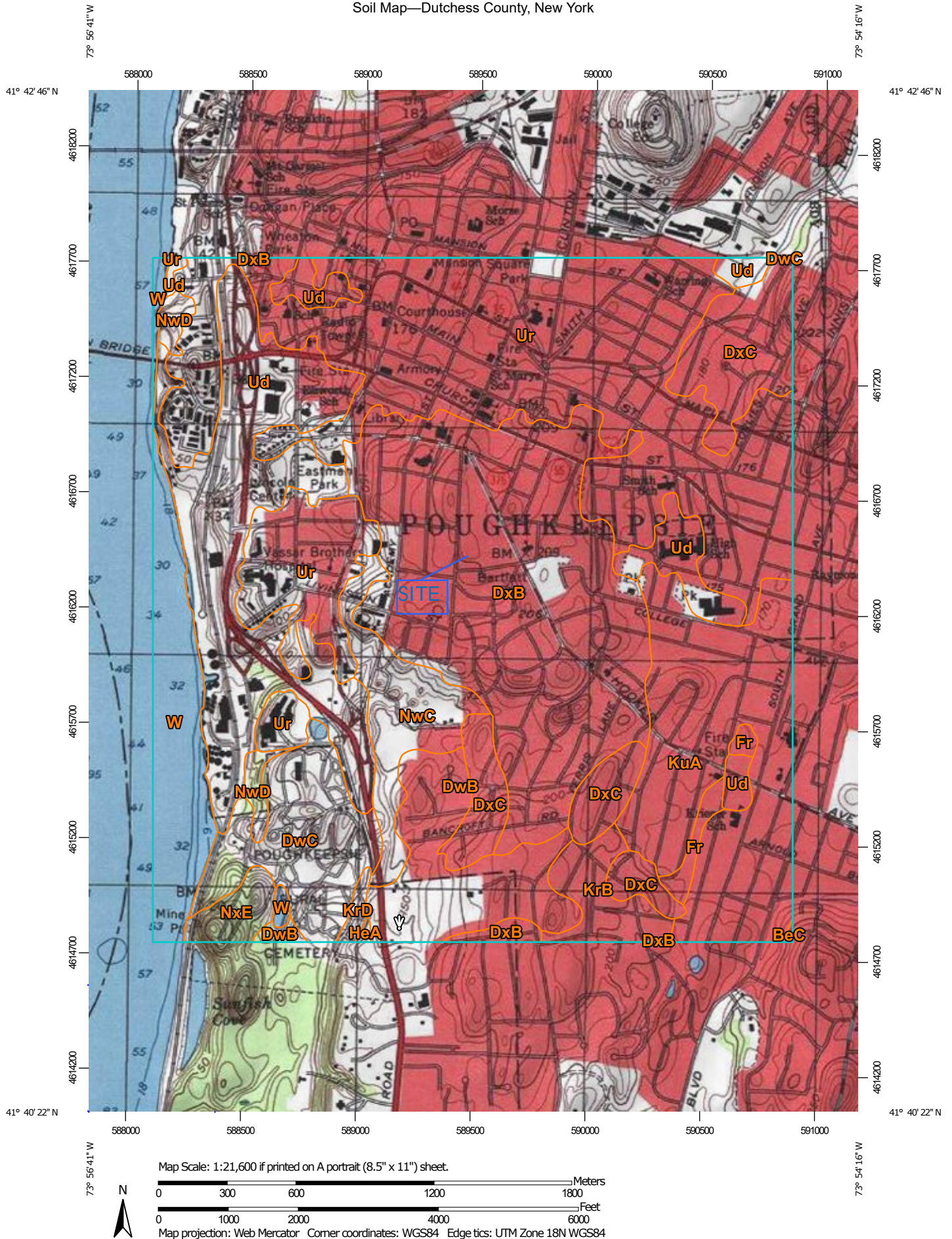
.3

Dated radiocarbon locality

MAP KEY NEW YORK STATE SURFICIAL GEOLOGIC MAP

KEVIN L. PATTON, P.E.

Soil Map—Dutchess County, New York



Natural Resources
Conservation Service

Web Soil Survey
National Cooperative Soil Survey

4/21/2023
Page 1 of 3
Page 333 of 752

MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features



Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow



Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot



Spoil Area



Stony Spot



Very Stony Spot



Wet Spot



Other



Special Line Features

Background



Topographic Map

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Dutchess County, New York

Survey Area Data: Version 19, Sep 10, 2022

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
BeC	Bernardston silt loam, 8 to 15 percent slopes	0.6	0.0%
DwB	Dutchess-Cardigan complex, undulating, rocky	102.3	5.0%
DwC	Dutchess-Cardigan complex, rolling, rocky	96.6	4.7%
DxB	Dutchess-Cardigan-Urban land complex, undulating, rocky	399.6	19.5%
DxC	Dutchess-Cardigan-Urban land complex, rolling, rocky	123.4	6.0%
Fr	Fredon silt loam	12.3	0.6%
HeA	Haven loam, nearly level	1.0	0.1%
KrB	Knickerbocker fine sandy loam, undulating	36.0	1.8%
KrD	Knickerbocker fine sandy loam, hilly	5.2	0.3%
KuA	Knickerbocker-Urban land complex, nearly level	214.3	10.5%
NwC	Nassau-Cardigan complex, rolling, very rocky	68.8	3.4%
NwD	Nassau-Cardigan complex, hilly, very rocky	14.7	0.7%
NxE	Nassau-Rock outcrop complex, steep	19.6	1.0%
Ud	Udorthents, smoothed	286.3	14.0%
Ur	Urban land	565.5	27.6%
W	Water	104.2	5.1%
Totals for Area of Interest		2,050.6	100.0%

Dutchess County, New York

DxB—Dutchess-Cardigan-Urban land complex, undulating, rocky

Map Unit Setting

National map unit symbol: 9rfr

Elevation: 20 to 1,000 feet

Mean annual precipitation: 41 to 47 inches

Mean annual air temperature: 45 to 50 degrees F

Frost-free period: 115 to 195 days

Farmland classification: Not prime farmland

Map Unit Composition

Dutchess and similar soils: 30 percent

Cardigan and similar soils: 25 percent

Urban land: 20 percent

Minor components: 25 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Dutchess

Setting

Landform: Ridges, hills

Landform position (two-dimensional): Summit

Landform position (three-dimensional): Crest

Down-slope shape: Convex

Across-slope shape: Convex

Parent material: Loamy till derived mainly from phyllite, slate, schist, and shale

Typical profile

H1 - 0 to 8 inches: silt loam

H2 - 8 to 28 inches: silt loam

H3 - 28 to 86 inches: channery silt loam

Properties and qualities

Slope: 1 to 6 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Capacity of the most limiting layer to transmit water

(Ksat): Moderately high to high (0.57 to 1.98 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Available water supply, 0 to 60 inches: High (about 9.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: B

Ecological site: F144AY034CT - Well Drained Till Uplands
Hydric soil rating: No

Description of Cardigan

Setting

Landform: Ridges, hills
Landform position (two-dimensional): Summit
Landform position (three-dimensional): Crest
Down-slope shape: Convex
Across-slope shape: Convex
Parent material: Loamy till or colluvium derived from phyllite, slate, shale, and schist

Typical profile

H1 - 0 to 8 inches: channery silt loam
H2 - 8 to 20 inches: channery loam
H3 - 20 to 30 inches: channery silt loam
H4 - 30 to 34 inches: unweathered bedrock

Properties and qualities

Slope: 1 to 6 percent
Depth to restrictive feature: 20 to 40 inches to lithic bedrock
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Low to moderately low (0.00 to 0.06 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Low (about 4.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 2e
Hydrologic Soil Group: C
Ecological site: F144AY034CT - Well Drained Till Uplands
Hydric soil rating: No

Description of Urban Land

Typical profile

H1 - 0 to 6 inches: variable

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 8s
Hydric soil rating: Unranked

Minor Components

Udorthents

Percent of map unit: 10 percent
Hydric soil rating: No

Georgia

Percent of map unit: 5 percent

Hydric soil rating: No

Nassau

Percent of map unit: 5 percent

Hydric soil rating: No

Massena

Percent of map unit: 4 percent

Hydric soil rating: No

Sun

Percent of map unit: 1 percent

Landform: Depressions

Hydric soil rating: Yes

Data Source Information

Soil Survey Area: Dutchess County, New York

Survey Area Data: Version 19, Sep 10, 2022

Engineering Properties

This table gives the engineering classifications and the range of engineering properties for the layers of each soil in the survey area.

Hydrologic soil group is a group of soils having similar runoff potential under similar storm and cover conditions. The criteria for determining Hydrologic soil group is found in the National Engineering Handbook, Chapter 7 issued May 2007(<http://directives.sc.egov.usda.gov/OpenNonWebContent.aspx?content=17757.wba>). Listing HSGs by soil map unit component and not by soil series is a new concept for the engineers. Past engineering references contained lists of HSGs by soil series. Soil series are continually being defined and redefined, and the list of soil series names changes so frequently as to make the task of maintaining a single national list virtually impossible. Therefore, the criteria is now used to calculate the HSG using the component soil properties and no such national series lists will be maintained. All such references are obsolete and their use should be discontinued. Soil properties that influence runoff potential are those that influence the minimum rate of infiltration for a bare soil after prolonged wetting and when not frozen. These properties are depth to a seasonal high water table, saturated hydraulic conductivity after prolonged wetting, and depth to a layer with a very slow water transmission rate. Changes in soil properties caused by land management or climate changes also cause the hydrologic soil group to change. The influence of ground cover is treated independently. There are four hydrologic soil groups, A, B, C, and D, and three dual groups, A/D, B/D, and C/D. In the dual groups, the first letter is for drained areas and the second letter is for undrained areas.

The four hydrologic soil groups are described in the following paragraphs:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

Depth to the upper and lower boundaries of each layer is indicated.

Texture is given in the standard terms used by the U.S. Department of Agriculture. These terms are defined according to percentages of sand, silt, and clay in the fraction of the soil that is less than 2 millimeters in diameter. "Loam," for example, is soil that is 7 to 27 percent clay, 28 to 50 percent silt, and less than 52 percent sand. If the content of particles coarser than sand is 15 percent or more, an appropriate modifier is added, for example, "gravelly."

Classification of the soils is determined according to the Unified soil classification system (ASTM, 2005) and the system adopted by the American Association of State Highway and Transportation Officials (AASHTO, 2004).

The Unified system classifies soils according to properties that affect their use as construction material. Soils are classified according to particle-size distribution of the fraction less than 3 inches in diameter and according to plasticity index, liquid limit, and organic matter content. Sandy and gravelly soils are identified as GW, GP, GM, GC, SW, SP, SM, and SC; silty and clayey soils as ML, CL, OL, MH, CH, and OH; and highly organic soils as PT. Soils exhibiting engineering properties of two groups can have a dual classification, for example, CL-ML.

The AASHTO system classifies soils according to those properties that affect roadway construction and maintenance. In this system, the fraction of a mineral soil that is less than 3 inches in diameter is classified in one of seven groups from A-1 through A-7 on the basis of particle-size distribution, liquid limit, and plasticity index. Soils in group A-1 are coarse grained and low in content of fines (silt and clay). At the other extreme, soils in group A-7 are fine grained. Highly organic soils are classified in group A-8 on the basis of visual inspection.

If laboratory data are available, the A-1, A-2, and A-7 groups are further classified as A-1-a, A-1-b, A-2-4, A-2-5, A-2-6, A-2-7, A-7-5, or A-7-6. As an additional refinement, the suitability of a soil as subgrade material can be indicated by a group index number. Group index numbers range from 0 for the best subgrade material to 20 or higher for the poorest.

Percentage of rock fragments larger than 10 inches in diameter and 3 to 10 inches in diameter are indicated as a percentage of the total soil on a dry-weight basis. The percentages are estimates determined mainly by converting volume percentage in the field to weight percentage. Three values are provided to identify the expected Low (L), Representative Value (R), and High (H).

Percentage (of soil particles) passing designated sieves is the percentage of the soil fraction less than 3 inches in diameter based on an oven-dry weight. The sieves, numbers 4, 10, 40, and 200 (USA Standard Series), have openings of 4.76, 2.00, 0.420, and 0.074 millimeters, respectively. Estimates are based on laboratory tests of soils sampled in the survey area and in nearby areas and on estimates made in the field. Three values are provided to identify the expected Low (L), Representative Value (R), and High (H).

Liquid limit and plasticity index (Atterberg limits) indicate the plasticity characteristics of a soil. The estimates are based on test data from the survey area or from nearby areas and on field examination. Three values are provided to identify the expected Low (L), Representative Value (R), and High (H).

References:

American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.

American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.

Report—Engineering Properties

Absence of an entry indicates that the data were not estimated. The asterisk "*" denotes the representative texture; other possible textures follow the dash. The criteria for determining the hydrologic soil group for individual soil components is found in the National Engineering Handbook, Chapter 7 issued May 2007(<http://directives.sc.egov.usda.gov/OpenNonWebContent.aspx?content=17757.wba>). Three values are provided to identify the expected Low (L), Representative Value (R), and High (H).



Engineering Properties--Dutchess County, New York														
Map unit symbol and soil name	Pct. of map unit	Hydrologic group	Depth	USDA texture	Classification		Pct Fragments		Percentage passing sieve number--				Liquid limit	Plasticity index
					Unified	AASHTO	>10 inches	3-10 inches	4	10	40	200		
			<i>In</i>				<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>
DxB--Dutchess-Cardigan-Urban land complex, undulating, rocky														
Dutchess	30	B	0-8	Silt loam	ML	A-4	0- 0- 0	0- 8- 15	95-98-100	90-93-95	80-88-95	55-70-85	15-24-33	NP-2 -4
			8-28	Silt loam, channery silt loam, gravelly loam	GM, ML, SM	A-4	0- 0- 0	0- 0- 20	70-92-95	65-90-95	55-80-95	40-70-85	15-24-33	NP-2 -4
			28-86	Channery silt loam, very channery fine sandy loam, very gravelly sandy loam	GM, ML, SM	A-1-b, A-4, A-2-4	0- 0- 0	5-15- 25	55-68-80	50-63-75	30-53-75	15-40-65	15-20-25	NP-2 -4
Cardigan	25	C	0-8	Channery silt loam	ML, SM	A-4	0- 0- 0	0- 3- 5	80-88-95	70-83-95	60-68-75	35-53-70	15-24-33	NP-3 -5
			8-20	Silt loam, channery silt loam, channery loam	ML, SM	A-4	0- 0- 0	0- 5- 10	80-88-95	70-83-95	60-68-75	35-53-70	15-24-33	NP-3 -5
			20-30	Silt loam, loam, channery silt loam	ML, SM	A-1, A-4, A-2	0- 0- 0	0- 5- 10	80-88-95	60-75-90	40-58-75	20-45-70	15-24-33	NP-3 -5
			30-34	Unweathered bedrock	—	—	0- 0- 0	0- 0- 0	—	—	—	—	—	—
Urban land	20		0-6	Variable	—	—	0- 0- 0	0- 0- 0	—	—	—	—	—	—

Data Source Information

Soil Survey Area: Dutchess County, New York
 Survey Area Data: Version 19, Sep 10, 2022



APPENDIX K



**New York State
DEPARTMENT OF ENVIRONMENTAL CONSERVATION**

Division of Water

Better Site Design

April 2008

**New York State
Department of Environmental Conservation**

Document Prepared by:

Horsley Witten Group
with assistance from the
Center for Watershed Protection

BETTER SITE DESIGN

1 INTRODUCTION

1.1 Purpose

The purpose of this document is to provide guidance to developers and designers to plan for and implement better site-design practices for new development and redevelopment projects. While reducing the effects of stormwater runoff may be achieved through both regulatory and non-regulatory techniques, this document focuses on the site-level planning and design tools available to the development community.

As research, technology and information transfer have improved over recent years, alternative approaches are being sought by the public and regulatory boards to reduce the effects of stormwater runoff from new development and redevelopment. Developers and designers also are seeking alternatives to expedite permitting processes, reduce construction costs, reduce long-term operation and maintenance costs and increase property values.

What is better site design, and how does it differ from conventional design? Better site design incorporates non-structural and natural approaches to new and redevelopment projects to reduce effects on watersheds by conserving natural areas, reducing impervious cover and better integrating stormwater treatment. For the purposes of this document, conventional design can be viewed as the style of suburban development that has evolved during the past 50 years and generally involves larger lot development, clearing and grading of significant portions of a site, wider streets and larger cul-de-sacs, enclosed drainage systems for stormwater conveyance and large “hole-in-the-ground” detention basins.

The aim of better site design is to reduce the environmental-impact “footprint” of the site while retaining and enhancing the owner/developer’s purpose and vision for the site. Many of the better site-design concepts employ non-structural on-site treatment that can reduce the cost of infrastructure while maintaining or even increasing the value of the property relative to conventional designed developments.

The goals of better site design include:

- Prevention of stormwater effects rather than having to mitigate for them
- Management of stormwater (quantity and quality) as close to the source as possible and minimization of the use of large or regional collection and conveyance
- Preservation of natural areas, native vegetation and reduction of the effect of on watershed hydrology
- Usage of natural drainage pathways as a framework for site design
- Utilization of simple, non-structural methods for stormwater management that are lower cost and lower maintenance than structural controls
- Creation of a multifunctional landscape

1.2 Scope and Context/How to Use this Document

The scope and context of this document is to present developers and site designers with a series of alternatives to conventional stormwater management practices to reduce the effect development has on the watershed (e.g., peak stream flow, stormwater runoff, habitat, etc.). The information presented is intended to provide guidance during the site-planning process on how to “re-think” the traditional site layout and design approach for both new and redevelopment projects.

This document provides an overview of the broad categories of better site design, the specific practices under each category, guidance on evaluating appropriate practices by weighing the benefits and risks of each practice and further guidance on each individual practice. Two case studies are also presented, one for a residential development and one for a commercial development, that illustrate conventional site designs versus “better” site designs. This document provides general guidance on how to choose the appropriate design technique but does not provide detailed design requirements and specifications for each of these practices. A list of resources on where to find this information is provided in the profile sheets in Section 2.4.

1.3 Key Terminology

Better site design - Incorporates non-structural and natural approaches to new and redevelopment projects to reduce effects on watersheds by conserving natural areas, reducing impervious cover and better integrating stormwater treatment.

Conservation design - Includes laying out the elements of a development project in such a way that the site design takes advantage of a site’s natural features, preserves the more sensitive areas and identifies any site constraints and opportunities to prevent effects.

Conventional site design - For the purposes of this document, conventional design can be viewed as the style of suburban development that has evolved during the past 50 years and generally involves larger lot development, clearing and grading of significant portions of a site, wider streets and larger cul-de-sacs, enclosed drainage systems for stormwater conveyance and large “hole-in-the-ground” detention basins.

Total impervious area - This is the total area within a watershed of all materials or structures on or above the ground surface that prevents water from infiltrating into the underlying soils. Impervious surfaces include, without limitation: paved parking lots, sidewalks, rooftops, patios, and paved, gravel and compacted-dirt surfaced roads. Gravel parking lots and/or compacted urban soils are often not included in total impervious area but may have hydrologic characteristics that closely resemble paved areas.

Natural areas - This is undisturbed land or previously disturbed land that has recovered and that retains pre-development hydrologic and water quality characteristics.

New development – Any construction or disturbance of a parcel of land that is currently undisturbed or unaltered by human activities and in a natural state.

Non-structural stormwater control – Natural measures that reduce pollution levels, do not require extensive construction or engineering efforts and/or promote pollutant reduction by eliminating the pollutant source.

Redevelopment – Any land disturbance for construction, alteration or improvement where the existing land use is commercial, industrial, institutional or multi-family residential.

Structural stormwater control – Devices that are engineered and constructed to provide temporary storage and treatment of stormwater runoff.

1.4 The Benefits of Better Site Design

The use of better site design can have a number of benefits that extend beyond improving water quality and stormwater runoff management, including:

- Reduced construction costs
- Reduced long-term operation and maintenance costs
- Increased property values
- Easier compliance with wetland and other resource protection regulations
- More open space for recreation
- More pedestrian-friendly neighborhoods
- Protection of sensitive forests, wetlands and habitats
- More aesthetically pleasing and naturally attractive landscape

1.5 The Obstacles of Better Site Design

Some obstacles exist or are perceived in the implementation of better site-design practices. Such as:

- Public perception of a particular practice may not be favorable
- Local codes may not allow for particular design elements
- Capital costs and/or operation and maintenance costs for some practices may not always be less expensive than conventional designs.

Typical perceived obstacles and realities specific to each practice are presented in the individual practice profile sheets in Section 2.4.

2 STORMWATER BETTER SITE DESIGN PRACTICES

2.1 Better Site-Design Categories and Listing of Practices

Stormwater better site-design practices and techniques covered in this document are grouped into the following three categories:

Preservation of Natural Features and Conservation Design: Preservation of natural features includes techniques to foster the identification and preservation of natural areas that can be used in the protection of water resources. Conservation design includes laying out the elements of a development project in such a way that the site design takes advantage of a site's natural features, preserves the more sensitive areas and identifies any site constraints and opportunities to prevent or reduce effects.

Reduction of Impervious Cover: Reduction of impervious cover includes methods to reduce the amount of rooftops, parking lots, roadways, sidewalks and other surfaces that do not allow rainfall to infiltrate into the soil, in order to reduce the volume of stormwater runoff, increase groundwater recharge, and reduce pollutant loadings that are generated from a site.

Use of Natural Features and Source Control for Stormwater Management: Use of natural features for stormwater management includes design strategies rather than structural stormwater controls to help manage and mitigate runoff. Source control for stormwater management includes elements to mitigate or manage stormwater in a natural or lower-impact manner.

Table 1 lists the specific better site-design practices and techniques presented in this document for each of the three categories. An evaluation of each practice is presented in Table 2, and further detail on each site-design practice is provided in the profile sheets in section 2.4.

Table 1: Better Site-Design General Categories and Specific Practices	
Preservation of Natural Features and Conservation Design	
1.	Preservation of Undisturbed Areas
2.	Preservation of Buffers
3.	Reduction of Clearing and Grading
4.	Locating Sites in Less Sensitive Areas
5.	Open Space Design
Reduction of Impervious Cover	
6.	Roadway Reduction
7.	Sidewalk Reduction
8.	Driveway Reduction
9.	Cul-de-sac Reduction
10.	Building Footprint Reduction
11.	Parking Reduction
Use of Natural Features and Source Control for Stormwater Management	
12.	Vegetated Buffer/Filter Strips
13.	Open Vegetated Channels
14.	Bioretention and Raingardens
15.	Infiltration
16.	Rooftop Runoff Reduction Mitigation
17.	Stream Daylighting for Redevelopment Projects
18.	Tree Planting

2.2 Better Site-Design Planning Process

Site design should be done in unison with the design and layout of stormwater infrastructure in attaining stormwater management and land use goals. The stormwater better site-design process used a three-step process as follows:

1. **Avoiding the Impacts** – Preserving natural features and use conservation design techniques
2. **Reducing the Impacts** – Reducing impervious cover.
3. **Managing the Impacts** – Using natural features and natural low-impact techniques to manage stormwater

The first step in the planning and design process is to avoid or minimize disturbance by preserving natural areas or strategically locating development based on the location of resource areas and physical conditions at a site. Once sensitive resource areas and site constraints have been avoided, the next step is to minimize the impact of land alteration by reducing impervious areas. Finally, for the areas that must be impervious, alternative and natural stormwater management techniques are chosen as opposed to the more routine structural, “pipe-to-pond,” approach.

2.3 Evaluating and Selecting Better Site Design Practices

Part of the planning process for better site design includes choosing the appropriate practice or practices for a given site. Table 2 illustrates the various criteria and factors used to evaluate the feasibility of a particular design practice and are ranked as either good, fair or poor, or as often, sometimes or rarely. The factors presented in Table 2 that will help a developer decide which practices to choose include:

How the Practice Applies To Meeting New York State Stormwater Criteria – Does the practice help meet the water quality volume (WQv) criteria? Does the practice help provide the quantity controls such as channel protection (C_p), overbank flood (Q_p), and extreme flooding (Q_f)? For descriptions of the criteria, see the New York State Stormwater Management Design Manual. (www.dec.state.ny.us/website/dow/toolbox/swmanual/#Downloads)

Economics – Does the practice decrease capital construction/infrastructure costs and decrease long-term operation and maintenance costs? Does the practice increase property values?

Public Perception – Is the practice well received by the public and something people will want to live with?

Local Codes – Do local codes, ordinances and regulations typically allow implementation of the practice?

Table 2: Better Site Design Practice Evaluation

Category	Technique	Applies To SW Criteria ¹			Economics			Public Per-ception	Allowed by Local Codes ⁴
		WQv	Cp _v , Qp, Qf	How Applies To Criteria	Lowers Capital Costs ²	Lowers O&M ³ Costs	Raises Property Value		
Preservation of Natural Features and Conservation Design	1. Preservation of Undisturbed Areas	⊙	⊙	Increases times of concentration, reduces CN ⁵	●	●	⊙	⊙	⊙
	2. Preservation of Buffers	⊙	⊙	Increases times of concentration, reduces CN	●	●	⊙	⊙	●
	3. Reduction of Clearing and Grading	⊙	⊙	Increases times of concentration, reduces CN	●	●	●	●	⊙
	4. Locating Sites in Less Sensitive Areas	⊙	⊙	Increases times of concentration, reduces CN	●	●	⊙	⊙	⊙
	5. Open-Space Design	⊙	⊙	Increases times of concentration, reduces CN	●	●	○	⊙	⊙
Reduction of Impervious Cover	6. Roadway Reduction	●	●	Reduces impervious area, which reduces WQv & flows	●	●	●	⊙	○
	7. Sidewalk Reduction	●	●	Reduces impervious area, which reduces WQv & flows	●	●	⊙	⊙	○
	8. Driveway Reduction	●	●	Reduces impervious area, which reduces WQv & flows	●	●	⊙	⊙	⊙
	9. Cul-de-sac Reduction	●	●	Reduces impervious area, which reduces WQv & flows	●	●	⊙	⊙	○
	10. Building-Footprint Reduction	●	●	Reduces impervious area, which reduces WQv & flows	●	●	⊙	●	●
	11. Parking Reduction	●	●	Reduces impervious area, which reduces WQv & flows	●	●	⊙	⊙	⊙
Use of Natural Features and Source Control for Stormwater Management	12. Vegetated Buffer/Filter Strips	⊙	⊙	Increases times of concentration, reduces CN	●	●	⊙	●	⊙
	13. Open Vegetated Channels	●	●	Stores WQv & Peak Flows	●	⊙	⊙	⊙	●
	14. Bioretention	●	●	Stores WQv & Peak Flows	●	⊙	⊙	●	●
	15. Infiltration	●	●	Stores WQv & Peak Flows	●	⊙	⊙	●	●
	16. Rooftop Runoff Reduction Mitigation	●	●	Stores WQv & Peak Flows	●	●	⊙	⊙	●
	17. Stream Daylighting for Redevelopment Projects	○	○	Increases travel times, decreases peak flows	○	⊙	●	●	●
	18. Tree Planting	⊙	⊙	Reduces volume of runoff, reduces CN	⊙	⊙	●	●	●
Key: ● = good/often ⊙ = fair/sometimes ○ = poor/rarely 1 - WQv = Water Quality Volume, Cp _v = Channel Protection, Qp = Overbank Flood, Qf = Extreme Flood 2 - "Lowers Capital Costs" is intended for general purposes. Capital costs may vary on a site-by-site basis.				3 - Operation and Maintenance 4 - "Allowed by Local Code" is intended for general purposes. User should consult with actual local planning codes. 5 - CN = Runoff Coefficient "Curve Number"					

2.4 Better Site Design Practice Profile Sheets

2.4.1 *Preservation of Natural Features and Conservation Design*

Preservation of natural features includes the techniques to foster the identification and preservation of natural areas that can be used in the protection of water resources by reducing stormwater runoff, providing runoff storage, reducing flooding, preventing soil erosion, promoting infiltration and removing stormwater pollutants. Conservation Design includes laying out the elements of a development project in such a way that the site design takes advantage of a site's natural features, including areas to be protected as conservation areas, preserves the more sensitive areas and identifies any site constraints and opportunities (e.g., topography, soils, natural vegetation, wetlands, floodplains, shallow bedrock, high water table, etc.) to prevent both on-site and downstream stormwater effects.

2.4.2 *Reduction of Impervious Cover*

Reduction of impervious cover includes methods to reduce the amount of rooftops, parking lots, roadways, sidewalks and other surfaces that do not allow rainfall to infiltrate into the soil, in order to reduce the volume of stormwater runoff, increase groundwater recharge and reduce pollutant loadings that are generated from a site.

2.4.3 *Utilization of Natural Features and Source Control for Stormwater Management*

Use of natural features for stormwater management includes design strategies that use existing or recreate natural features to help manage and mitigate runoff, rather than structural stormwater controls. Source control for stormwater management includes elements to mitigate or manage stormwater in a natural or lower-impact manner.

Better Site Design Practice #1: Preservation of Undisturbed

Preservation of Natural
Features and Conservation

Description: Important natural features and areas such as undisturbed forested and native vegetated areas, natural terrain, riparian corridors, wetlands and other important site features should be delineated and placed into permanent conservation areas.

Key Benefits

- Helps to preserve a site's natural hydrology and water balance
- Can act as a non-structural stormwater feature to promote additional filtration and infiltration
- Can help to preserve a site's natural character, habitat and aesthetic appeal
- Has been shown to increase property values for adjacent parcels
- Can reduce structural stormwater management storage requirement and may be used as a "stormwater credit"

Typical Perceived Obstacles and Realities

- Preserved conservation areas may limit the development potential of a site – *With clustering and other development incentives, development yield can be maintained.*
- Preserved habitats may harbor undesirable wildlife and insects – *Most people enjoy viewing wildlife; native vegetation does not provide a food source for most vermin; continued education is necessary to show that humans and wildlife can co-exist, even at the neighborhood level.*
- Preserved areas may represent a fire hazard – *Clearing setbacks and target vegetation around residential structures can reduce property damage potential.*

USING THIS PRACTICE

- Delineate and define natural conservation areas before performing site layout and design.
- Ensure that conservation areas and native vegetation are protected in an undisturbed state through the design, construction and occupancy stages.

Discussion

Conservation of natural areas such as undisturbed forested and native-vegetated areas, natural terrain, riparian corridors and wetlands on a development project can help to preserve pre-development hydrology of the site and aid in reducing stormwater runoff and pollutant load. Undisturbed vegetated areas also promote soil stabilization and provide for filtering and infiltration of runoff.

Natural conservation areas are typically identified through a site-analysis stage using mapping and field-reconnaissance assessments. Areas proposed for protection should be delineated early in the planning stage, long before any site design, clearing or construction begins. When done before the concept-plan phase, the planned conservation areas can be used to guide the layout of a project. Figure 1 shows components of a natural resources inventory map with proposed conservation areas delineated.

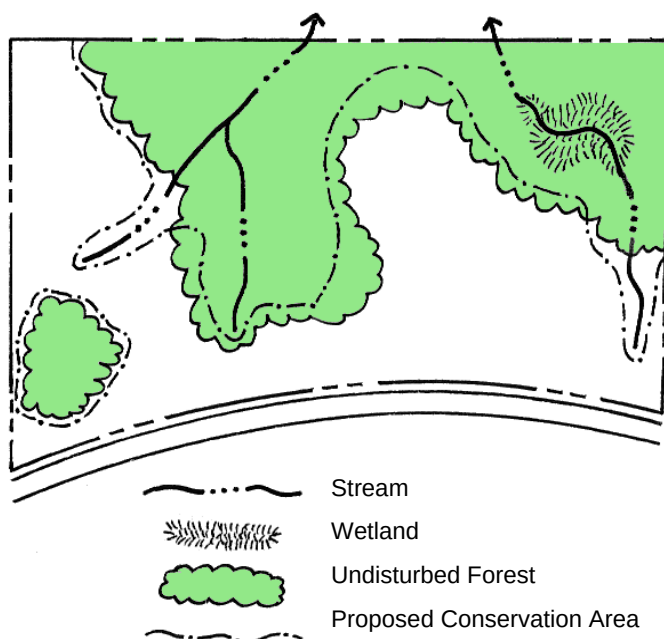


Figure 1: Example of Natural Resource Inventory Plan
(Source: Georgia Stormwater Manual, 2001)

Preservation areas should then be incorporated into site-development plans and clearly marked on all construction and grading plans to ensure that construction activities are kept out of these areas and that native vegetation is undisturbed. The boundaries of each conservation area should be mapped by carefully determining the limit which should not be crossed by construction activity.

Once established, natural conservation areas must be protected during construction and managed after occupancy by a responsible party able to maintain the areas in a natural state in perpetuity. Typically, conservation areas are protected by legally enforceable deed restrictions, conservation easements and a maintenance agreement. When all of these measures are applied, a permanently protected natural area can be applied as a “stormwater credit” to reduce the structural stormwater management measures (see Figure 2 for a representative project illustrating natural resource area protection).



Figure 2: Aerial Photograph of Development Project Illustrating Preservation of Undisturbed Natural Areas
(Source: Arendt, 1996)

Additional Guidance

Arendt, Randall. 1996. *Conservation Design for Subdivisions: A Practical Guide to Creating Open Space Networks*. American Planning Association. Chicago, IL. Available from the American Planning Association at www.planning.org

Center for Watershed Protection. 1998. *Better Site Design: A Handbook for Changing Development Rules in Your Community*. Available from www.cwp.org

Prince George's County, MD. June 1999. *Low-Impact Development Design Strategies: An Integrated Design Approach*. Prince George's County, Maryland, Department of Environmental Resources, Largo, Maryland. Available from www.epa.gov

Better Site Design Practice #2: Preservation of Buffers

Preservation of Natural Features
and Conservation Design

Description: Naturally vegetated buffers should be defined, delineated and preserved along perennial streams, rivers, shorelines and wetlands.

Key Benefits	Typical Perceived Obstacles and Realities
• Riparian buffers treat stormwater and improve water quality • Can be used as nonstructural stormwater infiltration zones • Keep structures out of the floodplain and provide a right-of-way for large flood events • Help to preserve riparian ecosystems and habitats • Can serve as recreational areas • A buffer credit can be taken if allowed by the local review authority.	• Buffers may result in a potential loss of developable land – <i>Regulatory tools or other incentives may be available to protect the interests of property owners</i> • Private landowners may be required to provide public access to privately held stream buffers – <i>Effective buffers can be maintained in private ownership through deed restrictions and conservation easements</i> • Excessive nuisance species will be present due to the natural buffer area - <i>Forested buffers do not encourage nuisance vegetative species, and animal habitation can be controlled at the outer zone of the buffer.</i>

USING THIS PRACTICE

- Delineate and preserve naturally vegetated riparian buffers (define the width, identify the target vegetation, designate methods to preserve the buffer indefinitely)
- Ensure that buffers and native vegetation are protected throughout planning, design, construction and occupancy
- Consult local planning authority for minimum buffer width and/or recommended width.

Discussion

A riparian buffer is a special type of natural conservation area along a stream, wetland or shoreline where development is restricted or prohibited. The primary function of buffers is to protect and physically separate a stream, lake, coastal shoreline or wetland from future disturbance or encroachment. If properly designed, a buffer can provide stormwater management functions, can act as a right-of-way during floods, and can sustain the integrity of water-resource ecosystems and habitats. An example of a riparian stream buffer is shown in Figure 3.



Figure 3: Riparian Stream Buffer

(Source: Georgia Stormwater Manual, 2001)

Forested riparian buffers should be maintained and reforestation should be encouraged where no wooded buffer exists. Proper restoration should include all layers of the forest plant community, including understory, shrubs and groundcover, not just trees. A riparian buffer can be of fixed or variable width but should be continuous and not interrupted by impervious areas that would allow stormwater to concentrate and flow into the stream without first flowing through the buffer.

Ideally, riparian buffers should be sized to include the 100-year floodplain as well as steep banks and freshwater wetlands. The buffer depth needed to perform properly will depend on the size of the stream and the surrounding conditions, but a minimum 25-foot undisturbed vegetative buffer is needed for even the smallest perennial streams, and a 50-foot or larger undisturbed buffer is ideal. Even with a 25-foot undisturbed buffer, additional zones can be added to extend the total buffer to at least 75 feet from the edge of the stream. The three distinct zones within the 75-foot depth are shown in Figure 4. The function, vegetative target and allowable uses vary by zone as described in Table 3.

These recommendations are minimum standards for most streams. Some streams and watersheds may benefit from additional measures to ensure adequate protection. In some areas, specific state laws or local ordinances already require stricter buffers than are described here. The buffer widths discussed are not intended to modify or supersede wider or more restrictive buffer requirements that are already in place.

Table 3: Riparian Buffer Management Zones (Source: Adapted from Schueler, 1995)			
	Streamside Zone	Middle Zone	Outer Zone
Width	Minimum 25 feet plus wetlands and critical habitat	Variable, depending on stream order, slope, and 100-year floodplain (min. 25 ft.)	25-foot minimum setback from structures
Vegetative Target	Undisturbed mature forest. Reforest if necessary.	Managed forest, some clearing allowed.	Forest encouraged, but usually turfgrass.
Allowable Uses	Very restricted (e.g., flood control, utility easements, footpaths)	Restricted (e.g., some recreational uses, some stormwater controls, bike paths)	Unrestricted (e.g., residential uses, including lawn, garden, most stormwater controls)

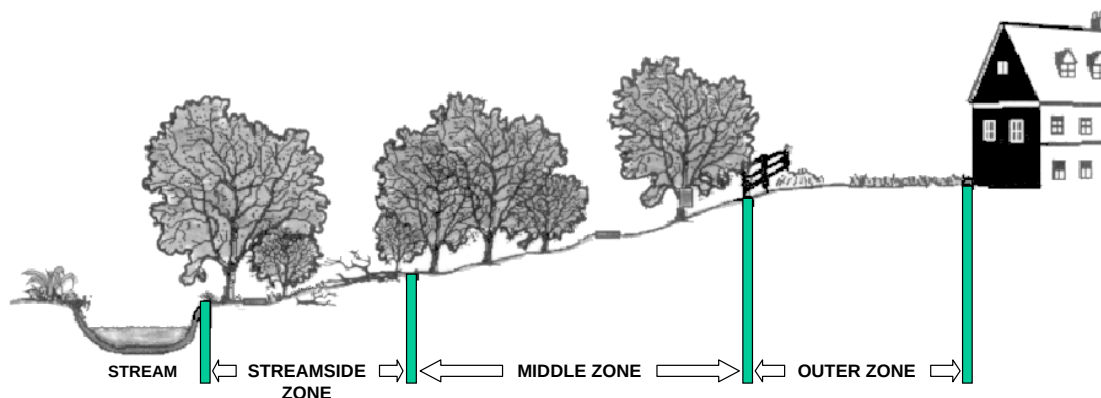


Figure 4: Three-Zone Stream Buffer System

(Source: Adapted from Schueler, 1995)

As stated above, the streamside or inner zone should consist of a minimum of 25 feet of undisturbed mature forest. In addition to runoff protection, this zone provides bank stabilization as well as shading and protection for the stream. This zone should also include wetlands and any critical habitats, and its width should be adjusted accordingly. The middle zone provides a transition between upland development and the inner zone and should consist of managed woodland that allows for infiltration and filtration of runoff. An outer zone allows more clearing and acts as a further setback for impervious surfaces. It also functions to prevent encroachment and filter runoff. It is here that flow into the buffer should be transformed from concentrated flow into sheet flow to maximize ground contact with the runoff.

Development within the riparian buffer should be limited only to those structures and facilities that are absolutely necessary. Such limited development should be specifically identified in any codes or ordinances enabling the buffers. When construction activities do occur within the riparian corridor, specific mitigation measures should be required, such as deeper buffers or riparian buffer improvements.

Generally, the riparian buffer should remain in its natural state. However, some maintenance is periodically necessary, such as planting to minimize concentrated flow, removal of exotic plant species when these species are detrimental to the vegetated buffer and removal of diseased or damaged trees.

Additional Guidance

Center for Watershed Protection. 1998. *Better Site Design: A Handbook for Changing Development Rules in Your Community*. Available from www.cwp.org

Berkshire Regional Planning Commission. 2003. *The Massachusetts Buffer Manual: Using Vegetated Buffers to Protect our Lakes and Rivers*. Prepared for the Massachusetts Department of Environmental Protection. Boston, MA. Available from www.berkshireplanning.org

Maine Department of Environmental Protection. 1998. *The Buffer Handbook: A Guide to Creating Vegetated Buffers for Lakefront Properties*. Maine DEP. Augusta, ME. Available from <http://www.state.me.us/dep/blwq/doclake/publake.htm>

Schueler, T. 1995. *Site Planning for Urban Stream Protection*. Prepared for: Metropolitan Washington Council of Governments. Washington, DC. Center for Watershed Protection, Ellicott City, MD. Available from www.cwp.org

Better Site Design Practice #3: Reduction of Clearing and Grading

Preservation of Natural Features
and Conservation Design

Description: Clearing and grading of the site should be limited to the minimum amount needed for the development function road access and infrastructure (e.g., utilities, wastewater disposal, stormwater management). Site foot-printing should be used to disturb the smallest possible land area on a site.

Key Benefits

- Preserves more undisturbed natural areas on a development site
- Areas of a site that are conserved in their natural state retain their natural hydrology and do not contribute to construction erosion
- Native trees, shrubs and grasses are important contributors to the overall quality and viability of the environment.

Typical Perceived Obstacles and Realities

- Preserving trees during construction is expensive – *Minimizing clearing during construction can reduce earth movement and erosion and sediment control costs*
- People prefer large lawns – *Lots with trees tend to have a higher value than those without*
- Vegetation near homes can be a fire risk – *Even if clearing is required near homes, this can be accommodated while minimizing clearing on the entire site*
- Native vegetation may harbor undesirable wildlife or insects - *Most people enjoy viewing wildlife; native vegetation does not provide a food source for most vermin; continued education is necessary to show that humans and wildlife can co-exist, even at the neighborhood level.*

USING THIS PRACTICE

- Restrict clearing to the minimum area required for building footprints, construction access, and safety setbacks
- Establish limits of disturbance for all development activities
- Use site foot-printing to minimize clearing and land disturbance
- Limit site mass grading approach.
- Use alternative site designs that use open-space or “cluster” developments.

Discussion

Minimal disturbance methods should be used to limit the amount of clearing and grading that takes place on a development site, preserving more of the undisturbed vegetation and natural hydrology of a site.

A limit of disturbance (LOD) should be established based on the maximum disturbance zone. These maximum distances should reflect reasonable construction techniques and equipment needs, together with the physical situation of the development site, such as slopes or soils. LOD

distances may vary by type of development, size of lot or site and by the specific development feature involved.

Site "foot-printing" should be used which maps all of the limits of disturbance to identify the smallest possible land area on a site which requires clearing or land disturbance. An example of site foot-printing is illustrated in figures 5 and 6.

Sites should be designed so that they fit the terrain (see practice #4). During construction, special procedures and equipment that reduce land disturbance should be used. Alternative site designs should be considered to minimize limits of clearing, such as "cluster" developments (see practice #5).

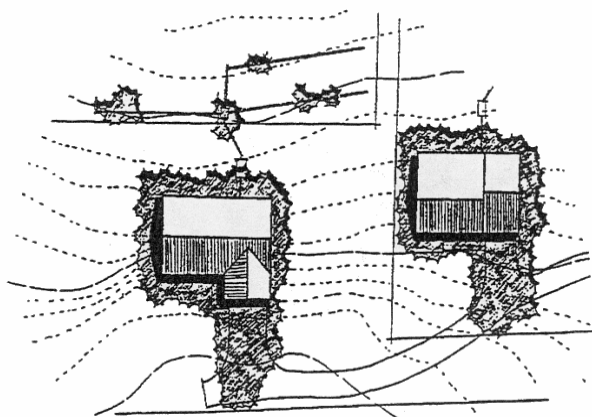


Figure 5: Establishing Limits of Clearing
(Source: DDNREC, 1997)



Figure 6: Example of Site Foot-Printing
(Source: Georgia Stormwater Manual, 2001)

Additional Guidance

Arendt, Randall. 1996. *Conservation Design for Subdivisions: A Practical Guide to Creating Open Space Networks*. American Planning Association. Chicago, IL. Available from the American Planning Association at www.planning.org

Center for Watershed Protection. 1998. *Better Site Design: A Handbook for Changing Development Rules in Your Community*. Available from www.cwp.org

Schueler, T. 1995. *Site Planning for Urban Stream Protection*. Prepared for: Metropolitan Washington Council of Governments. Washington, DC. Center for Watershed Protection, Ellicott City, MD. Available from www.cwp.org

Better Site Design Practice #4: Locating Sites in Less Sensitive Areas

Preservation of Natural Features
and Conservation Design

Description: Development sites should be located to avoid sensitive resource areas such as floodplains, steep slopes, erodible soils, wetlands, mature forests and critical habitat areas. Buildings, roadways and parking areas should be located to fit the terrain and in areas that will create the least impact.

Key Benefits

- Preserving floodplains provides a natural right-of-way and temporary storage for large flood events; keeps people and structures out of harm's way and helps to preserve riparian ecosystems and habitats.
- Preserving steep slopes and building on flatter areas helps to prevent soil erosion and minimizes stormwater runoff; helps to stabilize hillsides and soils and reduces the need for cut-and-fill and grading.
- Avoiding development on erodible soils can prevent sedimentation problems and water-quality degradation. Areas with highly permeable soils can be used as nonstructural stormwater infiltration zones
- Fitting the design to the terrain and in less sensitive areas helps to preserve the natural hydrology and drainageways of a site; reduces the need for grading and land disturbance, and provides a framework for site design and layout.

Typical Perceived Obstacles and Realities

- Costs will be higher for developments due to increased planning and design, localized construction and less developable land - *Developments that protect sensitive areas will likely have higher market value, less liability for potential natural disasters, such as flooding or slope failures and lower construction costs for areas that require less earthwork or difficult terrain, such as steep slopes or wetland areas to work around.*

USING THIS PRACTICE

- Ensure all development activities do not encroach on designated floodplain and/or wetland areas
- Avoid development on steep slope areas and minimize grading and flattening of hills and ridges
- Leave areas of porous or highly erodible soils as undisturbed conservation areas

- Develop roadway patterns to fit the site terrain, and locate buildings and impervious surfaces away from steep slopes, drainageways and floodplains
- Locate site in areas that are less sensitive to disturbance or have a lower value in terms of hydrologic function.

Discussion

Development in floodplain areas can reduce the ability of the floodplain to convey stormwater, potentially causing safety problems or significant damage to the site in question, as well as to both upstream and downstream properties. Ideally, the entire 100-year full-buildout floodplain should be avoided for clearing or building activities and should be preserved in a natural, undisturbed state where possible.

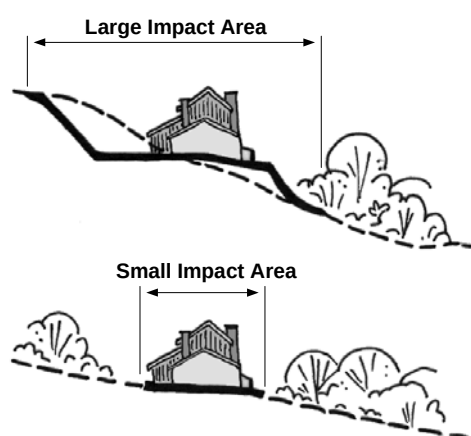


Figure 7: Grading that Creates Large Construction "Pads" Effects more Land than Contoured Grading on Smaller Areas at Flatter Slopes

(Source: MPCA, 1989)

Development on slopes with a grade of 15% or greater should be avoided, if possible, to limit soil loss, erosion, excessive stormwater runoff and the degradation of surface water. Excessive grading should be avoided on all slopes, as should the flattening of hills and ridges. Steep slopes should be kept in an undisturbed natural condition to help stabilize hillsides and soils. On slopes greater than 25%, no development, regarding or stripping of vegetation should be considered.

Areas of a site with hydrologic soil group A and B soils, such as sands and sandy loam soils, should be conserved as much as possible, and these areas should ideally be incorporated into undisturbed natural or open-space areas (Figure 8). Conversely, buildings and other impervious surfaces should be located on those portions of the site with the *least* permeable soils. Similarly, areas on a site with highly erodible or unstable soils should be avoided for land-disturbing activities and buildings to prevent erosion and sedimentation problems as well as potential structural problems. These areas should be left in an undisturbed and vegetated condition.

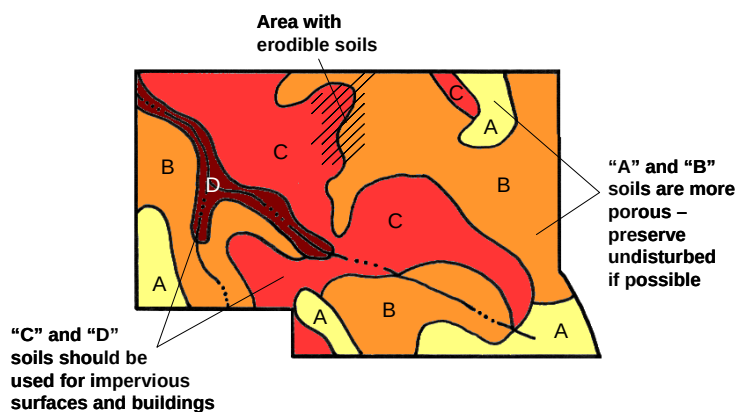


Figure 8: Soil Mapping Can Be Used to Guide Development

(Source: Georgia Stormwater Manual, 2001)

The layout of roadways and buildings on a site should generally conform to the landforms on a site (Figure 9). Natural drainageways and stream buffer areas should be preserved by designing road layouts around them. Buildings should be sited to use the natural grading and drainage system and avoid the unnecessary disturbance of vegetation and soils. Roadway patterns on a site should be chosen to provide access schemes which match the terrain. In rolling or hilly terrain, streets should be designed to follow natural contours to reduce clearing and grading. In flatter areas, a traditional grid pattern of streets or "fluid" grids which bend and may be interrupted by natural drainageways may be more appropriate.

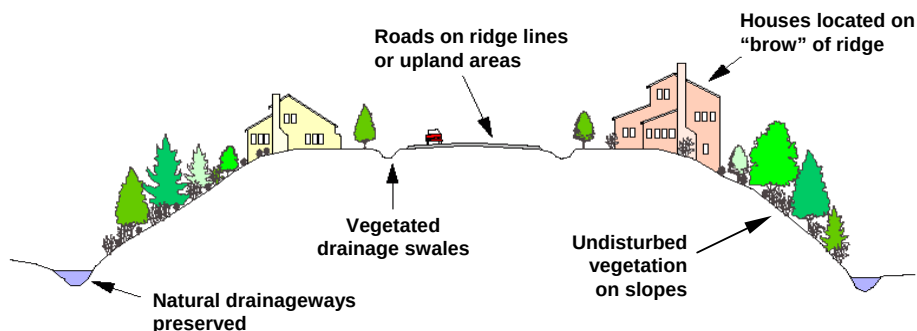


Figure 9: Preserving the Natural Topography of a Site

(Source: Adapted from Prince George's County, 1999)

In much the same way that a development should be designed to conform to the terrain of the site, layout should also be designed so that the areas of development are placed in the locations of the site that minimize the hydrologic impact of the project. This is accomplished by steering development to areas of the site that are less sensitive to land disturbance or have a lower value in terms of hydrologic function. Figure 10 shows a development site where the natural features have been mapped in order to delineate the hydrologically sensitive areas. Through careful site planning, sensitive areas can be set aside as natural open space areas. In many cases, such areas can be used as buffer spaces between land uses on the site or between adjacent sites.

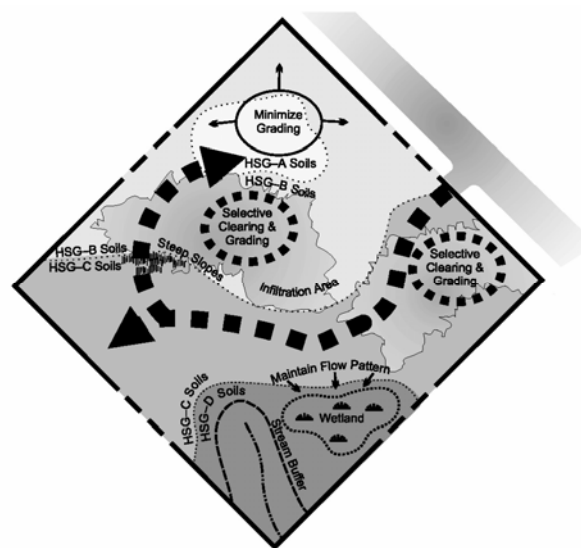


Figure 10: Guiding Development to Less Sensitive Areas of a Site

(Source: Adapted from Prince George's County, 1999)

Additional Guidance

Arendt, Randall. 1996. *Conservation Design for Subdivisions: A Practical Guide to Creating Open Space Networks*. American Planning Association. Chicago, IL. Available from the American Planning Association at www.planning.org

Environmental Protection Agency (EPA)
site on smart growth including a focus on community-based approaches to reducing sprawl.
www.epa.gov/ebtpages/envismartgrowth.html

Hart, Leslie. 1994. *Guiding Principles of Sustainable Design*. Prepared for the U.S Department of the Interior and the National Parks Service. Available from
<http://www.nps.gov/dsc/dsgncnstr/gpsd/>

Better Site Design Practice #5: Open-Space Design

Preservation of Natural Features
and Conservation Design

Description: Open-space site designs (also referred to as conservation development or clustering) incorporate smaller lot sizes to reduce overall impervious cover while providing more undisturbed open space and protection of water resources.

Key Benefits

- Preserves conservation areas on a development site
- Can be used to preserve natural hydrology and drainageways
- Can be used to help protect natural conservation areas and other site features
- Reduces the need for grading and land disturbance
- Reduces infrastructure needs and overall development costs
- Allows flexibility for developers to implement creative site designs, including better stormwater management practices

Typical Perceived Obstacles and Realities

- Smaller lot sizes and compact development may be perceived by developers as less marketable – *Open space designs are in fact highly desirable and have economic advantages such as cost savings and higher market appreciation*
- Lack of speed and certainty in the review process may be of concern – *Consult with the local review authority to review requirements; some communities are moving toward open-space design as a “by right” form of subdivision*
- Prospective homebuyers may be reluctant to purchase homes due to concerns regarding management of the community open space – *Proper methods and implementation of maintenance agreements are available; natural open space reduces maintenance costs and can help keep association fees down*
- Open-space developments appear incompatible with adjacent land uses and are equated with increased noise and traffic – *Open-space design allows preservation of natural areas, using less space for streets, sidewalks, parking lots and driveways; incorporating buffers into the design can help alleviate incompatibility with other competing land uses.*

USING THIS PRACTICE

- Use a site design which concentrates development and preserves open space and natural areas of the site
- Locate the developed portion of the cluster areas in the least sensitive areas of the site (see practice #4).
- Use reduced setbacks and frontages and narrower right-of-way widths to design non-traditional lot layouts within the cluster.

Discussion

Open-space development, also known as “open space residential design” (OSRD), or *conservation development* or *clustering*, is a better site-design technique that concentrates structures and impervious surfaces in a compact area in one portion of the development site in exchange for providing open space and natural areas elsewhere on the site. Typically smaller lots and/or nontraditional lot designs are used to cluster development and create more conservation areas on the site.

Open-space developments have many benefits compared with conventional commercial developments or residential subdivisions: they can reduce impervious cover, stormwater pollution, construction cost and the need for grading and landscaping, while providing for the conservation of natural areas. Figures 11 and 12 show examples of open space developments.

Along with reduced imperviousness, open-space designs provide a host of other environmental benefits lacking in most conventional designs. These developments reduce potential pressure to encroach on conservation and buffer areas because enough open space is usually reserved to accommodate these protection areas. As less land is cleared during the construction process, alteration of the natural hydrology and the potential for soil erosion are also greatly diminished. Perhaps most importantly open space design reserves 25 to 50 percent of the development site in conservation areas that would not otherwise be protected.

Open-space developments can also be significantly less expensive to build than conventional projects. Most of the cost savings are due to reduced infrastructure cost for roads and stormwater management controls and conveyances. While open-space developments are frequently less expensive to build, developers find that these properties often command higher prices than those in more conventional developments. Several studies estimate that residential properties in open-space developments garner premiums that are higher than conventional subdivisions and moreover, sell or lease at increased rates. Once established, common open-space and natural conservation areas must be managed by a responsible party able to maintain the areas in a natural state in perpetuity. Typically, the conservation areas are protected by legally enforceable deed restrictions, conservation easements and maintenance agreements.



Figure 11: Example of an Open Space or “Cluster” Subdivision
(Source: Georgia Stormwater Manual, 2001)



Figure 12: Aerial View of an Open Space or “Cluster” Subdivision
(Source: Georgia Stormwater Manual, 2001)

Flexible lot shapes and setback and frontage distances allow site designers to create attractive and unique lots that provide homeowners with enough space while allowing for the preservation of natural areas in a residential subdivision. A narrower right-of-way will consume less land that may be better used for housing lots and allow for a more compact site design. Figure 13 illustrates various nontraditional lot designs, and Figure 14 illustrates reduced front and side setbacks.

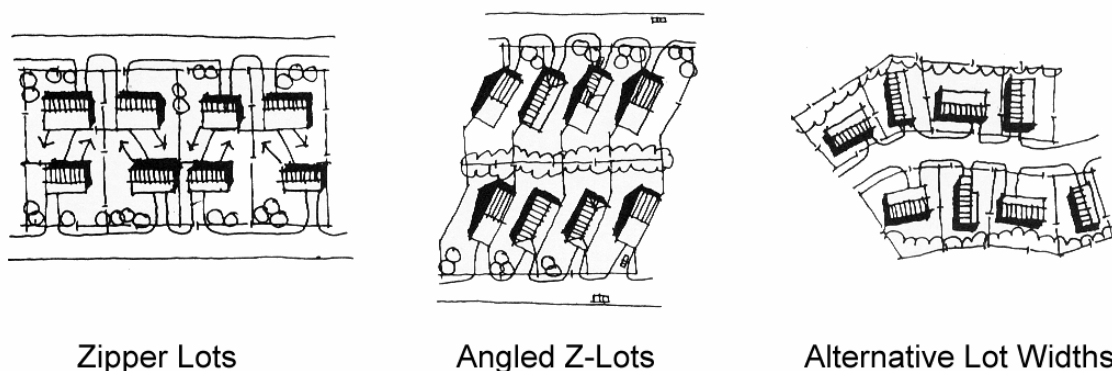


Figure 13: Nontraditional Lot Designs
(Source: ULI, 1992)



Figure 14: Lots with Reduced Front and Side Setbacks
(Source: Georgia Stormwater Manual, 2001)

Additional Guidance

Arendt, Randall. 1994. *Designing Open Space Subdivisions: A Practical Step-by-Step Approach*. Natural Lands Trust, Inc. Media, PA. Available from www.natlands.org or www.greenerprospects.com

Arendt, Randall. 1996. *Conservation Design for Subdivisions: A Practical Guide to Creating Open Space Networks*. American Planning Association. Chicago, IL. Available from the American Planning Association at www.planning.org

Center for Watershed Protection. 1998. *Better Site Design: A Handbook for Changing Development Rules in Your Community*. Available from www.cwp.org

A non-profit Massachusetts organization dedicated to educating people about OSRD development and implementation. <http://www.greenneighborhoods.org/site/Index.htm>

University of Michigan study finds homebuyers want view of woods, not large lawns. www.umich.edu/news/index.html?Releases/2004/Jun04/r062904a

Environmental Protection Agency (EPA)

site on smart growth including a focus on community based approaches to reducing sprawl. www.epa.gov/ebtpages/envismartgrowth.html

Better Site Design Practice #6: Roadway Reduction

Reduction of Impervious Cover

Description: Roadway lengths and widths should be minimized on a development site where possible to reduce overall imperviousness.

Key Benefits

- Reduces the amount of impervious cover and associated runoff and pollutants generated
- Reduces the costs associated with road construction and maintenance

Typical Perceived Obstacles and Realities

- Local codes may not permit shorter or narrower roads – *Meet with local officials to discuss waivers for alternative designs that will address concerns of access, snow stockpiling, and parking.*
- The public may view narrow roads as unsafe – *Narrower roads in fact reduce the speeds at which vehicles drive; many maintenance and emergency vehicles can in fact access narrow roads*
- Narrow and shorter roads do not have enough parking – *Provisions can be made in the design of a site to accommodate off-street parking.*

USING THIS PRACTICE

- Consider different site and road layouts that reduce overall street length
- Minimize street width by using narrower street designs that are a function of land use, density and traffic demand.
- Use smaller side-yard setbacks to reduce total road length.

Discussion

The use of alternative road layouts that reduce the total linear length of roadways can significantly reduce overall imperviousness of a development site. Site designers are encouraged to analyze different site and roadway layouts to see if they can reduce overall street length.

In addition, residential streets and private streets within commercial and other development should be designed for the minimum required pavement width needed to support travel lanes, on-street parking and emergency access. Figure 15 shows options for narrower street designs. In many instances, on-street parking can be reduced to one lane or eliminated on local access roads with less than 200 average daily trips (ADT) and on short cul-de-sacs street. One-way, single-lane, loop roads are another way to reduce the width of lower-traffic streets.

Further, reducing side yard setbacks and using narrower frontages can reduce total street length, which is especially important in cluster and open-space designs.

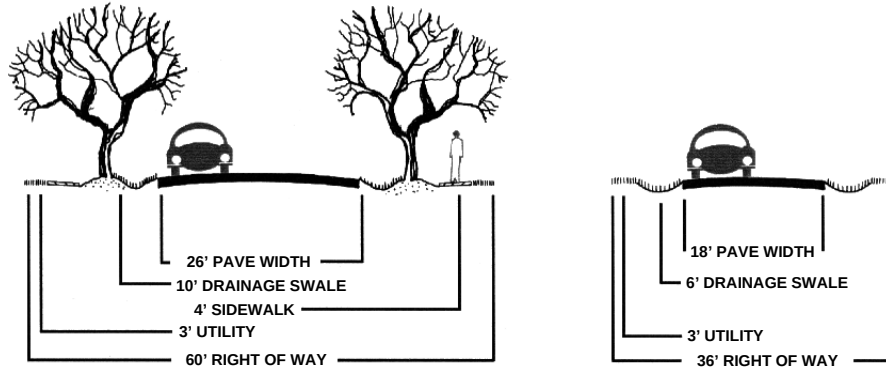


Figure 15: Potential Design Options for Narrower Roadway Widths
(Source: Georgia Stormwater Manual, 2001)

Additional Guidance

Arendt, Randall. 1994. *Designing Open Space Subdivisions: A Practical Step-by-Step Approach*. Natural Lands Trust, Inc. Media, PA. Available from www.natlands.org or www.greenerprospects.com

Arendt, Randall. 1996. *Conservation Design for Subdivisions: A Practical Guide to Creating Open Space Networks*. American Planning Association. Chicago, IL. Available from the American Planning Association at www.planning.org

Institute of Traffic Engineers (ITE). 2001. *Residential Streets, Third Edition*. Institute of Traffic Engineers, Publication No. LP-630. Available from www.ite.org

Institute of Traffic Engineers (ITE). 1999. *Traditional Neighborhood Development Street Design Guidelines*. Institute of Traffic Engineers, Publication No. RP-027A. Available from www.ite.org

Institute of Traffic Engineers (ITE). 1997. *Designing Neighborhood Streets*. Institute of Traffic Engineers, Publication No. VHS-027. Available from www.ite.org

Better Site Design Practice #7: Sidewalk Reduction

Reduction of Impervious Cover

Description: Sidewalk lengths and widths should be minimized on a development site where possible to reduce overall imperviousness.

Key Benefits

- Reduces the amount of impervious cover and associated runoff and pollutants generated
- Reduces the costs associated with construction and maintenance
- Reduces the individual homeowner's responsibility for maintenance, such as snow clearance

Typical Perceived Obstacles and Realities

- Sidewalks on only one side of the street may be perceived as unsafe – *Accident research shows sidewalks on one side are nearly as safe as sidewalks on both*
- Homebuyers are perceived to want sidewalks on both sides – *Some actually prefer not to have a sidewalk in front of their home, and there is no market difference between homes with and without sidewalks directly in front.*
- Local codes may not permit narrower, alternative, or the elimination of a sidewalk – *Meet with local officials to discuss waivers for alternative designs that will address concerns of accessibility and safety issues.*

USING THIS PRACTICE

- Locate sidewalks on only one side of the street.
- Provide common walkways linking pedestrian areas.
- Use alternative sidewalk and walkway surfaces.
- Shorten front setbacks to reduce walkway lengths.

Discussion

Most codes require that sidewalks be placed on both sides of residential streets (e.g., double sidewalks) and be constructed of impervious concrete or asphalt. Many subdivision codes also require sidewalks to be 4 to 6 feet wide and 2 to 10 feet from the street. These codes are enforced to provide sidewalks as a safety measure.

Developers may wish to consider allowing sidewalks on only one side of the street or eliminating them where they don't make sense. Sidewalks should be designed with the goal of improving pedestrian movement and diverting it away from the street. Developers may also consider reducing sidewalk widths and placing them farther from the street. In addition, sidewalks should be graded to drain to front yards rather than the street.

Alternative surfaces for sidewalks and walkways should be considered to reduce impervious cover (figures 16 and 17). In addition, building and home setbacks should be shortened to reduce the amount of impervious cover from entry walks.



Figure 16: Sidewalk with a permeable paver surface
(Source: MA EOEA, 2005)



Figure 17: Sidewalk with Common Walkways Linking Pedestrian Areas
(Source: MA EOEA, 2005)

Additional Guidance

Center for Watershed Protection. 1998. *Better Site Design: A Handbook for Changing Development Rules in Your Community*. Available from www.cwp.org

Website for Walkable Communities, Inc. www.walkablecommunities.org

Litman, Todd Alexander. 2004. *The Economic Value of Walkability*. Victoria Transport Policy Institute. Victoria, British Columbia. Available from <http://www.vtpi.org/walkability.pdf>

Better Site Design Practice #8: Driveway Reduction

Reduction of Impervious Cover

Description: Where driveway lengths and widths should be minimized on a development site possible, to reduce overall imperviousness.

Key Benefits

- Reduces the amount of impervious cover and associated runoff and pollutants generated

Typical Perceived Obstacles and Realities

- Alternative driveway surfaces make snow removal more difficult – *Careful site design, material selection and homeowner education can help alleviate the concern*
- Developers perceive alternative surfaces as less marketable – *“Green” development projects are increasingly being sought by consumers.*
- Homeowners have concerns regarding access with shared driveways – *Proper site design and homeowner education will alleviate access issues.*
- Local codes may not permit shorter or narrower driveways or driveways with porous surfaces – *Meet with local officials to discuss waivers for alternative designs.*

USING THIS PRACTICE

- Use shared driveways that connect two or more homes.
- Use alternative driveway surfaces.
- Use smaller lot front building setbacks to reduce total driveway length.

Discussion

Most local subdivision codes are not very explicit as to how driveways must be designed. Most simply require a standard apron to connect the street to the driveway but do not specify width or surface material for driveways. Typical residential driveways range from 12 feet wide for one-car driveways to 20 feet for two.

Shared driveways are discouraged or prohibited by many communities. Shared driveways can reduce impervious cover and should be encouraged with enforceable maintenance agreements and easements.

Secondly, the typical 400-800 square feet of impervious cover per driveway can be minimized by using narrower driveway widths, reducing the length of driveways, or using alternative surfaces such as double-tracks, reinforced grass or permeable paving materials.

Building and home setbacks should be shortened to reduce the amount of impervious cover from driveways and entry walks. A setback of 20 feet is more than sufficient to allow a car to park in a driveway without encroaching into the public right of way and reduces driveway and walk pavement by more than 30 percent compared with a setback of 30 feet (see Figure 18).

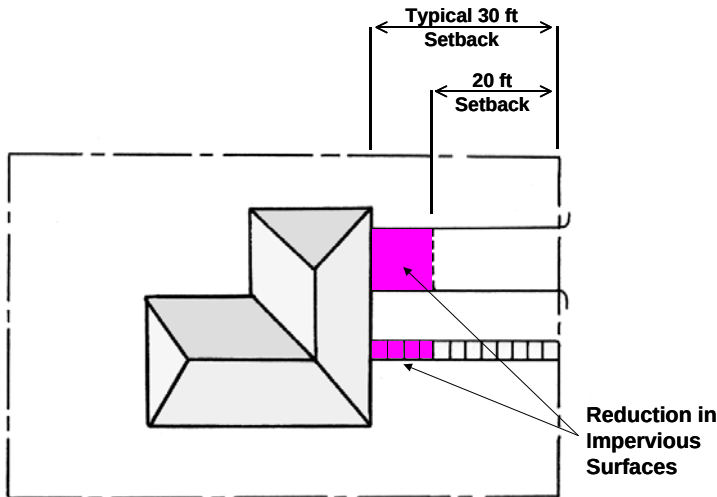


Figure 18: Reduced Driveway and Walkway Lengths by Using Reduced Setbacks
(Adapted from: MPCA, 1989)



Figure 19: Reduced Driveway Lengths by Using Shared Driveways
(Source: MA EOE, 2005)



Figure 20: Permeable Pavers as an Alternative Driveway Surface
(Source: MA EOE, 2005)

Additional Guidance

Center for Watershed Protection. 1998. *Better Site Design: A Handbook for Changing Development Rules in Your Community*. Available from www.cwp.org

Prince George's County, MD. June 1999. *Low-Impact Development Design Strategies: An Integrated Design Approach*. Prince George's County, Maryland, Department of Environmental Resources, Largo, Maryland. Available from www.epa.gov

Massachusetts Executive Office of Environmental Affairs (EOEA). 2005. *Smart Growth Toolkit*. Boston, MA. Available from <http://www.mass.gov/envir/>

Better Site Design Practice #9: Cul-de-sac Reduction

Reduction of Impervious Cover

Description: Minimize the number of cul-de-sacs and incorporate landscaped areas to reduce their impervious cover. The radius of a cul-de-sac should be the minimum required to accommodate emergency and maintenance vehicles. Alternative turnarounds should also be considered.

Key Benefits	Typical Perceived Obstacles and Realities
- Reduces the amount of impervious cover, associated runoff and pollutants generated - Increases aesthetics by allowing for natural or landscaped areas rather than pavement	- Emergency and maintenance vehicles require a large turning radius – <i>Many newer vehicles are available with small turning radii.</i> - School buses require a large turning radius - <i>Verify school bus pick-up plans. Not every cul-de-sac will need to accommodate school bus turning radii.</i> - Homeowners like the “end of the road” appeal of cul-de-sacs – <i>This appeal can be accommodated using loop roads or lots that back onto open space areas.</i> - Local codes may not permit smaller or alternative cul-de-sac designs – <i>Meet with local officials to discuss waivers for alternative designs that will address concerns of access.</i>

USING THIS PRACTICE

- Reduce the radius of the turnaround bulb or consider alternative cul-de-sac design, such as “tee” turn-a-rounds or looping lanes.
- Apply site design strategies that minimize dead-end streets.
- Create a pervious island or a stormwater bioretention area in the middle of the cul-de-sac to reduce impervious area.

Discussion

Alternative turnarounds are end of the street designs that replace fully-paved cul-de-sacs and reduce the amount of impervious cover created in developments. Cul-de-sacs are local access streets with a closed circular end that allows for vehicle turnarounds. Many of these cul-de-sacs can have a radius of more than 40 feet. From a stormwater perspective, cul-de-sacs create a huge bulb of impervious cover, increasing the amount of runoff. For this reason, reducing the size of cul-de-sacs through the use of alternative turnarounds or eliminating them altogether can reduce the amount of impervious cover created at a site.

Numerous alternatives create less impervious cover than the traditional 40-foot cul-de-sac. These alternatives include reducing cul-de-sacs to a 30-foot radius and creating hammerheads, loop roads and pervious islands in the cul-de-sac center (see figures 21, 22 and 23 below).

Sufficient turnaround area is a significant factor to consider in the design of cul-de-sacs. In particular, the types of vehicles entering the cul-de-sac should be considered. Firetrucks, service vehicles and schoolbuses are often cited as needing large turning radii. However, some firetrucks are designed for smaller turning radii. In addition, many newer largeservice vehicles are designed with a tri-axle (requiring a smaller turning radius), and many school buses usually do not enter individual cul-de-sacs.

Another option for designing cul-de-sacs involves the placement of a pervious island in the center. Vehicles only travel along the outside of the cul-de-sac when turning, leaving an unused “island” of pavement in the center. These islands can be attractively landscaped and also designed as bioretention areas to treat stormwater (see practice #14).

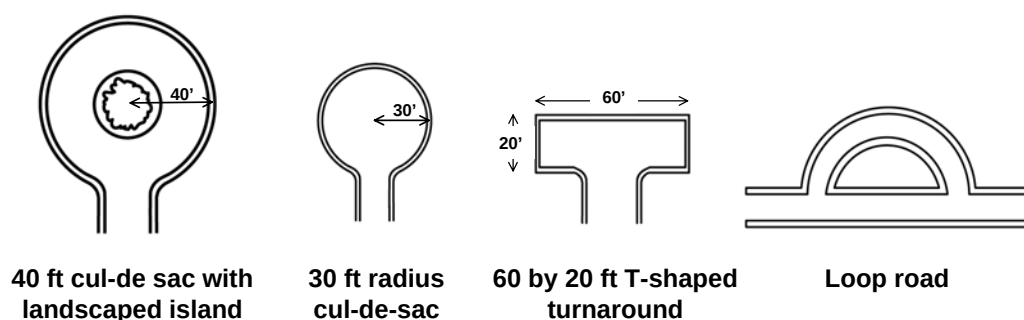


Figure 21: Turnaround Options for Residential Streets
(Source: Adapted from Schueler, 1995)



Figure 22: Loop Road Option
(Source: Center for Watershed Protection, 2005)



Figure 23: T-Shaped Turnaround Option
(Source: Center for Watershed Protection, 2005)

Additional Guidance

Center for Watershed Protection. 1998. *Better Site Design: A Handbook for Changing Development Rules in Your Community*. Available from www.cwp.org

Schueler, T. 1995. *Site Planning for Urban Stream Protection*. Prepared for: Metropolitan Washington Council of Governments. Washington, DC. Center for Watershed Protection, Ellicott City, MD. Available from www.cwp.org

Massachusetts Executive Office of Environmental Affairs (EOEA). 2005. *Smart Growth Toolkit*. Boston, MA. Available from <http://www.mass.gov/envir/>

Better Site Design Practice #10: Building Footprint Reduction

Reduction of Impervious Cover

Description: The impervious footprint of residences and commercial buildings can be reduced by using alternate or taller buildings while maintaining the same floor to area ratio.

Key Benefits

- Reduces the amount of impervious cover and associated runoff and pollutants generated

Typical Perceived Obstacles and Realities

- Taller buildings are perceived to have higher construction and maintenance costs – *Costs for taller buildings and associated parking may be offset by land costs.*
- Local codes may not permit taller buildings – *Consider alternative locations that do allow taller buildings, or meet with local officials to discuss waivers for alternative designs.*

USING THIS PRACTICE

- Use alternate or taller building designs to reduce the impervious footprint of buildings.
- Consolidate functions and buildings or segment facilities to reduce footprints of structures.
- Reduce directly connected impervious areas.

Discussion

In order to reduce the imperviousness associated with the footprint and rooftops of buildings and other structures, alternative and/or vertical (taller) building designs should be considered. Consolidate functions and buildings, as required, or segment facilities to reduce the footprint of individual structures. Figure 24 shows the reduction in impervious footprint by using a taller building design, and figures 25 and 26 show residential examples of reduced footprints.

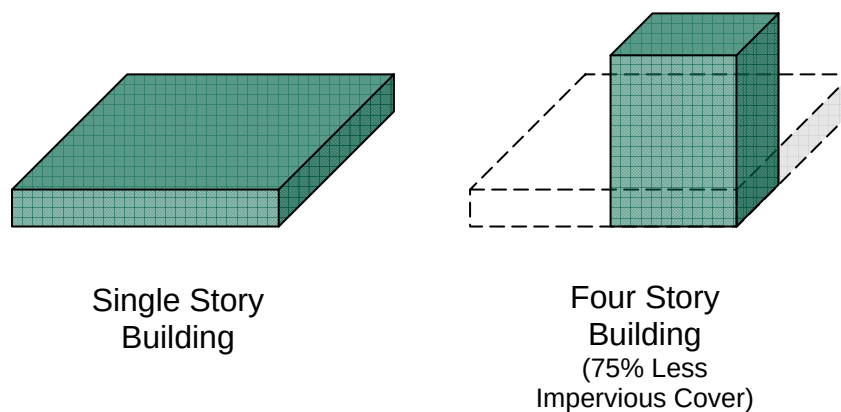


Figure 24: Reduction of Impervious Cover by Building Up Rather than Out

(Source: Georgia Stormwater Manual, 2001)



Figure 25: Taller Houses Create a Smaller Impervious Footprint

(Source: Center for Watershed Protection, 2005)



Figure 26: Taller Apartments Create a Smaller Impervious Footprint

(Source: City of Portland, OR, 2001)

Additional Guidance

Environmental Protection Agency (EPA)

site on smart growth including a focus on community based approaches to reducing sprawl.
www.epa.gov/ebtpages/envismartgrowth.html

Hart, Leslie. 1994. *Guiding Principles of Sustainable Design*. Prepared for the U.S Department of the Interior and the National Parks Service. Available from
<http://www.nps.gov/dsc/dsgncnstr/gpsd>

Better Site Design Practice #11: Parking Reduction

Reduction of Impervious Cover

Description: Reduce the overall imperviousness associated with parking lots by eliminating unneeded spaces, providing compact car spaces, minimizing stall dimensions, incorporating efficient parking lanes, using multi-storied parking decks and using porous paver surfaces or porous concrete in overflow parking areas where feasible.

Key Benefits

- Reduces the amount of impervious cover, associated runoff and pollutants generated
- Reduces construction costs, long-term operation and maintenance costs, and the need for larger stormwater facilities
- Improves aesthetics of an area by increasing vegetative surfaces and reducing the feeling of a large, paved urban area

Typical Perceived Obstacles and Realities

- Developers desire excess parking and fear losing customers during peaks – *The potential loss of customers due to reduced parking is unknown however, often times parking areas are not full during peak periods.*
- Parking may spill over into residential or commercial areas when full – *Include preferential parking provisions for residents or parking enforcement with meters.*
- Trend to larger vehicles such as SUVs – *Stall width requirements in most local parking codes are much larger than the widest SUVs.*
- Structured parking is more expensive than surface lots – *Costs for structured parking may be offset by land costs or by constructing garages above or below an actual building.*
- Porous pavement surfaces are more expensive to install and maintain – *Alternative surfaces may alleviate the need for larger stormwater treatment elsewhere on the site.*

USING THIS PRACTICE

- Reduce the number of unnecessary parking spaces by examining minimum parking ratio requirements, and set a maximum number of spaces.
- Reduce the number of un-needed parking spaces by examining the site's accessibility to mass transit.
- Minimize individual parking stall dimensions.
- Examine the traffic flow of the parking lot design to eliminate un-needed lanes / drive aisles
- Consider parking structures and shared parking arrangements between non-competing uses
- Use alternative porous surface for overflow areas or main parking areas if not a high-traffic parking lot.
- Use landscaping or vegetated stormwater practices in parking lot islands.
- Provide incentives for compact and hybrid cars.

Discussion

Setting maximums for parking spaces, minimizing stall dimensions, using structured parking, encouraging shared parking and using alternative porous surfaces can all reduce the overall parking footprint and site imperviousness.

Many parking lot designs result in far more spaces than actually required. This problem is exacerbated by a common practice of setting parking ratios to accommodate the highest hourly parking during the peak season. By determining average parking demand instead, a lower maximum number of parking spaces can be set to accommodate most of the demand. Table 4 provides examples of conventional parking requirements and compares them to average parking demand. In addition, the number of parking spaces needed may be reduced by a site's accessibility to public transportation.

Table 4: Conventional Minimum Parking Ratios <i>(Source: CWP, 1998)</i>			
Land Use	Parking Requirement		Actual Average Parking Demand
	Parking Ratio	Typical Range	
Single family homes	2 spaces per dwelling unit	1.5–2.5	1.11 spaces per dwelling unit
Shopping center	5 spaces per 1000 ft ² GFA	4.0–6.5	3.97 per 1000 ft ² GFA
Convenience store	3.3 spaces per 1000 ft ² GFA	2.0–10.0	--
Industrial	1 space per 1000 ft ² GFA	0.5–2.0	1.48 per 1000 ft ² GFA
Medical/dental office	5.7 spaces per 1000 ft ² GFA	4.5–10.0	4.11 per 1000 ft ² GFA
GFA = Gross floor area of a building without storage or utility spaces			

Another technique to reduce the parking footprint is to minimize the dimensions of the parking spaces. This can be accomplished by reducing both the length and width of the parking stall. Parking stall dimensions can be further reduced if compact spaces are provided. Another method to reduce the parking area is to incorporate efficient parking lanes such as using one-way drive aisles with angled parking rather than the traditional two-way aisles.

Structured parking decks are another method for significantly reducing the overall parking footprint by minimizing surface parking. Figure 27 shows a parking deck used for a commercial development.

Shared parking in mixed-use areas and structured parking are techniques that can further reduce the conversion of land to impervious cover. A shared parking arrangement could include usage of the same parking lot by an office space that experiences peak parking demand during the weekday with a church that experiences parking demands during the weekends and evenings.



Figure 27: Structured Parking at an Office Park

(Source: Georgia Stormwater Manual, 2001)

Using alternative surfaces such as porous pavers or porous concrete is an effective way to reduce the amount of runoff generated by parking lots. They can replace conventional asphalt or concrete in both new developments and redevelopment projects. Figure 28 is an example of porous pavers used at an overflow lot. Alternative pavers can also capture and treat runoff from other sites area.

When possible, expanses of parking should be broken up with landscaped islands which could include shade trees and shrubs (see Figure 29) or landscaped stormwater management “islands” such as filter strips, swales and bioretention areas (see practice #s 12, 13 & 14)

Additional Guidance

Center for Watershed Protection. 1998. *Better Site Design: A Handbook for Changing Development Rules in Your Community*. Available from www.cwp.org

Prince George’s County, MD. June 1999. *Low-Impact Development Design Strategies: An Integrated Design Approach*. Prince George’s County, Maryland, Department of Environmental Resources, Largo, Maryland. Available from www.epa.gov

Massachusetts Executive Office of Environmental Affairs (EOEA). 2005. *Smart Growth Toolkit*. Boston, MA. Available from <http://www.mass.gov/envir/>

Institute of Traffic Engineers (ITE). 1997. *The Aesthetics of Parking*. Institute of Traffic Engineers, Publication No. LP-090A. Available from www.ite.org

Institute of Traffic Engineers (ITE). 1994. *Guidelines for Parking Facility Location and Design*. Institute of Traffic Engineers, Publication No. RP-022A. Available from www.ite.org

U.S. Environmental Protection Agency. 1999. *Parking Alternatives: Making Way for Urban Infill and Brownfields Redevelopment*. U.S. EPA Urban and Economic Development Division. Washington, DC. Available from <http://www.epa.gov/smartgrowth/publications.htm#articles>



Figure 28: Grass Paver Used for Parking
(Source: Georgia Stormwater Manual, 2001)



Figure 29: Expanses of Parking Area “Broken-Up” with Landscape Features
(Source: MA EOEA, 2005)

**Better Site Design Practice #12:
Vegetated Buffer/Filter Strips**Use of Natural Features and Source
Control for Stormwater Management

Description: Undisturbed natural areas such as forested conservation areas and stream buffers or vegetated filter strips can be used to treat and control stormwater runoff from some areas of a development project.

Key Benefits

- Riparian buffers and undisturbed vegetated areas can be used to filter and infiltrate stormwater runoff
- Natural depressions can provide inexpensive storage and detention of stormwater flows
- A stormwater site design credit can be taken if allowed by the local review authority.

**Typical Perceived Obstacles
and Realities**

- Require space – *Use in areas where land is available and land costs are not significantly high.*
- May be inappropriate in areas of higher pollutant loading due to direct infiltration of pollutants– *Integrate with other practices to ensure adequate treatment prior to discharge.*
- Channelization and premature failure can occur – *This can be alleviated with proper design, construction and maintenance.*

USING THIS PRACTICE

- Direct runoff towards buffers and undisturbed areas using sheet flow or a level spreader to ensure sheet flow.
- Use natural depressions for runoff storage.
- Direct runoff and nature of runoff (sheet flow versus shallow concentrated flow) to buffer/filter strip areas.
- Examine the slope, soils and vegetative cover of the buffer/filter strip.
- Disconnect impervious areas to these areas.

Discussion

Runoff can be directed towards riparian buffers and other undisturbed natural areas delineated in the initial stages of site planning to infiltrate runoff, reduce runoff velocity and remove pollutants. Natural depressions can be used to temporarily store (detain) and infiltrate water, particularly in areas with more permeable (hydrologic soil groups A and B) soils.

The objective in using natural areas for stormwater infiltration is to intercept runoff before it has become substantially concentrated and then distribute this flow evenly (as sheet flow) to the buffer or natural area. This can typically be accomplished using a level spreader, as seen in Figure 30. A mechanism for the bypass of higher-flow events should be provided to reduce erosion or damage to a buffer or undisturbed natural area.

Carefully constructed berms can be placed around natural depressions and below undisturbed vegetated areas with porous soils to provide for additional runoff storage and/or infiltration of flows.

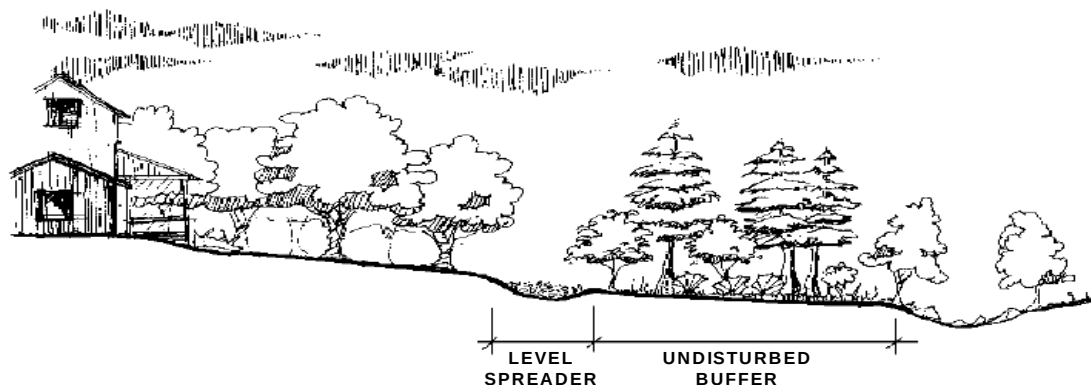


Figure 30: Use of a Level Spreader with a Riparian Buffer
(Source: Georgia Stormwater Manual, 2001)



Figure 31: Use of a Grassed Filter Strip
(Source: MA EOE, 2005)



Figure 32: Use of a Vegetated Filter Strip
(Source: MA EOE, 2005)

Additional Guidance

Center for Watershed Protection. 1998. *Better Site Design: A Handbook for Changing Development Rules in Your Community*. Available from www.cwp.org

Prince George's County, MD. June 1999. *Low-Impact Development Design Strategies: An Integrated Design Approach*. Prince George's County, Maryland, Department of Environmental Resources, Largo, Maryland. Available from www.epa.gov

City of Portland, Oregon. September 2004. *Stormwater Management Manual*. Bureau of Environmental Services, Portland, OR. Available from <http://www.portlandonline.com/bes/>

Better Site Design Practice #13: Open Vegetated Channels

Use of Natural Features and Source
Control for Stormwater Management

Description: The natural drainage paths of a site, or properly designed and constructed vegetated channels can be used instead of constructing underground storm sewers or concrete open channels. Where density, topography, soils, slope and safety issues permit, vegetated open channels can be used in the street right-of-way to convey and treat stormwater runoff from roadways.

Key Benefits

- Reduces the cost of road and storm sewer construction
- Provides for some runoff storage and infiltration, as well as treatment of stormwater
- A stormwater site design credit can be taken if allowed by the local review authority.
- Increases stormwater travel times and lowers peak discharges.

Typical Perceived Obstacles and Realities

- Local codes may not allow swales instead of curb and gutter or closed drainage pipes – *Meet with local officials to discuss waivers for alternative designs.*
- There is a strong perception that swales require more maintenance than curb and gutter or closed drainage pipes – *With the proper design, swales require less maintenance and are less prone to failure.*
- Lack of curbing may increase potential for failure of the pavement at the grass interface – *The potential for failure can be alleviated by hardening the interface by installing grass pavers, geosynthetics or placing a low-rising concrete strip along the pavement edge.*

USING THIS PRACTICE

- Preserve natural flow paths in the site design.
- Direct runoff to natural drainage ways, ensuring that peak flows and velocities will not cause channel erosion.
- Use vegetated open channels (enhanced wet or dry swales or grass channels) and pipes in place of curb and gutter, to convey and treat stormwater runoff.
- Ensure runoff volumes and velocities provide adequate residence times and non-erosive conditions (i.e., use of check dams).

Discussion

Open vegetated channels (see figures 33 and 34) remove pollutants by allowing infiltration and filtration to occur, unlike curb-and-gutter systems or closed piping systems which move water with virtually no treatment. Curb-and-gutter and storm drain systems allow for the quick transport of stormwater, which results in increased peak flow and flood volumes and reduced runoff infiltration. Curb-and-gutter systems also do not provide treatment of stormwater that is often polluted from vehicle emissions, pet waste, lawn runoff and litter. Engineering techniques have advanced the roadside ditches of the past, which suffered from erosion, standing water and

breakup of the road edge. Grass channels and enhanced dry swales are two such alternatives, and with proper installation under the right site conditions, they are excellent methods for treating stormwater on site. In addition, open vegetated channels can be less expensive to install than curb-and-gutter systems. Complete descriptions and design criteria for open channels are included in the New York State Stormwater Management Manual.



Figure 33: Examples of Open Vegetated Channels
(Source: Georgia Stormwater Manual, 2001)



Figure 34: Another Example of an Open Vegetated Channel
(Source: MA EOEA, 2005)

Additional Guidance

Center for Watershed Protection. 1998. *Better Site Design: A Handbook for Changing Development Rules in Your Community*. Available from www.cwp.org

Center for Watershed Protection. August 2003. *New York State Stormwater Management Design Manual*. Prepared for New York State Department of Environmental Conservation, Albany, NY. <http://www.dec.state.ny.us/website/dow/toolbox/swmanual/#Downloads>

Prince George's County, MD. June 1999. *Low-Impact Development Design Strategies: An Integrated Design Approach*. Prince George's County, Maryland, Department of Environmental Resources, Largo, Maryland. Available from www.epa.gov

City of Portland, Oregon. September 2004. *Stormwater Management Manual*. Bureau of Environmental Services, Portland, OR. Available from <http://www.portlandonline.com/bes/>

Massachusetts Executive Office of Environmental Affairs (EOEA). 2005. *Smart Growth Toolkit*. Boston, MA. Available from <http://www.mass.gov/envir/>

**Better Site Design Practice #14:
Bioretention and Rain Gardens**Use of Natural Features and Source
Control for Stormwater Management

Description: Provide stormwater treatment for runoff from impervious surfaces using bioretention areas or rain gardens that can be integrated into required landscaping areas and traffic islands.

Key Benefits

- Breaks up impervious cover, thus allowing for better infiltration and treatment from smaller drainage areas.
- Combines landscaping with stormwater treatment.
- Improves aesthetics.
- Reduces thermal impacts.

**Typical Perceived Obstacles
and Realities**

- Bioretention areas require regular maintenance – *Regular maintenance amounts to general landscaping duties such as trash removal, mulching, weeding and irrigation.*
- Bioretention areas may be expensive – *Using bioretention and other on-site treatment can significantly reduce the need for storm drains, thus reducing stormwater infrastructure costs*

USING THIS PRACTICE

- Integrate bioretention into a parking lot or roadway design.
- Integrate bioretention, or raingardens, into on-lot residential designs.
- Closely examine runoff volumes and velocities to ensure runoff enters bioretention in a distributed manner and in a non-erosive condition.
- Ensure the bioretention has proper pre-treatment.
- Carefully select the landscaping materials required.
- Use as a retrofit or in redevelopment projects.

Discussion

Bioretention areas are naturally vegetated, structural, stormwater-treatment practices that offer an aesthetically pleasing alternative to pavement or traditional detention stormwater facilities. Bioretention areas resemble landscaped depressions and can contain grasses, wildflowers or trees, depending on the size of the facility. Stormwater runoff is routed by slopes, curb cuts or piping into these depressions, where it is allowed to pond temporarily. Eventually, the retained runoff will seep through an organic underground filter system before discharging to an underdrain or infiltrating to the underlying soils. Treatment of stormwater includes attenuation of sediment, metals, bacteria and nutrients. Complete descriptions and design criteria for bioretention are included in the New York State Stormwater Management Manual (<http://www.dec.state.ny.us/website/dow/toolbox/swmanual/#Downloads>)

Bioretention facilities can receive runoff from areas as small as residential lawns or as large as commercial parking areas. These facilities are often used to replace conventional landscaping in parking areas or along roadways. Site limitations include steep slopes, high water tables or

excessively cold climates. Where unusually high sediment loading is expected, proper pretreatment, such as a sediment forebay designed for settling sediments, should be used to reduce the probability of clogging the subsurface filter.

Parking lots should be designed with landscaped stormwater management “islands” which reduce the connected impervious cover of the lot as well as provide for runoff treatment and control in stormwater facilities. Bioretention can be incorporated into roadway design as well, such as in the center of a cul-de-sac or within roadway rights-of-way or easements following sheet flow from the road surface.



Figure 35: Example of a Bioretention Facility in a Parking Lot Island



Figure 36: Example of a Bioretention Facility Along Roadway

Rain gardens are smaller versions of bioretention, typically considered for on-lot residential designs. Rain gardens can be landscaped depressions on the lot used to mitigate rooftop runoff, or can be designed as the low point of a lot to treat on-site stormwater. Rain gardens are constructed as shallow depressions where stormwater runoff will collect during and shortly after a rain event. These areas are vegetated with plants that are both aesthetically pleasing and well suited to an environment periodically inundated with water. Rain gardens can be designed at different scales to suit different levels of runoff. Adequate sizing of these gardens will allow for infiltration of the most common rain events, while runoff from larger events will overflow into other stormwater infrastructure or a receiving water body. Rain gardens are an appropriate practice as long as certain potential site constraints have been considered. Drainage areas cannot be too extensive for rain gardens, and slopes leading to them cannot be too steep because large volumes of rain or runoff moving at excessive velocities will simply overwhelm these facilities. Also, where subsurface soils do not naturally allow for good drainage, these soils should be replaced or mixed with sandier varieties.



Figure 37: Example of an On-Lot Rain Garden
Source (MA EOE, 2005)

Additional Guidance

Center for Watershed Protection. August 2003. *New York State Stormwater Management Design Manual*. Prepared for New York State Department of Environmental Conservation, Albany, NY. <http://www.dec.state.ny.us/website/dow/toolbox/swmanual/#Downloads>

Prince George's County, MD. June 1999. *Low-Impact Development Design Strategies: An Integrated Design Approach*. Prince George's County, Maryland, Department of Environmental Resources, Largo, Maryland. Available from www.epa.gov

City of Portland, Oregon. September 2004. *Stormwater Management Manual*. Bureau of Environmental Services, Portland, OR. Available from <http://www.portlandonline.com/bes/>

Massachusetts Executive Office of Environmental Affairs (EOEA). 2005. *Smart Growth Toolkit*. Boston, MA. Available from <http://www.mass.gov/envir/>

Low Impact Development (LID) Center website: <http://www.lowimpactdevelopment.org/>

Better Site Design Practice #15: Infiltration

Use of Natural Features and Source
Control for Stormwater Management

Description: Use infiltration trenches, basins or leaching chambers to provide groundwater recharge, mimic existing hydrologic conditions and reduce runoff and pollutant export. Permeable paving surfaces may also be used where site conditions are appropriate.

Key Benefits

- Increases recharge to the groundwater.
- Reduces stormwater runoff volume and peak runoff rates; therefore, pipes and basins are smaller.
- Can increase effective developable area on a site because portions of the stormwater system are located underneath the paved areas.
- Grass pavers can improve site appearance by providing vegetation where there would otherwise be only pavement.

Typical Perceived Obstacles and Realities

- Infiltration trenches and dry wells cannot receive untreated stormwater runoff, except rooftop runoff – *Provide proper pretreatment such as grass swales or filter strip.*
- Rehabilitation of failed infiltration trenches and dry wells requires complete reconstruction – *Proper system design, construction and ongoing operation and maintenance will prevent failure.*
- Permeable paving can be prone to clogging from sand and fine sediments that fill void spaces and the joints between pavers – *Avoid permeable paving in high traffic areas where frequent winter sanding is necessary; provide periodic maintenance.*
- Snow plows can catch the edge of grass pavers and some paving stones – *Avoid using in high traffic areas; and attach rollers to the bottom edge of a snowplow to prevent this problem.*

USING THIS PRACTICE

- May be used for roadway or parking impervious areas if adequate pre-treatment is provided; Rooftop runoff may discharge directly to drywells or infiltration chambers.
- The site must have soils with moderate to high infiltration capacities and must have adequate depth to groundwater.
- Certain sites (i.e., pollutant hotspots) require additional pretreatment prior to infiltration.
- Use porous pavers only in low-traffic areas or for pedestrian walkways/plazas.
- Poor soils may preclude aggressive infiltration.

Discussion

Infiltration trenches, dry wells and chambers are standard stormwater management structures that can play an important role in lower-impact site design. Dispersed around the site, these infiltration structures can recharge groundwater and help to maintain or restore the site's natural hydrology. Dry wells, infiltration trenches, and chambers all store water in the void between crushed stone or gravel; the water slowly percolates downward into the subsoil. An overflow outlet is needed for runoff from large storms that cannot be fully infiltrated by the trench or dry well. Complete descriptions and design criteria for infiltration trenches, basins, and drywells are included in the New York State Stormwater Management Manual.

(<http://www.dec.state.ny.us/website/dow/toolbox/swmanual/#Downloads>)



Figure 38: Dry Well
(Source: MA EOE, 2005)



Figure 39: Infiltration Trench
(Source: MA EOE, 2005)



Figure 40: Infiltration Chambers
(Source: MA EOE, 2005)

Because impervious pavement is the primary source of stormwater runoff, better site-design strategies offer permeable paving as an option for parking areas and other hard surfaces. Permeable paving allows rainwater to percolate through the paving and into the ground before it runs off. This approach reduces stormwater runoff volumes and minimizes the pollutants introduced into stormwater runoff from parking areas. All permeable paving systems consist of a durable, load bearing, pervious surface overlying a crushed-stone base that stores rainwater before it infiltrates into the underlying soil.

Permeable paving techniques include porous asphalt, pervious concrete, paving stones and manufactured “grass pavers” made of concrete or plastic. Permeable paving may be used for walkways, patios, plazas, driveways, parking stalls and overflow parking areas.

Permeable paving is appropriate for pedestrian-only areas and for low-volume, low-speed areas such as overflow parking areas, residential driveways, alleys and parking stalls. It can be constructed where the underlying soils have a permeability of at least 0.5 inch per hour. Permeable paving is an excellent technique for dense urban areas because it does not require any additional land. With proper design, cold climates are not a major limitation; porous pavement has been used successfully in Norway, incorporating design features to reduce frost heave.



Figure 41: Examples of Permeable Pavers
(Source: MA EOE, 2005)

Additional Guidance

Center for Watershed Protection. August 2003. *New York State Stormwater Management Design Manual*. Prepared for New York State Department of Environmental Conservation, Albany, NY. <http://www.dec.state.ny.us/website/dow/toolbox/swmanual/#Downloads>

Prince George's County, MD. June 1999. *Low-Impact Development Design Strategies: An Integrated Design Approach*. Prince George's County, Maryland, Department of Environmental Resources, Largo, Maryland. Available from www.epa.gov

Low Impact Development (LID) Center website: <http://www.lowimpactdevelopment.org/>

Metropolitan Area Planning Council (MAPC). 2005. Massachusetts Low Impact Development Toolkit Fact Sheets. Metropolitan Area Planning Council. Boston, MA. Available from www.mapc.org/lid

Better Site Design Practice #16: Rooftop Runoff Reduction Mitigation

Use of Natural Features and Source
Control for Stormwater Management

Description: Direct runoff from residential rooftop areas to pervious areas, lower-impact practices or use “green-roof” (specially designed vegetated rooftops) strategies to reduce rooftop runoff volumes and rates.

Key Benefits

- Sending runoff to pervious areas, lower-impact practices or using green roofs increases overland flow time and reduces peak flows.
- Vegetated and pervious areas can filter and infiltrate stormwater runoff, thus increasing water quality.
- A stormwater site design credit can be taken if allowed by the local review authority.

Typical Perceived Obstacles and Realities

- Wet basements will result from re-directing rooftop runoff – *Careful design and construction inspection will minimize this condition.*
- Re-directed rooftop runoff may increase a property owner’s maintenance burden – *When designed properly, on-lot rain gardens do not require supplemental water.*
- Alternative rooftop runoff mitigation may be costly – *Rain barrels, in fact, are inexpensive and will reduce water-use costs; green roofs reduce heating and cooling costs and roof replacement costs.*

USING THIS PRACTICE

- Direct rooftop runoff to pervious areas such as yards, open channels or vegetated areas.
- Direct rooftop runoff to lower-impact practices such as rain barrels, cisterns, drywells, rain gardens, or stormwater planters.
- Use “green roofs” to reduce stormwater runoff from rooftops.

Discussion

Stormwater quantity and quality benefits can be achieved by routing the runoff from rooftop areas to pervious areas such as lawns, landscaping, filter strips and vegetated channels. Much like the use of undisturbed buffers and natural areas (see practices #1 & 2), revegetated areas such as lawns and engineered filter strips and vegetated channels can act as biofilters for stormwater runoff and provide for infiltration in more permeable soils (hydrologic groups A and B).

Cisterns and rain barrels are designed to retain water that runs off of roofs for an extended period. Rain barrels are smaller structures ranging generally from 20 to 100 gallons, while cisterns can store thousands of gallons, depending on the design. Construction material for rain barrels is generally plastic although other materials such as wooden barrels have been used. Cisterns can be constructed of metal, wood, concrete or plastic. Stormwater stored in these structures is generally used for irrigation,



Figure 42: Rain Barrel
(Source: MA EOE, 2005)

although more complex designs incorporate that water into everyday uses such as toilet and shower water. Rain barrels and cisterns have historically been used in more arid climates where water is scarce for much of the year. More recently, however, these technologies have been applied in less arid climates because of the savings in water costs and overall increases in water demands.

Drywells, as described under practice #15, are underground chambers surrounded by crushed stone, typically used to infiltrate runoff from rooftops. Drywells are well suited for residential applications or small buildings.

Rain gardens, as described under practice #14, are constructed as shallow depressions where stormwater runoff will collect during and shortly after a rain event. These areas are vegetated with plantings that are both aesthetically pleasing and well suited to an environment periodically inundated with water. Rain gardens are well suited for small drainage areas such as residential building rooftops.



Figure 43: Stormwater Planters
(Source: MA EOE, 2005)

Stormwater planters are a small-scale engineered management strategy designed to treat limited volumes of stormwater runoff in discrete areas. Planters generally look like large vaulted plant boxes and can contain anything from basic wildflower communities to complex arrangements of trees and flowering shrubs. Other stormwater planters, known as “tree boxes,” are simply modified side tree enclosures that are installed below the surface of the sidewalk. Stormwater is generally routed into these systems from roofs via downspouts, where it runs through various filtering media and is also subject to uptake from vegetation. Because of the compact and self-contained nature of these practices, they are best suited to handling rooftop runoff. Multiple units can be used to treat large-scale commercial developments.

Green rooftops are rooftop areas that have been landscaped with grasses, shrubs and, in some cases, trees. “Intensive” rooftops are designed with pedestrian access and deep soil layers to provide for complex planting schemes. “Extensive” rooftops are designed with a more shallow soil foundation and generally do not incorporate pedestrian access. Stormwater runoff is either retained until uptake can occur, or eventually runs off the roof with considerably less pollution than would be contained in runoff from a standard impervious rooftop. Beyond stormwater pretreatment, benefits of green rooftops include reduction of the “heat island” effect, extended life of the rooftop, aesthetic appeal and increased useable area. Green rooftops are best suited for

the construction of new buildings because the special structural considerations necessary for these applications can be incorporated early in the design phase. Retrofits to older buildings are often possible, however, owing to the fact that the rooftops were designed well beyond the minimum necessary support capacity.

Additional Guidance



Figure 44: Green Roof
(Source: MA EOE, 2005)

Prince George's County, MD. June 1999. *Low-Impact Development Design Strategies: An Integrated Design Approach*. Prince George's County, Maryland, Department of Environmental Resources, Largo, Maryland. Available from www.epa.gov

Rhode Island Department of Environmental Management. January 2005. *The Urban Environmental Design Manual*. Rhode Island Department of Environmental Management, Providence, Rhode Island. Available from <http://www.dem.state.ri.us/programs/bpoladm/suswshed/pubs.htm>

City of Portland, Oregon. September 2004. *Stormwater Management Manual*. Bureau of Environmental Services, Portland, OR. Available from <http://www.portlandonline.com/bes/>

Green Roofs for Healthy Cities website - www.greenroofs.org

The international greenroof industry's resource and online information portal.
www.greenroofs.com

Better Site Design Practice #17: Stream Daylighting for Redevelopment Projects

Utilization of Natural Features and Source
Control for Stormwater Management

Description: Stream Daylight previously-culverted/piped streams to restore natural habitats, better attenuate runoff and help reduce pollutant loads where feasible and practical.

Key Benefits

- The aesthetic appeal of daylighted streams can be expected to add appeal to neighborhoods or urban areas.
- Improves water quality.
- Prevents flooding by increasing storage and reducing peak flows.
- Increases habitat and wildlife value.
- Increases pedestrian traffic and general public use.
- Increases property values.

Typical Perceived Obstacles and Realities

- Daylighting a stream can be expensive - *Costs for daylighting streams are often comparable to costs for replacing culverts.*
- Maintenance of daylighted stream areas can be intensive during the first years the stream is established – *Once the banks are well established, regular maintenance is similar to that required in any public green space such as trash removal, mowing and general housekeeping.*
- Finding the original stream channel may be difficult – *examine historic records, soils, and up and downstream channel characteristics.*
- Political backing and public support is more difficult for daylighting streams than for surface restoration because the culvert is not seen – *Provide proper public education and outreach about the benefits and how safety issues will be addressed.*

USING THIS PRACTICE

- Daylighting should be considered when a culvert replacement is scheduled.
- Restore historic drainage patterns by removing closed drainage systems and constructing stabilized, vegetated streams.
- Carefully examine flooding potential, utility impacts and/or prior contaminated sites.
- Consider runoff pretreatment and erosion potential of restored streams/rivers.

Discussion

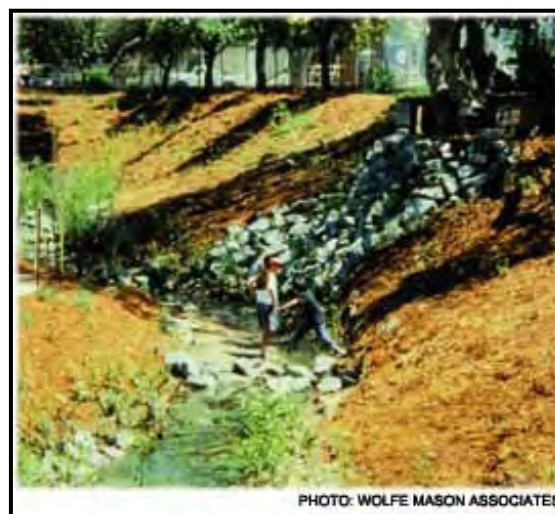
Stream daylighting involves uncovering a stream or a section of a stream that had been artificially enclosed in the past to accommodate development. The original enclosure of rivers and streams often took place in urbanized areas through the use of large culvert operations that often integrated the stormsewer system and combined sanitary sewers. The daylighting operation, therefore, often requires overhauls or updating of storm-drain systems and re-

establishing stream banks where culverts once existed. When the operation is complete, what was once a linear pipe of heavily polluted water can become a meandering stream with dramatic improvements to both aesthetics and water quality. In some cases, instead of creating a natural channel for the daylighted stream, the culvert is simply replaced with a concrete channel.

Where combined sewer overflow (CSO) separation and other upgrades to storm-sewer systems are part of a daylighting project, significant water-quality improvements can be expected during wet-weather events. Also, as ultraviolet radiation is one of the most effective ways to eliminate pathogens in water, exposing these streams to sunlight could significantly decrease pathogen counts in the surface water.

Stream daylighting can play an integral role in neighborhood restoration and site redevelopment efforts. Aside from improvements to infrastructure, stream daylighting can restore floodplain and aquatic habitat areas, reduce runoff velocities and be integrated into pedestrian walkway or bike-path design.

Stream daylighting can generally be applied most successfully to sites with considerable open or otherwise vacant space. This space is required to: 1) Potentially reposition the stream in its natural stream bed; 2) Accommodate the meandering that will be required if a natural channel is being designed and 3) Provide adjacent floodplain area to store water in large storm-flow situations. However, where a concrete channel will replace a culverted stream, these projects require significantly less space than those designed for a natural streambed.



**Figure 45: Before and After Daylighting
Blackberry Creek in Berkeley, CA**
(Source: Stormwater Magazine, Nov/Dec 2001)



Figure 46: Daylighting Arcadia Creek in an Urban Area in Kalamazoo, MI
(Source: Stormwater Magazine, Nov/Dec 2001)

Additional Guidance

Rhode Island Department of Environmental Management. January 2005. *The Urban Environmental Design Manual*. Rhode Island Department of Environmental Management, Providence, Rhode Island. Available from <http://www.dem.state.ri.us/programs/bpoladm/suswshed/pubs.htm>

Blankinship, Donna Gordon. Jan/Feb 2005. *Creeks are Coming Back into the Light*. Article from Stormwater Magazine Vol. 6, No. 1. Forester Communications. Caledonia, MI. Available from www.stormh2o.com

Pinkham, Richard. Nov/Dec 2001. *Daylighting: New Life for Buried Streams*. Article from Stormwater Magazine Vol. 2, No. 6. Forester Communications. Caledonia, MI. Available from www.stormh2o.com

Better Site Design Practice #18: Tree Planting

Description: Plant or conserve trees at new or redevelopment sites to reduce stormwater runoff, increase nutrient uptake, provide bank stabilization, provide shading and provide wildlife habitat. Trees can be used for applications such as landscaping, stormwater management practice areas, conservation areas and erosion and sediment control.

Key Benefits	Typical Perceived Obstacles and Realities
• Reduces construction and maintenance costs. • Increases property values. • Reduces urban heat island, decreases heating and cooling costs, blocks UV radiation. • Buffers wind and noise. • Planting trees in stormwater management practices can increase nutrient uptake, reduce runoff through rainfall interception and evapotranspiration (ET), aid infiltration, provide wildlife habitat, provide shading, discourage geese and reduce mowing costs. • Tree planting can be applied to stormwater credit #6, "Riparian Reforestation."	• Local codes may restrict trees in certain areas – <i>Consult with local officials to discuss waivers for alternative designs.</i> • Trees may not survive through construction or in certain urban environments – <i>Trees will survive with proper tree selection, landscape design and protection during construction.</i> • Planting or preserving trees may be expensive – <i>Conserving or planting trees increases property values.</i> • Native vegetation may harbor undesirable wildlife and insects - <i>Most people enjoy viewing wildlife; native vegetation does not provide a food source for most vermin; continued education is necessary to show that humans and wildlife can co-exist, even at the neighborhood level.</i>

USING THIS PRACTICE

- Conserve existing trees during construction by performing an inventory of the existing forest and identifying trees to protect.
- Design the development with tree conservation in mind, protect trees during construction and protect trees after construction.
- Plant trees at development sites by first selecting the planting sites and then evaluate and improve the planting sites. Trees should be planted in stormwater-management practices and other open spaces.
- Tree types and locations should be chosen to withstand the constraints of an urban setting.

Discussion

Few communities require that trees and native vegetation be conserved during the development process. However, native trees, shrubs, and grasses are important contributors to the overall quality and viability of the environment. Some tools that can be used for tree conservation include open space development practices, planting of vegetation in street, clearing and grading restrictions that include preservation of trees and native vegetation and the addition of vegetation to parking-lot islands.

Developers, engineers or landscape architects can incorporate more trees into a development site, using a three-prong approach: Conserving existing trees during construction; planting trees in stormwater management practices; and Planting trees in other open spaces.

Trees can be conserved and planted at both new development and redevelopment or infill projects. On currently forested development sites, it is most important to conserve existing trees, particularly high-quality stands or large, mature trees (Figure 47). To conserve existing trees, developers should inventory existing forest at the site to identify the best trees and forest to protect, design the development around these trees and take measures to ensure the protection of trees both during and after construction.

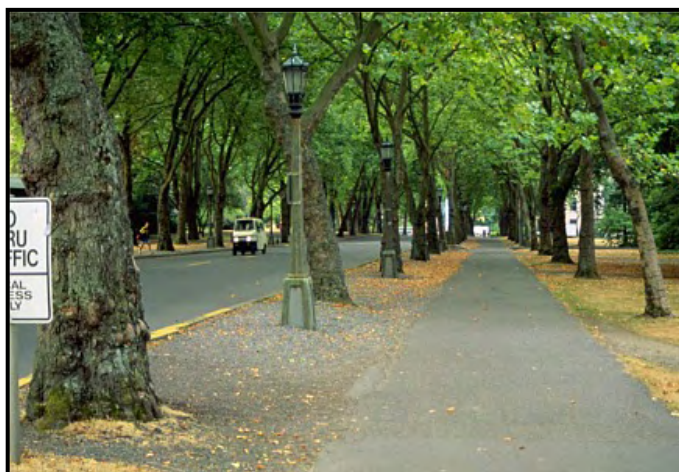


Figure 47: Mature Trees Conserved During Development

(Photo Sources: Randall Arendt and Ed Gilman)

Where tree conservation is not an option, urban development sites provide many opportunities to plant new trees, such as in stormwater management practices (SMPs) and other pervious areas of the site. Some SMPs are not traditionally considered appropriate for tree planting; however, planting trees and shrubs in certain areas of specific SMPs can enhance the aesthetic appeal and even improve their performance.

The remaining pervious areas of the site that make good candidates for tree planting and are often overlooked and include local road rights-of-way, landscaped islands in cul-de-sacs or traffic circles and parking lots. Private lawns may also constitute a significant portion of green space at the site, and the developer should certainly strive to conserve or plant trees in these areas

as well. These urban planting sites may have harsh soil and environmental conditions that should be addressed through appropriate species selection or proper site preparation prior to planting.

Conserving or planting trees at development sites can be done to meet forest conservation, landscaping or other site-design requirements to enhance the appeal of the development and, therefore, increase land and housing values, to reduce costs of construction and stormwater management and to provide many additional benefits summarized above.

Additional Guidance

Cappiella, K., T. Schueler, T. Wright. 2004. *Urban Watershed Forestry Manual*. Available from www.cwp.org

American Forests website: www.americanforests.org

City of Toronto Tree Advocacy Planting Program website:
<http://www.city.toronto.on.ca/parks/treeadvocacy.htm>

2.3 BETTER SITE DESIGN CASE STUDIES

The following case studies illustrate how better site-design practices can be successfully incorporated into site planning. A comparison to a conventional design approach illustrates the opportunities presented by better site-design practices to meet stormwater management criteria in addition to identifying the obstacles for implementing such practices.

2.3.1 *Medium Density Residential Subdivision Case Study*

A conventional residential subdivision design on a parcel is shown in Figure 48. The entire parcel, except for the subdivision amenity area (clubhouse and tennis courts), is used for lots. The entire site is cleared and mass graded, and no attempt is made to fit the road layout to the existing topography. Because of the clearing and grading, all of the existing tree cover and vegetation and topsoil are removed, dramatically altering both the natural hydrology and drainage of the site. The wide residential streets create unnecessary impervious cover and a curb-and-gutter system that carries stormwater flows to the storm sewersystem. No provision for non-structural stormwater treatment is provided on the subdivision site.

A residential subdivision employing stormwater better site design practices is presented in Figure 49. This subdivision configuration preserves a quarter of the property as undisturbed open space and vegetation. The road layout is designed to fit the topography of the parcel, following the high points and ridgelines. The natural drainage patterns of the site are preserved and are used to provide natural stormwater treatment and conveyance. Narrower streets reduce impervious cover, and open vegetated channels provide for treatment and conveyance of roadway and driveway runoff. Bioretention islands at the ends of cul-de-sacs also reduce impervious cover and provide stormwater treatment functions. When constructing and building homes, only the building envelopes of the individual lots are cleared and graded, further preserving the natural hydrology of the site.

2.3.2 *Commercial Development Case Study*

Figure 50 shows a conventional commercial development containing a supermarket, drugstore, smaller shops and a restaurant on an adjacent lot. The majority of the parcel is a concentrated parking-lot area. The only pervious area is a small, replanted vegetation area acting as a buffer between the shopping center and adjacent land uses. Stormwater quality and quantity control are provided by a wet, extended-detention pond in the corner of the parcel.

A better site-design commercial development can be seen in Figure 51. Here the retail buildings are dispersed on the property, providing more of an “urban village” feel, with pedestrian access between the buildings. The parking is broken up, and bioretention areas for stormwater treatment are built into parking-lot islands. A large bioretention area which serves as open green space is located at the main entrance to the shopping center. A larger undisturbed buffer has been preserved on the site. Because the bioretention areas and buffer provide water-quality treatment, only a dry, extended-detention basin is needed for water-quantity control.



Figure 48: Residential Subdivision - Conventional Design

(Source: Georgia Stormwater Manual, 2001)

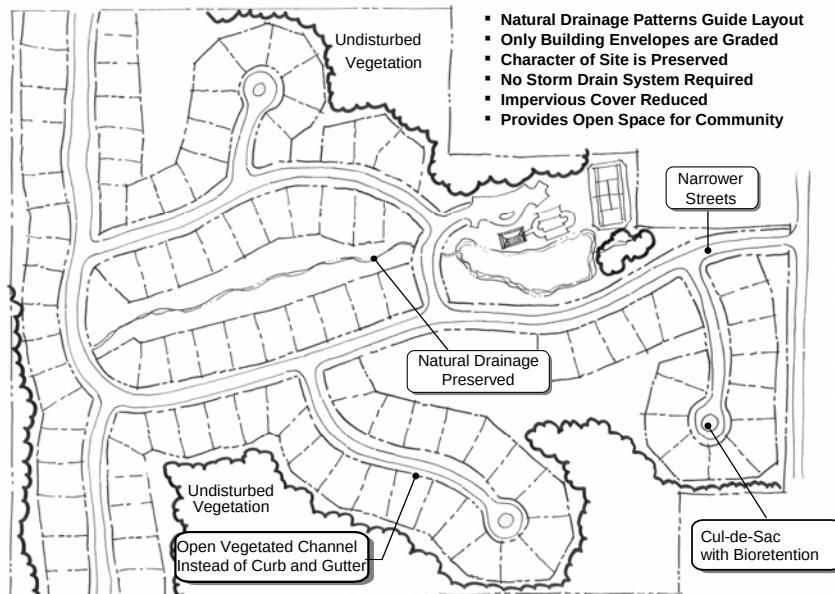


Figure 49: Residential Subdivision - Better Site Design

(Source: Georgia Stormwater Manual, 2001)

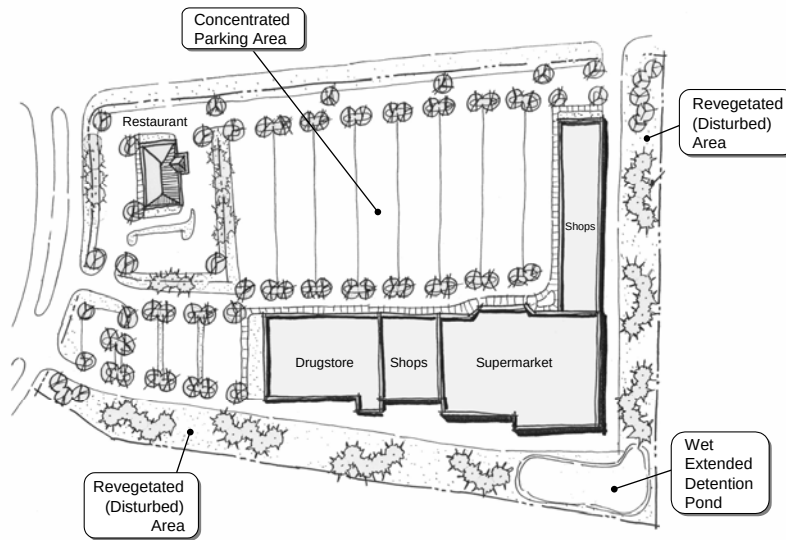


Figure 50: Commercial Development - Conventional Design
(Source: Georgia Stormwater Manual, 2001)

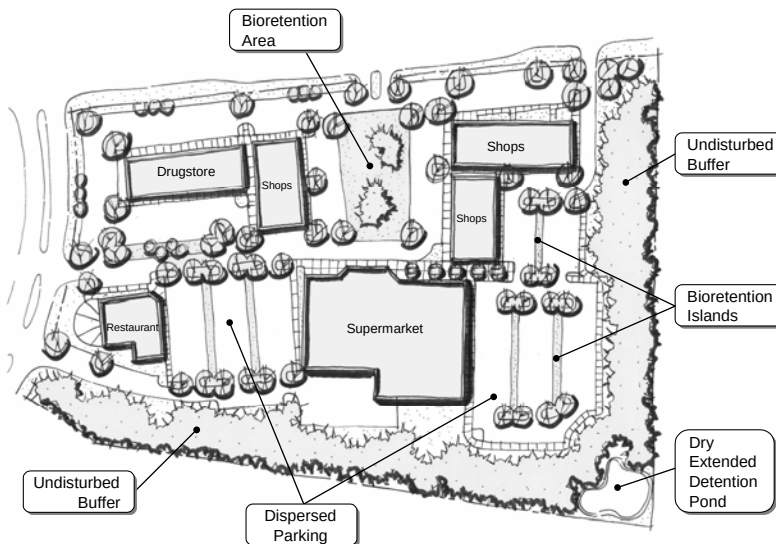


Figure 51: Commercial Development - Better Site Design
(Source: Georgia Stormwater Manual, 2001)

4 REFERENCES

- Arendt, Randall. 1994. *Rural by Design: Maintaining Small Town Character*. Planners Press, American Planning Association. Chicago, IL.
- Arendt, Randall. 1994. *Designing Open Space Subdivisions: A Practical Step-by-Step Approach*. Natural Lands Trust, Inc. Media, PA.
- Arendt, Randall. 1996. *Conservation Design for Subdivisions: A Practical Guide to Creating Open Space Networks*. American Planning Association. Chicago, IL.
- Atlanta Regional Commission. August 2001. *Georgia Stormwater Management Manual*. Atlanta, GA.
- Berkshire Regional Planning Commission. 2003. *The Massachusetts Buffer Manual: Using Vegetated Buffers to Protect our Lakes and Rivers*. Prepared for the Massachusetts Department of Environmental Protection. Boston, MA.
- Blankinship, Donna Gordon. Jan/Feb 2005. *Creeks are Coming Back into the Light*. Article from Stormwater Magazine Vol. 6, No. 1. Forester Communications. Caledonia, MI.
- Cappiella, K., T. Schueler, T. Wright. 2004. *Urban Watershed Forestry Manual*.
- Center for Watershed Protection. August 2003. *New York State Stormwater Management Design Manual*. Prepared for New York State Department of Environmental Conservation, Albany, NY. <http://www.dec.state.ny.us/website/dow/toolbox/swmanual/#Downloads>
- Center for Watershed Protection. 1998. *Better Site Design: A Handbook for Changing Development Rules in Your Community*. Center for Watershed Protection, Ellicott City, MD.
- Center for Watershed Protection. July 1998. *Nutrient Loading from Conventional and Innovative Site Development*. Prepared for: Chesapeake Research Consortium. Center for Watershed Protection, Ellicott City, MD.
- Center for Watershed Protection. 1998. *Better Site Design: An Assessment of the Better Site Design Principles for Communities Implementing the Chesapeake Bay Preservation Act..*. Center for Watershed Protection, Ellicott City, MD.
- Center for Watershed Protection. 2000. *Introduction to Better Site Design Slideshow*. Available from www.stormwatercenter.net
- Center for Watershed Protection. October 2001. *Redevelopment Roundtable Consensus Agreement: Smart Site Practices for Redevelopment and Infill Practices*. Center for Watershed Protection, Ellicott City, MD.

Center for Watershed Protection. 2005. *Better Site Design and Changing the Development Rules Slideshow*. Presented at New Windsor, New York, June 21-22, 2005.

DDNREC. 1997. *Conservation Design for Stormwater Management*. Delaware Department of Natural Resources and Environmental Control and the Environmental Management Centre of the Bradywine Conservancy.

Green Roofs for Healthy Cities website: www.greenroofs.org

Green Roofs.com website: www.greenroofs.com - The international greenroof industry's resource and online information portal.

Hart, Leslie. 1994. *Guiding Principles of Sustainable Design*. Prepared for the U.S Department of the Interior and the National Parks Service.

Institute of Traffic Engineers (ITE). 2001. *Residential Streets, Third Edition*. Institute of Traffic Engineers, Publication No. LP-630.

Institute of Traffic Engineers (ITE). 1999. *Traditional Neighborhood Development Street Design Guidelines*. Institute of Traffic Engineers, Publication No. RP-027A.

Institute of Traffic Engineers (ITE). 1997. *The Aesthetics of Parking*. Institute of Traffic Engineers, Publication No. LP-090A.

Institute of Traffic Engineers (ITE). 1997. *Designing Neighborhood Streets*. Institute of Traffic Engineers, Publication No. VHS-027.

Institute of Traffic Engineers (ITE). 1994. *Guidelines for Parking Facility Location and Design*. Institute of Traffic Engineers, Publication No. RP-022A.

Litman, Todd Alexander. 2004. *The Economic Value of Walkability*. Victoria Transport Policy Institute. Victoria, British Columbia.

Low Impact Development (LID) Center website: <http://www.lowimpactdevelopment.org/>

Maine Department of Environmental Protection. 1998. *The Buffer Handbook: A Guide to Creating Vegetated Buffers for Lakefront Properties*. Maine DEP. Augusta, ME.

Massachusetts Executive Office of Environmental Affairs (EOEA). 2005. *Smart Growth Toolkit*. Boston, MA.

Metropolitan Area Planning Council (MAPC). 2005. *Massachusetts Low Impact Development Toolkit Fact Sheets*. Metropolitan Area Planning Council. Boston, MA.

MPCA. 1989. *Best Management Practices for Minnesota*. Minnesota Pollution Control Agency. Minneapolis, MN.

Pinkham, Richard. Nov/Dec 2001. *Daylighting: New Life for Buried Streams*. Article from Stormwater Magazine Vol. 2, No. 6. Forester Communications. Caledonia, MI.

City of Portland, Oregon. September 2004. *Stormwater Management Manual*. Bureau of Environmental Services, Portland, OR.

Prince George's County, MD. June 1999. *Low-Impact Development Design Strategies: An Integrated Design Approach*. Prince George's County, Maryland, Department of Environmental Resources, Largo, Maryland.

Rhode Island Department of Environmental Management. January 2005. *The Urban Environmental Design Manual*. Rhode Island Department of Environmental Management, Providence, Rhode Island.

Schueler, T. 1995. *Site Planning for Urban Stream Protection*. Prepared for: Metropolitan Washington Council of Governments. Washington, DC. Center for Watershed Protection, Ellicott City, MD.

Urban Land Institute (ULI). 1992. *Density by Design*. James W. Wetling and Lloyd W. Bookout, editors. Urban Land Institute, Washington, DC.

U.S. Environmental Protection Agency. 1999. *Parking Alternatives: Making Way for Urban Infill and Brownfields Redevelopment*. U.S. EPA Urban and Economic Development Division. Washington, DC.



**New York State
DEPARTMENT OF ENVIRONMENTAL CONSERVATION**

Division of Water

Deep-Ripping and Decompaction

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**New York State
Department of Environmental Conservation**

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Alternative Stormwater Management Deep-Ripping and Decompaction

Description

The two-phase practice of 1) “Deep Ripping;” and 2) “Decompaction” (deep subsoiling), of the soil material as a step in the cleanup and restoration/landscaping of a construction site, helps mitigate the physically induced impacts of soil compression; i.e.: soil compaction or the substantial increase in the bulk density of the soil material.

Deep Ripping and Decompaction are key factors which help in restoring soil pore space and permeability for water infiltration. Conversely, the physical actions of cut-and-fill work, land grading, the ongoing movement of construction equipment and the transport of building materials throughout a site alter the architecture and structure of the soil, resulting in: the mixing of layers (horizons) of soil materials, compression of those materials and diminished soil porosity which, if left unchecked, severely impairs the soil’s water holding capacity and vertical drainage (rainfall infiltration), from the surface downward.

In a humid climate region, compaction damage on a site is virtually guaranteed over the duration of a project. Soil in very moist to wet condition when compacted, will have severely reduced permeability. Figure 1 displays the early stage of the deep-ripping phase (Note that all topsoil was stripped prior to construction access, and it remains stockpiled until the next phase – decompaction – is complete). A heavy-duty tractor is pulling a three-shank ripper on the first of several series of incrementally deepening passes through the construction access corridor's densely compressed subsoil material. Figure 2 illustrates the approximate volumetric composition of a loam surface soil when conditions are good for plant growth, with adequate natural pore space for fluctuating moisture conditions.



Fig. 1. A typical deep ripping phase of this practice, during the first in a series of progressively deeper “rips” through severely compressed subsoil.

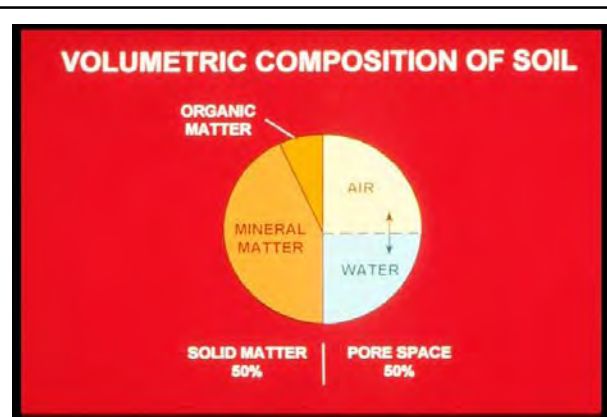


Fig. 2. About 50% of the volume of undisturbed loam surface soil is pore space, when soil is in good condition for plant growth. Brady, 2002.

Recommended Application of Practice

The objective of Deep Ripping and Decompaction is to effectively fracture (vertically and laterally) through the thickness of the physically compressed subsoil material (see Figure 3), restoring soil porosity and permeability and aiding infiltration to help reduce runoff. Together with topsoil stripping, the “two-phase” practice of Deep Ripping and Decompaction first became established as a “best management practice” through ongoing success on commercial farmlands affected by heavy utility construction right-of-way projects (transmission pipelines and large power lines).

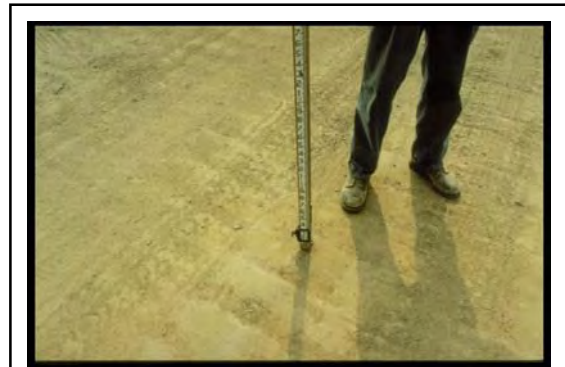


Fig. 3. Construction site with significant compaction of the deep basal till subsoil extends 24 inches below this exposed cut-and-fill work surface.

Soil permeability, soil drainage and cropland productivity were restored. For broader construction application, the two-phase practice of Deep Ripping and Decompaction is best adapted to areas impacted with significant soil compaction, on contiguous open portions of large construction sites and inside long, open construction corridors used as temporary access over the duration of construction. Each mitigation area should have minimal above-and-below-ground obstructions for the easy avoidance and maneuvering of a large tractor and ripping/decompacting implements. Conversely, the complete two-phase practice is not recommended in congested or obstructed areas due to the limitations on tractor and implement movement.

Benefits

Aggressive “deep ripping” through the compressed thickness of exposed subsoil before the replacement/respreading of the topsoil layer, followed by “decompaction,” i.e.: “sub-soiling,” through the restored topsoil layer down into the subsoil, offers the following benefits:

- Increases the project (larger size) area’s direct surface infiltration of rainfall by providing the open site’s mitigated soil condition and lowers the demand on concentrated runoff control structures
- Enhances direct groundwater recharge through greater dispersion across and through a broader surface than afforded by some runoff-control structural measures
- Decreases runoff volume generated and provides hydrologic source control
- May be planned for application in feasible open locations either alone or in

conjunction with plans for structural practices (e.g., subsurface drain line or infiltration basin) serving the same or contiguous areas

- Promotes successful long-term revegetation by restoring soil permeability, drainage and water holding capacity for healthy (rather than restricted) root-system development of trees, shrubs and deep rooted ground cover, minimizing plant drowning during wet periods and burnout during dry periods.

Feasibility / Limitations

The effectiveness of Deep Ripping and Decompaction is governed mostly by site factors such as: the original (undisturbed) soil's hydrologic characteristics; the general slope; local weather/timing (soil moisture) for implementation; the space-related freedom of equipment/implement maneuverability (noted above in **Recommended Application of Practice**), and by the proper selection and operation of tractor and implements (explained below in **Design Guidance**). The more notable site-related factors include:

Soil

In the undisturbed condition, each identified soil type comprising a site is grouped into one of four categories of soil hydrology, Hydrologic Soil Group A, B, C or D, determined primarily by a range of characteristics including soil texture, drainage capability when thoroughly wet, and depth to water table. The natural rates of infiltration and transmission of soil-water through the undisturbed soil layers for Group A is "high" with a low runoff potential while soils in Group B are moderate in infiltration and the transmission of soil-water with a moderate runoff potential, depending somewhat on slope. Soils in Group C have slow rates of infiltration and transmission of soil-water and a moderately high runoff potential influenced by soil texture and slope; while soils in Group D have exceptionally slow rates of infiltration and transmission of soil-water, and high runoff potential.

In Figure 4, the profile displays the undisturbed horizons of a soil in Hydrologic Soil Group C and the naturally slow rate of infiltration through the subsoil. The slow rate of infiltration begins immediately below the topsoil horizon (30 cm), due to the limited amount of macro pores, e.g.: natural subsoil fractures, worm holes and root channels. Infiltration after the construction-induced mixing and compression of such subsoil material is virtually absent; but can be restored back to this natural level with the two-phase practice of deep ripping and decompaction, followed by the permanent establishment of an appropriate, deep taproot

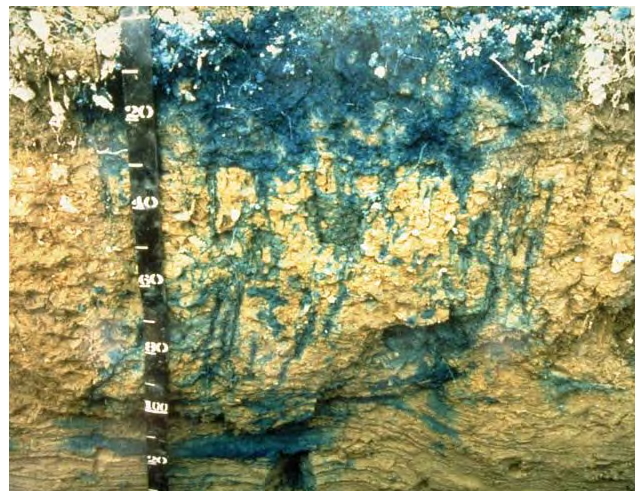


Fig. 4. Profile (in centimeters) displaying the infiltration test result of the natural undisturbed horizons of a soil in Hydrologic Soil Group C.

lawn/ground cover to help maintain the restored subsoil structure. Infiltration after construction-induced mixing and compression of such subsoil material can be notably rehabilitated with the Deep Ripping and Decompaction practice, which prepares the site for the appropriate long-term lawn/ground cover mix including deep taproot plants such as clover, fescue or trefoil, etc. needed for all rehabilitated soils.

Generally, soils in Hydrologic Soil Groups A and B, which respectively may include deep, well-drained, sandy-gravelly materials or deep, moderately well-drained basal till materials, are among the easier ones to restore permeability and infiltration, by deep ripping and decompaction. Among the many different soils in Hydrologic Soil Group C are those unique glacial tills having a natural fragipan zone, beginning about 12 to 18 inches (30 – 45cm), below surface. Although soils in Hydrologic Soil Group C do require a somewhat more carefully applied level of the Deep Ripping and Decompaction practice, it can greatly benefit such affected areas by reducing the runoff and fostering infiltration to a level equal to that of pre-disturbance.

Soils in Hydrologic Soil Group D typically have a permanent high water table close to the surface, influenced by a clay or other highly impervious layer of material. In many locations with clay subsoil material, the bulk density is so naturally high that heavy trafficking has little or no added impact on infiltration; and structural runoff control practices rather than Deep Ripping and Decompaction should be considered.

The information about Hydrologic Soil Groups is merely a general guideline. Site-specific data such as limited depths of cut-and-fill grading with minimal removal or translocation of the inherent subsoil materials (as analyzed in the county soil survey) or, conversely, the excavation and translocation of deeper, unconsolidated substratum or consolidated bedrock materials (unlike the analyzed subsoil horizons' materials referred to in the county soil survey) should always be taken into account.

Sites made up with significant quantities of large rocks, or having a very shallow depth to bedrock, are not conducive to deep ripping and decompaction (subsoiling); and other measures may be more practical.

Slope

The two-phase application of 1) deep ripping and 2) decompaction (deep subsoiling), is most practical on flat, gentle and moderate slopes. In some situations, such as but not limited to temporary construction access corridors, inclusion areas that are moderately steep along a project's otherwise gentle or moderate slope may also be deep ripped and decompacted. For limited instances of moderate steepness on other projects, however, the post-construction land use and the relative alignment of the potential ripping and decompaction work in relation to the lay of the slope should be reviewed for safety and practicality. In broad construction areas predominated by moderately steep or steep slopes, the practice is generally not used.

Local Weather/Timing/Soil Moisture

Effective fracturing of compressed subsoil material from the exposed work surface, laterally and vertically down through the affected zone is achieved only when the soil material is moderately dry to moderately moist. Neither one of the two-phases, deep ripping nor decompaction (deep

subsoiling), can be effectively conducted when the soil material (subsoil or replaced topsoil) is in either a “plastic” or “liquid” state of soil consistency. Pulling the respective implements legs through the soil when it is overly moist only results in the “slicing and smearing” of the material or added “squeezing and compression” instead of the necessary fracturing. Ample drying time is needed for a “rippable” soil condition not merely in the material close to the surface, but throughout the material located down to the bottom of the physically compressed zone of the subsoil.

The “poor man’s Atterberg field test” for soil plasticity is a simple “hand-roll” method used for quick, on-site determination of whether or not the moisture level of the affected soil material is low enough for: effective deep ripping of subsoil; respreading of topsoil in a friable state; and final decompaction (deep subsoiling). Using a sample of soil material obtained from the planned bottom depth of ripping, e.g.: 20 - 24 inches below exposed subsoil surface, the sample is hand rolled between the palms down to a 1/8-inch diameter thread. (Use the same test for stored topsoil material before respreading on the site.) If the respective soil sample crumbles apart in segments no greater than 3/8 of an inch long, by the time it is rolled down to 1/8 inch diameter, it is low enough in moisture for deep ripping (or topsoil replacement), and decompaction. Conversely, as shown in Figure 5, if the rolled sample stretches out in increments greater than 3/8 of an inch long before crumbling, it is in a “plastic” state of soil consistency and is too wet for subsoil ripping (as well as topsoil replacement) and final decompaction.

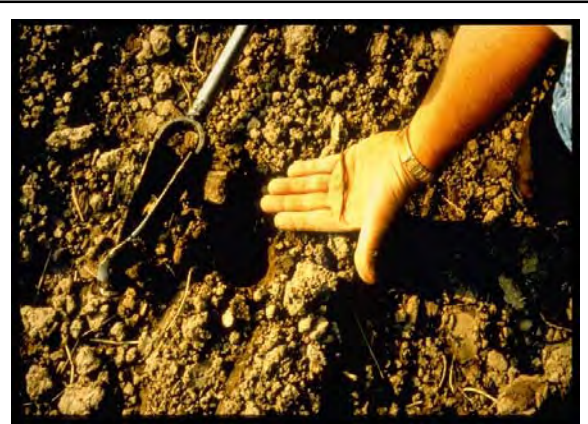


Fig. 5. Augered from a depth of 19 inches below the surface of the replaced topsoil, this subsoil sample was hand rolled to a 1/8-inch diameter. The test shows the soil at this site stretches out too far without crumbling; it indicates the material is in a plastic state of consistence, too wet for final decompaction (deep subsoiling) at this time.

Design Guidance

Beyond the above-noted site factors, a vital requirement for the effective Deep Ripping and Decompaction (deep subsoiling), is implementing the practice in its distinct, two-phase process:

- 1) Deep rip the affected thickness of exposed subsoil material (see Figure 10 and 11), aggressively fracturing it before the protected topsoil is reapplied on the site (see Figure 12); and
- 2) Decompact (deep subsoil), simultaneously through the restored topsoil layer and the upper half of the affected subsoil (Figure 13). The second phase, “decompaction,” mitigates the partial recompaction which occurs during the heavy process of topsoil spreading/grading. Prior to deep ripping and decompacting the site, all construction activity, including construction equipment and material storage, site cleanup and trafficking (Figure 14), should be finished; and the site closed off to further disturbance. Likewise, once the practice is underway and the area’s soil permeability and

rainfall infiltration are being restored, a policy limiting all further traffic to permanent travel lanes is maintained.

The other critical elements, outlined below, are: using the proper implements (deep, heavy-duty rippers and subsoilers), and ample pulling-power equipment (tractors); and conducting the practice at the appropriate speed, depth and pattern(s) of movement.

Note that an appropriate plan for the separate practice of establishing a healthy perennial ground cover, with deep rooting to help maintain the restored soil structure, should be developed in advance. This may require the assistance of an agronomist or landscape horticulturist.

Implements

Avoid the use of all undersize implements. The small-to-medium, light-duty tool will, at best, only “scarify” the uppermost surface portion of the mass of compacted subsoil material. The term “chisel plow” is commonly but incorrectly applied to a broad range of implements. While a few may be adapted for the moderate subsoiling of non-impacted soils, the majority are less durable and used for only lighter land-fitting (see Figure 6).



Fig. 6. A light duty chisel implement, not adequate for either the deep ripping or decompaction (deep subsoiling) phase.



Fig. 7. One of several variations of an agricultural ripper. This unit has long, rugged shanks mounted on a steel V-frame for deep, aggressive fracturing through Phase 1.

Use a “heavy duty” agricultural-grade, deep ripper (see Figures 7,9,10 and 11) for the first phase: the lateral and vertical fracturing of the mass of exposed and compressed subsoil, down and through, to the bottom of impact, prior to the replacement of the topsoil layer. (Any oversize rocks which are uplifted to the subsoil surface during the deep ripping phase are picked and removed.) Like the heavy-duty class of implement for the first phase, the decompaction (deep subsoiling) of Phase 2 is conducted with the heavy-duty version of the deep subsoiler. More preferable is the angled-leg variety of deep subsoiler (shown in Figures 8 and 13). It minimizes the inversion of the subsoil and topsoil layers while laterally and vertically fracturing the upper half of the previously ripped subsoil layer and all of the topsoil layer by delivering a momentary, wave-like “lifting and shattering” action up through the soil layers as it is pulled.

Pulling-Power of Equipment

Use the following rule of thumb for tractor horsepower (hp) whenever deep ripping and decompacting a significantly impacted site: For both types of implement, have at least 40 hp of tractor pull available for each mounted shank/ leg.

Using the examples of a 3-shank and a 5-shank implement, the respective tractors should have 120 and 200 hp available for fracturing down to the final depth of 20-to-24 inches per phase. Final depth for the deep ripping in Phase 1 is achieved incrementally by a progressive series of passes (see Depth and Patterns of Movement, below); while for Phase 2, the full operating depth of the deep subsoiler is applied from the beginning.

The operating speed for pulling both types of implement should not exceed 2 to 3 mph. At this slow and managed rate of operating speed, maximum functional performance is sustained by the tractor and the implement performing the soil fracturing. Referring to Figure 8, the implement is the 6-leg version of the deep angled-leg subsoiler. Its two outside legs are “chained up” so that only four legs will be engaged (at the maximum depth), requiring no less than 160 hp, (rather than 240 hp) of pull. The 4-wheel drive, articulated-frame tractor in Figure 8 is 174 hp. It will be decompacting this unobstructed, former construction access area simultaneously through 11 inches of replaced topsoil and the upper 12 inches of the previously deep-ripped subsoil. In constricted areas of Phase 1) Deep Ripping, a medium-size tractor with adequate hp, such as the one in Figure 9 pulling a 3-shank deep ripper, may be more maneuverable.

Some industrial-grade variations of ripping implements are attached to power graders and bulldozers. Although highly durable, they are generally not recommended. Typically, the shanks or “teeth” of these rippers are too short and stout; and they are mounted too far apart to achieve the well-distributed type of lateral and vertical fracturing of the soil materials necessary to restore soil permeability and infiltration. In addition, the power graders and bulldozers, as pullers, are far less maneuverable for turns and patterns than the tractor.



Fig. 8. A deep, angled-leg subsoiler, ideal for Phase 2 decompaction of after the topsoil layer is graded on top of the ripped subsoil.



Fig. 9. This medium tractor is pulling a 3-shank deep ripper. The severely compacted construction access corridor is narrow, and the 120 hp tractor is more maneuverable for Phase 1 deep ripping (subsoil fracturing), here.

Depth and Patterns of Movement

As previously noted both Phase 1 Deep Ripping through significantly compressed, exposed subsoil and Phase 2 Decomposition (deep subsoiling) through the replaced topsoil and upper subsoil need to be performed at maximum capable depth of each implement. With an implement's guide wheels attached, some have a "normal" maximum operating depth of 18 inches, while others may go deeper. In many situations, however, the tractor/implement operator must first remove the guide wheels and other non essential elements from the implement. This adapts the ripper or the deep subsoiler for skillful pulling with its frame only a few inches above surface, while the shanks or legs, fracture the soil material 20-to-24 inches deep.

There may be construction sites where the depth of the exposed subsoil's compression is moderate, e.g.: 12 inches, rather than deep. This can be verified by using a $\frac{3}{4}$ inch cone penetrometer and a shovel to test the subsoil for its level of compaction, incrementally, every three inches of increasing depth. Once the full thickness of the subsoil's compacted zone is finally "pieced" and there is a significant drop in the psi measurements of the soil penetrometer, the depth/thickness of compaction is determined. This is repeated at several representative locations of the construction site. If the thickness of the site's subsoil compaction is verified as, for example, ten inches, then the Phase 1 Deep Ripping can be correspondingly reduced to the implement's minimum operable depth of 12 inches. However, the Phase 2 simultaneous Decomposition (subsoiling) of an 11 inch thick layer of replaced topsoil and the upper subsoil should run at the subsoiling implements full operating depth.



Fig. 10. An early pass with a 3-shank deep ripper penetrating only 8 inches into this worksite's severely compressed subsoil.



Fig. 11. A repeat run of the 3-shank ripper along the same patterned pass area as Fig. 9; here, incrementally reaching 18 of the needed 22 inches of subsoil fracture.

Typically, three separate series (patterns) are used for both the Phase 1 Deep Ripping and the Phase 2 Decomposition on significantly compacted sites. For Phase 1, each series begins with a moderate depth of rip and, by repeat-pass, continues until full depth is reached. Phase 2 applies the full depth of Decomposition (subsoiling), from the beginning.

Every separate series (pattern) consists of parallel, forward-and-return runs, with each progressive

pass of the implement's legs or shanks evenly staggered between those from the previous pass. This compensates for the shank or leg-spacing on the implement, e.g., with 24-to-30 inches between each shank or leg. The staggered return pass ensures lateral and vertical fracturing actuated every 12 to 15 inches across the densely compressed soil mass.

Large, Unobstructed Areas

For larger easy areas, use the standard patterns of movement:

- The first series (pattern) of passes is applied lengthwise, parallel with the longest spread of the site; gradually progressing across the site's width, with each successive pass.
- The second series runs obliquely, crossing the first series at an angle of about 45 degrees.
- The third series runs at right angle (or 90 degrees), to the first series to complete the fracturing and shattering on severely compacted sites, and avoid leaving large unbroken blocks of compressed soil material. (In certain instances, the third series may be optional, depending on how thoroughly the first two series loosen the material and eliminate large chunks/blocks of material as verified by tests with a $\frac{3}{4}$ -inch cone penetrometer.)



Fig. 12. Moderately dry topsoil is being replaced on the affected site now that Phase 1 deep ripping of the compressed subsoil is complete.



Fig. 13. The same deep, angled-leg subsoiler shown in Fig. 7 is engaged at maximum depth for Phase 2, decompaction (deep soiling), of the replaced topsoil and the upper subsoil materials.

Corridors

In long corridors of limited width and less maneuverability than larger sites, e.g.: along compacted areas used as temporary construction access, a modified series of pattern passes are used.

- First, apply the same initial lengthwise, parallel series of passes described above.

- A second series of passes makes a broad “S” shaped pattern of rips, continually and gradually alternating the “S” curves between opposite edges inside the compacted corridor.
- The third and final series again uses the broad, alternating S pattern, but it is “flip-flopped” to continually cross the previous S pattern along the corridor’s centerline. This final series of the S pattern curves back along the edge areas skipped by the second series.

Maintenance and Cost

Once the two-phase practice of Deep Ripping and Decompanation is completed, two items are essential for maintaining a site’s soil porosity and permeability for infiltration. They are: planting and maintaining the appropriate ground cover with deep roots to maintain the soil structure (see Figure 15); and keeping the site free of traffic or other weight loads.

Note that site-specific choice of an appropriate vegetative ground-cover seed mix, including the proper seeding ratio of one or more perennial species with a deep taproot system and the proper amount of lime and soil nutrients (fertilizer mix) adapted to the soil-needs, are basic to the final practice of landscaping, i.e: surface tillage, seeding/planting/fertilizing and culti-packing or mulching is applied. The "maintenance" of an effectively deep-ripped and decompacted area is generally limited to the successful perennial (long-term) landscape ground cover; as long as no weight-bearing force of soil compaction is applied.



Fig. 14. The severely compacted soil of a temporary construction yard used daily by heavy equipment for four months; shown before deep ripping, topsoil replacement, and decompaction.



Fig. 15. The same site as Fig. 14 after deep ripping of the exposed subsoil, topsoil replacement, decompaction through the topsoil and upper subsoil and final surface tillage and revegetation to maintain soil permeability and infiltration.

The Deep Ripping and Decompaction practice is, by necessity, more extensive than periodic subsoiling of farmland. The cost of deep ripping and decompacting (deep subsoiling), will vary according to the depth and severity of soil-material compression and the relative amount of tractor and implement time that is required. In some instances, depending on open maneuverability, two-to-three acres of compacted project area may be deep-ripped in one day. In other situations of more severe compaction and - or less maneuverability, as little as one acre may be fully ripped in a day. Generally, if the Phase 1) Deep Ripping is fully effective, the Phase 2) Decompaction should be completed in $\frac{2}{3}$ to $\frac{3}{4}$ of the time required for Phase 1.

Using the example of two acres of Phase 1) Deep Ripping in one day, at \$1800 per day, the net cost is \$900 per acre. If the Phase 2) Decompacting or deep subsoiling takes $\frac{3}{4}$ the time as Phase 1, it costs \$675 per acre for a combined total of \$1575 per acre to complete the practice (these figures do not include the cost of the separate practice of topsoil stripping and replacement). Due to the many variables, it must be recognized that cost will be determined by the specific conditions or constraints of the site and the availability of proper equipment.

Resources

Publications:

- American Society of Agricultural Engineers. 1971. *Compaction of Agricultural Soils*. ASAE.
- Brady, N.C., and R.R. Weil. 2002. *The Nature and Properties of Soils*. 13th ed. Pearson Education, Inc.
- Baver, L.D. 1948. *Soil Physics*. John Wiley & Sons.
- Carpachi, N. 1987 (1995 fifth printing). *Excavation and Grading Handbook, Revised*. 2nd ed. Craftsman Book Company
- Ellis, B. (Editor). 1997. *Safe & Easy Lawn Care: The Complete Guide to Organic Low Maintenance Lawn*. Houghton Mifflin.
- Harpstead, M.I., T.J. Sauer, and W.F. Bennett. 2001. *Soil Science Simplified*. 4th ed. Iowa State University Press.
- Magdoff, F., and H. van Es. 2000. *Building Soils for Better Crops*. 2nd ed. Sustainable Agricultural Networks
- McCarthy, D.F. 1993. *Essentials of Soil Mechanics and Foundations, Basic Geotechnics* 4th ed. Regents/Prentice Hall.
- Plaster, E.J. 1992. *Soil Science & Management*. 3rd ed. Delmar Publishers.
- Union Gas Limited, Ontario, Canada. 1984. *Rehabilitation of Agricultural Lands, Dawn-Kerwood Loop Pipeline; Technical Report*. Ecological Services for Planning, Ltd.; Robinson, Merritt & Devries, Ltd. and Smith, Hoffman Associates, Ltd.
- US Department of Agriculture in cooperation with Cornell University Agricultural Experiment Station. Various years. *Soil Survey of (various names) County, New York*. USDA.

Internet Access:

- Examples of implements:
V-Rippers. Access by internet search of *John Deere Ag -New Equipment for 915* (larger-frame model) *V-Ripper*; and, *for 913* (smaller-frame model) *V-Ripper*. Deep, angled-leg subsoiler. Access by internet search of: *Bigham Brothers Shear Bolt Paratill-Subsoiler*.
http://salesmanual.deere.com/sales/salesmanual/en_NA/primary_tillage/2008/feature/rippers/915v_pattern_frame.html?sbu=ag&link=prodcut Last visited March 08.
- Soils data of USDA Natural Resources Conservation Service. *NRCS Web Soil Survey*.
<http://websoilsurvey.nrcs.usda.gov/app/> and *USDA-NRCS Official Soil Series Descriptions; View by Name*. <http://ortho.ftw.nrcs.usda.gov/cgi-bin/osd/osdname.cgi> . Last visited Jan. 08.
- Soil penetrometer information. Access by internet searches of: *Diagnosing Soil Compaction using a Penetrometer (soil compaction tester)*, PSU Extension; as well as *Dickey-john Soil Compaction Tester*.
<http://www.dickey-johnproducts.com/pdf/SoilCompactionTest.pdf> and <http://cropsoil.psu.edu/Extension/Facts/uc178pdf> Last visited Sept. 07

APPENDIX K

STANDARD AND SPECIFICATIONS FOR CHECK DAM



Definition

Small barriers or dams constructed of stone, bagged sand or gravel, or other durable material across a drainage way.

Purpose

To reduce erosion in a drainage channel by restricting the velocity of flow in the channel.

Condition Where Practice Applies

This practice is used as a temporary or emergency measure to limit erosion by reducing velocities in small open channels that are degrading or subject to erosion and where permanent stabilization is impractical due to short period of usefulness and time constraints of construction.

Design Criteria

Drainage Area: Maximum drainage area above the check dam shall not exceed two (2) acres.

Height: Not greater than 2 feet. Center shall be maintained 9 inches lower than abutments at natural ground elevation.

Side Slopes: Shall be 2:1 or flatter.

Spacing: The check dams shall be spaced as necessary in the channel so that the crest of the downstream dam is at the

elevation of the toe of the upstream dam. This spacing is equal to the height of the check dam divided by the channel slope.

Therefore:

$$S = h/s$$

Where:

S = spacing interval (ft.)

h = height of check dam (ft.)

s = channel slope (ft./ft.)

Example:

For a channel with a 4% slope and 2 ft. high stone check dams, they are spaced as follows:

$$S = \frac{2 \text{ ft.}}{.04 \text{ ft./ft.}} = 50 \text{ ft.}$$

Stone size: Use a well graded stone matrix 2 to 9 inches in size (NYS – DOT Light Stone Fill meets these requirements).

The overflow of the check dams will be stabilized to resist erosion that might be caused by the check dam. See Figure 5A.9 on page 5A.24 for details.

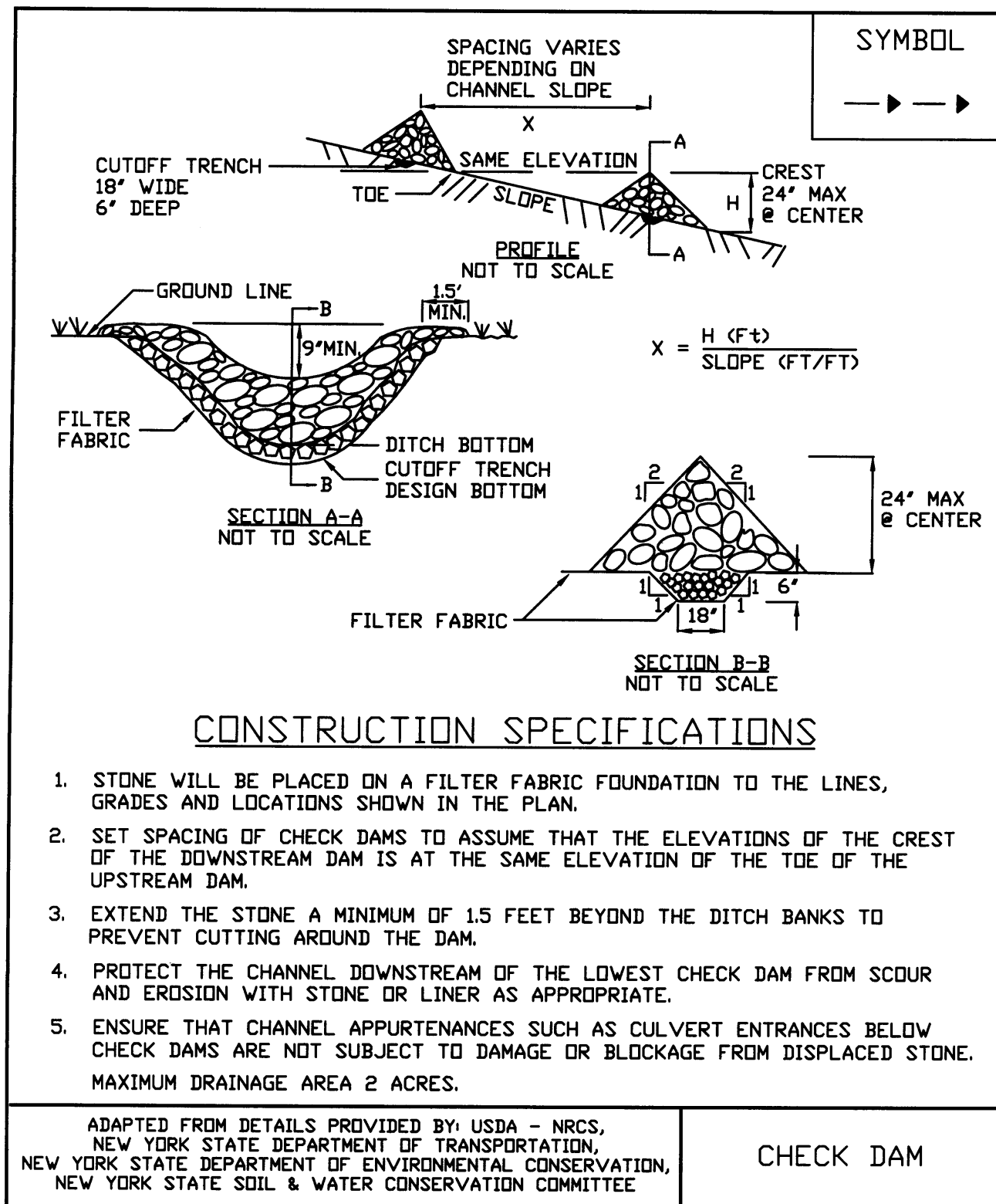
Check dams should be anchored in the channel by a cutoff trench 1.5 ft. wide and 0.5 ft. deep and lined with filter fabric to prevent soil migration.

Maintenance

The check dams should be inspected after each runoff event. Correct all damage immediately. If significant erosion has occurred between structures, a liner of stone or other suitable material should be installed in that portion of the channel.

Remove sediment accumulated behind the dam as needed to allow channel to drain through the stone check dam and prevent large flows from carrying sediment over the dam. Replace stones as needed to maintain the design cross section of the structures.

Figure 5A.9
Check Dam



STANDARD AND SPECIFICATIONS FOR STABILIZED CONSTRUCTION ENTRANCE



Definition

A stabilized pad of aggregate underlain with geotextile located at any point where traffic will be entering or leaving a construction site to or from a public right-of-way, street, alley, sidewalk, or parking area.

Purpose

The purpose of stabilized construction entrance is to reduce or eliminate the tracking of sediment onto public rights-of-way or streets.

Conditions Where Practice Applies

A stabilized construction entrance shall be used at all points of construction ingress and egress.

Design Criteria

See Figure 5A.35 on page 5A.76 for details.

Aggregate Size: Use a matrix of 1-4 inch stone, or reclaimed or recycled concrete equivalent.

Thickness: Not less than six (6) inches.

Width: 12-foot minimum but not less than the full width of points where ingress or egress occurs. 24-foot minimum if there is only one access to the site.

Length: As required, but not less than 50 feet (except on a single residence lot where a 30 foot minimum would apply).

Geotextile: To be placed over the entire area to be covered with aggregate. Filter cloth will not be required on a single-family residence lot. Piping of surface water under entrance shall be provided as required. If piping is impossible, a mountable berm with 5:1 slopes will be permitted.

Criteria for Geotextile

The geotextile shall be woven or nonwoven fabric consisting only of continuous chain polymeric filaments or yarns of polyester. The fabric shall be inert to commonly encountered chemicals, hydro-carbons, mildew, rot resistant, and conform to the fabric properties as shown:

Fabric Properties ³	Light Duty ¹ Roads Grade Subgrade	Heavy Duty ² Haul Roads Rough Graded	Test Method
Grab Tensile Strength (lbs)	200	220	ASTM D1682
Elongation at Failure (%)	50	60	ASTM D1682
Mullen Brust Strength (lbs)	190	430	ASTM D3786
Puncture Strength (lbs)	40	125	ASTM D751 modified
Equivalent Opening Size	40-80	40-80	US Std Sieve CW-02215
Aggregate Depth	6	10	--

¹Light Duty Road: Area sites that have been graded to subgrade and where most travel would be single axle vehicles and an occasional multi-axle truck. Acceptable materials are Trevira Spunbond 1115, Mirafi 100X, Typar 3401, or equivalent.

²Heavy Duty Road: Area sites with only rough grading, and where most travel would be multi-axle vehicles. Acceptable materials are Trevira Spunbond 1135, Mirafi 600X, or equivalent.

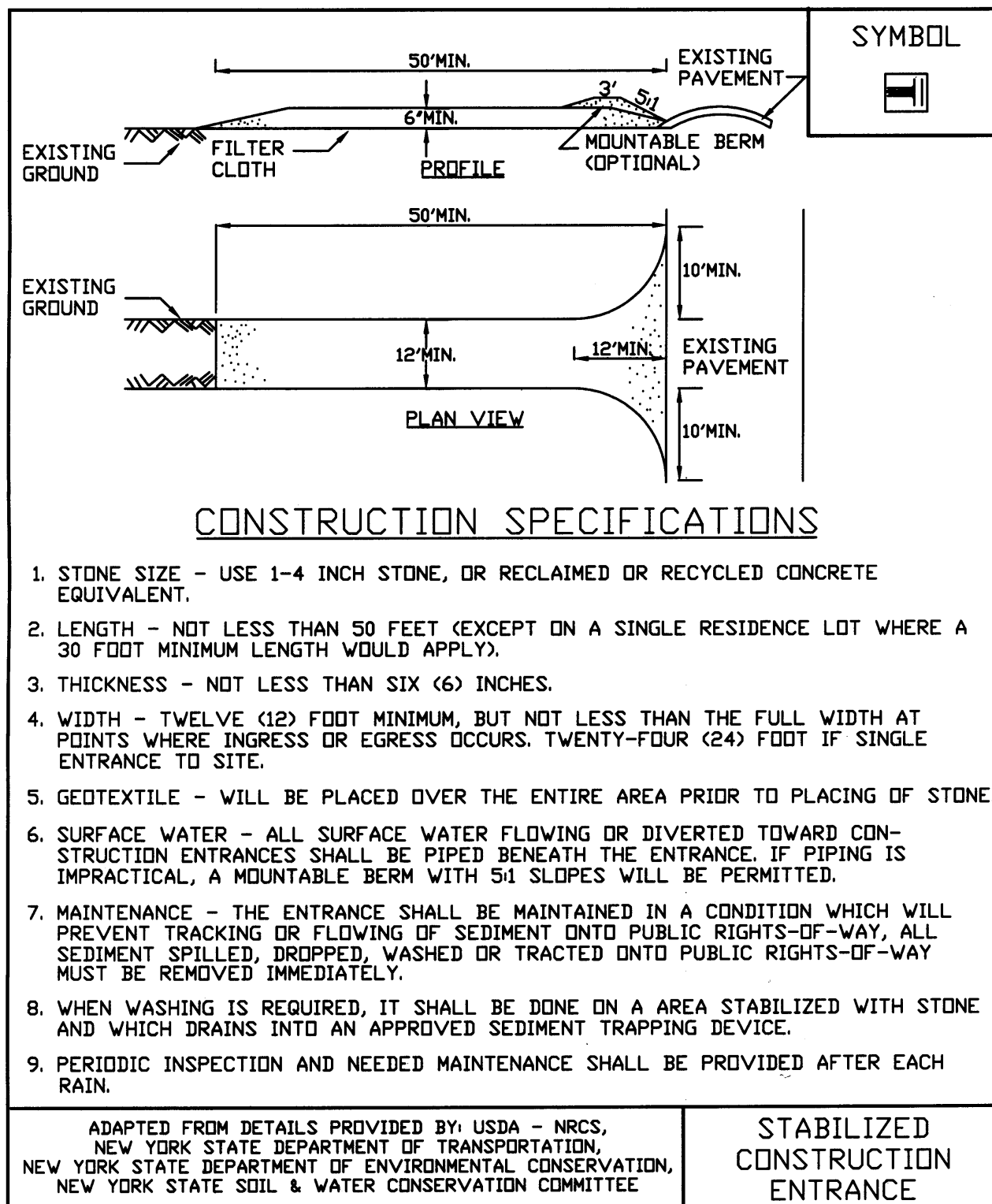
³Fabrics not meeting these specifications may be used only when design procedure and supporting documentation are supplied to determine aggregate depth and fabric strength.

Maintenance

The entrance shall be maintained in a condition which will prevent tracking of sediment onto public rights-of-way or streets. This may require periodic top dressing with additional aggregate. All sediment spilled, dropped, or washed onto public rights-of-way must be removed immediately.

When necessary, wheels must be cleaned to remove sediment prior to entrance onto public rights-of-way. When washing is required, it shall be done on an area stabilized with aggregate, which drains into an approved sediment-trapping device. All sediment shall be prevented from entering storm drains, ditches, or watercourses.

Figure 5A.35
Stabilized Construction Entrance



STANDARD AND SPECIFICATIONS FOR CONSTRUCTION ROAD STABILIZATION



Definition

The stabilization of temporary construction access routes, on-site vehicle transportation routes, and construction parking areas.

Purpose

To control erosion on temporary construction routes and parking areas.

Condition Where Practice Applies

All traffic routes and parking areas for temporary use by construction traffic.

Design Criteria

Construction roads should be located to reduce erosion potential, minimize impact on existing site resources, and maintain operations in a safe manner. Highly erosive soils, wet or rocky areas, and steep slopes should be avoided. Roads should be routed where seasonal water tables are deeper than 18 inches. Surface runoff and control should be in accordance with other standards.

Road Grade – A maximum grade of 12% is recommended, although grades up to 15% are possible for short distances.

Road Width – 14 foot minimum for one-way traffic or 24 foot minimum for two-way traffic.

Side Slope of Road Embankment – 2:1 or flatter.

Ditch Capacity – On-site roadside ditch and culvert capacities shall be the 10 yr. peak runoff.

Composition – Use a 6-inch layer of NYS DOT sub-base Types 1,2,3, 4 or equivalent as specified in NYS – Standards and Specifications for Highways.

Construction Specifications

1. Clear and strip roadbed and parking areas of all vegetation, roots, and other objectionable material.
2. Locate parking areas on naturally flat areas as available. Keep grades sufficient for drainage, but not more than 2 to 3 percent.
3. Provide surface drainage and divert excess runoff to stabilized areas.
4. Maintain cut and fill slopes to 2:1 or flatter and stabilized with vegetation as soon as grading is accomplished.
5. Spread 6-inch layer of sub-base material evenly over the full width of the road and smooth to avoid depressions.
6. Provide appropriate sediment control measures to prevent offsite sedimentation.

Maintenance

Inspect construction roads and parking areas periodically for condition of surface. Topdress with new gravel as needed. Check ditches for erosion and sedimentation after rainfall events. Maintain vegetation in a health, vigorous condition. Areas producing sediment should be treated immediately.

STANDARD AND SPECIFICATIONS FOR STORM DRAIN INLET PROTECTION



Definition

A temporary, somewhat permeable barrier, installed around inlets in the form of a fence, berm or excavation around an opening, trapping water and thereby reducing the sediment content of sediment laden water by settling.

Purpose

To prevent heavily sediment laden water from entering a storm drain system through inlets.

Conditions Where Practice Applies

This practice shall be used where the drainage area to an inlet is disturbed, it is not possible to temporarily divert the storm drain outfall into a trapping device, and watertight blocking of inlets is not advisable. **It is not to be used in place of sediment trapping devices.** This may be used in conjunction with storm drain diversion to help prevent siltation of pipes installed with low slope angle.

Types of Storm Drain Inlet Practices

There are four (4) specific types of storm drain inlet protection practices that vary according to their function, location, drainage area, and availability of materials:

- I. Excavated Drop Inlet Protection
- II. Fabric Drop Inlet Protection
- III. Stone & Block Drop Inlet Protection
- IV. Curb Drop Inlet Protection

Design Criteria

Drainage Area – The drainage area for storm drain inlets shall not exceed one acre. The crest elevations of these practices shall provide storage and minimize bypass flow.

Type I – Excavated Drop Inlet Protection

See details for Excavated Drop Inlet Protection in Figure 5A.11 on page 5A.29.

Limit the drainage area to the inlet device to 1 acre. Excavated side slopes shall be no steeper than 2:1. The minimum depth shall be 1 foot and the maximum depth 2 feet as measured from the crest of the inlet structure. Shape the excavated basin to fit conditions with the longest dimension oriented toward the longest inflow area to provide maximum trap efficiency. The capacity of the excavated basin should be established to contain 900 cubic feet per acre of disturbed area. Weep holes, protected by fabric and stone, should be provided for draining the temporary pool.

Inspect and clean the excavated basin after every storm. Sediment should be removed when 50 percent of the storage volume is achieved. This material should be incorporated into the site in a stabilized manner.

Type II – Fabric Drop Inlet Protection

See Figure 5A.12 for details on Filter Fabric Drop Inlet Protection on page 5A.30.

Limit the drainage area to 1 acre per inlet device. Land area slope immediately surrounding this device should not exceed 1 percent. The maximum height of the fabric above the inlet crest shall not exceed 1.5 feet unless reinforced.

The top of the barrier should be maintained to allow overflow to drop into the drop inlet and not bypass the inlet to unprotected lower areas. Support stakes for fabric shall be a minimum of 3 feet long, spaced a maximum 3 feet apart. They should be driven close to the inlet so any overflow drops into the inlet and not on the unprotected soil. Improved performance and sediment storage volume can be obtained by excavating the area.

Inspect the fabric barrier after each rain event and make repairs as needed. Remove sediment from the pool area as

necessary with care not to undercut or damage the filter fabric. Upon stabilization of the drainage area, remove all materials and unstable sediment and dispose of properly. Bring the adjacent area of the drop inlet to grade, smooth and compact and stabilize in the appropriate manner to the site.

If straw bales are used in lieu of filter fabric, they should be placed tight with the cut edge adhering to the ground at least 3 inches below the elevation of the drop inlet. Two anchor stakes per bale shall be driven flush to bale surface. Straw bales will be replaced every 4 months until the area is stabilized.

Type III – Stone and Block Drop Inlet Protection

See Figure 5A.13 for details on Stone and Block Drop Inlet Protection on page 5A.31.

Limit the drainage area to 1 acre at the drop inlet. The stone barrier should have a minimum height of 1 foot and a maximum height of 2 feet. Do not use mortar. The height should be limited to prevent excess ponding and bypass flow.

Recess the first course of blocks at least 2 inches below the crest opening of the storm drain for lateral support. Subsequent courses can be supported laterally if needed by placing a 2x4 inch wood stud through the block openings perpendicular to the course. The bottom row should have a few blocks oriented so flow can drain through the block to dewater the basin area.

The stone should be placed just below the top of the blocks on slopes of 2:1 or flatter. Place hardware cloth of wire mesh with ½ inch openings over all block openings to hold stone in place.

As an optional design, the concrete blocks may be omitted and the entire structure constructed of stone, ringing the outlet (“doughnut”). The stone should be kept at a 3:1 slope toward the inlet to keep it from being washed into the inlet.

A level area 1 foot wide and four inches below the crest will further prevent wash. Stone on the slope toward the inlet should be at least 3 inches in size for stability and 1 inch or smaller away from the inlet to control flow rate. The elevation of the top of the stone crest must be maintained 6 inches lower than the ground elevation down slope from the inlet to ensure that all storm flows pass over the stone into the storm drain and not past the structure. Temporary diking should be used as necessary to prevent bypass flow.

The barrier should be inspected after each rain event and repairs made where needed. Remove sediment as necessary to provide for accurate storage volume for subsequent rains. Upon stabilization of contributing drainage area, remove all materials and any unstable soil and dispose of properly.

Bring the disturbed area to proper grade, smooth, compact and stabilized in a manner appropriate to the site.

Type IV – Curb Drop Inlet Protection

See Figure 5A. 14 for details on Curb Drop Inlet Protection on page 5A.32.

The drainage area should be limited to 1 acre at the drop inlet. The wire mesh must be of sufficient strength to support the filter fabric and stone with the water fully impounded against it. Stone is to be 2 inches in size and clean. The filter fabric must be of a type approved for this purpose with an equivalent opening size (EOS) of 40-85. The protective structure will be constructed to extend beyond the inlet 2 feet in both directions. Assure that storm flow does not bypass the inlet by installing temporary dikes (such as sand bags) directing flow into the inlet. Make sure that the overflow weir is stable. Traffic safety shall be integrated with the use of this practice.

The structure should be inspected after every storm event. Any sediment should be removed and disposed of on the site. Any stone missing should be replaced. Check materials for proper anchorage and secure as necessary.

Figure 5A.11
Excavated Drop Inlet Protection

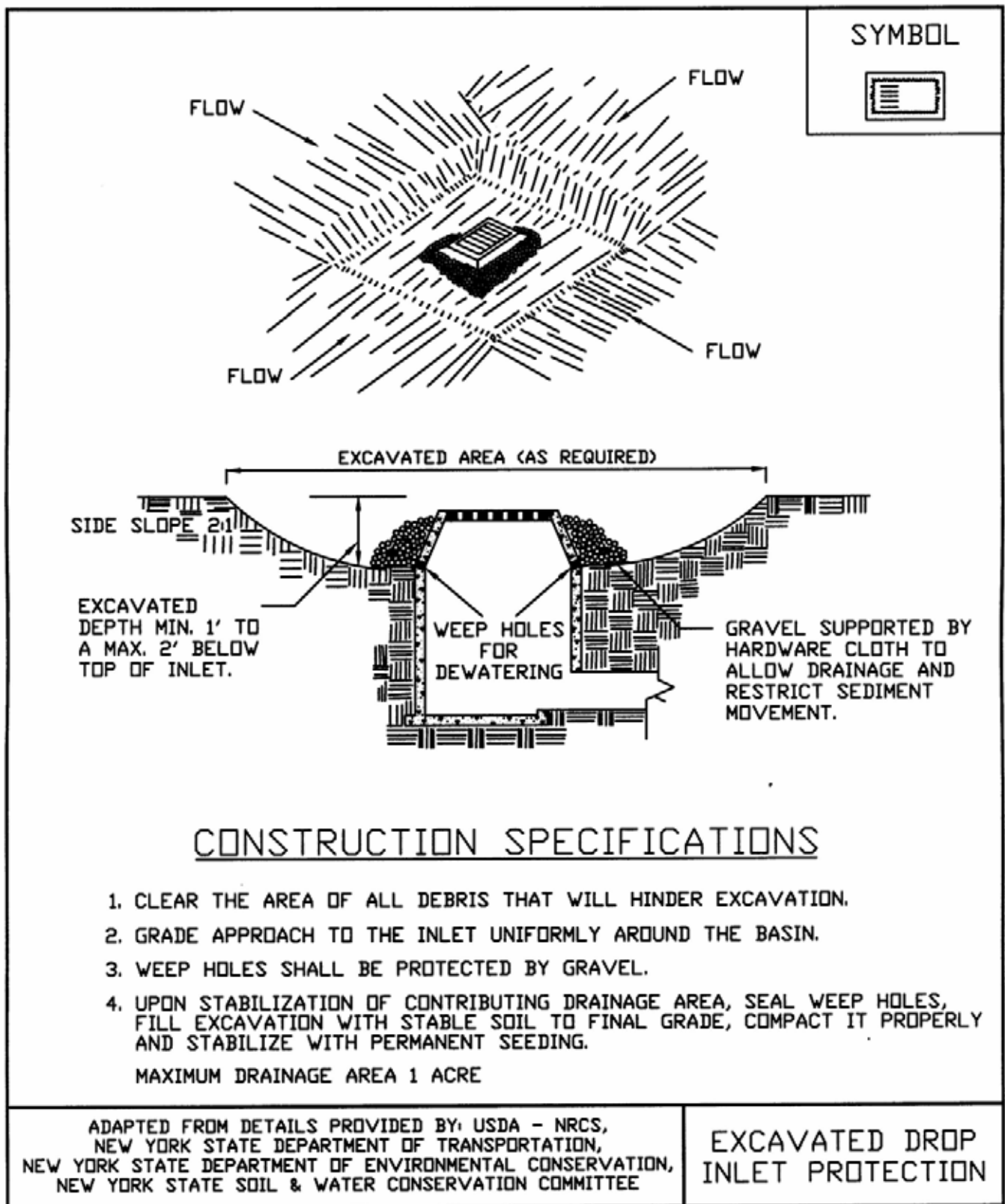


Figure 5A.12
Filter Fabric Drop Inlet Protection

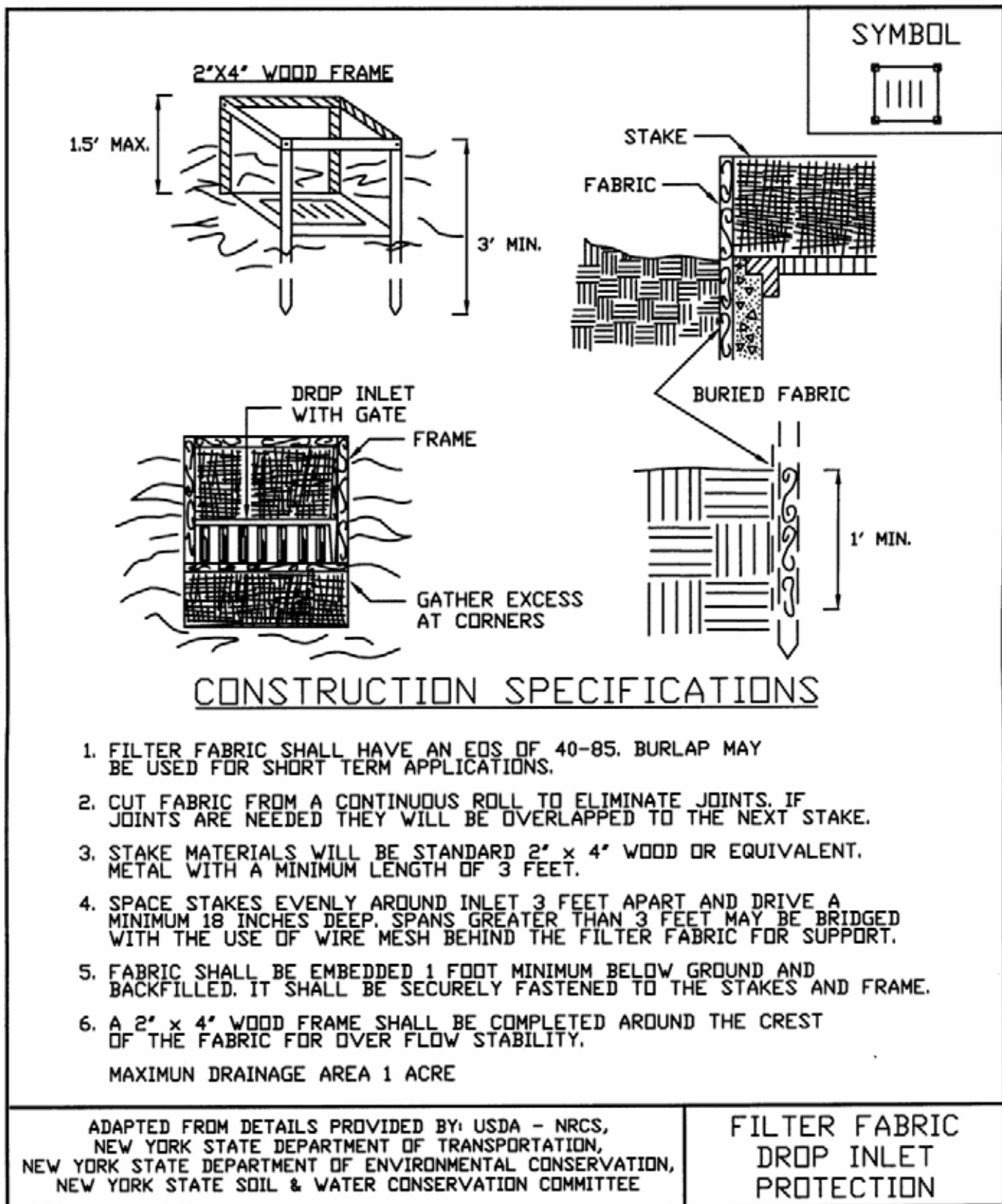


Figure 5A.13
Stone & Block Drop Inlet Protection

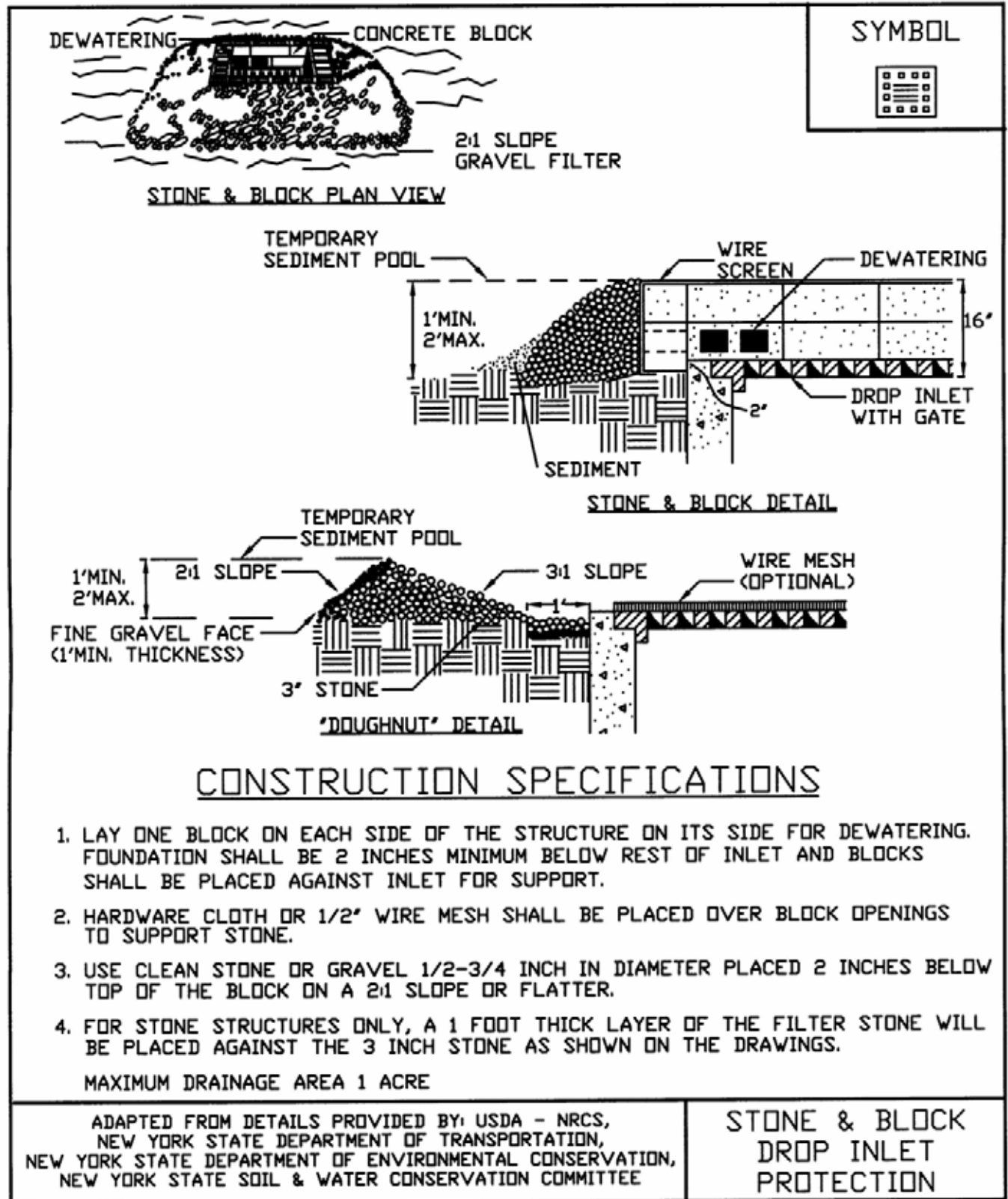
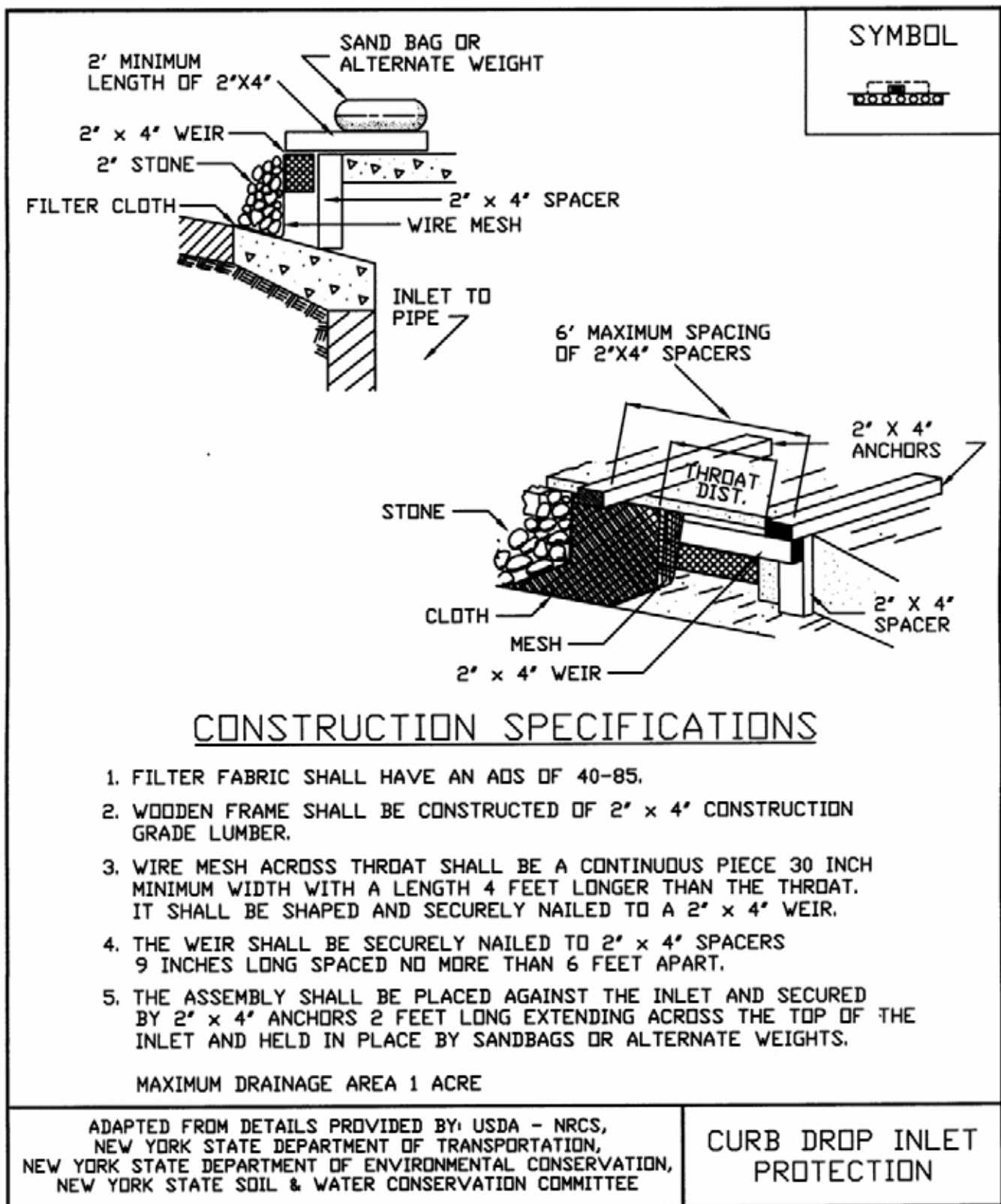


Figure 5A.14
Curb Drop Inlet Protection



STANDARD AND SPECIFICATIONS FOR DUST CONTROL



Definition

The control of dust resulting from land-disturbing activities.

Purpose

To prevent surface and air movement of dust from disturbed soil surfaces that may cause off-site damage, health hazards, and traffic safety problems.

Conditions Where Practice Applies

On construction roads, access points, and other disturbed areas subject to surface dust movement and dust blowing where off-site damage may occur if dust is not controlled.

Design Criteria

Construction operations should be scheduled to minimize the amount of area disturbed at one time. Buffer areas of vegetation should be left where practical. Temporary or permanent stabilization measures shall be installed. No specific design criteria is given; see construction specifications below for common methods of dust control.

Water quality must be considered when materials are selected for dust control. Where there is a potential for the material to wash off to a stream, ingredient information must be provided to the local permitting authority.

Construction Specifications

A. Non-driving Areas – These areas use products and materials applied or placed on soil surfaces to prevent airborne migration of soil particles.

Vegetative Cover – For disturbed areas not subject to traffic, vegetation provides the most practical method of dust control (see Section 3).

Mulch (including gravel mulch) – Mulch offers a fast effective means of controlling dust. This can also include rolled erosion control blankets.

Spray adhesives – These are products generally composed of polymers in a liquid or solid form that are mixed with water to form an emulsion that is sprayed on the soil surface with typical hydroseeding equipment. The mixing ratios and application rates will be in accordance with the manufacturer's recommendations for the specific soils on the site. In no case should the application of these adhesives be made on wet soils or if there is a probability of precipitation within 48 hours of its proposed use. Material Safety Data Sheets will be provided to all applicators and others working with the material.

B. Driving Areas – These areas utilize water, polymer emulsions, and barriers to prevent dust movement from the traffic surface into the air.

Sprinkling – The site may be sprayed with water until the surface is wet. This is especially effective on haul roads and access routes.

Polymer Additives – These polymers are mixed with water and applied to the driving surface by a water truck with a gravity feed drip bar, spray bar or automated distributor truck. The mixing ratios and application rates will be in accordance with the manufacturer's recommendations. Incorporation of the emulsion into the soil will be done to the appropriate depth based on expected traffic. Compaction after incorporation will be by vibratory roller to a minimum of 95%. The prepared surface shall be moist and no application of the polymer will be made if there is a probability of precipitation within 48 hours of its proposed use. Material Safety Data Sheets will be provided to all applicators working with the material.

Barriers – Woven geotextiles can be placed on the driving surface to effectively reduce dust throw and particle migration on haul roads. Stone can also be used for construction roads for effective dust control.

Windbreak – A silt fence or similar barrier can control air currents at intervals equal to ten times the barrier height. Preserve existing wind barrier vegetation as much as practical.

All Stormwater Pollution Prevention Plans must contain the NYS DEC issued “Conditions for Use” and “Application Instructions” for any polymers used on the site. This information can be obtained from the NYS DEC website.

Maintenance

Maintain dust control measures through dry weather periods until all disturbed areas are stabilized.

STANDARD AND SPECIFICATIONS FOR FIBER ROLL



Definition

A fiber roll is a coir (coconut fiber), straw, or excelsior woven roll encased in netting of jute, nylon, or burlap.

Purpose

To dissipate energy along streambanks, channels, and bodies of water and reduce sheet flow on slopes.

Conditions Where Practice Applies

Fiber rolls are used where the water surface levels are relatively constant. Artificially controlled streams for hydropower are not good candidates for this technique. The rolls provide a good medium for the introduction of herbaceous vegetation. Planting in the fiber roll is appropriate where the roll will remain continuously wet.

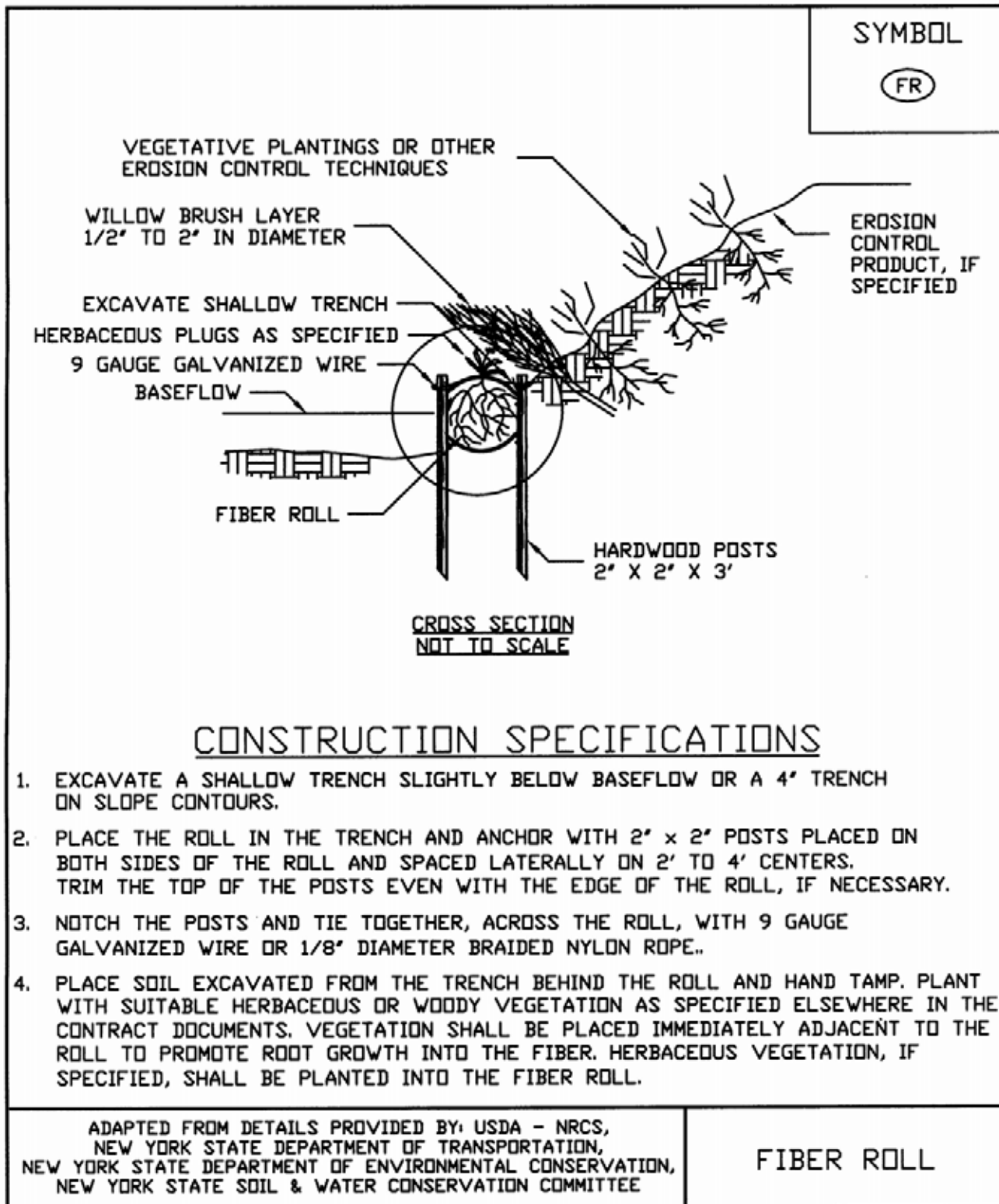
Design Criteria

1. The roll is placed in a shallow trench dug below baseflow or in a 4 inch trench on the slope contour and anchored by 2" x 2", 3-foot long posts driven on each side of the roll (see Figure 4.9).
2. The roll is contained by a 9-gauge non-galvanized wire placed over the roll from post to post. Braided nylon rope (1/8" thick) may be used.
3. The anchor posts shall be spaced laterally 4 feet on center on both sides of the roll, staggered, and driven down to the top of the roll.
4. Soil is placed behind the roll and planted with suitable herbaceous or woody vegetation. If the roll will be continuously saturated, wetland plants may be planted into voids created in the upper surface of the roll.
5. Where water levels may fall below the bottom edge of the roll, a brush layer of willow should be installed so as to lay across the top edge of the roll.

Maintenance

Due to the susceptibility of plant materials to the physical constraints of the site, climate conditions, and animal populations, it is necessary to inspect installations frequently. This is especially important during the first year or two of establishment. Plant materials missing or damaged should be replaced as soon as possible. Sloughs or breaks in drainage pattern should be reestablished for the site as quickly as possible to maintain stability.

Figure 4.9
Fiber Roll



References

1. Gray, Donald H. and A.T. Leiser. 1982. Biotechnical Slope Protection and Erosion Control. Van Nostrand Reinhold Company. New York.
2. Dickerson, John A. and D.W. Lake, Jr. 1989. ASAE Paper No. 892 654. Cost Effective Biotechnical Slope Protection Trials in New York.
3. Dickerson, John A. and M. Van der Grinten. 1991. ICEA paper Biotechnical Slope Protection Systems on Dry Soils in the Northeast United States.
4. Dickerson, et. al. 1990. ICEA paper. Obtaining Plant Materials for Biotechnical Work.

STANDARD AND SPECIFICATIONS FOR MULCHING



Definition

Applying coarse plant residue or chips, or other suitable materials, to cover the soil surface.

Purpose

The primary purpose is to provide initial erosion control while a seeding or shrub planting is establishing. Mulch will conserve moisture and modify the surface soil temperature and reduce fluctuation of both. Mulch will prevent soil surface crusting and aid in weed control. Mulch is also used alone for temporary stabilization in non-growing months.

Conditions Where Practice Applies

On soils subject to erosion and on new seedlings and shrub plantings. Mulch is useful on soils with low infiltration rates by retarding runoff.

Criteria

Site preparation prior to mulching requires the installation of necessary erosion control or water management practices and drainage systems.

Slope, grade and smooth the site to fit needs of selected mulch products.

Remove all undesirable stones and other debris to meet the needs of the anticipated land use and maintenance required.

Apply mulch after soil amendments and planting is accomplished or simultaneously if hydroseeding is used.

Select appropriate mulch material and application rate or material needs. Determine local availability.

Select appropriate mulch anchoring material.

NOTE: The best combination for grass/legume establishment is straw (cereal grain) mulch applied at 2 ton/acre (90 lbs./1000sq.ft.) and anchored with wood fiber mulch (hydromulch) at 500 – 750 lbs./acre (11 – 17 lbs./1000 sq. ft.). The wood fiber mulch must be applied through a hydroseeder immediately after mulching.

Table 3.7
Guide to Mulch Materials, Rates, and Uses

Mulch Material	Quality Standards	per 1000 Sq. Ft.	per Acre	Depth of Application	Remarks
Wood chips or shavings	Air-dried. Free of objectionable coarse material	500-900 lbs.	10-20 tons	2-7"	Used primarily around shrub and tree plantings and recreation trails to inhibit weed competition. Resistant to wind blowing. Decomposes slowly.
Wood fiber cellulose (partly digested wood fibers)	Made from natural wood usually with green dye and dispersing agent	50 lbs.	2,000 lbs.	—	Apply with hydromulcher. No tie down required. Less erosion control provided than 2 tons of hay or straw.
Gravel, Crushed Stone or Slag	Washed; Size 2B or 3A—1 1/2"	9 cu. yds.	405 cu. yds.	3"	Excellent mulch for short slopes and around plants and ornamentals. Use 2B where subject to traffic. (Approximately 2,000 lbs./cu. yd.). Frequently used over filter fabric for better weed control.
Hay or Straw	Air-dried; free of undesirable seeds & coarse materials	90-100 lbs. 2-3 bales	2 tons (100-120 bales)	cover about 90% surface	Use small grain straw where mulch is maintained for more than three months. Subject to wind blowing unless anchored. Most commonly used mulching material. Provides the best micro-environment for germinating seeds.
Jute twisted yarn	Undyed, unbleached plain weave. Warp 78 ends/yd., Weft 41 ends/yd. 60-90 lbs./roll	48" x 50 yds. or 48" x 75 yds.	—	—	Use without additional mulch. Tie down as per manufacturers specifications. Good for center line of concentrated water flow.
Excelsior wood fiber mats	Interlocking web of excelsior fibers with photodegradable plastic netting	8" x 100" 2-sided plastic, 48" x 180" 1-sided plastic	—	—	Use without additional mulch. Excellent for seeding establishment. Tie down as per manufacturers specifications. Approximately 72 lbs./roll for excelsior with plastic on both sides. Use two sided plastic for centerline of waterways.
Compost	Up to 3" pieces, moderately to highly stable	3-9 cu. yds.	134-402 cu. yds.	1-3"	Coarser textured mulches may be more effective in reducing weed growth and wind erosion.
Straw or coconut fiber, or combination	Photodegradable plastic net on one or two sides	Most are 6.5 ft. x 3.5 ft.	81 rolls	—	Designed to tolerate higher velocity water flow, centerlines of waterways, 60 sq. yds. per roll.

Table 3.8
Mulch Anchoring Guide

Anchoring Method or Material	Kind of Mulch to be Anchored	How to Apply
1. Peg and Twine	Hay or straw	After mulching, divide areas into blocks approximately 1 sq. yd. in size. Drive 4-6 pegs per block to within 2" to 3" of soil surface. Secure mulch to surface by stretching twine between pegs in criss-cross pattern on each block. Secure twine around each peg with 2 or more tight turns. Drive pegs flush with soil. Driving stakes into ground tightens the twine.
2. Mulch netting	Hay or straw	Staple the light-weight paper, jute, wood fiber, or plastic nettings to soil surface according to manufacturer's recommendations. Should be biodegradable. Most products are not suitable for foot traffic.
3. Wood cellulose fiber	Hay or straw	Apply with hydroseeder immediately after mulching. Use 500 lbs. wood fiber per acre. Some products contain an adhesive material ("tackifier"), possibly advantageous.
4. Mulch anchoring tool	Hay or straw	Apply mulch and pull a mulch anchoring tool (blunt, straight discs) over mulch as near to the contour as possible. Mulch material should be "tucked" into soil surface about 3".
5. Tackifier	Hay or straw	Mix and apply polymeric and gum tackifiers according to manufacturer's instructions. Avoid application during rain. A 24-hour curing period and a soil temperature higher than 45 ⁰ Fahrenheit are required.

STANDARD AND SPECIFICATIONS FOR ROCK OUTLET PROTECTION



Definition

A section of rock protection placed at the outlet end of the culverts, conduits, or channels.

Purpose

The purpose of the rock outlet protection is to reduce the depth, velocity, and energy of water, such that the flow will not erode the receiving downstream reach.

Scope

This standard applies to the planning, design, and construction of rock riprap and gabions for protection of downstream areas. It does not apply to rock lining of channels or streams.

Conditions Where Practice Applies

This practice applies where discharge velocities and energies at the outlets of culverts, conduits, or channels are sufficient to erode the next downstream reach. This applies to:

1. Culvert outlets of all types.
2. Pipe conduits from all sediment basins, dry storm water ponds, and permanent type ponds.
3. New channels constructed as outlets for culverts and conduits.

Design Criteria

The design of rock outlet protection depends entirely on the location. Pipe outlet at the top of cuts or on slopes steeper than 10 percent, cannot be protected by rock aprons or riprap sections due to re-concentration of flows and high velocities encountered after the flow leaves the apron.

Many counties and state agencies have regulations and design procedures already established for dimensions, type and size of materials, and locations where outlet protection is required. Where these requirements exist, they shall be followed.

Tailwater Depth

The depth of tailwater immediately below the pipe outlet must be determined for the design capacity of the pipe. If the tailwater depth is less than half the diameter of the outlet pipe, and the receiving stream is wide enough to accept divergence of the flow, it shall be classified as a Minimum Tailwater Condition; see Figure 5B.12 on page 5B.25 as an example. If the tailwater depth is greater than half the pipe diameter and the receiving stream will continue to confine the flow, it shall be classified as a Maximum Tailwater Condition; see Figure 5B.13 on page 5B.26 as an example. Pipes which outlet onto flat areas with no defined channel may be assumed to have a Minimum Tailwater Condition; see Figure 5B.12 on page 5B.25 as an example.

Apron Size

The apron length and width shall be determined from the curves according to the tailwater conditions:

Minimum Tailwater – Use Figure 5B.12 on page 5B.25
Maximum Tailwater – Use Figure 5B.13 on page 5B.26

If the pipe discharges directly into a well defined channel, the apron shall extend across the channel bottom and up the channel banks to an elevation one foot above the maximum tailwater depth or to the top of the bank, whichever is less.

The upstream end of the apron, adjacent to the pipe, shall have a width two (2) times the diameter of the outlet pipe, or conform to pipe end section if used.

Bottom Grade

The outlet protection apron shall be constructed with no slope along its length. There shall be no overfall at the end of the apron. The elevation of the downstream end of the apron shall be equal to the elevation of the receiving channel or adjacent ground.

Alignment

The outlet protection apron shall be located so that there are no bends in the horizontal alignment.

Materials

The outlet protection may be done using rock riprap, grouted riprap, or gabions.

Riprap shall be composed of a well-graded mixture of stone size so that 50 percent of the pieces, by weight, shall be larger than the d_{50} size determined by using the charts. A well-graded mixture, as used herein, is defined as a mixture composed primarily of larger stone sizes, but with a sufficient mixture of other sizes to fill the smaller voids between the stones. The diameter of the largest stone size in such a mixture shall be 1.5 times the d_{50} size.

Thickness

The minimum thickness of the riprap layer shall be 1.5 times the maximum stone diameter for d_{50} of 15 inches or less; and 1.2 times the maximum stone size for d_{50} greater than 15 inches. The following chart lists some examples:

D_{50} (inches)	d_{max} (inches)	Minimum Blanket Thickness (inches)
4	6	9
6	9	14
9	14	20
12	18	27
15	22	32
18	27	32
21	32	38
24	36	43

Stone Quality

Stone for riprap shall consist of field stone or rough unhewn quarry stone. The stone shall be hard and angular and of a quality that will not disintegrate on exposure to water or weathering. The specific gravity of the individual stones shall be at least 2.5.

Recycled concrete equivalent may be used provided it has a

density of at least 150 pounds per cubic foot, and does not have any exposed steel or reinforcing bars.

Filter

A filter is a layer of material placed between the riprap and the underlying soil surface to prevent soil movement into and through the riprap. Riprap shall have a filter placed under it in all cases.

A filter can be of two general forms: a gravel layer or a plastic filter cloth. The plastic filter cloth can be woven or non-woven monofilament yarns, and shall meet these base requirements: thickness 20-60 mils, grab strength 90-120 lbs; and shall conform to ASTM D-1777 and ASTM D-1682.

Gravel filter blanket, when used, shall be designed by comparing particle sizes of the overlying material and the base material. Design criteria are available in Standard and Specification for Riprap Slope Protection on page 5B.57.

Gabions

Gabions shall be made of hexagonal triple twist mesh with heavily galvanized steel wire. The maximum linear dimension of the mesh opening shall not exceed 4 ½ inches and the area of the mesh opening shall not exceed 10 square inches.

Gabions shall be fabricated in such a manner that the sides, ends, and lid can be assembled at the construction site into a rectangular basket of the specified sizes. Gabions shall be of single unit construction and shall be installed according to manufacturers recommendations.

The area on which the gabion is to be installed shall be graded as shown on the drawings. Foundation conditions shall be the same as for placing rock riprap, and filter cloth shall be placed under all gabions. Where necessary, key, or tie, the structure into the bank to prevent undermining of the main gabion structure.

Maintenance

Once a riprap outlet has been installed, the maintenance needs are very low. It should be inspected after high flows for evidence of scour beneath the riprap or for dislodged stones. Repairs should be made immediately.

Design Procedure

1. Investigate the downstream channel to assure that nonerosive velocities can be maintained.
2. Determine the tailwater condition at the outlet to establish which curve to use.
3. Enter the appropriate chart with the design discharge to

determine the riprap size and apron length required. It is noted that references to pipe diameters in the charts are based on full flow. For other than full pipe flow, the parameters of depth of flow and velocity must be used to adjust the design discharges.

4. Calculate apron width at the downstream end if a flare section is to be employed.

Examples

Example 1: Pipe Flow (full) with discharge to unconfined section.

Given: A circular conduit flowing full.

$Q = 280$ cfs, diam. = 66 in., tailwater (surface) is 2 ft. above pipe invert (minimum tailwater condition).

Find: Read $d_{50} = 1.2$ and apron length (L_a) = 38 ft.

Apron width = diam. + $L_a = 5.5 + 38 = 43.5$ ft.

Use: $d_{50} = 15''$, $d_{max} = 22''$, blanket thickness = 32"

Example 2: Box Flow (partial) with high tailwater

Given: A box conduit discharging under partial flow conditions. A concrete box 5.5 ft. x 10 ft. flowing 5.0 ft. deep,

$Q = 600$ cfs and tailwater surface is 5 ft. above invert (max. tailwater condition).

Since this is not full pipe and does not directly fit the nomograph assumptions of Figure 7B.13 substitute depth as the diameter, to find a discharge equal to full pipe flow for that diameter, in this case 60 inches.

Since, $Q = AV$ and $A = \frac{\pi D^2}{4}$

First, compute velocity:

$V = (Q/A) = (600/(5) (10)) = 12$ fps

Then substituting:

$$Q = \frac{\pi D^2}{4} \times V = \frac{3.14 (5 \text{ ft})^2}{4} \times 12 \text{ fps} = 236 \text{ cfs}$$

At the intersection of the curve $d = 60$ in. and $Q = 236$ cfs, read $d_{50} = 0.4$ ft.

Then reading the $d = 60$ in. curve, read apron length (L_a) = 40 ft.

Apron width, $W = \text{conduit width} + (6.4)(L_a) = 10 + (0.4)(40) = 26$ ft.

Example 3: Open Channel Flow with Discharge to Unconfined Section

Given: A trapezoidal concrete channel 5 ft. wide with 2:1 side slopes is flowing 2 ft. deep, $Q = 180$ cfs (velocity = 10 fps) and the tailwater surface downstream is 0.8 ft. (minimum tailwater condition).

Find: Using similar principles as Example 2, compute equivalent discharge for a 2 foot, using depth as a diameter, circular pipe flowing full at 10 feet per second.

Velocity:

$$Q = \frac{\pi (2 \text{ ft})^2}{4} \times 10 \text{ fps} = 31.4 \text{ cfs}$$

At intersection of the curve, $d = 24$ in. and $Q = 32$ cfs, read $d_{50} = 0.6$ ft.

Then reading the $d = 24$ in. curve, read apron length (L_a) = 20 ft.

Apron width, $W = \text{bottom width of channel} + L_a = 5 + 20 = 25$ ft.

Example 4: Pipe flow (partial) with discharge to a confined section

Given: A 48 in. pipe is discharging with a depth of 3 ft. $Q = 100$ cfs, and discharge velocity of 10 fps (established from partial flow analysis) to a confined trapezoidal channel with a 2 ft. bottom, 2:1 side slopes, $n = .04$, and grade of 0.6%.

Calculation of the downstream channel (by Manning's Equation) indicates a normal depth of 3.1 ft. and normal velocity of 3.9 fps.

Since the receiving channel is confined, the maximum tailwater condition controls.

Find: discharge using previous principles:

$$Q = \frac{\pi (3 \text{ ft})^2}{4} \times 10 \text{ fps} = 71 \text{ cfs}$$

At the intersection of $d = 36$ in. and $Q = 71$ cfs, read $d_{50} = 0.3$ ft.

Reading the $d = 36''$ curve, read apron length (L_a) = 30 ft.

Since the maximum flow depth in this reach is 3.1 ft., that is the minimum depth of riprap to be maintained for the entire length.

Construction Specifications

1. The subgrade for the filter, riprap, or gabion shall be prepared to the required lines and grades. Any fill required in the subgrade shall be compacted to a density of approximately that of the surrounding undisturbed material.
2. The rock or gravel shall conform to the specified grading limits when installed respectively in the riprap or filter.
3. Filter cloth shall be protected from punching, cutting, or tearing. Any damage other than an occasional small hole shall be repaired by placing another piece of cloth over the damaged part or by completely replacing the cloth. All overlaps, whether for repairs or for joining two pieces of cloth shall be a minimum of one foot.
4. Stone for the riprap or gabion outlets may be placed by equipment. Both shall each be constructed to the full course thickness in one operation and in such a manner as to avoid displacement of underlying materials. The stone for riprap or gabion outlets shall be delivered and placed in a manner that will ensure that it is reasonably homogenous with the smaller stones and spalls filling the voids between the larger stones. Riprap shall be placed in a manner to prevent damage to the filter blanket or filter cloth. Hand placement will be required to the extent necessary to prevent damage to the permanent works.

Figure 5B.12
Outlet Protection Design—Minimum Tailwater Condition
(Design of Outlet Protection from a Round Pipe Flowing Full,
Minimum Tailwater Condition: $T_w < 0.5D_o$) (USDA - NRCS)

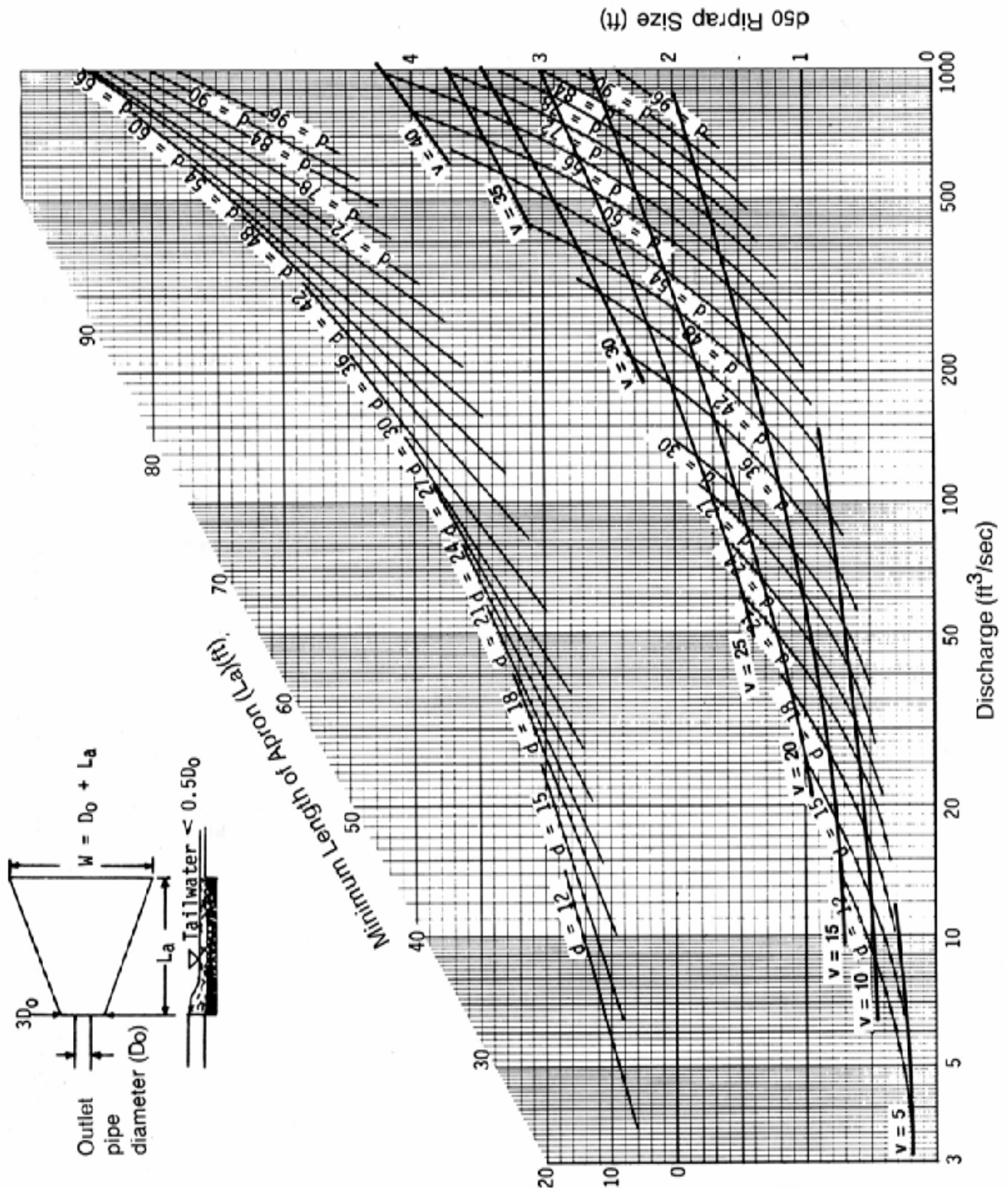


Figure 5B.13
Outlet Protection Design—Maximum Tailwater Condition
(Design of Outlet Protection from a Round Pipe Flowing Full,
Maximum Tailwater Condition: $T_w \geq 0.5D_o$) (USDA - NRCS)

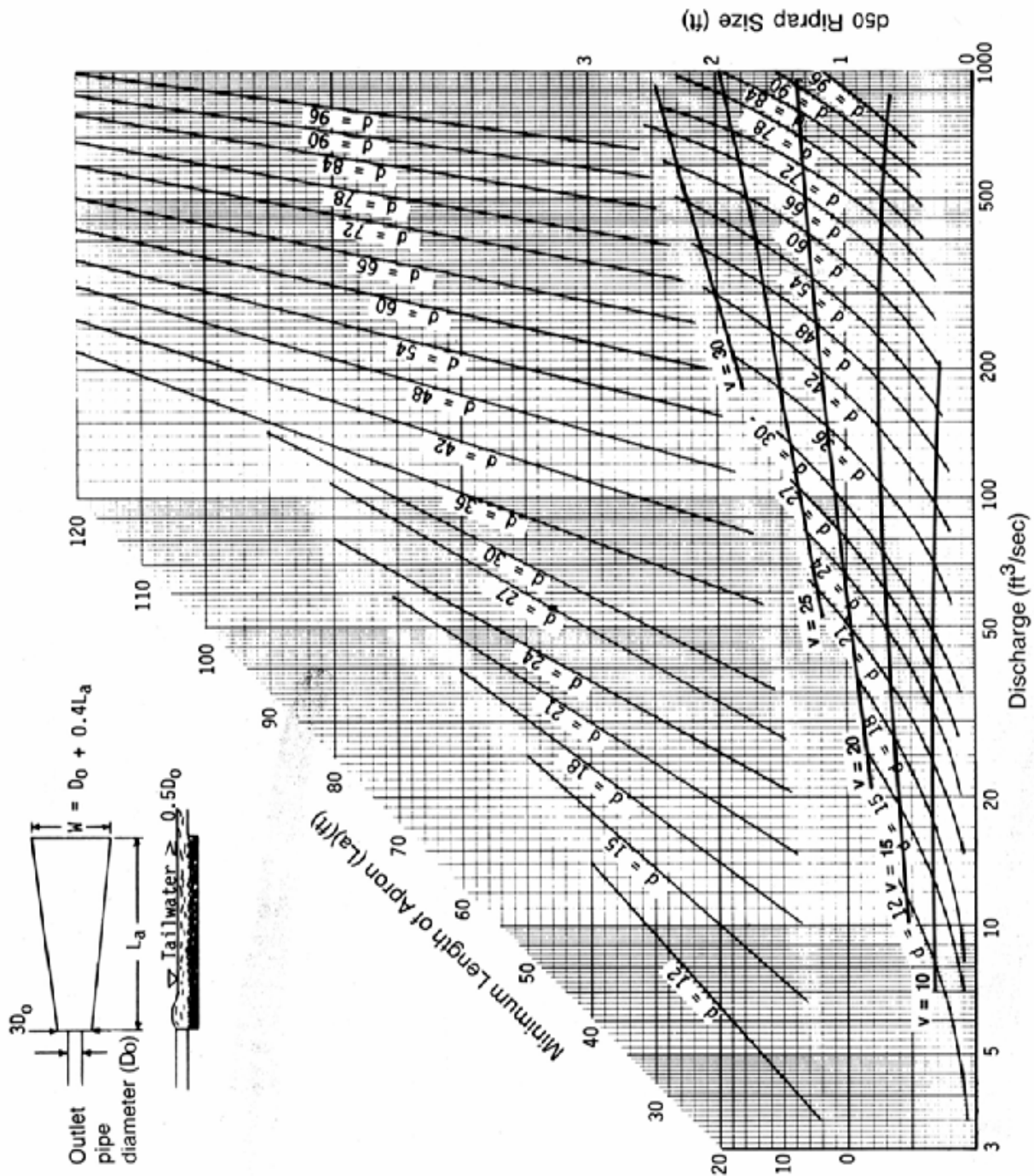


Figure 5B.14
Riprap Outlet Protection Detail (1)

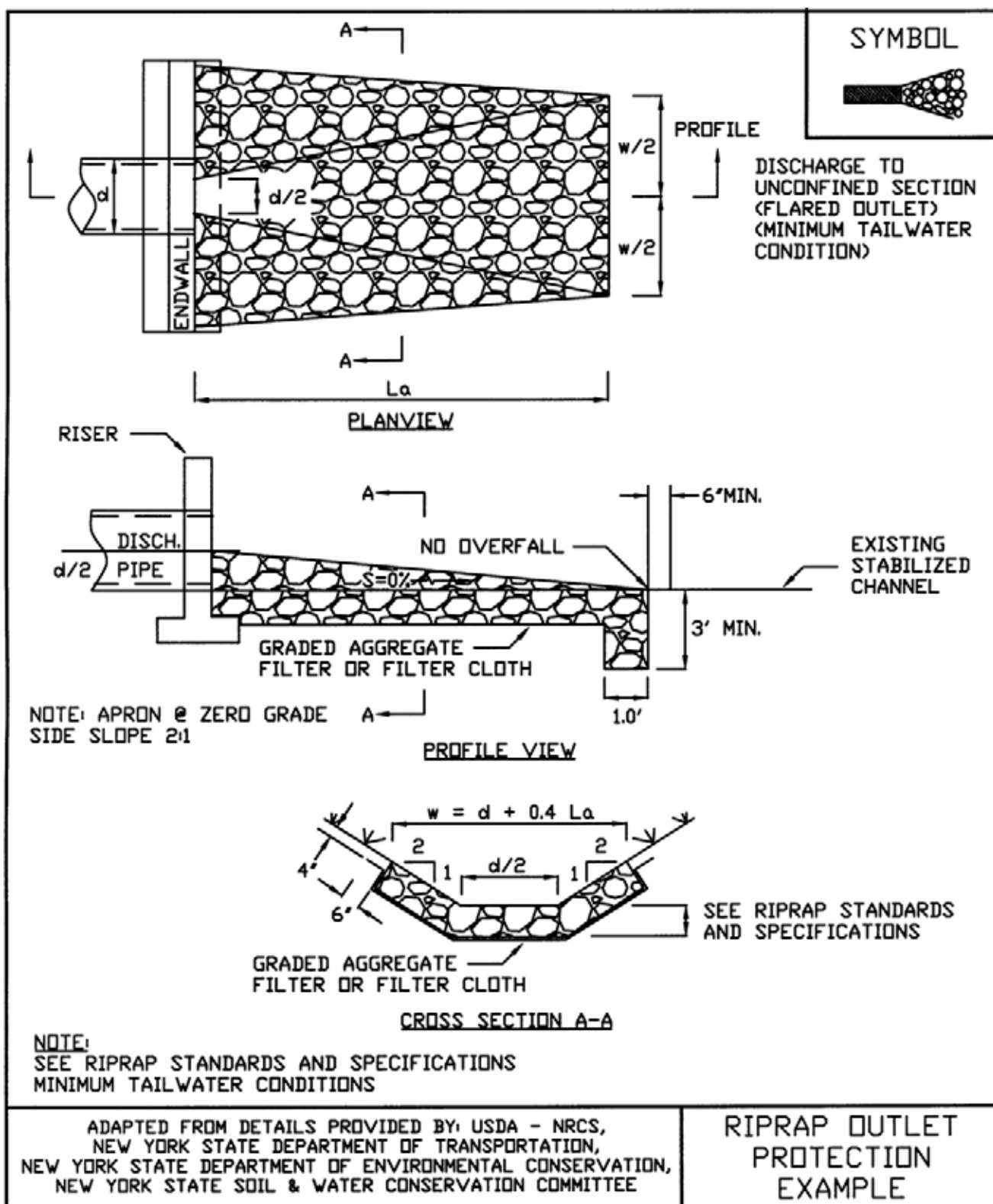


Figure 5B.15
Riprap Outlet Protection Detail (2)

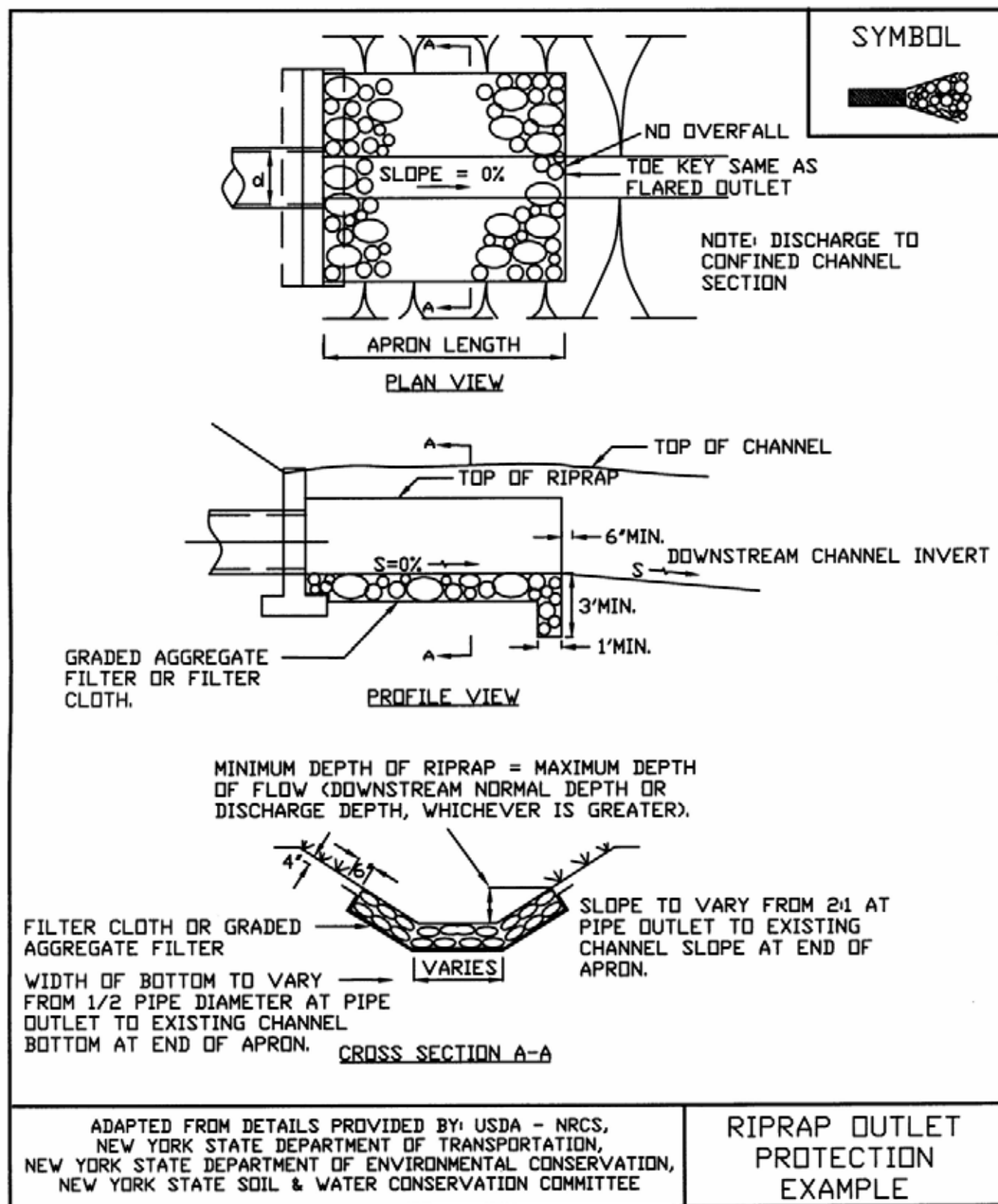
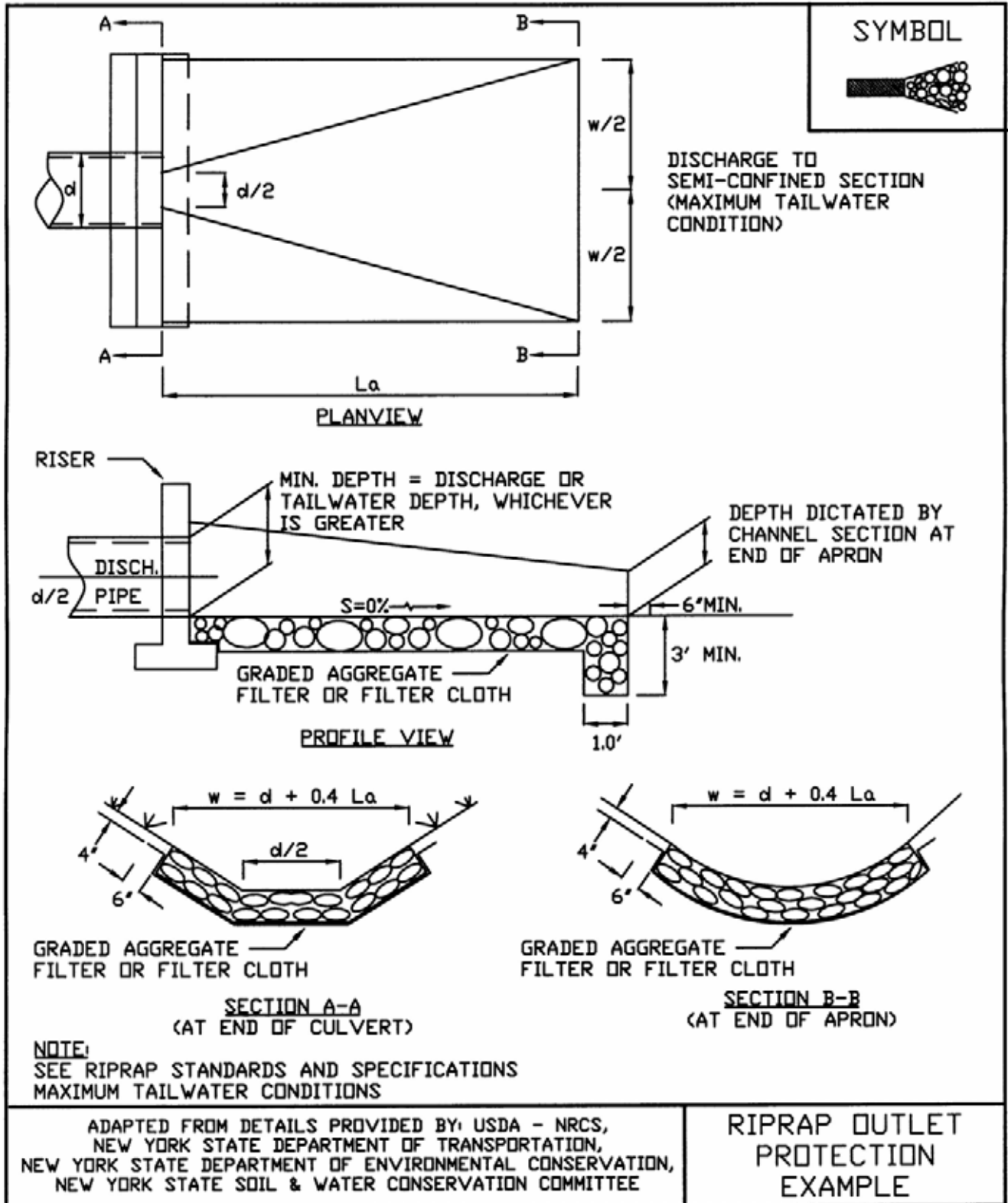


Figure 5B.16
Riprap Outlet Protection Detail (3)



STANDARD AND SPECIFICATIONS FOR SEDIMENT TRAP



Definition

A temporary sediment control device formed by excavation and/or embankment to intercept sediment laden runoff and retain the sediment.

Purpose

The purpose of the structure is to intercept sediment-laden runoff and trap the sediment in order to protect drainage ways, properties, and rights-of-way below the sediment trap from sedimentation.

Conditions Where Practice Applies

A sediment trap is usually installed in a drainage way, at a storm drain inlet, or other points of collection from a disturbed area.

Sediment traps should be used to artificially break up the natural drainage area into smaller sections where a larger device (sediment basin) would be less effective.

Design Criteria

If any of the design criteria presented here cannot be met, see Standard and Specification for Sediment Basin on page 5A.49.

Drainage Area

The drainage area for sediment traps shall be in accordance with the specific type of sediment trap used (Type I through V).

Location

Sediment traps shall be located so that they can be installed

prior to grading or filling in the drainage area they are to protect. Traps must not be located any closer than 20 feet from a proposed building foundation if the trap is to function during building construction. Locate traps to obtain maximum storage benefit from the terrain and for ease of cleanout and disposal of the trapped sediment.

Trap Size

The volume of a sediment trap as measured at the elevation of the crest of the outlet shall be at least 3,600 cubic feet per acre of drainage area. The volume of a constructed trap shall be calculated using standard mathematical procedures. The volume of a natural sediment trap may be approximated by the equation: Volume (cu.ft.) = 0.4 x surface area (sq.ft.) x maximum depth (ft.).

Trap Cleanout

Sediment shall be removed and the trap restored to the original dimensions when the sediment has accumulated to ½ of the design depth of the trap. Sediment removed from the trap shall be deposited in a protected area and in such a manner that it will not erode.

Embankment

All embankments for sediment traps shall not exceed five (5) feet in height as measured at the low point of the original ground along the centerline of the embankment. Embankments shall have a minimum four (4) foot wide top and side slopes of 2:1 or flatter. The embankment shall be compacted by traversing with equipment while it is being constructed. The embankment shall be stabilized with seed and mulch as soon as it is completed

The elevation of the top of any dike directing water to any sediment trap will equal or exceed the maximum height of the outlet structure along the entire length of the trap.

Excavation

All excavation operations shall be carried out in such a manner that erosion and water pollution shall be minimal. Excavated portions of sediment traps shall have 1:1 or flatter slopes.

Outlet

The outlet shall be designed, constructed, and maintained in such a manner that sediment does not leave the trap and that erosion at or below the outlet does not occur.

Sediment traps must outlet onto stabilized (preferable undisturbed) ground, into a watercourse, stabilized channel, or into a storm drain system. Distance between inlet and outlet should be maximized to the longest length practicable.

Trap Details Needed on Erosion and Sediment Control Plans

Each trap shall be delineated on the plans in such a manner that it will not be confused with any other features. Each trap on a plan shall indicate all the information necessary to properly construct and maintain the structure. If the drawings are such that this information cannot be delineated on the drawings, then a table shall be developed. If a table is developed, then each trap on a plan shall have a number and the numbers shall be consecutive.

The following information shall be shown for each trap in a summary table format on the plans.

- 1. Trap number
- 2. Type of trap
- 3. Drainage area
- 4. Storage required
- 5. Storage provided (if applicable)
- 6. Outlet length or pipe sizes
- 7. Storage depth below outlet or cleanout elevation
- 8. Embankment height and elevation (if applicable)

Type of Sediment Traps

There are five (5) specific types of sediment traps which vary according to their function, location, or drainage area.

- I. Pipe Outlet Sediment Trap
- II. Grass Outlet Sediment Trap
- III. Catch Basin Sediment Trap
- IV. Stone Outlet Sediment Trap
- V. Riprap Outlet Sediment Trap

I. Pipe Outlet Sediment Trap

A Pipe Outlet Sediment Trap consists of a trap formed by embankment or excavation. The outlet for the trap is through a perforated riser and a pipe through the embankment. The outlet pipe and riser shall be made of steel, corrugated metal or other suitable material. The top of the embankment shall be at least 1 ½ feet above the crest of the riser. The top 2/3 of the riser shall be perforated with one (1) inch nominal diameter holes or slits spaced six (6) inches vertically and horizontally placed in the concave portion of the corrugated pipe.

No holes or slits will be allowed within six (6) inches of the top of the horizontal barrel. All pipe connections shall be watertight. The riser shall be wrapped with ½ to ¼ inch hardware cloth wire then wrapped with filter cloth with a sieve size between #40-80 and secured with strapping or

connecting band at the top and bottom of the cloth. The cloth shall cover an area at least six (6) inches above the highest hole and six (6) inches below the lowest hole. The top of the riser pipe shall not be covered with filter cloth. The riser shall have a base with sufficient weight to prevent flotation of the riser. Two approved bases are:

- 1. A concrete base 12 in. thick with the riser embedded 9 in. into the concrete base, or
- 2. One quarter inch, minimum, thick steel plate attached to the riser by a continuous weld around the circumference of the riser to form a watertight connection. The plate shall have 2.5 feet of stone, gravel, or earth placed on it to prevent flotation. In either case, each side of the square base measurement shall be the riser diameter plus 24 inches.

Pipe outlet sediment traps shall be limited to a five (5) acre maximum drainage area. Pipe outlet sediment traps may be interchangeable in the field with stone outlet or riprap sediment traps provided that these sediment traps are constructed in accordance with the detail and specifications for that trap.

Select pipe diameter from the following table:

Minimum Sizes

Barrel Diameter ¹ (in.)	Riser Diameter ¹ (in.)	Maximum Drainage Area (ac.)
12	15	1
15	18	2
18	21	3
21	24	4
21	27	5

¹ Barrel diameter may be same size as riser diameter.

See details for Pipe Outlet Sediment Trap ST-I in Figure 5A.16 (1) and 5A.16 (2) on pages 5A.38 and 5A.39.

II. Grass Outlet Sediment Trap

A Grass Outlet Sediment Trap consists of a trap formed by excavating the earth to create a holding area. The trap has a discharge point over natural existing grass. The outlet crest width (feet) shall be equal to four (4) times the drainage area (acres) with a minimum width of four (4) feet. The outlet shall be free of any restrictions to flow. The outlet lip must remain undisturbed and level. The volume of this trap shall be computed at the elevation of the crest of the outlet. Grass outlet sediment traps shall be limited to a five (5) acre maximum drainage area.

See details for Grass Outlet Sediment Trap ST-II in Figure 5A.17 on page 5A.40.

III. Catch Basin Sediment Trap

A Catch Basin Sediment Trap consists of a basin formed by excavation on natural ground that discharges through an opening in a storm drain inlet structure. This opening can either be the inlet opening or a temporary opening made by omitting bricks or blocks in the inlet.

A yard drain inlet or an inlet in the median strip of a dual highway could use the inlet opening for the type outlet. The trap should be out of the roadway so as not to interfere with future compaction or construction. Placing the trap on the opposite side of the opening and diverting water from the roadway to the trap is one means of doing this. Catch basin sediment traps shall be limited to a three (3) acre maximum drainage area. The volume of this trap is measured at the elevation of the crest of the outlet (invert of the inlet opening).

See details for Catch Basin Sediment Trap ST-III in Figure 5A.18 on page 5A.41.

IV. Stone Outlet Sediment Trap

A Stone Outlet Sediment Trap consists of a trap formed by an embankment or excavation. The outlet of this trap is over a stone section placed on level ground. The minimum length (feet) of the outlet shall be equal to four (4) times the drainage area (acres).

Required storage shall be 3,600 cubic feet per acre of drainage area.

The outlet crest (top of stone in weir section) shall be level, at least one (1) foot below top of embankment and no more than one (1) foot above ground beneath the outlet. Stone used in the outlet shall be small riprap (4 in. x 8 in.). To provide more efficient trapping effect, a layer of filter cloth should be embedded one (1) foot back into the upstream face of the outlet stone or a one (1) foot thick layer of two (2) inch or finer aggregate shall be placed on the upstream face of the outlet.

Stone Outlet Sediment Traps may be interchangeable in the field with pipe or riprap outlet sediment traps provided they are constructed in accordance with the detail and specifications for those traps. Stone outlet sediment traps shall be limited to a five (5) acre maximum drainage area.

See details for Stone Outlet Sediment Trap ST-IV in Figure 5A.19 on page 5A.42.

V. Riprap Outlet Sediment Trap

A Riprap Outlet Sediment Trap consists of a trap formed by an excavation and embankment. The outlet for this trap

shall be through a partially excavated channel lined with riprap. This outlet channel shall discharge onto a stabilized area or to a stable watercourse. The riprap outlet sediment trap may be used for drainage areas of up to a maximum of 15 acres.

Design Criteria for Riprap Outlet Sediment Trap

1. The total contributing drainage area (disturbed or undisturbed either on or off the developing property) shall not exceed 15 acres.
2. The storage needs for this trap shall be computed using 3600 cubic feet of required storage for each acre of drainage area. The storage volume provided can be figured by computing the volume of storage area available behind the outlet structure up to an elevation of one (1) foot below the level weir crest.
3. The maximum height of embankment shall not exceed five (5) feet.
4. The elevation of the top of any dike directing water to a riprap outlet sediment trap will equal or exceed the minimum elevation of the embankment along the entire length of this trap.

Riprap Outlet Sediment Trap ST-V (for Stone Lined Channel)

Contributing Drainage Area (ac.)	Depth of Channel (a) (ft.)	Length of Weir (b) (ft.)
1	1.5	4.0
2	1.5	5.0
3	1.5	6.0
4	1.5	10.0
5	1.5	12.0
6	1.5	14.0
7	1.5	16.0
8	2.0	10.0
9	2.0	10.0
10	2.0	12.0
11	2.0	14.0
12	2.0	14.0
13	2.0	16.0
14	2.0	16.0
15	2.0	18.0

See details for Riprap Outlet Sediment Trap ST-V on Figures 5A.20(1) and 5A.20(2) on pages 5A.43 and 5A.44.

Optional Dewatering Methods

Optional dewatering devices may be designed for use with sediment traps. Included are two methods, which may be used. See Figure 5A.21 on page 5A.45 for details.

Figure 5A.16(1)
Pipe Outlet Sediment Trap: ST-I

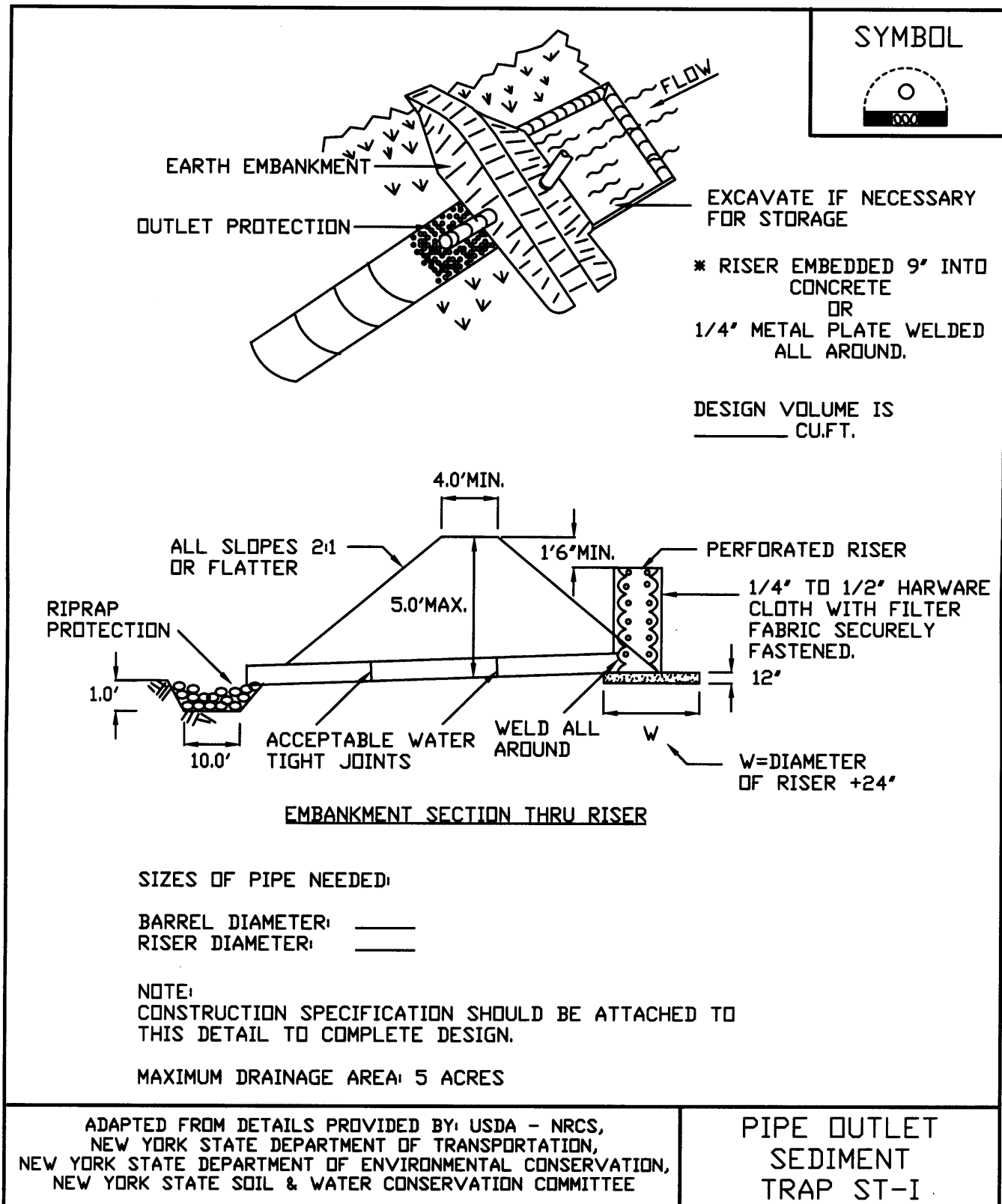


Figure 5A.16(2)

Pipe Outlet Sediment Trap: ST-I—Construction Specifications

CONSTRUCTION SPECIFICATIONS	SYMBOL 
1. AREA UNDER EMBANKMENT SHALL BE CLEARED, GRUBBED AND STRIPPED OF ANY VEGETATION AND ROOT MAT. THE POOL AREA SHALL BE CLEARED. 2. THE FILL MATERIAL FOR THE EMBANKMENT SHALL BE FREE OF ROOTS OR OTHER WOODY VEGETATION AS WELL AS OVER-SIZED STONES, ROCKS, ORGANIC MATERIAL, OR OTHER OBJECTIONABLE MATERIAL. THE EMBANKMENT SHALL BE COMPACTED BY TRAVERSING WITH EQUIPMENT WHILE IT IS BEING CONSTRUCTED. 3. VOLUME OF SEDIMENT STORAGE SHALL BE 3600 CUBIC FEET PER ACRE OF CONTRIBUTORY DRAINAGE. 4. SEDIMENT SHALL BE REMOVED AND TRAP RESTORED TO ITS ORIGINAL DIMENSIONS WHEN THE SEDIMENT HAS ACCUMULATED TO 1/2 THE DESIGN DEPTH OF THE TRAP. REMOVED SEDIMENT SHALL BE DEPOSITED IN A SUITABLE AREA AND STABILIZED. 5. THE STRUCTURE SHALL BE INSPECTED AFTER EACH RAIN AND REPAIRS MADE AS NEEDED. 6. CONSTRUCTION OPERATIONS SHALL BE CARRIED OUT IN SUCH A MANNER THAT EROSION AND SEDIMENT ARE CONTROLLED. 7. THE STRUCTURE SHALL BE REMOVED AND AREA STABILIZED WHEN THE DRAINAGE AREA HAS BEEN PROPERLY STABILIZED. 8. ALL FILL SLOPES SHALL BE 2:1 OR FLATTER; CUT SLOPES 1:1 OR FLATTER. 9. ALL PIPE CONNECTIONS SHALL BE WATERTIGHT. 10. THE TOP 2/3 OF THE RISER SHALL BE PERFORATED WITH ONE (1) INCH DIAMETER HOLES OR SLITS SPACED SIX (6) INCHES VERTICALLY AND HORIZONTALLY AND PLACED IN THE CONCAVE PORTION OF PIPE. NO HOLES WILL BE ALLOWED WITHIN SIX (6) INCHES OF THE HORIZONTAL BARREL. 11. THE RISER SHALL BE WRAPPED WITH 1/4 TO 1/2 INCH HARDWARE CLOTH WIRE THEN WRAPPED WITH FILTER CLOTH (HAVING AN EQUIVALENT SIEVE SIZE OF 40-80). THE FILTER CLOTH SHALL EXTEND SIX (6) INCHES ABOVE THE HIGHEST HOLE AND SIX (6) INCHES BELOW THE LOWEST HOLE. WHERE ENDS OF THE FILTER CLOTH COME TOGETHER, THEY SHALL BE OVER-LAPPED, FOLDED AND STAPLED TO PREVENT BYPASS. 12. STRAPS OR CONNECTING BANDS SHALL BE USED TO HOLD THE FILTER CLOTH AND WIRE FABRIC IN PLACE. THEY SHALL BE PLACED AT THE TOP AND BOTTOM OF THE CLOTH. 13. FILL MATERIAL AROUND THE PIPE SPILLWAY SHALL BE HAND COMPACTED IN FOUR (4) INCH LAYERS. A MINIMUM OF TWO (2) FEET OF HAND COMPACTED BACKFILL SHALL BE PLACED OVER THE PIPE SPILLWAY BEFORE CROSSING IT WITH CONSTRUCTION EQUIPMENT. 14. THE RISER SHALL BE ANCHORED WITH EITHER A CONCRETE BASE OR STEEL PLATE BASE TO PREVENT FLOTATION. FOR CONCRETE BASED THE DEPTH SHALL BE TWELVE (12) INCHES WITH THE RISER EMBEDDED NINE (9) INCHES. A 1/4 INCH MINIMUM THICKNESS STEEL PLATE SHALL BE ATTACHED TO THE RISER BY A CONTINUOUS WELD AROUND THE BOTTOM TO FORM A WATERTIGHT CONNECTION AND THEN PLACE TWO (2) FEET OF STONE, GRAVEL, OR TAMPED EARTH ON THE PLATE.	
ADAPTED FROM DETAILS PROVIDED BY: USDA - NRCS, NEW YORK STATE DEPARTMENT OF TRANSPORTATION, NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION, NEW YORK STATE SOIL & WATER CONSERVATION COMMITTEE	
PIPE OUTLET SEDIMENT TRAP ST-I	

Figure 5A.17
Grass Outlet Sediment Trap: ST-II

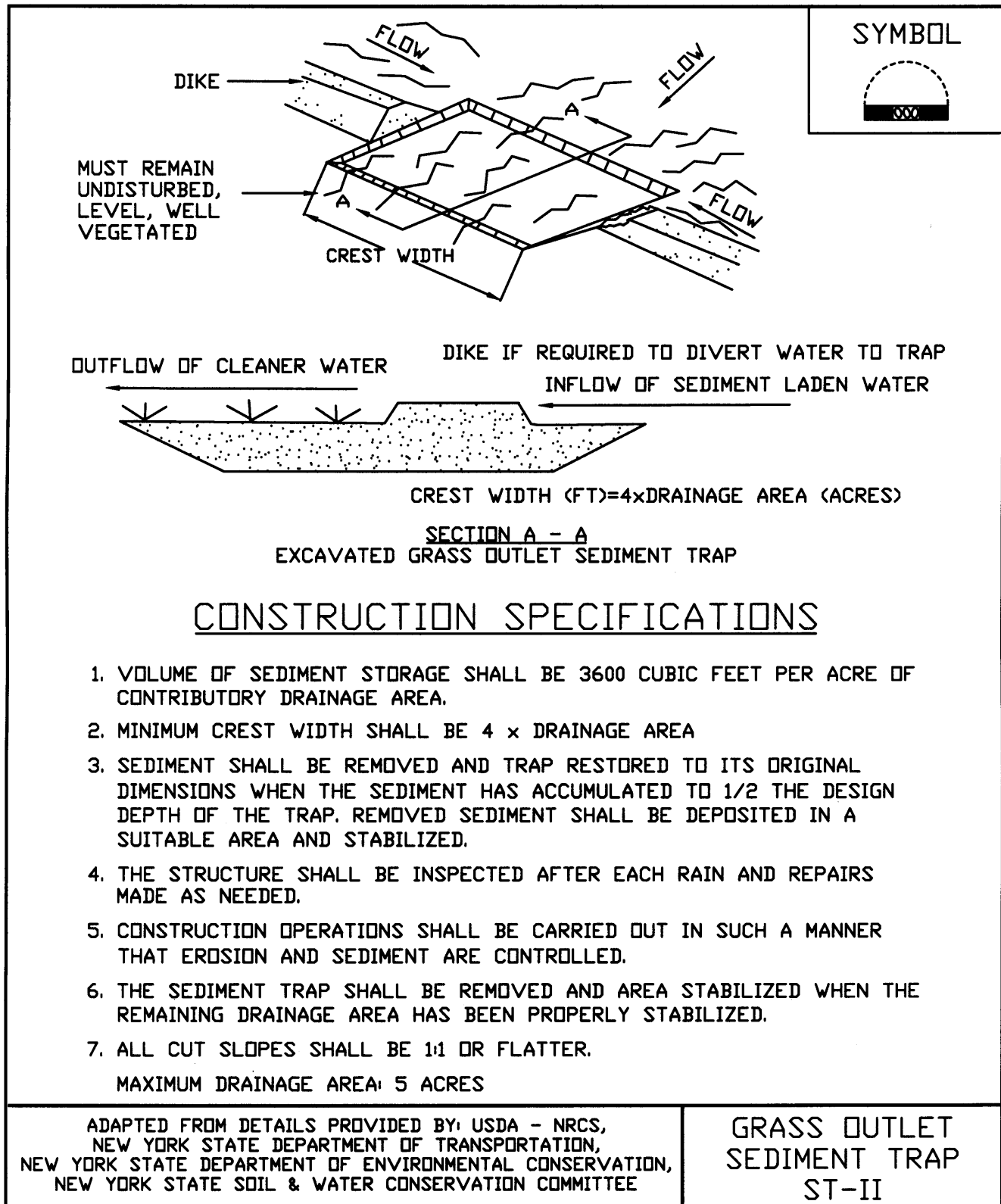


Figure 5A.18
Catch Basin Sediment Trap: ST-III

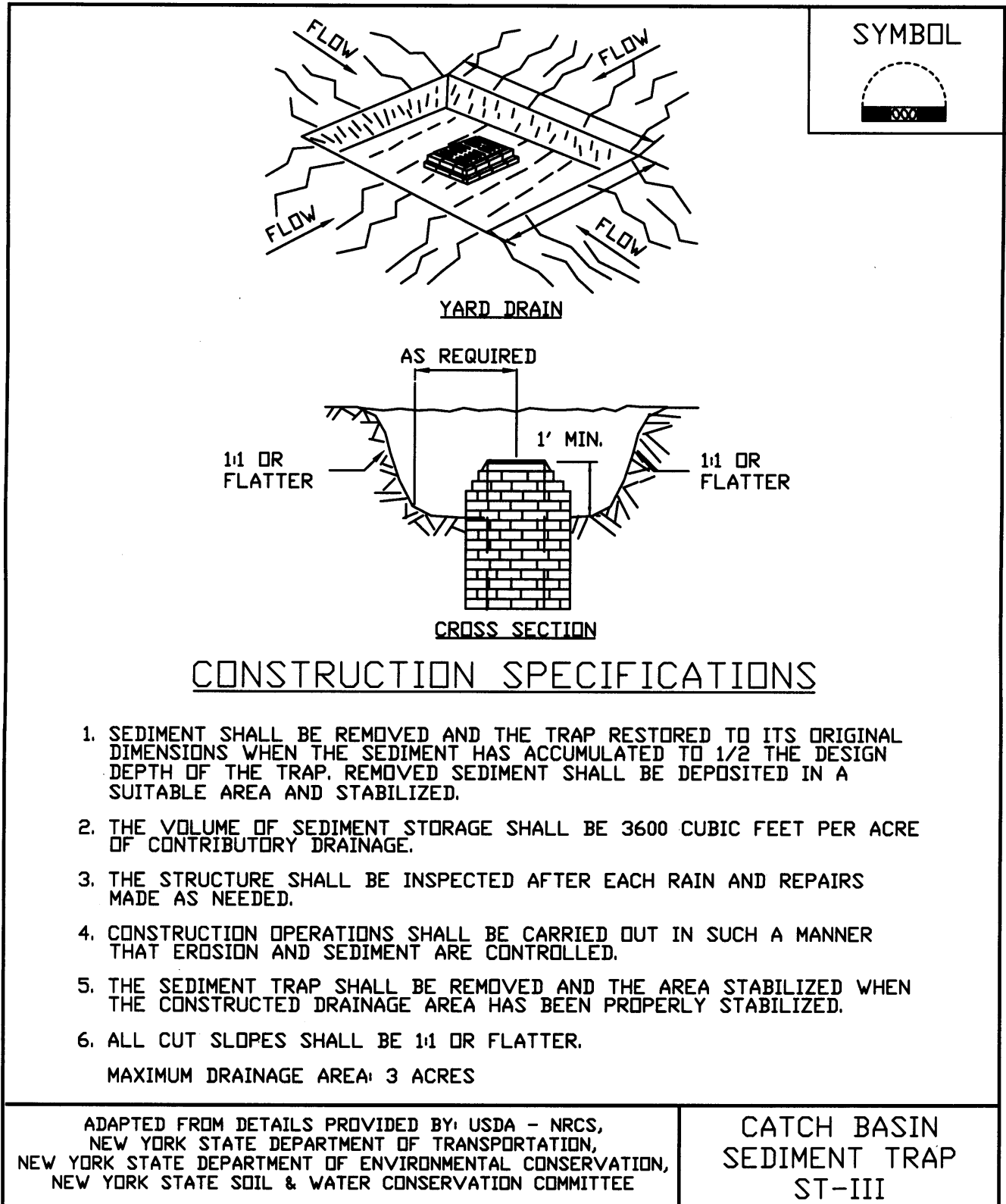


Figure 5A.19
Stone Outlet Sediment Trap: ST-IV

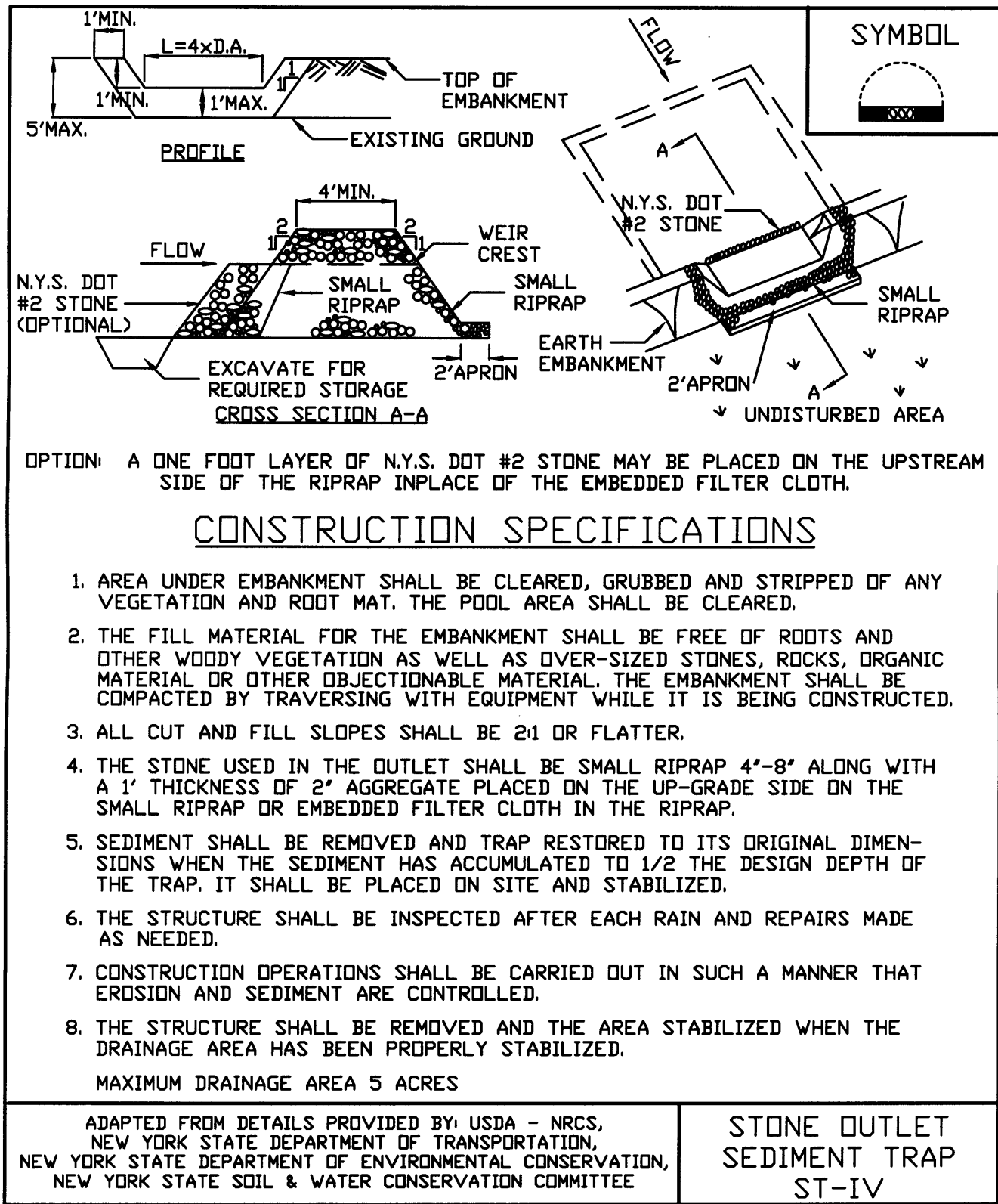


Figure 5A.20(1)
Riprap Outlet Sediment Trap: ST-V

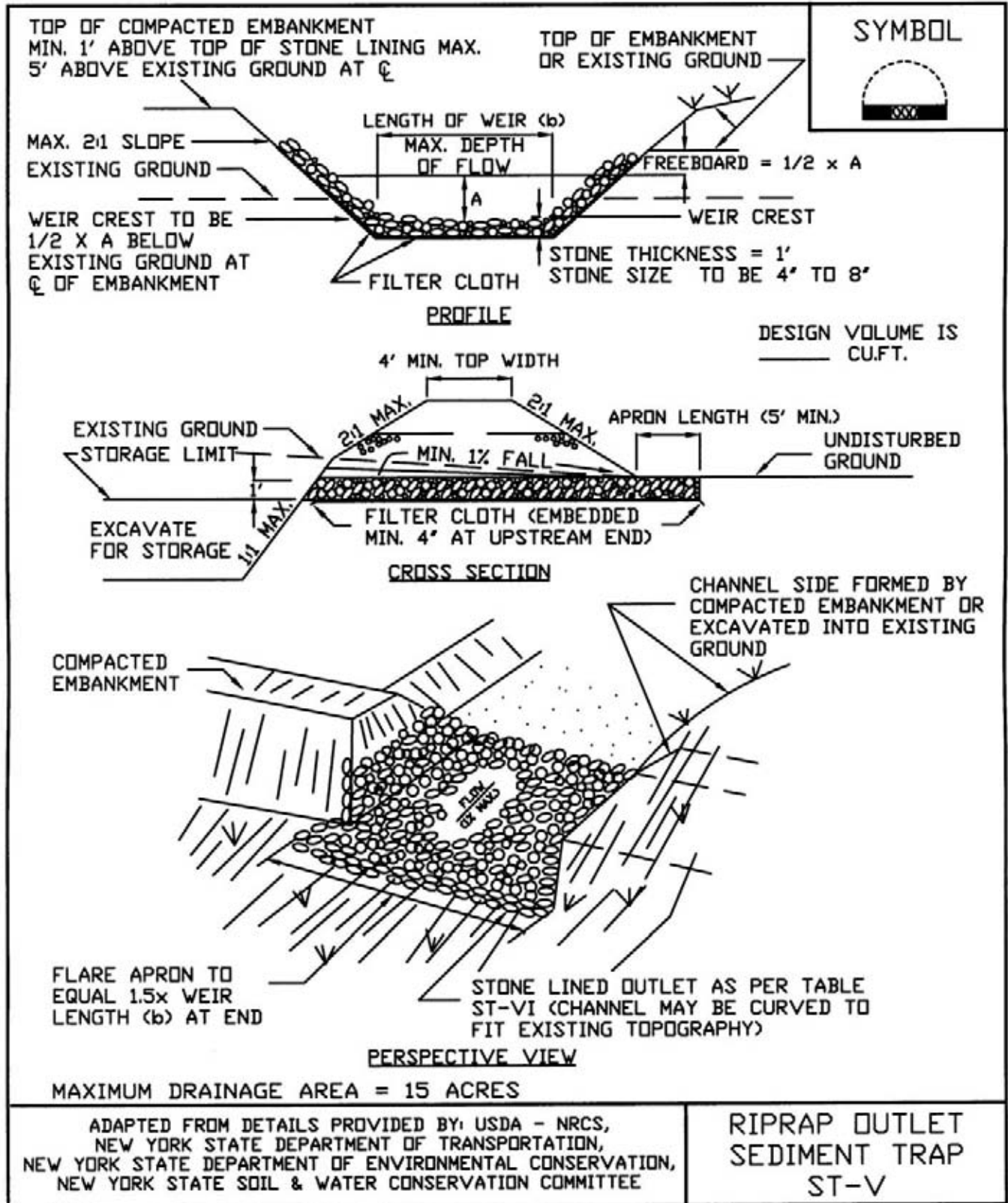
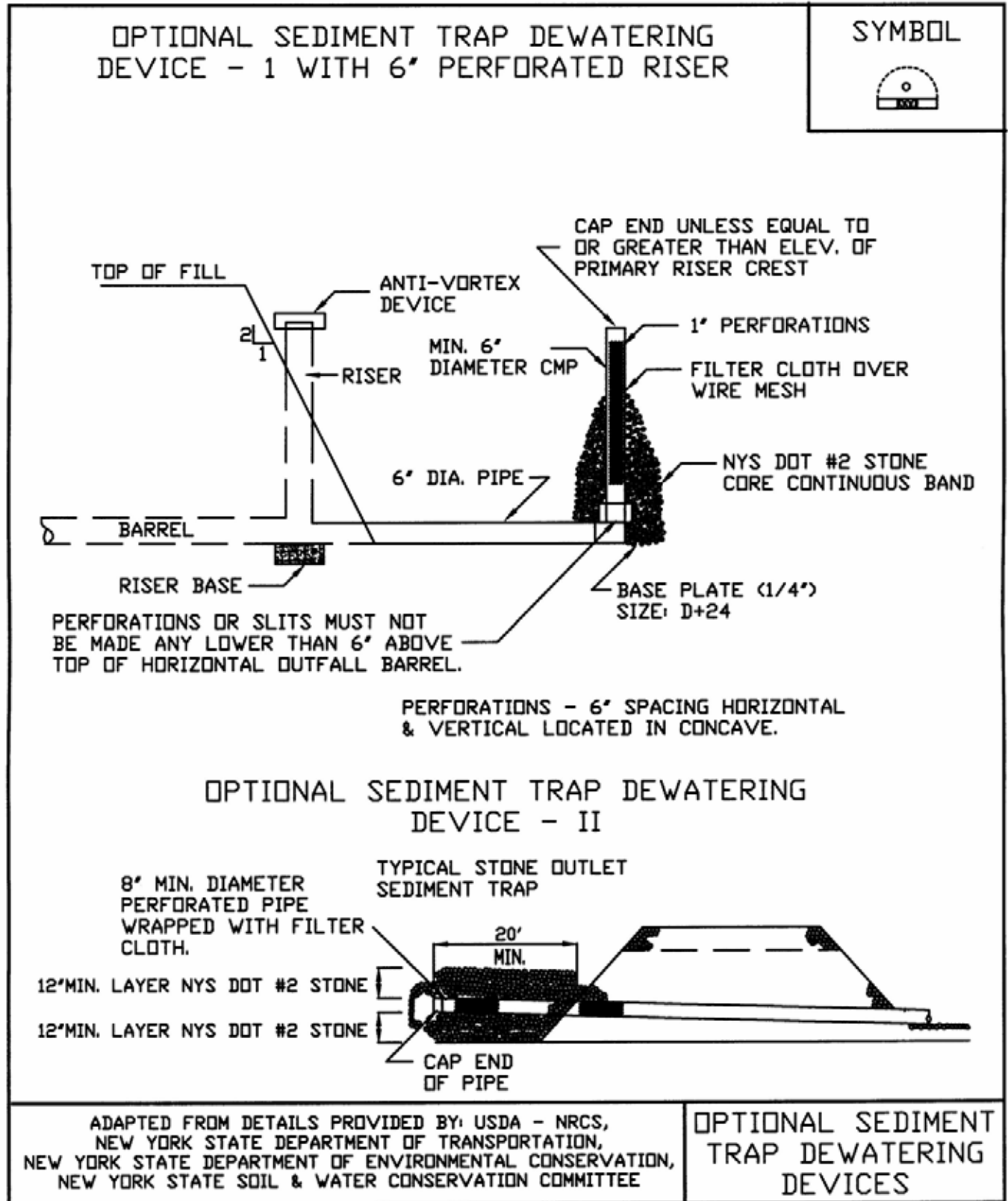


Figure 5A.202)

Riprap Outlet Sediment Trap: ST-V—Construction Specifications

	SYMBOL 
<u>CONSTRUCTION SPECIFICATIONS</u> 1. THE AREA UNDER EMBANKMENT SHALL BE CLEARED, GRUBBED AND STRIPPED OF ANY VEGETATION AND ROOT MAT. THE POOL AREA SHALL BE CLEARED. 2. THE FILL MATERIAL FOR THE EMBANKMENT SHALL BE FREE OF ROOTS OR OTHER WOODY VEGETATION AS WELL AS OVER-SIZED STONES, ROCKS, ORGANIC MATERIAL OR OTHER OBJECTIONABLE MATERIAL. THE EMBANKMENT SHALL BE COMPACTED BY TRAVERSING WITH EQUIPMENT WHILE IT IS BEING CONSTRUCTED. MAXIMUM HEIGHT OF OF EMBANKMENT SHALL BE FIVE (5) FEET, MEASURED AT CENTERLINE OF EMBANKMENT. 3. ALL FILL SLOPES SHALL BE 2:1 OR FLATTER, CUT SLOPES 1:1 OR FLATTER. 4. ELEVATION OF THE TOP OF ANY DIKE DIRECTING WATER INTO TRAP MUST EQUAL OR EXCEED THE HEIGHT OF EMBANKMENT. 5. STORAGE AREA PROVIDED SHALL BE FIGURED BY COMPUTING THE VOLUME AVAILABLE BEHIND THE OUTLET CHANNEL UP TO AN ELEVATION OF ONE (1) FOOT BELOW THE LEVEL WEIR CREST. 6. FILTER CLOTH SHALL BE PLACED OVER THE BOTTOM AND SIDES OF THE OUTLET CHANNEL PRIOR TO PLACEMENT OF STONE. SECTIONS OF FABRIC MUST OVERLAP AT LEAST ONE (1) FOOT WITH SECTION NEAREST THE ENTRANCE PLACED ON TOP. FABRIC SHALL BE EMBEDDED AT LEAST SIX (6) INCHES INTO EXISTING GROUND AT ENTRANCE OUTLET CHANNEL. 7. STONE USED IN THE OUTLET CHANNEL SHALL BE FOUR (4) TO EIGHT (8) INCH RIPRAP. TO PROVIDE A FILTERING EFFECT, A LAYER OF FILTER CLOTH SHALL BE EMBEDDED ONE (1) FOOT WITH SECTION NEAREST ENTRANCE PLACED ON TOP. FABRIC SHALL BE EMBEDDED AT LEAST SIX (6) INCHES INTO EXISTING GROUND AT ENTRANCE OF OUTLET CHANNEL. 8. SEDIMENT SHALL BE REMOVED AND TRAP RESTORED TO ITS ORIGINAL DIMENSIONS WHEN SEDIMENT HAS ACCUMULATED TO 1/2 THE DESIGN DEPTH OF THE TRAP. REMOVED SEDIMENT SHALL BE DEPOSITED IN A SUITABLE AREA AND IN SUCH A MANNER THAT IT WILL NOT ERODE. 9. THE STRUCTURE SHALL BE INSPECTED AFTER EACH RAIN AND REPAIRED AS NEEDED. 10. CONSTRUCTION OPERATIONS SHALL BE CARRIED OUT IN SUCH A MANNER THAT EROSION AND WATER POLLUTION ARE MINIMIZED. 11. THE STRUCTURE SHALL BE REMOVED AND THE AREA STABILIZED WHEN DRAINAGE AREA HAS BEEN PROPERLY STABILIZED. 12. DRAINAGE AREA FOR THIS PRACTICE IS LIMITED TO 15 ACRES OR LESS.	
ADAPTED FROM DETAILS PROVIDED BY: USDA - NRCS, NEW YORK STATE DEPARTMENT OF TRANSPORTATION, NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION, NEW YORK STATE SOIL & WATER CONSERVATION COMMITTEE	RIPRAP OUTLET SEDIMENT TRAP ST-V

Figure 5A.21
Optional Sediment Trap Dewatering Devices



STANDARD AND SPECIFICATIONS FOR SILT FENCE



Definition

A temporary barrier of geotextile fabric installed on the contours across a slope used to intercept sediment laden runoff from small drainage areas of disturbed soil.

Purpose

The purpose of a silt fence is to reduce runoff velocity and effect deposition of transported sediment load. Limits imposed by ultraviolet stability of the fabric will dictate the maximum period the silt fence may be used (approximately one year).

Conditions Where Practice Applies

A silt fence may be used subject to the following conditions:

1. Maximum allowable slope lengths contributing runoff to a silt fence placed on a slope are:

<u>Slope Steepness</u>	<u>Maximum Length (ft.)</u>
2:1	25
3:1	50
4:1	75
5:1 or flatter	100

2. Maximum drainage area for overland flow to a silt fence shall not exceed ¼ acre per 100 feet of fence, with maximum ponding depth of 1.5 feet behind the fence; and
3. Erosion would occur in the form of sheet erosion; and
4. There is no concentration of water flowing to the barrier.

Design Criteria

Design computations are not required for installations of 1 month or less. Longer installation periods should be designed for expected runoff. All silt fences shall be placed as close to the areas as possible, but at least 10 feet from the toe of a slope to allow for maintenance and roll down. The area beyond the fence must be undisturbed or stabilized.

Sensitive areas to be protected by silt fence may need to be reinforced by using heavy wire fencing for added support to prevent collapse.

Where ends of filter cloth come together, they shall be overlapped, folded and stapled to prevent sediment bypass. A detail of the silt fence shall be shown on the plan. See Figure 5A.8 on page 5A.21 for details.

Criteria for Silt Fence Materials

1. Silt Fence Fabric: The fabric shall meet the following specifications unless otherwise approved by the appropriate erosion and sediment control plan approval authority. Such approval shall not constitute statewide acceptance.

<u>Fabric Properties</u>	<u>Minimum Acceptable Value</u>	<u>Test Method</u>
Grab Tensile Strength (lbs)	90	ASTM D1682
Elongation at Failure (%)	50	ASTM D1682

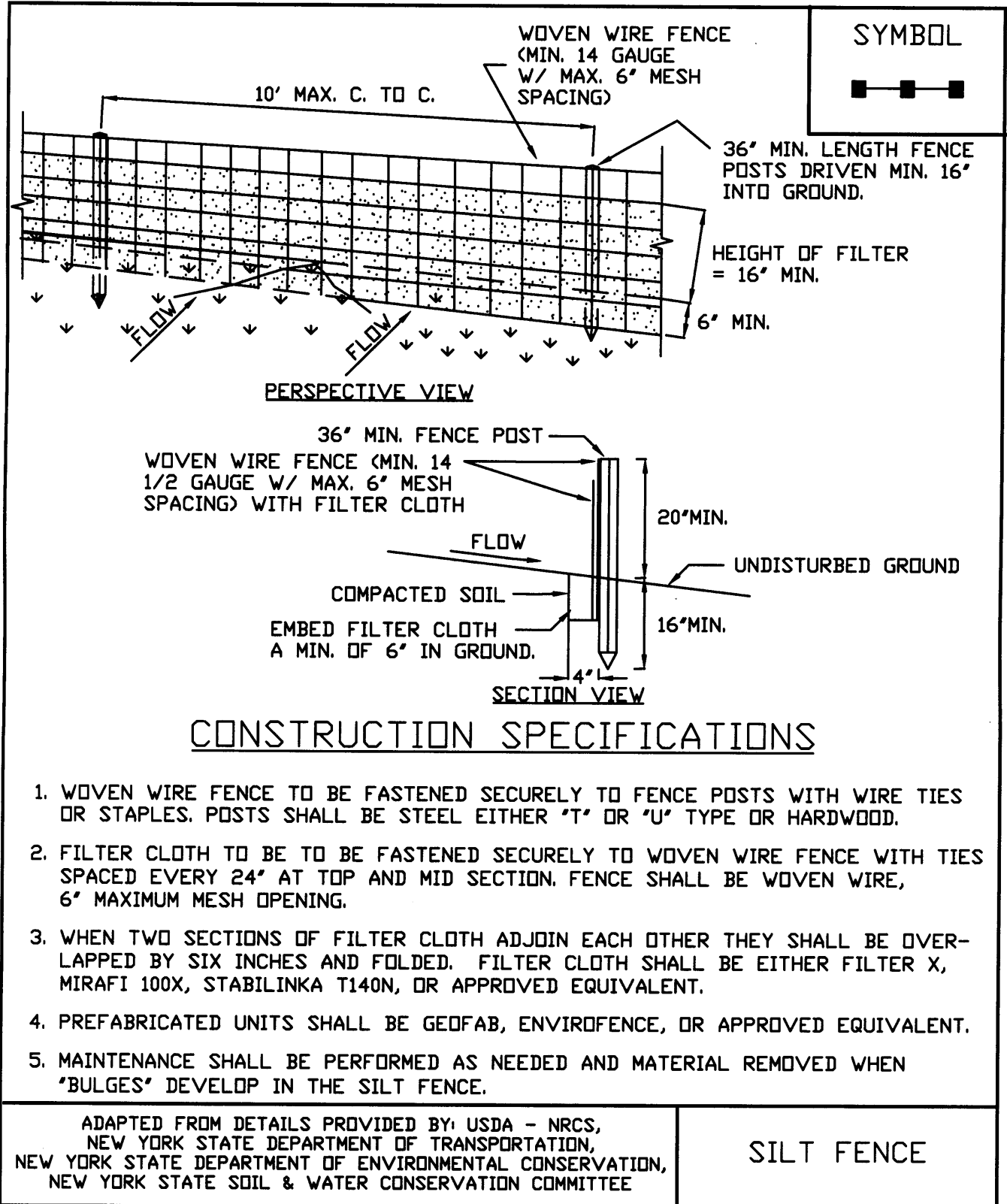
Mullen Burst Strength (PSI)	190	ASTM D3786
Puncture Strength (lbs)	40	ASTM D751 (modified)
Slurry Flow Rate (gal/min/sf)	0.3	
Equivalent Opening Size	40-80	US Std Sieve CW-02215
Ultraviolet Radiation Stability (%)	90	ASTM G-26

2. Fence Posts (for fabricated units): The length shall be a minimum of 36 inches long. Wood posts will be of sound quality hardwood with a minimum cross sectional area of 3.0 square inches. Steel posts will be standard T and U section weighing not less than 1.00 pound per linear foot.

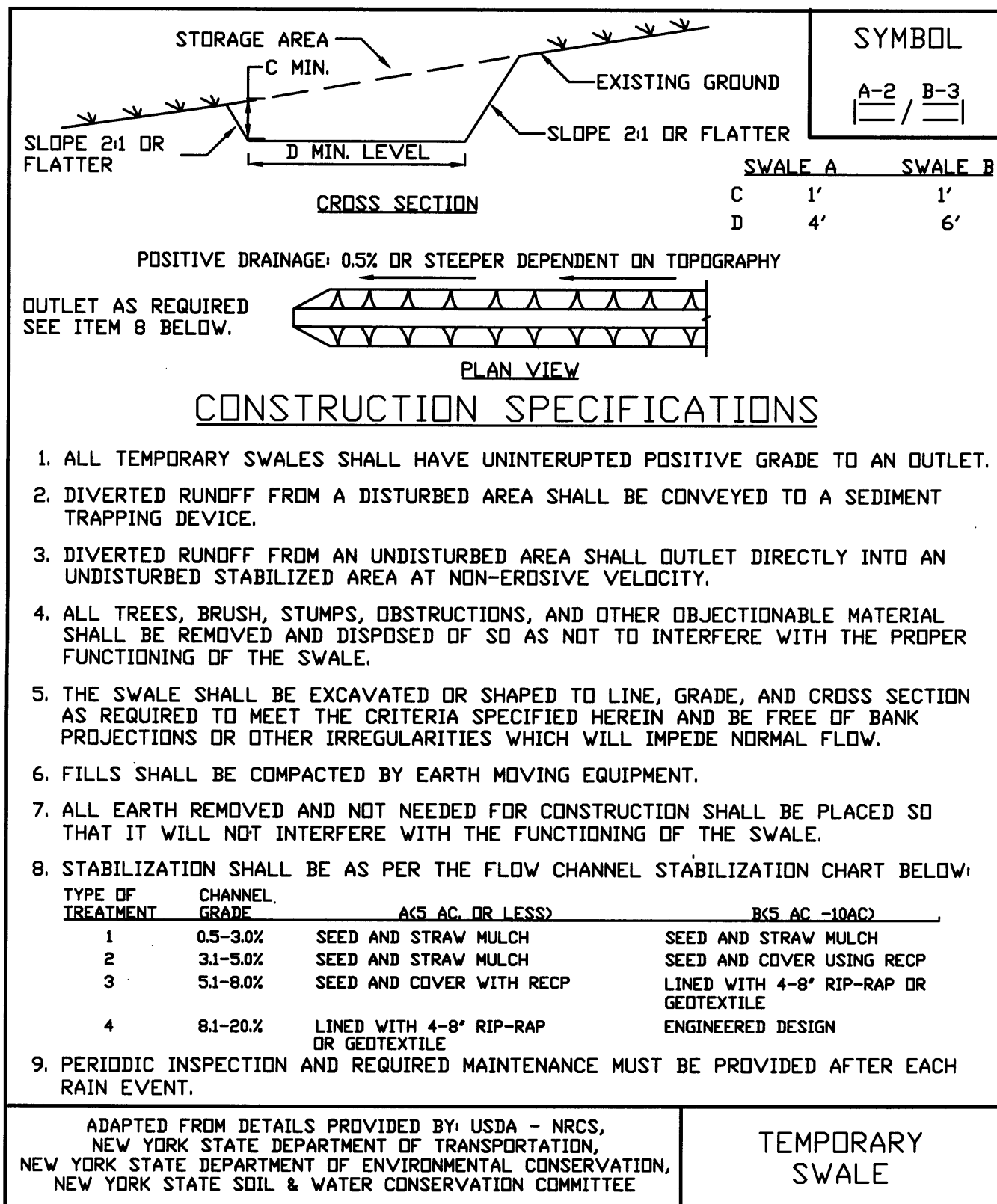
3. Wire Fence (for fabricated units): Wire fencing shall be a minimum 14 gage with a maximum 6 in. mesh opening, or as approved.

4. Prefabricated Units: Envirofence, Geofab, or approved equal, may be used in lieu of the above method providing the unit is installed per details shown in Figure 5A.8.

Figure 5A.8
Silt Fence



**Figure 5A.2
Temporary Swale**



STANDARD AND SPECIFICATIONS FOR TEMPORARY SWALE



Definition

A temporary excavated drainage way.

Purpose

The purpose of a temporary swale is to prevent runoff from entering disturbed areas by intercepting and diverting it to a stabilized outlet or to intercept sediment laden water and divert it to a sediment trapping device.

Conditions Where Practice Applies

Temporary swales are constructed:

1. to divert flows from entering a disturbed area.
2. intermittently across disturbed areas to shorten overland flow distances.
3. to direct sediment laden water along the base of slopes to a trapping device.
4. to transport offsite flows across disturbed areas such as rights-of-way.

Swales collecting runoff from disturbed areas shall remain in place until the disturbed areas are permanently stabilized.

Design Criteria

See Figure 5A.2 on page 5A.5 for details.

	<u>Swale A</u>	<u>Swale B</u>
Drainage Area	<5 Ac	5-10 Ac
Bottom Width of Flow Channel	4 ft	6 ft
Depth of Flow Channel	1 ft	1 ft
Side Slopes	2:1 or flatter	2:1 or flatter
Grade	0.5% Min. 20% Max.	0.5% Min. 20% Max.

For drainage areas larger than 10 acres, refer to the Standard and Specification for Waterways on page 5B.11.

Stabilization

Stabilization of the swale shall be completed within 7 days of installation in accordance with the appropriate standard and specifications for vegetative stabilization or stabilization with mulch as determined by the time of year. The flow channel shall be stabilized as per the following criteria:

<u>Type of Treatment</u>	<u>Channel Grade¹</u>	<u>Flow Channel</u>	
		<u>A (<5 Ac.)</u>	<u>B (5-10 Ac)</u>
1	0.5-3.0%	Seed & Straw Mulch	Seed & Straw Mulch
2	3.1-5.0%	Seed & Straw Mulch	Seed and cover with RECP, Sod, or lined with plastic or 2 in. stone
3	5.1-8.0%	Seed and cover with RECP, Sod, or line with plastic or 2 in. stone	Line with 4-8 in. or stone or Recycled Concrete Equivalent ² or geotextile
4	8.1-20%	Line with 4-8 in. stone or Recycled Concrete Equivalent ² or geotextile	Site Specific Engineering Design

¹ In highly erodible soils, as defined by the local approving agency, refer to the next higher slope grade for type of stabilization.

² Recycled Concrete Equivalent shall be concrete broken into the required size, and shall contain no steel reinforcement.

Outlet

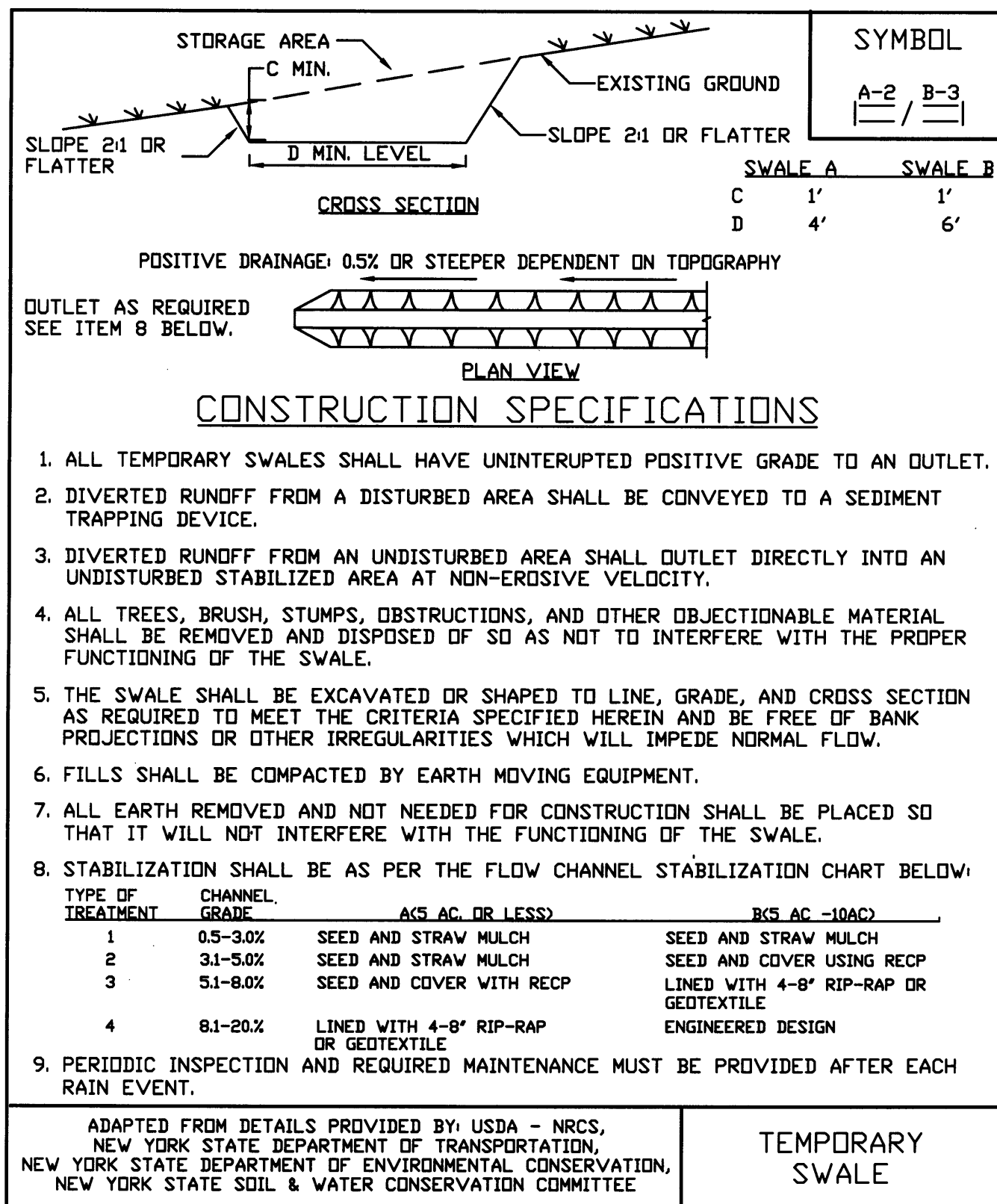
Swale shall have an outlet that functions with a minimum of erosion, and dissipates runoff velocity prior to discharge off the site.

Runoff shall be conveyed to a sediment trapping device such as a sediment trap or sediment basin until the drainage area above the swale is adequately stabilized.

The on-site location may need to be adjusted to meet field conditions in order to utilize the most suitable outlet condition.

If a swale is used to divert clean water flows from entering a disturbed area, a sediment trapping device may not be needed.

**Figure 5A.2
Temporary Swale**



STANDARD AND SPECIFICATIONS FOR TOPSOILING



Definition

Spreading a specified quality and quantity of topsoil materials on graded or constructed subsoil areas.

Purpose

To provide acceptable plant cover growing conditions, thereby reducing erosion; to reduce irrigation water needs; and to reduce the need for nitrogen fertilizer application.

Conditions Where Practice Applies

Topsoil is applied to subsoils that are droughty (low available moisture for plants), stony, slowly permeable, salty or extremely acid. It is also used to backfill around shrub and tree transplants. This standard does not apply to wetland soils.

Design Criteria

1. Preserve existing topsoil in place where possible, thereby reducing the need for added topsoil.
2. Conserve by stockpiling topsoil and friable fine textured subsoils that must be stripped from the excavated site and applied after final grading where vegetation will be established.
3. Refer to USDA Soil Conservation Service (presently Natural Resource Conservation Service) soil surveys or soil interpretation record sheets for further soil texture information for selecting appropriate design topsoil depths.

Site Preparation

1. As needed, install erosion control practices such as diversions, channels, sediment traps, and stabilizing measures, or maintain if already installed.
2. Complete rough grading and final grade, allowing for depth of topsoil to be added.
3. Scarify all compact, slowly permeable, medium and fine textured subsoil areas. Scarify at approximately right angles to the slope direction in soil areas that are steeper than 5 percent. Areas that have been overly compacted shall be decompacted to a minimum depth of 12 inches with a deep ripper or chisel plow prior to topsoiling.
4. Remove refuse, woody plant parts, stones over 3 inches in diameter, and other litter.

Topsoil Materials

1. Topsoil shall have at least 6 percent by weight of fine textured stable organic material, and no greater than 20 percent. Muck soil shall not be considered topsoil.
2. Topsoil shall have not less than 20 percent fine textured material (passing the NO. 200 sieve) and not more than 15 percent clay.
3. Topsoil treated with soil sterilants or herbicides shall be so identified to the purchaser.
4. Topsoil shall be relatively free of stones over 1 1/2 inches in diameter, trash, noxious weeds such as nut sedge and quackgrass, and will have less than 10 percent gravel.
5. Topsoil containing soluble salts greater than 500 parts per million shall not be used.

Application and Grading

1. Topsoil shall be distributed to a uniform depth over the area. It shall not be placed when it is partly frozen, muddy, or on frozen slopes or over ice, snow, or standing water puddles.
2. Topsoil placed and graded on slopes steeper than 5 percent shall be promptly fertilized, seeded, mulched, and stabilized by "tracking" with suitable equipment.

3. Apply topsoil in the following amounts:

Site Conditions	Intended Use	Minimum Topsoil Depth
1. Deep sand or loamy sand	Mowed lawn	6 in.
	Tall legumes, unmowed	2 in.
	Tall grass, unmowed	1 in.
2. Deep sandy loam	Mowed lawn	5 in.
	Tall legumes, unmowed	2 in.
	Tall grass, unmowed	none
3. Six inches or more: silt loam, loam, or silt	Mowed lawn	4 in.
	Tall legumes, unmowed	1 in.
	Tall grass, unmowed	1 in.

STANDARD AND SPECIFICATIONS FOR WATER BAR



Definition

A ridge or ridge and channel constructed diagonally across a sloping road or utility right-of-way that is subject to erosion.

Purpose

To limit the accumulation of erosive velocity of water by diverting surface runoff at pre-designed intervals.

Conditions Where Practice Applies

Where runoff protection is needed to prevent erosion on sloping access right-of-ways or either long, narrow sloping areas generally less than 100 feet in width.

Design Criteria

Design computations are not required.

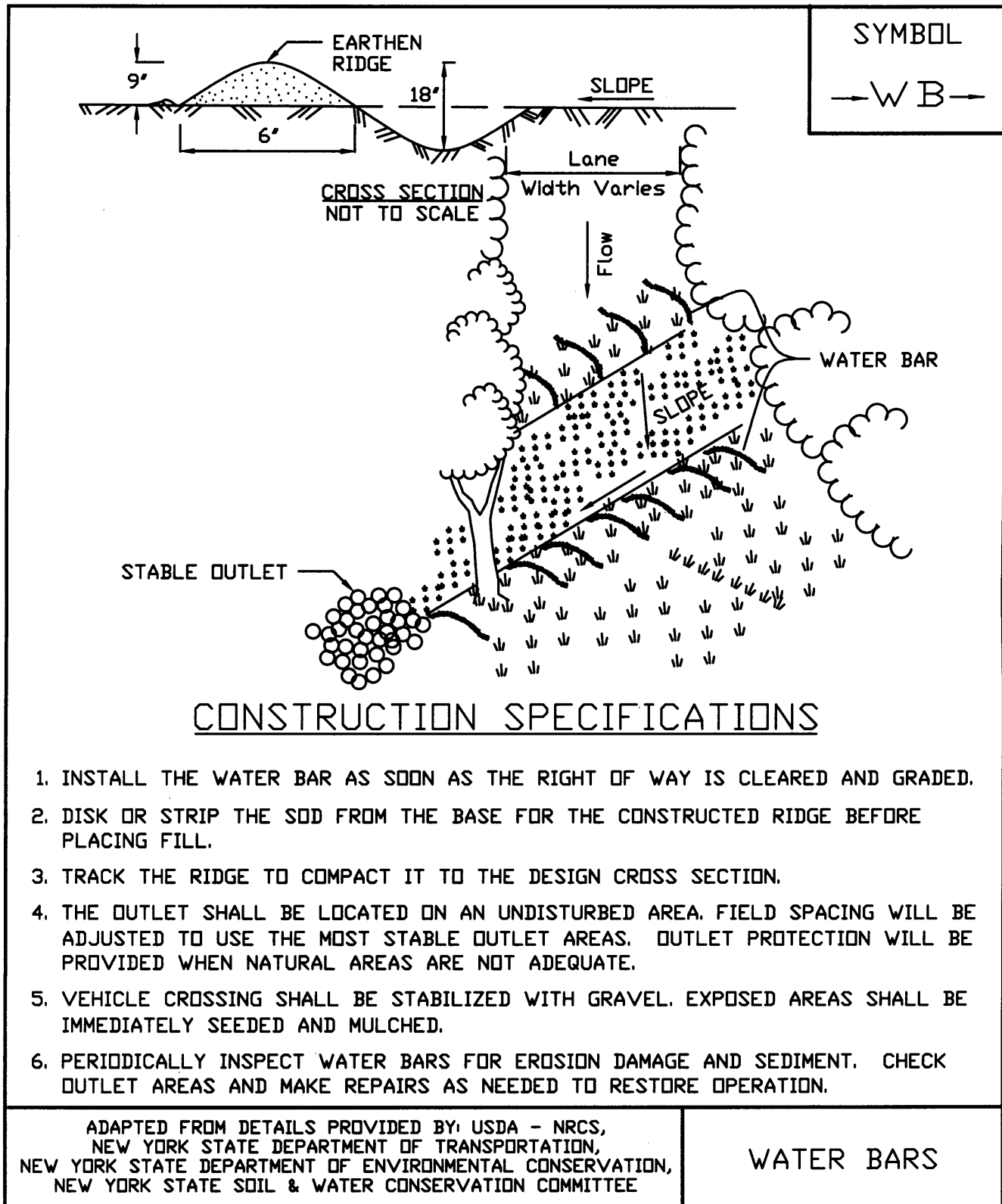
1. The design height shall be minimum of 12 inches measured from channel bottom to ridge top.
2. The side slopes shall be 2:1 or flatter, a minimum of 4:1 where vehicles cross.
3. The base width of the ridge shall be six feet minimum.
4. The spacing of the water bars shall be as follows:

<u>Slope (%)</u>	<u>Spacing (ft)</u>
<5	125
5 TO 10	100
10 TO 20	75
20 TO 35	50
>35	25

5. The positive grade of the water bar shall not exceed 2%. A crossing angle of approximately 60 degrees is preferred.
6. Once diverted, water must be conveyed to a stable system (i.e. vegetated swale or storm sewer system). Water bars should have stable outlets, either natural or constructed. Site spacing may need to be adjusted for field conditions to use the most suitable areas for water disposal.

See Figure 5A.4 for details.

Figure 5A.4
Water Bar



APPENDIX L

EMERGENCY CONTACT LIST

Operator/Owner

Company: _____
Address: _____

Emergency Number: _____
Local Number: _____
Contact Person: _____
Email: _____

Municipal Emergency Agencies

Department: _____
Address: _____

Emergency Number: Dial 911
Local Number: _____
Contact Person: _____

County Sheriff/Police Authority

Department: _____
Address: _____

Emergency Number: Dial 911
Local Number: _____
Contact Person: _____

New York Department of Environmental Protection

Regional Office Number: _____
Contact Person: _____

United States Environmental Protection Agency

Phone Number: (617) 918-1111
Contact Person: _____

Additional Contacts or Agency's

Department: _____
Address: _____

Emergency Number: Dial 911
Local Number: _____
Contact Person: _____

Additional Contacts or Agency's

Department: _____
Address: _____

Emergency Number: Dial 911
Local Number: _____
Contact Person: _____

**STORMWATER POLLUTION PREVENTION PLAN
PLAN CHANGES, AUTHORIZATION AND CHANGE CERTIFICATION**

CHANGES REQUIRED TO THE POLLUTION PREVENTION PLAN:

REASONS FOR CHANGES:

REQUESTED BY: _____

DATE: _____

AUTHORIZED BY: _____

DATE: _____

CERTIFICATION OF CHANGES:

I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate and complete. I am aware that false statements made herein are punishable as a Class A misdemeanor pursuant to Section 210.45 of the Penal Law.

SIGNATURE _____ DATE _____

APPENDIX M

OPERATIONS AND MAINTENANCE MANUAL

FOR THE DEVELOPMENT OF
SOUTH HAMILTON

CITY OF POUGHKEEPSIE
DUTCHESS COUNTY, NEW YORK

PREPARED FOR:

O'NEILL GROUP HAMILTON, LLC
241 HUDSON AVENUE
HACKENSACK, NEW JERSEY 07601

OCTOBER 3, 2023

PREPARED BY:

Arico Associates
1407 Route 9, Bld.2, Suite 6
Clifton Park, New York 12065

ARICO ASSOCIATES PROJECT ID: 22.10



General

The owner shall overtake and become responsible for all inspecting, monitoring and maintaining erosions control features and drainage structures over the lifetime of the feature. The inspection of on-site stabilization measures will become part of routine and preventive maintenance practices developed by the owner and/or his representatives.

Inspection

The entire site should be inspected regularly for the presence of erosion gullies, cracking and wash-outs caused by heavy storm events. The cause of any irregularities should be identified and addressed in a timely manner. At a minimum, eroded areas should be topsoiled, seeded and mulched, as necessary, depending upon the identified irregularity.

The stormwater management system (i.e. roadway drainage ditches, stormwater management ponds, pipe inlets and outlets, etc.) shall be inspected at least yearly for sediment build-up, debris and structural integrity. Stormwater management pond outlet control structures should be checked more frequently for debris blockages. Vacuum truck services should be secured by the owner for periodic removal of sediment within the pond system, the outlet control structure and drainage structures.

The inspection should include, but not be limited to the following:

- Accumulation of pollutants such as grease and oils
- Stabilization and condition of vegetative ground cover areas
- Cracking, settlement, sliding or gully erosions of embankments
- Sedimentation of downstream water bodies, culvert or swales
- Sedimentation of lawns, pavement areas or catch basin sumps
- Presences of animal borrows, clogging or sedimentation at inlets or outlets
- Erosion or disruption of flow paths from stormwater management areas
- Structural integrity of spillways and obstruction of overflow
- Inlet and outlet riprap for scouring or dislodged stones or obstructions
- Forebays and aquatic benches to ensure proper and structural integrity.

Any sediment build-up should be removed and disposed of utilizing acceptable practices. Care should be exercised to avoid storing snow on any stormwater facilities, including the outfall swale.

Maintenance

The permittee shall at all time properly operate and maintain all facilities and systems of treatment and control, as well as related appurtenances, which are install or to be used by the permittee to achieve compliance with the conditions of this permit and within the requirements of SWPPP's. Maintenance shall occur on a regular basis and should include, but not be limited to the following:

- Mowing of lawn areas at least three times per year to prevent over and vigorous growth

- Mowing of steep slopes, embankments, overflow spillways at least three times per year to prevent overgrowth
- Sweeping common paved areas at least three times per year, especially during early spring to remove excessive deicing material and sediment
- Removing litter and debris that has accumulated during mowing and sweeping operations
- Repairing catch basins, storm sewer components upon immediately addressing and identifying structural deficiency
- Clean catch basin sumps once accumulation is within six inches of the outlet pipe invert
- Repair of riprap lined swales, inlets and outlets, replacing stone where signs of scouring has occurred
- Remove trees and excessive growth from inlets and outlets
- Basins and forebays shall be monitored periodically for sediment accumulations. Once the accumulations have reduced the water quality volume by more than 25% or when 12" has accumulated around the inlets or outlets, sediment shall be removed. Removal of sediment shall be in a manner to re-establish original grades as shown on the original construction plans.

Preventative Measures to Reduce Maintenance Costs

The most effective way to maintain your water quality facility is to prevent the pollutants from entering the facility in the first place. Common pollutants include sediment, trash & debris, chemicals, dog wastes, runoff from stored materials, illicit discharges into the storm drainage system and many others.

- Educate property owners/residents to be aware of how their actions affect water quality, and how they can help reduce maintenance costs.
- Keep properties, streets and gutters, and parking lots free of trash, debris, and lawn clippings.
- Ensure the proper disposal of hazardous wastes and chemicals.
- Plan lawn care to minimize the use of chemicals and pesticides.
- Sweep paved surfaces and put the sweepings back on the lawn.
- Be aware of automobiles leaking fluids. Use absorbents such as cat litter to soak up drippings – dispose of properly.
- Re-vegetate disturbed and bare areas to maintain vegetative stabilization.
- Clean out the upstream components of the storm drainage system, including inlets, storm sewers and outfalls.
- Do not store materials outdoors (including landscaping materials) unless properly protected from runoff.

Access and Easements

All stormwater management facilities located on the site have both a designated access location as well as a maintenance easement. Refer to the Stormwater Facilities Map located in Appendix G for access and easement locations.

Safety

Keep safety considerations at the forefront of inspection procedures at all times. Likely hazards should be anticipated and avoided. Never enter a confined space (outlet structure, manhole, etc) without proper training or equipment. A confined space should never be entered without at least one additional person present. If a toxic or flammable substance is discovered, leave the immediate area and contact the local Sheriff at 911.

Potentially dangerous (e.g., fuel, chemicals, hazardous materials) substances found in the areas must be referred to the local Sheriff's Office immediately for response by the Hazardous Materials Unit. The emergency contact number is 911.

Vertical drops may be encountered in areas located within and around the facility. Avoid walking on top of retaining walls or other structures that have a significant vertical drop. If a vertical drop is identified within the pond that is greater than 48" in height, make the appropriate note/comment on the maintenance inspection form.

If any hazard is found within the facility area that poses an immediate threat to public safety, contact the local Sheriff's Office immediately.

Field Inspection Equipment

It is imperative that the appropriate equipment is taken to the field with the inspector(s). This is to ensure the safety of the inspector and allow the inspections to be performed as efficiently as possible. Below is a list of the equipment that may be necessary to perform the inspections of all Stormwater Management Facilities:

- Protective clothing and boots.
- Safety equipment (vest, hard hat, confined space entry equipment).
- Communication equipment.
- Operation and Maintenance Manual for the site including stormwater management facility location maps.
- Clipboard.
- Stormwater Facility Maintenance Inspection Forms (See Appendix D).
- Manhole Lid Remover
- Shovel.

Some of the items identified above need not be carried by the inspector (manhole lid remover, shovel, and confined space entry equipment). However, this equipment should be available in the vehicle driven to the site.

Inspection Procedures

All stormwater management facilities are required to be inspected by a qualified individual at a minimum of once per year. Inspections should follow the inspection guidance found in the SOP for the specific type of facility.

Inspection Report

The person(s) conducting the inspection activities shall complete the appropriate inspection report for the specific facility. Inspection reports are located in this Appendix.

The following information explains how to fill out the Inspection Forms:

General Information

This section identifies the facility location, person conducting the inspection, the date and time the facility was inspected, and approximate days since the last rainfall. Property classification is identified as single-family residential, multi-family residential, commercial, or other.

The reason for the inspection is also identified on the form depending on the nature of the inspection. All facilities should be inspected on an annual basis at a minimum. In addition, all facilities should be inspected after a significant precipitation event to ensure the facility is draining appropriately and to identify any damage that occurred as a result of the increased runoff.

Verification of Inspection and Form Submittal

The Stormwater Management Facility Inspection Form provides a record of inspection of the facility. Inspection Forms for each facility type are provided in this appendix. Verification of the inspection of the stormwater facilities, the facility inspection form(s), and Inspector Qualifications shall be provided to the proper authority on an annual basis. The verification and the inspection form(s) shall be reviewed and submitted by the property owner or property manager.

Routine Work

The majority of this work consists of scheduled mowing's and trash and debris pickups for stormwater management facilities during the growing season. This includes items such as the removal of debris/material that may be clogging the outlet structure well screens and trash racks. It also includes activities such as weed control, mosquito treatment, and algae treatment. These activities normally will be performed numerous times during the year.

Restoration Work

This work consists of a variety of isolated or small-scale maintenance and work needed to address Operational problems. Most of this work can be completed by a small crew, with minor tools, and small equipment.

Rehabilitation Work

This work consists of large-scale maintenance and major improvements needed to address failures within the stormwater management facilities. This work requires consultation and may require an Engineering design with construction plans to be prepared for review and approval. This work may also require more specialized maintenance equipment, surveying, construction permits or assistance through private contractors and consultants.

Maintenance Personnel

Maintenance personnel must be qualified to properly maintain stormwater management facilities. Inadequately trained personnel can cause additional problems resulting in additional maintenance costs.

Maintenance Forms

The Stormwater Management Facility Maintenance Form provides a record of maintenance activities. Maintenance Forms for each facility type are provided in this appendix. Maintenance Forms shall be completed by the contractor completing the required maintenance items. The form shall then be reviewed by the property owner or an authorized agent of the property owner and submitted on an annual basis to the proper authority.

ESTIMATED ANNUAL OPERATION AND MAINTENANCE COST

Cost

6-months	Inspect outlet and inlet control structure openings; Mow extended detention basins	\$3,500.00
1-year	Inspection & Report by Qualified Professional Groom and repair basin side walls and grass	\$3,500.00
5-year	Remove and dispose of sediment Repair grass, side slopes and bottom as needed	\$5,500.00
10-year	Remove and dispose of sediment Repair grass, side slopes and bottom as needed Repair outlet control devices, as needed	\$3,500.00
30-year	Remove and dispose of sediment Repair grass, side slopes and bottom as needed Repair outlet control devices, misc. repairs as needed	\$4,000.00

(*costs are in 2023 dollars. Adjustments should be adjusted for inflation at 3% annually)

October 4, 2023



One Key Development LLC
241 Hudson Street
Attn: George Brandt

RE: Traffic Impact Study for Proposed Multi-Family Residential Building, City of Poughkeepsie, Dutchess County, New York; CM Project No. 123-200

Dear Mr. Brandt:

As requested, Creighton Manning Engineering, LLP (CM) has completed a Traffic Impact Study for the proposed residential development located on South Hamilton Street in the City of Poughkeepsie, Dutchess County, NY. This study is based on traffic engineering industry standards and the Concept Plan prepared by Arico Associates, last revised April 4, 2023, which is included under Attachment A.

1.0 Project Description

The subject site is defined on the Dutchess County Tax Map as Section 6161, Block 30, Lot 151782, and undeveloped. The site has an existing curb cut on South Hamilton Street approximately 80 feet south of Franklin Street. The proposed project consists of the construction of a three-story residential building with 58 units comprised of 29 one-bedroom units and 29 two-bedroom units. The site will be accessed via a 24-foot-wide full-movement driveway at the same location as the existing curb cut and a 17-foot-wide access driveway on Dwight Street. The development will be supported by 102 parking spaces inclusive of six ADA-accessible spaces, which exceeds the City of Poughkeepsie's parking requirements. The proposed development is expected to be completed and occupied by 2026. A map illustrating the site location is shown in Exhibit 1.



Exhibit 1 – Site Location

2.0 Existing Conditions

Roadways Serving the Site

- **Hooker Avenue** is classified as an Urban Minor Arterial roadway and is under the jurisdiction of the City of Poughkeepsie. The roadway runs primarily east-west from NYS Route 376 to South Hamilton Street within the City of Poughkeepsie and neighboring Town of Poughkeepsie. In the vicinity of the site, Hooker Avenue provides one 12-foot-wide travel lane in each direction with on-street parking on some sections of the road. Turn lanes are provided at key intersections. Sidewalks are provided on both sides of the roadway.
- **Montgomery Street** is classified as an Urban Minor Arterial roadway and is under the jurisdiction of the City of Poughkeepsie. The roadway runs primarily east-west from South Clinton Street to Lincoln Avenue (beyond which it continues as Pine Street) within the City of Poughkeepsie. In the vicinity of the site, Montgomery Street provides one 12-foot-wide travel lane in each direction with on-street parking on some sections of the road. Turn lanes are provided at key intersections. Sidewalks are provided on both sides of the roadway.
- **South Hamilton Street** is classified as an Urban Major Arterial roadway and is under the jurisdiction of the City of Poughkeepsie. The roadway runs primarily north-south from Main Street (beyond which it continues as North Hamilton Street) to Livingston Street within the City of Poughkeepsie. In the vicinity of the site, Montgomery Street provides one 12-foot-wide travel lane in each direction with on-street parking on some sections of the road. Turn lanes are provided at key intersections. Sidewalks are provided on both sides of the roadway.
- **Dwight Street** is classified as an Urban Local roadway and is under the jurisdiction of the City of Poughkeepsie. The roadway runs east-west from Hooker Avenue to South Hamilton Street for approximately 1,000 feet. In the vicinity of the site, Dwight Street provides one 12-foot-wide travel lane in each direction with on-street parking on both sides of the road. Sidewalks are provided on both sides of the roadway.

Study Intersections

- **Hooker Avenue/Montgomery Street/South Hamilton Street:** This is a four-leg signalized intersection. The eastbound Montgomery Street approach provides an exclusive left-turn lane and a shared through/right-turn lane. The westbound Hooker Avenue approach provides a shared left-turn/through lane and an exclusive right-turn lane. The northbound South Hamilton Street approach provides a shared left-turn/through/right-turn lane. The southbound South Hamilton Street approach provides a shared left-turn/through/right-turn lane. Marked pedestrian crossings are present on all legs of the intersection. During a field visit to the site, CM found that all the pedestrian signal and countdown timers do not work. The level of service analysis herein was conducted using the traffic signal timings and phasing directive obtained from the City of Poughkeepsie Engineering Department. Exhibit 2 is a Nearmap image that shows the study intersection.



Exhibit 3 – Hooker Avenue/Montgomery Street/ South Hamilton Street Intersection

- **Hooker Avenue/Dwight Street/Hanscom Avenue:** This is a five-leg signalized intersection. The eastbound Dwight Street approach provides a shared left-turn/through/right-turn lane. The westbound Hanscom Avenue approach provides an exclusive left-turn lane and a shared through/right-turn lane. The northbound Hooker Ave approach provides a shared left-turn/through lane. The southbound Hooker Avenue approach provides a shared through/right-turn lane. Marked pedestrian crossings are present on all legs of the intersection. The level of service analysis herein was conducted using the traffic signal timings and phasing directive obtained from the City of Poughkeepsie Engineering Department. Exhibit 3 is a Nearmap image that shows the study intersection.



Exhibit 3 – Hooker Avenue/Dwight Street/Hanscom Avenue Intersection

- **South Hamilton Street/Dwight Street:** This is a three-leg unsignalized intersection operating with stop control on the westbound approach. The westbound Dwight Street approach provides a shared left-turn/right turn. The northbound South Hamilton Street approach provides one shared through/right-turn lane. The southbound South Hamilton Street approach provides a shared left-turn/through lane. Curb ramps are present on the northeast and southeast corners of the intersection. Exhibit 4 depicts the intersection.



Exhibit 4 – South Hamilton Street/Dwight Street Intersection

Data Collection

Turning Movement Counts (TMCs) were conducted on Thursday, July 6, 2023, during the weekday morning (7:00AM - 9:00AM) and weekday evening (4:00PM - 6:00PM) peak periods, and on Saturday July 8, 2023, during the midday (11:00AM - 2:00PM) peak period. These periods coincide with the anticipated peak-hour operation times of the proposed use as well as the adjacent street traffic. The observed peak hours were 8:00AM to 9:00AM and 4:30PM to 5:30PM on the weekday, and 1:00PM to 2:00PM on the Saturday. Counts were performed at the following intersections:

- Hooker Avenue/Montgomery Street/South Hamilton Street
- Hooker Avenue/Dwight Street/Hanscom Avenue

- South Hamilton Street/Dwight Street

Figure 1 shows the 2023 Existing traffic volumes for the study area. The raw TMC data is included under Attachment B.

3.0 Traffic Assessment

Trip Generation

Trip generation determines the quantity of traffic expected to travel to/from a given site. The Institute of Transportation Engineers' (ITE) *Trip Generation Manual*, 11th Edition, is the industry-standard resource used for estimating trip generation for proposed land uses based on data collected at similar uses. Upon review of the *Trip Generation Manual*, Land Use Code (LUC) 220 "Multifamily Housing (Low-Rise)" most accurately describe the proposed use. Table 1 summarizes the trip generation for the weekday AM, weekday PM, and Saturday Midday peak hours.

Table 1 – Trip Generation Summary for Proposed Use

Land Use	Independent Variable	Weekday AM Peak Hour			Weekday PM Peak Hour			Saturday Midday Peak Hour		
		Enter	Exit	Total	Enter	Exit	Total	Enter	Exit	Total
Multifamily Housing (Low-Rise) – LUC 220	58 Units	10	31	41	29	16	45	15	9	24

Table 1 shows that the project is expected to generate 41 total trips during weekday AM peak hour, 45 total trips during the weekday PM peak hour, and 24 trips during the Saturday midday peak hour. It is important to note that there is no "pass-by" component of the traffic associated with the proposed development. Additionally, the magnitude of the new traffic associated with this development is less than the NYSDOT and ITE threshold of 100-site-generated trips on any one intersection, which is an industry threshold indicating whether a proposed development will have a significant impact on off-site intersections. While the anticipated trip generation falls below that threshold, the study herein analyzes three off-site intersections.

Future Traffic Volumes

To evaluate the impact of the proposed project, traffic projections were prepared for the anticipated year of completion – 2026. Historic traffic volume data on Hooker Avenue and South Hamilton Street indicates that traffic volumes along the roadway have increased by 0.24% annually.¹ In order to conservatively forecast the 2026 traffic volumes, a +0.5% growth rate was applied to the existing traffic volumes and compounded annually for three years. These volumes represent 2026 No-Build conditions. These 2026 No-Build conditions are shown on Figure 2 and represent the expected traffic volumes in 2026 *without* the proposed development.

Traffic generated by the project was distributed on the adjacent roadway network based on existing observed travel patterns in the project area and the likely paths of travel of future residents and visitors. The proximity of the site to US Route 9 and US Route 44/US Route 55 (collectively "The Arterials") are expected to influence trip-making behavior along with the Poughkeepsie Train Station and FDR Mid-Hudson Bridge. The trip distribution of the proposed multifamily residential development is shown on Figure 3. The associated site-generated traffic volumes are shown on Figure 4. The site-generated trips were then added to the 2026 No-Build traffic volumes, resulting in the 2026 Build traffic volumes as shown on Figure 5.

¹ Based on NYSDOT ATR Station ID 828764 | ID 821080 | ID 828768 | ID 821002 | ID 821084 | . Study years: 2009, 2012, 2015, 2019.

Traffic Operations

Intersection Level of Service (LOS) and capacity analysis relate traffic volumes to the physical characteristics of an intersection. Intersection evaluations were made using Synchro Version 11 software, which automates the procedures contained in the Highway Capacity Manual. Table 2 summarizes the results of the level of service calculations for the Existing, No-Build, and Build conditions during the weekday AM peak hour, weekday PM peak hour, and Saturday Midday peak hour. The detailed level of service analyses are included under Attachment C.

Table 2 – Level of Service Summary

Intersection	Control	Weekday AM Peak Hour			Weekday PM Hour			Saturday Midday Peak Hour		
		2023 Existing	2026 No-Build	2026 Build	2023 Existing	2026 No-Build	2026 Build	2023 Existing	2026 No-Build	2026 Build
Hooker Ave/Montgomery St/S. Hamilton St	S									
Montgomery St, EB L		A (4.2)	A (4.2)	A (4.2)	A (5.0)	A (5.0)	A (5.2)	A (4.5)	A (4.5)	A (4.6)
TR		A (4.4)	A (4.4)	A (4.4)	A (5.4)	A (5.4)	A (5.6)	A (4.7)	A (4.7)	A (4.8)
Hooker Ave, WB LT		A (9.1)	A (9.1)	A (9.1)	B (11.0)	B (11.0)	B (11.1)	A (9.4)	A (9.4)	A (9.4)
R		A (8.0)	A (8.0)	A (8.1)	A (9.7)	A (9.7)	A (9.9)	A (8.7)	A (8.7)	A (8.7)
S. Hamilton St, NB LTR		B (13.0)	B (13.1)	B (13.5)	B (10.2)	B (10.2)	B (10.3)	A (10.0)	A (10.0)	A (10.0)
S. Hamilton St, SB LTR		B (14.2)	B (14.3)	B (14.4)	B (12.1)	B (12.3)	B (12.3)	B (10.8)	B (10.9)	B (10.9)
Overall		A (9.5)	A (9.5)	A (9.7)	A (9.6)	A (9.6)	A (9.7)	A (8.7)	A (8.7)	A (8.7)
Hooker Ave/Dwight St/Crescent Rd/Hanscom Ave	S									
Dwight St, EB LTR		B (14.4)	B (14.5)	B (15.0)	B (12.3)	B (12.4)	B (12.7)	B (10.7)	B (10.9)	B (11.0)
Hanscom Ave, WB L		B (12.5)	B (12.5)	B (12.7)	B (12.4)	B (12.5)	B (12.8)	A (9.9)	B (10.0)	B (10.0)
R		B (12.7)	B (12.7)	B (12.9)	B (12.4)	B (12.5)	B (12.7)	A (9.7)	A (9.8)	A (9.8)
Hooker Ave, NB LTR		A (6.9)	A (6.9)	A (6.9)	A (7.4)	A (7.3)	A (7.2)	A (8.4)	A (8.3)	A (8.4)
Hooker Ave, SB LTR		A (6.1)	A (6.1)	A (6.1)	A (7.7)	A (7.6)	A (7.5)	A (8.1)	A (8.0)	A (8.0)
Crescent Rd, SB LTR		B (13.9)	B (13.9)	B (14.1)	B (15.0)	B (15.1)	B (15.4)	B (11.0)	B (11.1)	B (11.1)
Overall		A (7.6)	A (7.5)	A (7.6)	A (8.3)	A (8.3)	A (8.3)	A (8.6)	A (8.6)	A (8.6)
S. Hamilton St/Dwight St	U									
Dwight St, WB LR		A (9.6)	A (9.6)	A (9.8)	A (9.6)	A (9.7)	A (9.8)	A (9.4)	A (9.4)	A (9.4)
S. Hamilton St, SB L		A (7.4)	A (7.4)	A (7.4)	A (7.5)	A (7.5)	A (7.5)	A (7.4)	A (7.4)	A (7.4)
S. Hamilton St/Site Driveway	U									
Site Driveway, WB LR		--	--	A (9.1)	--	--	A (9.3)	--	--	A (9.1)
S. Hamilton, NB L		--	--	A (7.4)	--	--	A (7.4)	--	--	A (7.4)

U = Unsignalized intersection

S = Signalized intersection

EB, WB, NB, SB = Eastbound, Westbound, Northbound, and Southbound intersection approaches

L, T, R = Left-turn, Through, and/or Right-turn movements

X (Y.Y) = Level of service (Average delay in seconds per vehicle)

The impact of the project can be described by comparing the analysis of the No-Build and Build operating conditions. The following observation are evident from the analysis:

- **Hooker Avenue/Montgomery Street/S. Hamilton Street:** The level of service analysis indicates that the intersection currently operates at an overall LOS A during all peak hours and will continue to do so in the Build conditions.
- **Hooker Avenue/Dwight Street Crescent Road/Hanscom Avenue:** The level of service analysis indicates that the intersection currently operates at an overall LOS A or better and will continue to do so in the Build conditions.
- **S. Hamilton Street/Dwight Street:** The level of service analysis indicates that the minor approach currently operates at a LOS A and will continue to do so in the Build Condition during the study peak hours.

- **S. Hamilton Street/Site Driveway:** The level of service analysis indicates that the intersection will operate at a LOS A during the study peak hours. The 95th-percentile queue at the driveway exit approach is calculated to be one vehicle during the study peak hours.

4.0 Site Access, Circulation, and Parking

CM reviewed the site access, site circulation and parking as shown on the Concept Plan prepared by Arico Associates, last revised April 4, 2023. The site will be accessed via a full-movement driveway on South Hamilton Street. Vehicles will have a minimum 22-foot-wide drive aisle on site to provide access to the 102 spaces. The site will be supported by 102 parking spaces inclusive of six ADA-accessible spaces. The proposed number of parking spaces meets the City of Poughkeepsie zoning requirements.²

Similar to trip generation, the ITE also provides parking generation data in its publication entitled Parking Generation, 5th Edition. Based on the ITE LUC 220, the proposed development would have a peak parking demand of 65 spaces and the 85th percentile demand is 88 spaces. This indicates that the proposed 102 spaces proposed for the residential development would be adequate to accommodate the peak parking demand.

5.0 Conclusion

The subject site is defined on the Dutchess County Tax Map as Section 6161, Block 30, Lot 151782. The proposed project consists of the construction of a three-story residential building with 58 units comprised of 29 one-bedroom units and 29 two-bedroom units. The site will be accessed via a full-movement driveway on South Hamilton Street and a 17-foot-wide driveway that will be accessed via Dwight Street. The development will be supported by 102 spaces inclusive of six ADA-accessible spaces. The following is noted regarding the proposed project:

- Turning movement counts were collected during a typical weekday and typical Saturday at the study intersections.
- Upon review of the *Trip Generation Manual*, Land Use Code (LUC) 220 “Multifamily Housing (Low-Rise)” most closely described the anticipated use on site.
- The development is expected to generate a total of 41 trips during the AM peak hour, a total of 45 trips during the PM peak hour, and a total of 24 trips during the Saturday midday peak hour.
- The level of service analysis indicates that the Build condition of the study intersections will operate at the levels of service consistent with the No-Build conditions. LOS A and LOS B are optimal operating levels and considered acceptable.
- The project is not expected to have a significant adverse impact on the surrounding roadway network.
- The site provides 102 parking spaces and meets the City of Poughkeepsie parking requirements.
- CM recommends that the City of Poughkeepsie investigate the inoperable pedestrian crossing signals and pushbuttons at the intersection of Hooker Avenue/Montgomery Street/South Hamilton Street.
- CM recommends that the site driveway provide a Stop sign (MUTCD R1-1) on exit approach to South Hamilton Street.

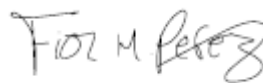
Please do not hesitate to call our office if you have any questions or comments, or require additional information.

Respectfully submitted,
Creighton Manning Engineering, LLP

² Multifamily Dwelling – 1.5 spaces/1-bedroom + 2 spaces/2-bedroom apartment = 1.5*29 + 2 * 29 = 102 spaces required.

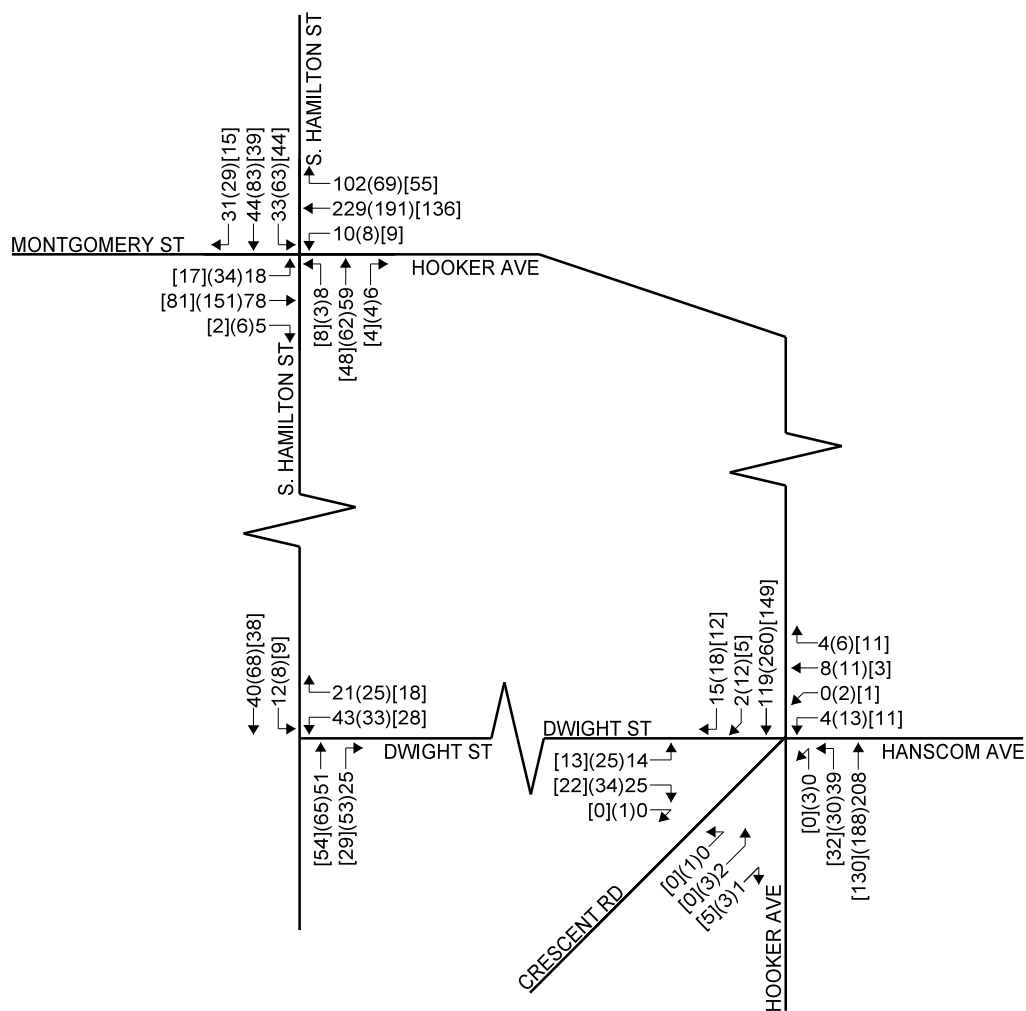
A handwritten signature in blue ink, appearing to read 'Frank A. Filiciotto'.

Frank A. Filiciotto, PE
Associate

A handwritten signature in blue ink, appearing to read 'Fior M. Perez'.

Fior M. Perez, EIT
Assistant Project Engineer

cc:



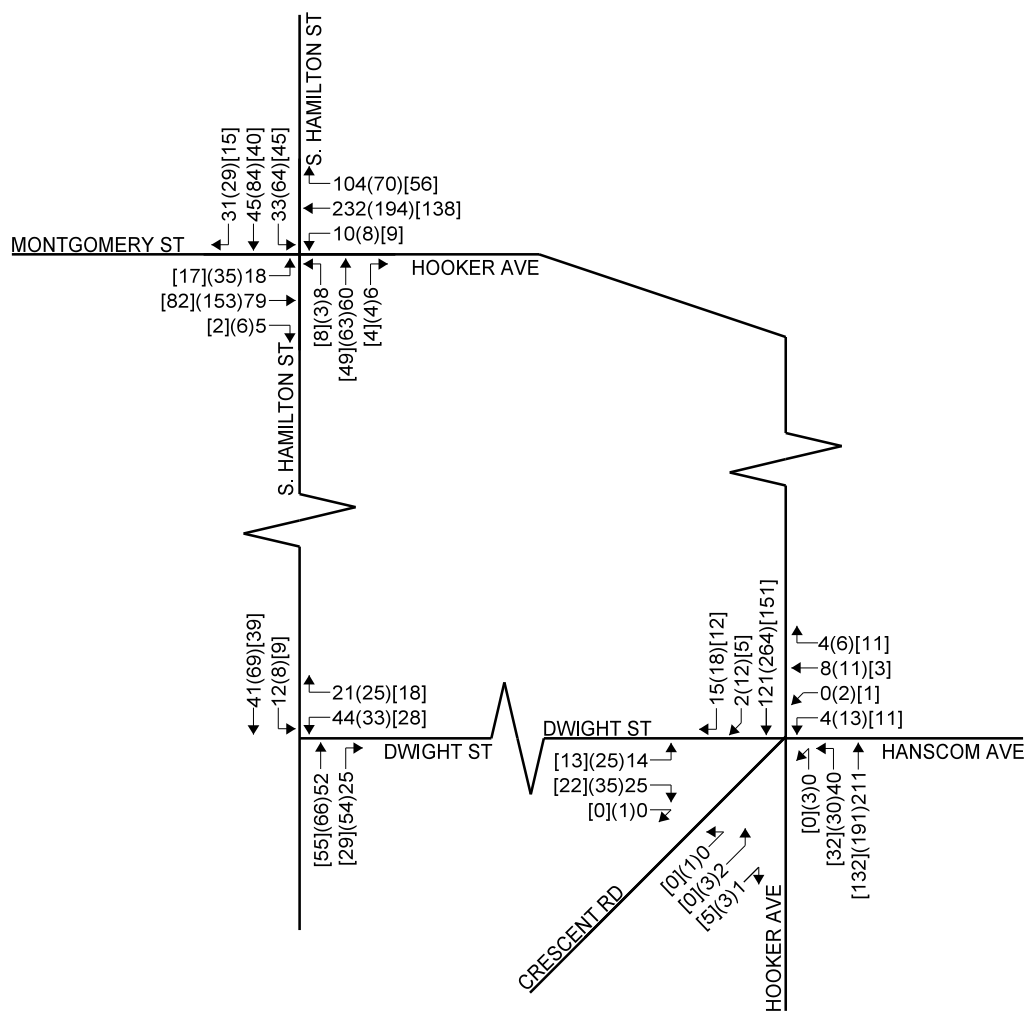
WEEKDAY AM (WEEKDAY PM) [SATURDAY MIDDAY]

2023 EXISTING
TRAFFIC VOLUMES

ONE KEY DEVELOPMENT, LLC
CITY OF POUGHKEEPSIE
DUTCHESS COUNTY, NEW YORK



PROJECT: 123-200 DATE: 07/2023 FIGURE: 1



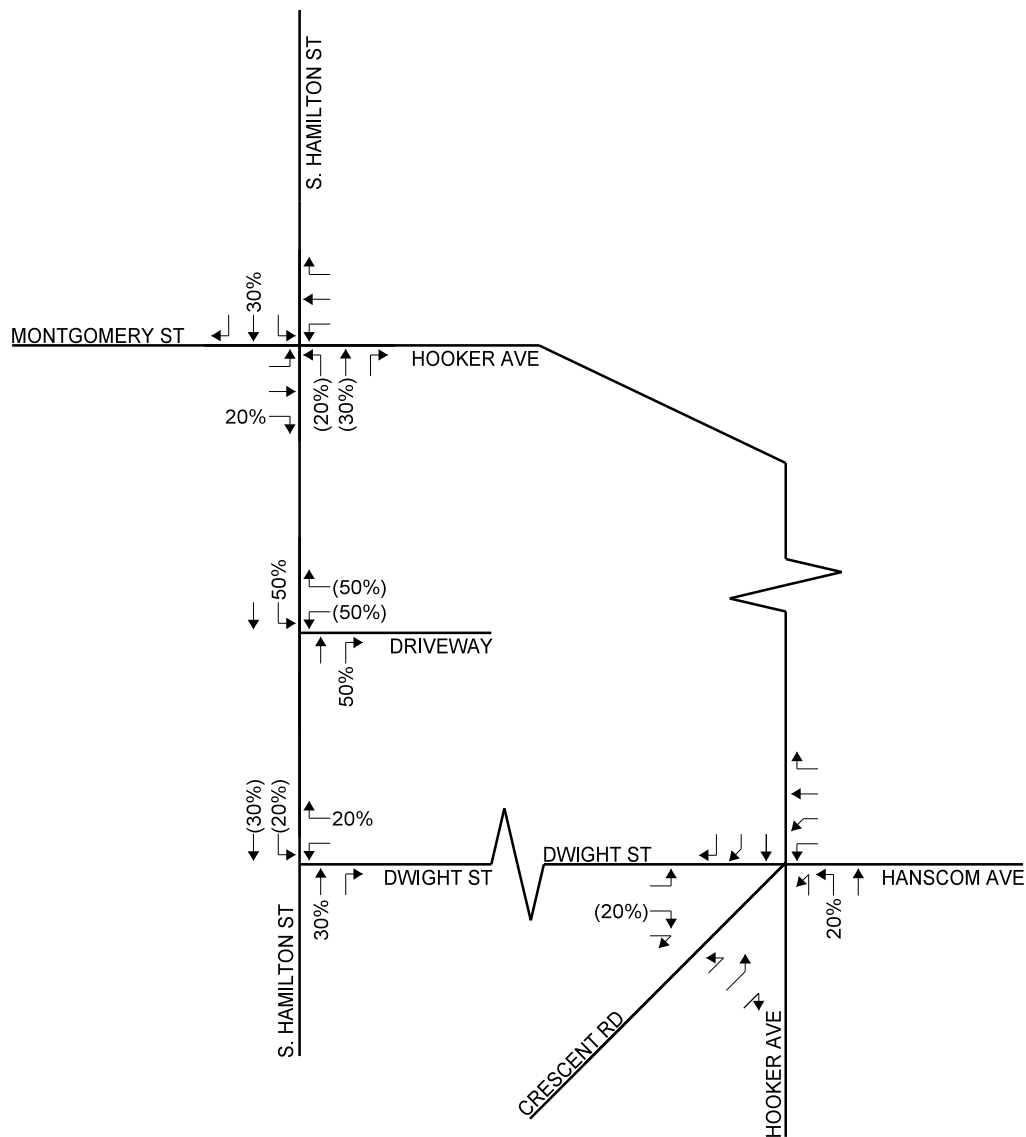
WEEKDAY AM (WEEKDAY PM) [SATURDAY MIDDAY]

2026 NO-BUILD
TRAFFIC VOLUMES

ONE KEY DEVELOPMENT, LLC
CITY OF POUGHKEEPSIE
DUTCHESS COUNTY, NEW YORK



PROJECT: 123-200 DATE: 07/2023 FIGURE: 2



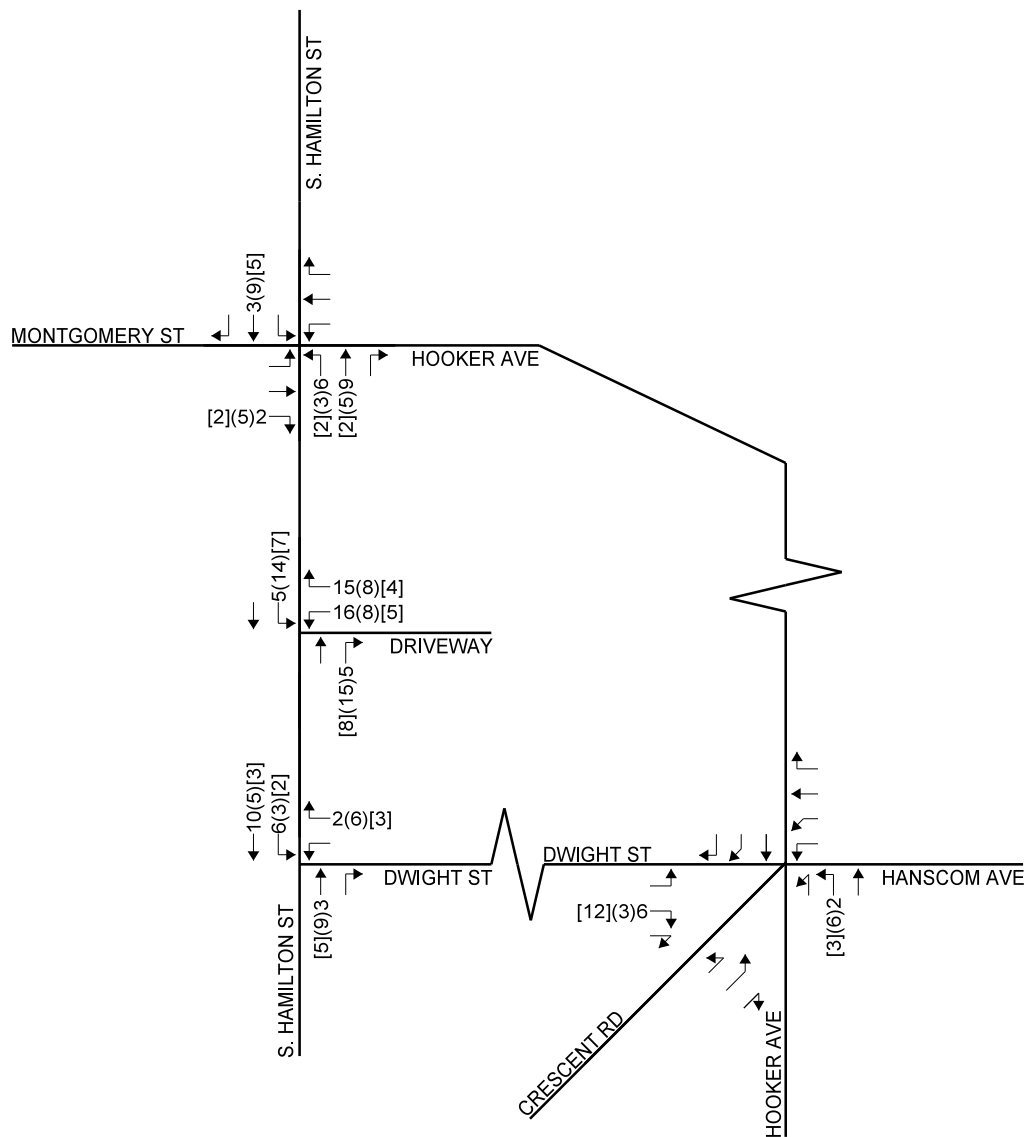
ENTERING(EXITING)

TRIP DISTRIBUTION

ONE KEY DEVELOPMENT, LLC
CITY OF POUGHKEEPSIE
DUTCHESS COUNTY, NEW YORK



PROJECT: 123-200 DATE: 07/2023 FIGURE: 3



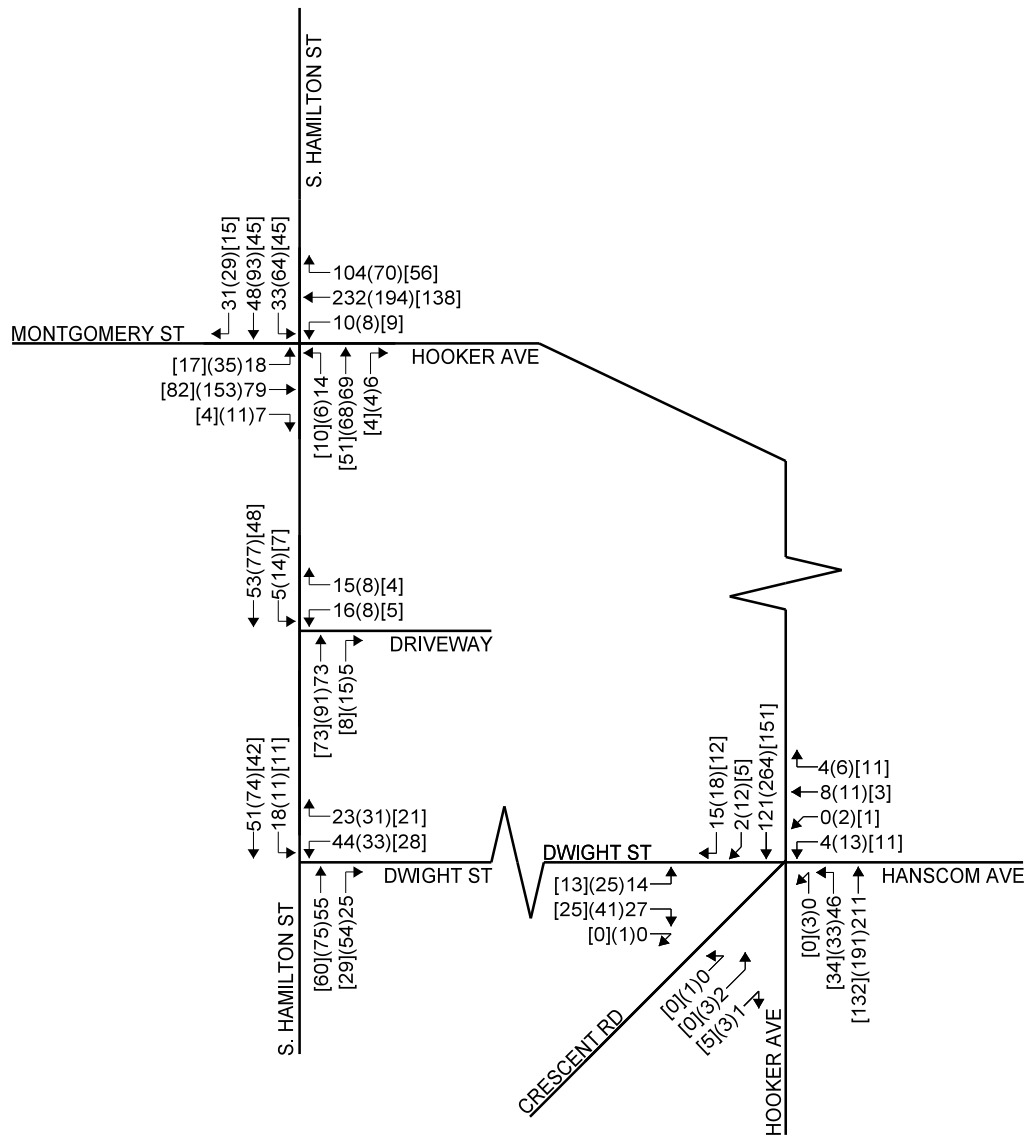
WEEKDAY AM (WEEKDAY PM) [SATURDAY MIDDAY]

TRIP ASSIGNMENT

ONE KEY DEVELOPMENT, LLC
CITY OF POUGHKEEPSIE
DUTCHESS COUNTY, NEW YORK



PROJECT: 123-200 DATE: 07/2023 FIGURE: 4



WEEKDAY AM (WEEKDAY PM) [SATURDAY MIDDAY]

2026 BUILD
TRAFFIC VOLUMES

ONE KEY DEVELOPMENT, LLC
CITY OF POUGHKEEPSIE
DUTCHESS COUNTY, NEW YORK



PROJECT: 123-200 DATE: 07/2023 FIGURE: 5

ATTACHMENT A
CONCEPT PLAN

City of Poughkeepsie
Dutchess County, New York

ATTACHMENT B
TURNING MOVEMENT COUNTS

City of Poughkeepsie
Dutchess County, New York

National Data & Surveying Services Intersection Turning Movement Count

Location: S Hamilton St & Dwight St
City: Poughkeepsie
Control: 1-Way Stop(WB)

Project ID: 23-380020-001
Date: 7/6/2023

Data - Total

NS/EW Streets:	S Hamilton St				S Hamilton St				Dwight St				Dwight St				
AM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
7:00 AM	0	4	4	0	1	5	0	0	0	0	0	0	11	0	2	0	27
7:15 AM	0	12	3	0	1	10	0	0	0	0	0	0	5	0	5	0	36
7:30 AM	0	17	5	0	2	0	0	0	0	0	0	0	13	0	6	0	43
7:45 AM	0	10	12	0	0	8	0	0	0	0	0	0	18	0	5	0	53
8:00 AM	0	9	5	0	2	5	0	0	0	0	0	0	7	0	4	0	32
8:15 AM	0	16	5	0	1	11	0	0	0	0	0	0	13	0	4	0	50
8:30 AM	0	15	6	0	4	10	0	0	0	0	0	0	14	0	5	0	54
8:45 AM	0	11	9	0	5	14	0	0	0	0	0	0	9	0	8	0	56
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
APPROACH %'s :	0	94	49	0	16	63	0	0	0	0	0	0	90	0	39	0	351
	0.00%	65.73%	34.27%	0.00%	20.25%	79.75%	0.00%	0.00%	0	0	0	0	69.77%	0.00%	30.23%	0.00%	
PEAK HR :	08:00 AM - 09:00 AM				12	40	0	0	0	0	0	0	43	0	21	0	TOTAL
PEAK HR VOL :	0	51	25	0	12	40	0	0	0	0	0	0	43	0	21	0	192
PEAK HR FACTOR :	0.000	0.797	0.694	0.000	0.600	0.714	0.000	0.000	0.000	0.000	0.000	0.000	0.768	0.000	0.656	0.000	0.857
	0.905				0.684								0.842				

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
4:00 PM	0	18	19	1	0	14	0	0	0	0	0	0	6	0	3	0	61
4:15 PM	0	21	17	0	0	7	0	0	0	0	0	0	6	0	4	0	55
4:30 PM	0	17	16	0	1	24	0	0	0	0	0	0	9	0	6	0	73
4:45 PM	0	11	12	0	3	19	0	0	0	0	0	0	8	0	8	0	61
5:00 PM	0	21	14	0	2	11	0	0	0	0	0	0	7	0	6	0	61
5:15 PM	0	16	11	0	2	14	0	0	0	0	0	0	9	0	5	0	57
5:30 PM	0	14	12	0	3	13	0	0	0	0	0	0	17	0	6	0	65
5:45 PM	0	14	8	0	0	11	0	0	0	0	0	0	7	0	4	0	44
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
APPROACH %'s :	0	132	109	1	11	113	0	0	0	0	0	0	69	0	42	0	477
	0.00%	54.55%	45.04%	0.41%	8.87%	91.13%	0.00%	0.00%	0	0	0	0	62.16%	0.00%	37.84%	0.00%	
PEAK HR :	04:30 PM - 05:30 PM				8	68	0	0	0	0	0	0	33	0	25	0	TOTAL
PEAK HR VOL :	0	65	53	0	8	68	0	0	0	0	0	0	33	0	25	0	252
PEAK HR FACTOR :	0.000	0.774	0.828	0.000	0.667	0.708	0.000	0.000	0.000	0.000	0.000	0.000	0.917	0.000	0.781	0.000	0.863
	0.843				0.760								0.906				

National Data & Surveying Services Intersection Turning Movement Count

Location: S Hamilton St & Dwight St
City: Poughkeepsie
Control: 1-Way Stop(WB)

Project ID: 23-380020-001
Date: 7/6/2023

Data - Cars

NS/EW Streets:	S Hamilton St				S Hamilton St				Dwight St				Dwight St				
AM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
7:00 AM	0	4	4	0	1	5	0	0	0	0	0	0	11	0	2	0	27
7:15 AM	0	12	3	0	1	9	0	0	0	0	0	0	5	0	5	0	35
7:30 AM	0	17	4	0	2	0	0	0	0	0	0	0	13	0	6	0	42
7:45 AM	0	9	12	0	0	6	0	0	0	0	0	0	18	0	5	0	50
8:00 AM	0	9	4	0	2	5	0	0	0	0	0	0	6	0	4	0	30
8:15 AM	0	14	5	0	1	10	0	0	0	0	0	0	12	0	4	0	46
8:30 AM	0	14	6	0	4	10	0	0	0	0	0	0	13	0	5	0	52
8:45 AM	0	11	8	0	5	14	0	0	0	0	0	0	9	0	6	0	53
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
APPROACH %'s :	0	90	46	0	16	59	0	0	0	0	0	0	87	0	37	0	335
	0.00%	66.18%	33.82%	0.00%	21.33%	78.67%	0.00%	0.00%	0	0	0	0	70.16%	0.00%	29.84%	0.00%	
PEAK HR :	08:00 AM - 09:00 AM																TOTAL
PEAK HR VOL :	0	48	23	0	12	39	0	0	0	0	0	0	40	0	19	0	181
PEAK HR FACTOR :	0.000	0.857	0.719	0.000	0.600	0.696	0.000	0.000	0.000	0.000	0.000	0.000	0.769	0.000	0.792	0.000	0.854
	0.888				0.671								0.819				

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
4:00 PM	0	18	18	1	0	14	0	0	0	0	0	0	6	0	3	0	60
4:15 PM	0	19	17	0	0	7	0	0	0	0	0	0	6	0	4	0	53
4:30 PM	0	17	16	0	1	24	0	0	0	0	0	0	9	0	6	0	73
4:45 PM	0	11	12	0	3	19	0	0	0	0	0	0	8	0	8	0	61
5:00 PM	0	21	14	0	2	11	0	0	0	0	0	0	7	0	6	0	61
5:15 PM	0	16	11	0	2	14	0	0	0	0	0	0	9	0	5	0	57
5:30 PM	0	14	12	0	3	13	0	0	0	0	0	0	17	0	6	0	65
5:45 PM	0	14	8	0	0	11	0	0	0	0	0	0	7	0	4	0	44
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
APPROACH %'s :	0	130	108	1	11	113	0	0	0	0	0	0	69	0	42	0	474
	0.00%	54.39%	45.19%	0.42%	8.87%	91.13%	0.00%	0.00%	0	0	0	0	62.16%	0.00%	37.84%	0.00%	
PEAK HR :	04:30 PM - 05:30 PM																TOTAL
PEAK HR VOL :	0	65	53	0	8	68	0	0	0	0	0	0	33	0	25	0	252
PEAK HR FACTOR :	0.000	0.774	0.828	0.000	0.667	0.708	0.000	0.000	0.000	0.000	0.000	0.000	0.917	0.000	0.781	0.000	0.863
	0.843				0.760								0.906				

National Data & Surveying Services Intersection Turning Movement Count

Location: S Hamilton St & Dwight St
City: Poughkeepsie
Control: 1-Way Stop(WB)

Project ID: 23-380020-001
Date: 7/6/2023

Data - HT

NS/EW Streets:	S Hamilton St				S Hamilton St				Dwight St				Dwight St				
AM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
7:30 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1
7:45 AM	0	1	0	0	0	2	0	0	0	0	0	0	0	0	0	0	3
8:00 AM	0	0	1	0	0	0	0	0	0	0	0	0	1	0	0	0	2
8:15 AM	0	2	0	0	0	1	0	0	0	0	0	0	1	0	0	0	4
8:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	1	0	0	0	2
8:45 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	2	0	3
TOTAL VOLUMES :	NL 0	NT 4	NR 3	NU 0	SL 0	ST 4	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 3	WT 0	WR 2	WU 0	TOTAL 16
APPROACH %'s :	0.00%	57.14%	42.86%	0.00%	0.00%	100.00%	0.00%	0.00%					60.00%	0.00%	40.00%	0.00%	
PEAK HR :	08:00 AM - 09:00 AM																TOTAL
PEAK HR VOL :	0	3	2	0	0	1	0	0	0	0	0	0	3	0	2	0	11
PEAK HR FACTOR :	0.000	0.375	0.500	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.000	0.750	0.000	0.250	0.000	0.688
	0.625				0.250								0.625				
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
4:00 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1
4:15 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES :	NL 0	NT 2	NR 1	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 3
APPROACH %'s :	0.00%	66.67%	33.33%	0.00%													
PEAK HR :	04:30 PM - 05:30 PM																TOTAL
PEAK HR VOL :	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	

National Data & Surveying Services Intersection Turning Movement Count

Location: S Hamilton St & Dwight St
City: Poughkeepsie
Control: 1-Way Stop(WB)

Project ID: 23-380020-001
Date: 7/6/2023

Data - Bikes

NS/EW Streets:	S Hamilton St				S Hamilton St				Dwight St				Dwight St				
AM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	2
TOTAL VOLUMES :	NL 0	NT 0	NR 0	NU 0	SL 1	ST 1	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 1	WU 0	TOTAL 4
APPROACH %'s :	0.00%	100.00%	0.00%	0.00%	50.00%	50.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	100.00%	0.00%	
PEAK HR :	08:00 AM - 09:00 AM																TOTAL 2
PEAK HR VOL :	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	
PEAK HR FACTOR :	0.000	0.250	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250
	0.250				0.250												
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
4:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES :	NL 0	NT 1	NR 0	NU 0	SL 1	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 1	WU 0	TOTAL 3
APPROACH %'s :	0.00%	100.00%	0.00%	0.00%	100.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	100.00%	0.00%	
PEAK HR :	04:30 PM - 05:30 PM																TOTAL 2
PEAK HR VOL :	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	
PEAK HR FACTOR :	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.500
	0.250												0.250				

National Data & Surveying Services Intersection Turning Movement Count

Location: S Hamilton St & Dwight St
City: Poughkeepsie

Project ID: 23-380020-001
Date: 7/6/2023

Data - Pedestrians (Crosswalks)

NS/EW Streets:	S Hamilton St		S Hamilton St		Dwight St		Dwight St		
AM	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	1	0	0	1
7:15 AM	1	0	0	0	0	1	0	0	2
7:30 AM	0	0	0	0	0	2	0	0	2
7:45 AM	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	1	1	0	1	0	0	3
8:30 AM	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	1	0	0	1
TOTAL VOLUMES :	EB 1	WB 0	EB 1	WB 1	NB 0	SB 6	NB 0	SB 0	TOTAL 9
APPROACH %'s :	100.00%	0.00%	50.00%	50.00%	0.00%	100.00%			
PEAK HR :	08:00 AM - 09:00 AM								TOTAL
PEAK HR VOL :	0	0	1	1	0	2	0	0	4
PEAK HR FACTOR :			0.250	0.250		0.500			0.333

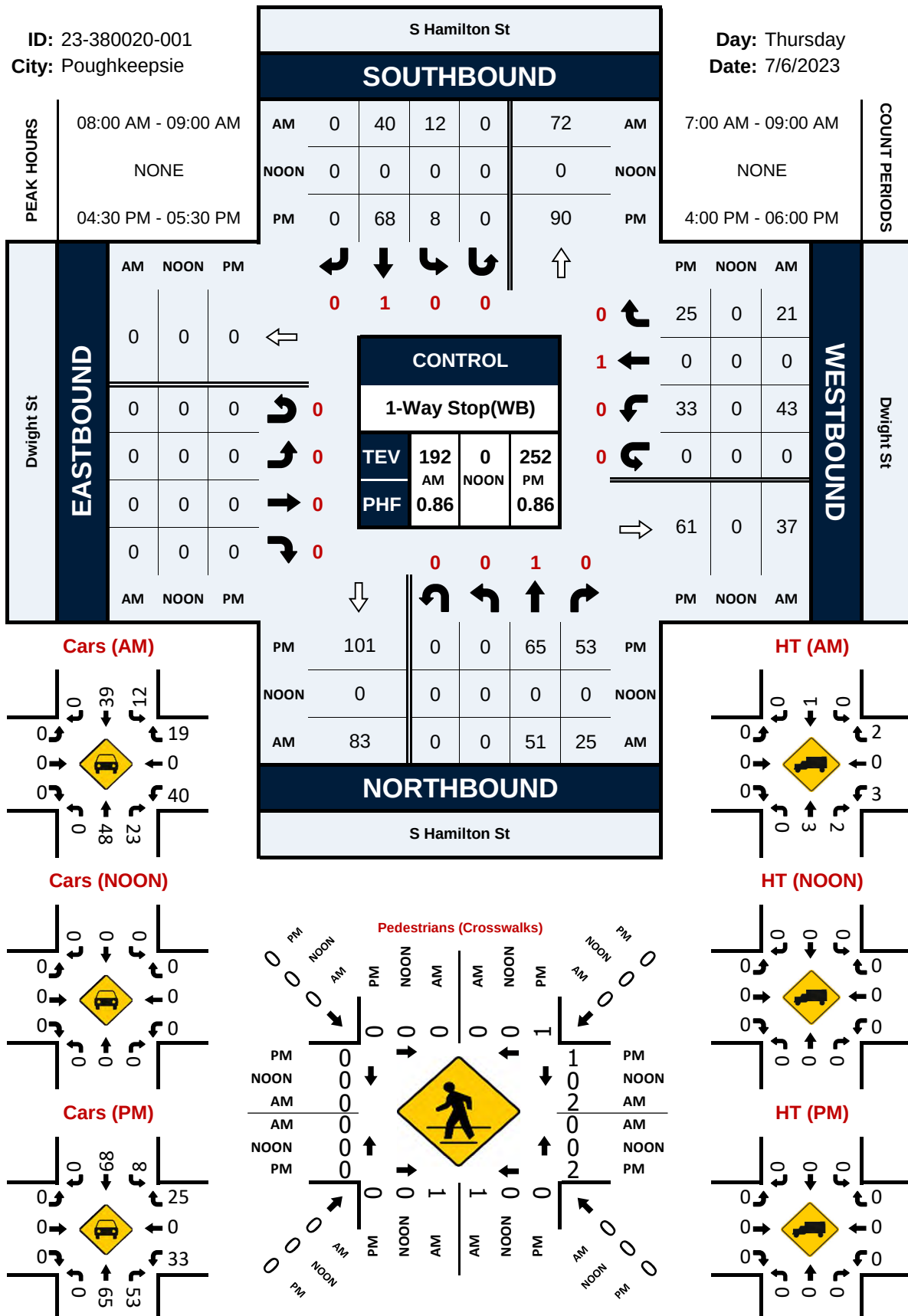
PM	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	
4:00 PM	0	0	0	0	0	0	0	0	0
4:15 PM	0	1	0	0	1	0	0	0	2
4:30 PM	0	0	0	0	1	0	0	0	1
4:45 PM	0	0	0	0	0	1	0	0	1
5:00 PM	0	1	0	0	1	0	0	0	2
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	1	0	1	0	0	0	2
5:45 PM	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES :	EB 0	WB 2	EB 1	WB 0	NB 4	SB 1	NB 0	SB 0	TOTAL 8
APPROACH %'s :	0.00%	100.00%	100.00%	0.00%	80.00%	20.00%			
PEAK HR :	04:30 PM - 05:30 PM								TOTAL
PEAK HR VOL :	0	1	0	0	2	1	0	0	4
PEAK HR FACTOR :		0.250			0.500	0.250			0.500

S Hamilton St & Dwight St

Peak Hour Turning Movement Count

ID: 23-380020-001
City: Poughkeepsie

Day: Thursday
Date: 7/6/2023



National Data & Surveying Services Intersection Turning Movement Count

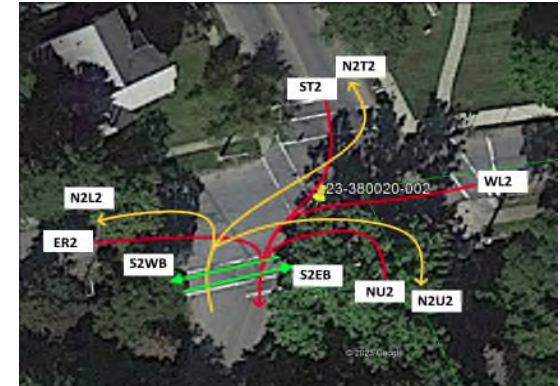
Location: Hooker Ave/Crescent Rd & Dwight St
City: Poughkeepsie
Control: Signalized

Project ID: 23-380020-002
Date: 7/6/2023

Data - Total

NS/EW Streets:		Hooker Ave/Crescent Rd					Hooker Ave/Crescent Rd					Dwight St					Dwight St									
AM		NORTHBOUND					SOUTHBOUND					EASTBOUND					WESTBOUND					NORTHBOUND2				
		0	1	0	0	0	0	1	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	0	
		NL	NT	NR	NU	NU2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	WL	WT	WR	WU	WL2	N2L2	N2T2	N2U2	TOTAL	
7:00 AM	7	28	0	0	0	0	22	5	0	1	3	0	2	0	0	0	2	0	0	0	0	1	1	72		
7:15 AM	4	24	0	0	0	0	14	0	0	0	1	0	4	0	0	0	5	0	0	0	0	0	0	52		
7:30 AM	8	28	0	0	0	0	24	3	0	0	5	0	3	0	0	0	6	1	0	0	0	1	0	79		
7:45 AM	12	34	0	0	1	1	16	8	0	1	4	0	5	0	0	2	5	0	0	0	0	0	4	92		
8:00 AM	7	47	0	0	0	0	31	2	0	0	4	0	6	0	0	0	1	0	0	0	0	1	0	99		
8:15 AM	11	51	0	0	0	0	28	4	0	0	3	0	2	0	0	0	1	0	0	0	0	1	0	101		
8:30 AM	11	57	0	0	0	0	25	4	0	0	3	0	8	0	0	0	4	4	4	0	0	0	0	120		
8:45 AM	10	53	0	0	0	0	35	5	0	2	4	0	9	0	0	0	0	2	0	0	0	0	1	121		
TOTAL VOLUMES:		NL	NT	NR	NU	NU2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	WL	WT	WR	WU	WL2	N2L2	N2T2	N2U2	TOTAL	
APPROACH %:		70	322	0	0	1	0	195	31	0	4	27	0	39	0	0	6	26	5	0	0	0	4	6	736	
PEAK HR		17.81	81.93%	0.00%	0.00%	0.25%	0.00%	84.78%	13.48%	0.00%	1.74%	40.91%	0.00%	59.09%	0.00%	0.00%	16.22%	70.27%	13.51%	0.00%	0.00%	0.00%	40.00%	60.00%		
PEAK HR VOL:		39	208	0	0	0	0	119	15	0	2	14	0	25	0	0	4	8	4	0	0	0	2	1	441	
PEAK HR FACTOR:		0.886	0.912	0.000	0.000	0.000	0.000	0.850	0.750	0.000	0.250	0.875	0.000	0.694	0.000	0.000	0.250	0.500	0.250	0.000	0.000	0.500	0.250	0.911		
		08:00 AM - 09:00 AM																								
PM		NORTHBOUND					SOUTHBOUND					EASTBOUND					WESTBOUND									
		0	1	0	0	0	0	1	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0		
		NL	NT	NR	NU	NU2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	WL	WT	WR	WU	WL2	N2L2	N2T2	N2U2	TOTAL	
4:00 PM	6	44	0	0	1	0	46	3	0	1	7	0	12	0	0	0	0	3	0	0	1	0	1	125		
4:15 PM	4	45	0	0	1	0	37	3	0	1	7	0	10	0	0	2	2	5	0	1	1	2	0	121		
4:30 PM	9	40	0	0	1	0	64	4	0	0	6	0	10	0	0	7	2	3	0	1	0	0	1	148		
4:45 PM	6	55	0	0	1	0	53	5	0	4	5	0	11	0	0	1	5	1	0	0	0	2	2	151		
5:00 PM	6	48	0	0	1	0	78	2	0	2	8	0	7	0	1	2	3	2	0	1	1	0	0	162		
5:15 PM	9	45	0	0	0	0	64	7	0	1	6	0	6	0	0	3	1	0	0	0	1	0	1	144		
5:30 PM	12	46	0	0	0	0	45	6	0	2	4	0	12	0	0	0	3	0	0	0	0	1	0	131		
5:45 PM	10	33	0	0	1	0	36	1	0	1	2	0	6	0	0	0	1	0	0	1	0	0	1	93		
TOTAL VOLUMES:		NL	NT	NR	NU	NU2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	WL	WT	WR	WU	WL2	N2L2	N2T2	N2U2	TOTAL	
APPROACH %:		62	356	0	0	6	1	423	31	0	12	45	0	74	0	1	15	17	14	0	4	3	6	5	1075	
PEAK HR		14.62%	83.96%	0.00%	0.00%	1.42%	0.21%	90.58%	6.64%	0.00%	2.57%	37.50%	0.00%	61.67%	0.00%	0.83%	30.00%	34.00%	28.00%	0.00%	8.00%	21.43%	42.86%	35.71%		
PEAK HR VOL:		30	188	0	0	3	1	259	18	0	7	25	0	34	0	1	13	11	6	0	2	1	3	3	605	
PEAK HR FACTOR:		0.833	0.855	0.000	0.000	0.750	0.250	0.830	0.643	0.000	0.438	0.781	0.000	0.773	0.000	0.250	0.464	0.550	0.500	0.000	0.500	0.250	0.375	0.375	0.934	
		04:30 PM - 05:30 PM																								

Explanation for extra leg movements	
Movements entering the extra leg	
ER2	Movements coming from EB on Dwight St entering into the Extra Leg (Crescent Rd)
ST2	Movements coming from SB on Hooker Ave entering into the Extra Leg (Crescent Rd)
WL2	Movements coming from WB on Hanscom Ave entering into the Extra Leg (Crescent Rd)
NU2	Movements coming from NB on Hooker Ave entering into the Extra Leg (Crescent Rd)
Movements exiting the extra leg	
N2L2	Movements exiting from Extra Leg (Crescent Rd) entering into Dwight St going WB
N2T2	Movements exiting from Extra Leg (Crescent Rd) entering into Hooker Ave going NB
N2U2	Movements exiting from Extra Leg (Crescent Rd) entering into Hooker Ave going SB



National Data & Surveying ServicesIntersection Turning Movement Count

Location: Hooker Ave/Crescent Rd & Dwight St
City: Poughkeepsie
Control: Signalized

Project ID: 23-380020-002
Date: 7/6/2023

Data - Cars

NS/EW Streets:		Hooker Ave/Crescent Rd					Hooker Ave/Crescent Rd					Dwight St					Dwight St					Dwight St				
AM	NORTHBOUND					SOUTHBOUND					EASTBOUND					WESTBOUND					NORTHBOUND					
	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0				
	NL	NT	NR	NU	N2U2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	WL	WT	WR	WU	WL2	N2L2	N2T2	N2U2			
7:00 AM	7	28	0	0	0	0	19	5	0	1	3	0	2	0	0	0	2	0	0	0	0	1	1			
7:15 AM	4	22	0	0	0	0	13	0	0	0	1	0	4	0	0	0	5	0	0	0	0	0	0			
7:30 AM	8	28	0	0	0	0	23	3	0	0	4	0	3	0	0	0	6	1	0	0	0	1	0			
7:45 AM	1	34	0	1	1	0	15	34	0	0	4	0	15	8	0	2	5	0	5	0	1	4	2			
8:00 AM	6	46	0	0	0	0	27	2	0	0	4	0	5	0	0	0	1	0	0	0	0	1	0			
8:15 AM	10	51	0	0	0	0	28	4	0	0	3	0	2	0	0	0	1	0	0	0	0	1	0			
8:30 AM	11	57	0	0	0	0	22	4	0	0	3	0	8	0	0	4	3	4	0	0	0	0	0			
8:45 AM	9	52	0	0	0	0	34	4	0	0	3	0	9	0	0	0	2	0	0	0	0	1	0			
TOTAL VOLUMES:	NL	NT	NR	NU	N2U2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	WL	WT	WR	WU	WL2	N2L2	N2T2	N2U2			
APPROACH %s:	17.36%	82.38%	0.00%	0.00%	0.26%	0.00%	84.19%	13.95%	0.00%	1.86%	39.68%	0.00%	60.32%	0.00%	0.00%	16.67%	69.44%	13.89%	0.00%	0.00%	0.00%	50.40%	50.00%			
PEAK HR:	08:00 AM - 09:00 AM																									
PEAK HR VOL:	36	206	0	0	0	0	111	14	0	2	13	0	24	0	0	4	7	4	0	0	0	2	1			
PEAK HR FACTOR:	0.818	0.904	0.000	0.000	0.000	0.000	0.816	0.875	0.000	0.250	0.813	0.000	0.667	0.000	0.000	0.250	0.583	0.250	0.000	0.000	0.000	0.500	0.250			
	0.890					0.794					0.771					0.341					0.914					

PM	NORTHBOUND					SOUTHBOUND					EASTBOUND					WESTBOUND					NORTHBOUND				
	0	1	0	0	0	0	1	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0			
	NL	NT	NR	NU	N2U2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	WL	WT	WR	WU	WL2	N2L2	N2T2	N2U2		
4:00 PM	6	43	0	0	1	0	43	3	0	1	6	0	12	0	0	0	12	0	0	0	1	1	0		
4:15 PM	4	45	0	0	1	0	35	3	0	1	7	0	10	0	0	0	2	5	0	0	1	2	0		
4:30 PM	9	38	0	0	1	0	64	4	0	0	6	0	10	0	0	7	2	3	0	1	0	0	1		
4:45 PM	6	53	0	0	1	0	51	5	0	4	5	0	11	0	0	1	5	1	0	0	2	2	1		
5:00 PM	6	48	0	0	0	0	78	2	0	2	8	0	7	0	1	2	3	2	0	1	1	0	0		
5:15 PM	9	44	0	0	1	0	64	7	0	0	6	0	6	0	0	1	6	1	0	0	0	1	0		
5:30 PM	12	45	0	0	0	0	43	6	0	2	4	0	12	0	0	0	3	0	0	0	0	1	0		
5:45 PM	10	31	0	0	1	0	36	1	0	1	2	0	6	0	0	0	1	0	0	1	0	1	0		
TOTAL VOLUMES:	NL	NT	NR	NU	N2U2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	WL	WT	WR	WU	WL2	N2L2	N2T2	N2U2		
APPROACH %s:	14.94%	83.61%	0.00%	0.00%	1.45%	0.22%	90.46%	6.72%	0.00%	2.60%	36.97%	0.00%	62.18%	0.00%	0.84%	30.00%	34.00%	28.00%	0.00%	8.00%	21.43%	42.86%	35.71%		
PEAK HR:	04:30 PM - 05:30 PM																								
PEAK HR VOL:	30	183	0	0	3	1	257	18	0	7	25	0	34	0	0	13	11	6	0	0	1	3	3		
PEAK HR FACTOR:	0.833	0.863	0.000	0.000	0.750	0.250	0.824	0.643	0.000	0.438	0.781	0.000	0.773	0.000	0.250	0.464	0.550	0.500	0.000	0.500	0.250	0.375	0.375		
	0.900					0.863					0.938					0.615					0.922				

National Data & Surveying ServicesIntersection Turning Movement Count

Location: Hooker Ave/Crescent Rd & Dwight St
City: Poughkeepsie
Control: Signalized

Project ID: 23-380020-002
Date: 7/6/2023

Data - HT

NS/EW Streets:	Hooker Ave/Crescent Rd						Hooker Ave/Crescent Rd						Dwight St						Dwight St						Dwight St														
AM	NORTHBOUND						SOUTHBOUND						EASTBOUND						WESTBOUND						NORTHBOUND2														
	0	1	NT	NR	0	NU2	0	SL	1	ST	SR	0	SU	0	ST2	0	EL	1	ET	ER	0	EU	0	ER2	0	WL	1	WT	WR	0	WU	0	WL2	0	N2L2	0	N2T2	0	N2U2
7:00 AM	0	0	0	0	0	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3
7:15 AM	0	2	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	
7:30 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
7:45 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	
8:00 AM	1	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7	
8:15 AM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
8:30 AM	0	0	0	0	0	0	0	0	3	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	4	
8:45 AM	1	1	0	0	0	0	0	0	1	1	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	
TOTAL VOLUMES:	NL	NT	NR	NU	NU2		SL	ST	SR	SU	ST2		EL	ET	ER	EU	ER2		WL	WT	WR	WU	WL2		N2L2	N2T2	N2U2		TOTAL										
APPROACH %'s:	3	4	0	0	0		0	14	1	0	0	0		66.67%	0.00%	33.33%	0.00%	0.00%		0.00%	100.00%	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	28	
PEAK HR:	42.86%	57.14%	0.00%	0.00%	0.00%		0.00%	93.33%	6.67%	0.00%	0.00%		66.67%	0.00%	33.33%	0.00%	0.00%		0.00%	100.00%	0.00%	0.00%	0.00%	0.00%		0.00%	0.00%	100.00%		TOTAL									
PEAK HR VOL:	3	2	0	0	0		0	8	1	0	0	0		1	0	1	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	17	
PEAK HR FACTOR:	0.750	0.500	0.000	0.000	0.000		0.000	0.500	0.250	0.000	0.000		0.250	0.000	0.250	0.000	0.000		0.000	0.250	0.000	0.000	0.000	0.000		0.000	0.000	0.000		TOTAL									
																																						0.607	

PM	NORTHBOUND						SOUTHBOUND						EASTBOUND						WESTBOUND						WESTBOUND														
	0	1	NT	NR	0	NU2	0	SL	1	ST	SR	0	SU	0	ST2	0	EL	1	ET	ER	0	EU	0	ER2	0	WL	1	WT	WR	0	WU	0	WL2	0	N2L2	0	N2T2	0	N2U2
4:00 PM	0	1	0	0	0	0	0	0	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
4:15 PM	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
4:30 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2		
4:45 PM	0	2	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4		
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
5:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1		
5:30 PM	0	1	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1		
5:45 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2		
TOTAL VOLUMES:	NL	NT	NR	NU	NU2		SL	ST	SR	SU	ST2		EL	ET	ER	EU	ER2		WL	WT	WR	WU	WL2		N2L2	N2T2	N2U2		TOTAL										
APPROACH %'s:	0	9	0	0	0		0	6	0	0	0		1	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	16		
PEAK HR:	0	5	0	0	0		0	2	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7		
PEAK HR VOL:	0	5	0	0	0		0	0.250	0.000	0.000	0.000	0.000		0.000	0.000	0.000	0.000	0.000		0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.438		
PEAK HR FACTOR:	0.000	100.00%	0.000	0.000	0.000		0.000	100.00%	0.00%	0.00%	0.00%	0.00%		100.00%	0.00%	0.00%	0.00%	0.00%		0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		

National Data & Surveying ServicesIntersection Turning Movement Count

Location: Hooker Ave/Crescent Rd & Dwight St
City: Poughkeepsie
Control: Signalized

Project ID: 23-380020-002
Date: 7/6/2023

Data - Bikes

NS/EW Streets:	Hooker Ave/Crescent Rd					Hooker Ave/Crescent Rd					Dwight St					Dwight St									
AM	NORTHBOUND					SOUTHBOUND					EASTBOUND					WESTBOUND					NORTHBOUND2				
	0	1	0	0	0	0	1	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	0	
	NL	NT	NR	NU	NU2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	WL	WT	WR	WU	WL2	N2L2	N2T2	N2U2	TOTAL	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	
8:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
8:30 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
8:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
TOTAL VOLUMES	NL	NT	NR	NU	NU2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	WL	WT	WR	WU	WL2	N2L2	N2T2	N2U2	TOTAL	
APPROACH %'s	0.00%	2	0	0	0	0.00%	100.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	100.00%	0.00%	0.00%	50.00%	1	50.00%	0.00%	0.00%	0.00%	0	0	6
PEAK HR	08:00 AM - 09:00 AM																								
PEAK HR VOL	0	2	0	0	0	0	1	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	4	
PEAK HR FACTOR	0.000	0.500	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.000	0.000	1.000	

PM	NORTHBOUND					SOUTHBOUND					EASTBOUND					WESTBOUND									
	0	1	0	0	0	0	1	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	0	
	NL	NT	NR	NU	NU2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	WL	WT	WR	WU	WL2	N2L2	N2T2	N2U2	TOTAL	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	
4:15 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:45 PM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
5:45 PM	0	0	1	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	
TOTAL VOLUMES	NL	NT	NR	NU	NU2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	WL	WT	WR	WU	WL2	N2L2	N2T2	N2U2	TOTAL	
APPROACH %'s	33.33%	33.33%	33.33%	0.00%	0.00%	0.00%	100.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	100.00%	0.00%	0.00%	0	0	0	0	0	0	0	7	
PEAK HR	04:30 PM - 05:30 PM																								
PEAK HR VOL	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
PEAK HR FACTOR	0.250	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	

National Data & Surveying Services Intersection Turning Movement Count

Location: Hooker Ave/Crescent Rd & Dwight St
City: Poughkeepsie

Project ID: 23-380020-002
Date: 7/6/2023

Data - Pedestrians (Crosswalks)

NS/EW Streets:	Hooker Ave/Crescent Rd		Hooker Ave/Crescent Rd		Dwight St		Dwight St				
AM	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		SOUTH LEG 2		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	EB	WB	
7:00 AM	0	0	0	0	1	0	1	0	0	0	2
7:15 AM	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	1	0	0	0	0	0	1
7:45 AM	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	3	0	0	0	0	0	0	0	4	0	7
8:15 AM	0	0	2	1	0	0	1	1	0	1	6
8:30 AM	0	0	1	0	0	0	1	1	0	0	3
8:45 AM	0	0	0	1	0	0	1	0	0	1	3
TOTAL VOLUMES : APPROACH %'s :	EB 3 100.00%	WB 0 0.00%	EB 3 60.00%	WB 2 40.00%	NB 2 100.00%	SB 0 0.00%	NB 4 66.67%	SB 2 33.33%	EB 4 66.67%	WB 2 33.33%	TOTAL 22
PEAK HR :	08:00 AM - 09:00 AM										TOTAL
PEAK HR VOL :	3	0	3	2	0	0	3	2	4	2	19
PEAK HR FACTOR :	0.250		0.375	0.500			0.750	0.500	0.250	0.500	0.679
	0.250		0.417				0.625		0.375		

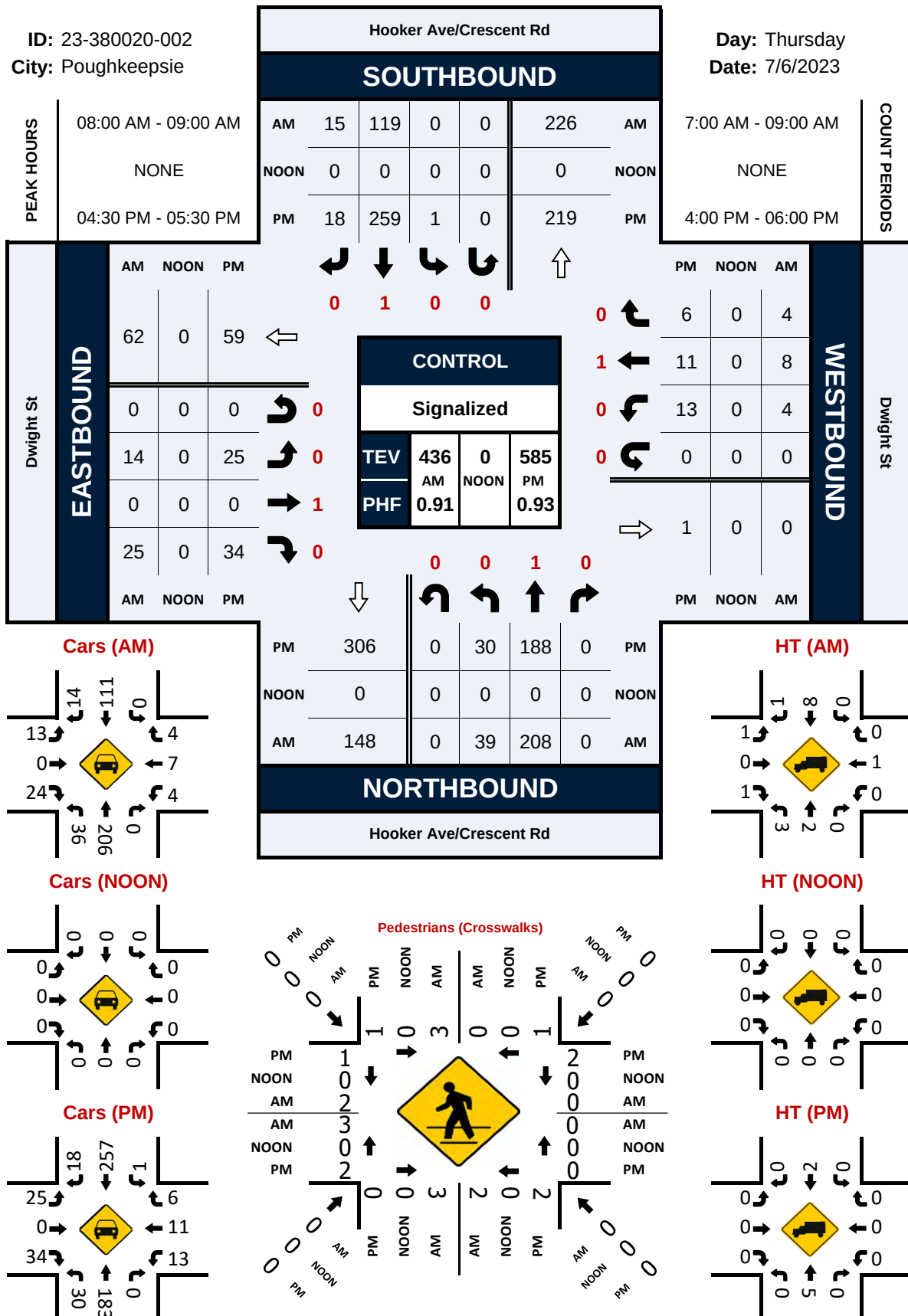
PM	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		SOUTH LEG 2		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	EB	WB	
4:00 PM	0	0	0	0	0	0	1	1	0	1	3
4:15 PM	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	1	0	0	0	0	0	0	1	0	0	2
4:45 PM	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	1	0	2	0	2	0	0	0	0	5
5:15 PM	0	0	0	0	0	0	2	0	0	0	2
5:30 PM	0	0	0	1	0	2	1	0	0	1	5
5:45 PM	1	0	0	0	0	0	1	0	0	0	2
TOTAL VOLUMES : APPROACH %'s :	EB 2 66.67%	WB 1 33.33%	EB 0 0.00%	WB 3 100.00%	NB 0 0.00%	SB 4 100.00%	NB 5 71.43%	SB 2 28.57%	EB 0 0.00%	WB 2 100.00%	TOTAL 19
PEAK HR :	04:30 PM - 05:30 PM										TOTAL
PEAK HR VOL :	1	1	0	2	0	2	2	1	0	0	9
PEAK HR FACTOR :	0.250	0.250		0.250		0.250	0.250	0.250			0.450
	0.500		0.250		0.250		0.375				

Hooker Ave/Crescent Rd & Dwight St

Peak Hour Turning Movement Count

ID: 23-380020-002
City: Poughkeepsie

Day: Thursday
Date: 7/6/2023



National Data & Surveying Services Intersection Turning Movement Count

Location: S Hamilton St & Hooker Ave/Montgomery St
City: Poughkeepsie
Control: Signalized

Project ID: 23-380020-003
Date: 7/6/2023

Data - Total

NS/EW Streets:	S Hamilton St				S Hamilton St				Hooker Ave/Montgomery St				Hooker Ave/Montgomery St				
AM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	2 WT	0 WR	0 WU	
7:00 AM	1	4	1	0	4	4	1	0	9	11	2	0	1	28	11	0	77
7:15 AM	1	10	2	0	7	8	6	0	4	12	1	0	1	36	7	0	95
7:30 AM	1	20	3	0	3	2	1	0	5	15	1	0	1	35	10	0	97
7:45 AM	3	9	0	0	7	11	2	0	4	20	0	0	0	36	17	0	109
8:00 AM	2	12	2	0	7	5	5	0	1	17	1	0	1	47	24	0	124
8:15 AM	2	12	1	0	9	13	7	0	6	23	1	0	2	54	25	0	155
8:30 AM	3	18	0	0	6	8	8	0	5	14	2	0	3	59	21	0	147
8:45 AM	1	17	3	0	11	18	11	0	6	24	1	0	4	69	32	0	197
TOTAL VOLUMES :	NL 14	NT 102	NR 12	NU 0	SL 54	ST 69	SR 41	SU 0	EL 40	ET 136	ER 9	EU 0	WL 13	WT 364	WR 147	WU 0	TOTAL 1001
APPROACH %'s :	10.94%	79.69%	9.38%	0.00%	32.93%	42.07%	25.00%	0.00%	21.62%	73.51%	4.86%	0.00%	2.48%	69.47%	28.05%	0.00%	
PEAK HR :	08:00 AM - 09:00 AM																TOTAL
PEAK HR VOL :	8	59	6	0	33	44	31	0	18	78	5	0	10	229	102	0	623
PEAK HR FACTOR :	0.667	0.819	0.500	0.000	0.750	0.611	0.705	0.000	0.750	0.813	0.625	0.000	0.625	0.830	0.797	0.000	0.791
	0.869				0.675				0.815				0.812				
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	2 WT	0 WR	0 WU	
4:00 PM	2	16	4	0	9	16	7	0	7	21	3	0	3	46	18	0	152
4:15 PM	1	20	4	0	7	9	10	0	5	15	0	0	6	41	15	0	133
4:30 PM	0	18	0	0	15	30	8	0	9	35	1	0	5	42	20	0	183
4:45 PM	1	9	3	0	10	15	6	0	9	34	1	0	1	55	18	0	162
5:00 PM	1	21	0	0	19	17	9	0	9	41	1	0	1	54	18	0	191
5:15 PM	1	14	1	0	19	21	6	0	7	41	3	0	1	40	13	0	167
5:30 PM	1	9	2	0	9	14	10	0	4	23	2	0	4	43	21	0	142
5:45 PM	3	15	1	0	12	20	3	0	5	19	1	0	3	24	15	0	121
TOTAL VOLUMES :	NL 10	NT 122	NR 15	NU 0	SL 100	ST 142	SR 59	SU 0	EL 55	ET 229	ER 12	EU 0	WL 24	WT 345	WR 138	WU 0	TOTAL 1251
APPROACH %'s :	6.80%	82.99%	10.20%	0.00%	33.22%	47.18%	19.60%	0.00%	18.58%	77.36%	4.05%	0.00%	4.73%	68.05%	27.22%	0.00%	
PEAK HR :	04:30 PM - 05:30 PM																TOTAL
PEAK HR VOL :	3	62	4	0	63	83	29	0	34	151	6	0	8	191	69	0	703
PEAK HR FACTOR :	0.750	0.738	0.333	0.000	0.829	0.692	0.806	0.000	0.944	0.921	0.500	0.000	0.400	0.868	0.863	0.000	0.920
	0.784				0.825				0.936				0.905				

National Data & Surveying Services Intersection Turning Movement Count

Location: S Hamilton St & Hooker Ave/Montgomery St
City: Poughkeepsie
Control: Signalized

Project ID: 23-380020-003
Date: 7/6/2023

Data - Cars

NS/EW Streets:	S Hamilton St				S Hamilton St				Hooker Ave/Montgomery St				Hooker Ave/Montgomery St				
AM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	2 WT	0 WR	0 WU	
7:00 AM	1	4	1	0	4	4	1	0	9	9	2	0	1	28	11	0	75
7:15 AM	1	10	2	0	7	8	6	0	3	12	1	0	1	35	5	0	91
7:30 AM	1	20	3	0	3	2	1	0	5	14	1	0	0	32	10	0	92
7:45 AM	3	9	0	0	6	9	2	0	4	20	0	0	0	36	17	0	106
8:00 AM	2	10	2	0	6	4	5	0	1	15	1	0	1	46	24	0	117
8:15 AM	2	12	1	0	9	12	6	0	6	23	1	0	2	53	25	0	152
8:30 AM	2	16	0	0	6	7	8	0	4	12	2	0	3	58	21	0	139
8:45 AM	1	16	3	0	11	18	10	0	6	22	1	0	4	68	32	0	192
TOTAL VOLUMES :	NL 13	NT 97	NR 12	NU 0	SL 52	ST 64	SR 39	SU 0	EL 38	ET 127	ER 9	EU 0	WL 12	WT 356	WR 145	WU 0	TOTAL 964
APPROACH %'s :	10.66%	79.51%	9.84%	0.00%	33.55%	41.29%	25.16%	0.00%	21.84%	72.99%	5.17%	0.00%	2.34%	69.40%	28.27%	0.00%	
PEAK HR :	08:00 AM - 09:00 AM																TOTAL
PEAK HR VOL :	7	54	6	0	32	41	29	0	17	72	5	0	10	225	102	0	600
PEAK HR FACTOR :	0.875	0.844	0.500	0.000	0.727	0.569	0.725	0.000	0.708	0.783	0.625	0.000	0.625	0.827	0.797	0.000	0.781
	0.838				0.654				0.783				0.810				
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	2 WT	0 WR	0 WU	
4:00 PM	2	16	4	0	9	16	7	0	7	21	3	0	3	45	18	0	151
4:15 PM	1	20	3	0	7	9	10	0	5	15	0	0	6	41	15	0	132
4:30 PM	0	18	0	0	15	29	8	0	9	34	1	0	5	41	20	0	180
4:45 PM	1	9	3	0	10	15	6	0	9	33	1	0	1	54	18	0	160
5:00 PM	1	21	0	0	19	17	9	0	9	41	1	0	1	54	18	0	191
5:15 PM	1	14	1	0	18	21	6	0	7	41	3	0	1	39	13	0	165
5:30 PM	1	9	2	0	9	14	10	0	4	22	2	0	4	43	20	0	140
5:45 PM	3	15	1	0	12	20	3	0	5	19	1	0	3	24	14	0	120
TOTAL VOLUMES :	NL 10	NT 122	NR 14	NU 0	SL 99	ST 141	SR 59	SU 0	EL 55	ET 226	ER 12	EU 0	WL 24	WT 341	WR 136	WU 0	TOTAL 1239
APPROACH %'s :	6.85%	83.56%	9.59%	0.00%	33.11%	47.16%	19.73%	0.00%	18.77%	77.13%	4.10%	0.00%	4.79%	68.06%	27.15%	0.00%	
PEAK HR :	04:30 PM - 05:30 PM																TOTAL
PEAK HR VOL :	3	62	4	0	62	82	29	0	34	149	6	0	8	188	69	0	696
PEAK HR FACTOR :	0.750	0.738	0.333	0.000	0.816	0.707	0.806	0.000	0.944	0.909	0.500	0.000	0.400	0.870	0.863	0.000	0.911
	0.784				0.832				0.926				0.908				

National Data & Surveying Services Intersection Turning Movement Count

Location: S Hamilton St & Hooker Ave/Montgomery St
City: Poughkeepsie
Control: Signalized

Project ID: 23-380020-003
Date: 7/6/2023

Data - HT

NS/EW Streets:	S Hamilton St				S Hamilton St				Hooker Ave/Montgomery St				Hooker Ave/Montgomery St				
AM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	2 WT	0 WR	0 WU	
7:00 AM	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2
7:15 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	1	2	0	4
7:30 AM	0	0	0	0	0	0	0	0	0	1	0	0	1	3	0	0	5
7:45 AM	0	0	0	0	1	2	0	0	0	0	0	0	0	0	0	0	3
8:00 AM	0	2	0	0	1	1	0	0	0	2	0	0	0	1	0	0	7
8:15 AM	0	0	0	0	0	1	1	0	0	0	0	0	0	1	0	0	3
8:30 AM	1	2	0	0	0	1	0	0	1	2	0	0	0	1	0	0	8
8:45 AM	0	1	0	0	0	0	1	0	0	2	0	0	0	1	0	0	5
TOTAL VOLUMES :	NL 1	NT 5	NR 0	NU 0	SL 2	ST 5	SR 2	SU 0	EL 2	ET 9	ER 0	EU 0	WL 1	WT 8	WR 2	WU 0	TOTAL 37
APPROACH %'s :	16.67%	83.33%	0.00%	0.00%	22.22%	55.56%	22.22%	0.00%	18.18%	81.82%	0.00%	0.00%	9.09%	72.73%	18.18%	0.00%	
PEAK HR :	08:00 AM - 09:00 AM																TOTAL
PEAK HR VOL :	1	5	0	0	1	3	2	0	1	6	0	0	0	4	0	0	23
PEAK HR FACTOR :	0.250	0.625	0.000	0.000	0.250	0.750	0.500	0.000	0.250	0.750	0.000	0.000	0.000	1.000	0.000	0.000	0.719
	0.500				0.750				0.583				1.000				
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	2 WT	0 WR	0 WU	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
4:15 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
4:30 PM	0	0	0	0	0	1	0	0	0	1	0	0	0	1	0	0	3
4:45 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	0	2
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	1	0	0	2
5:30 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	2
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
TOTAL VOLUMES :	NL 0	NT 0	NR 1	NU 0	SL 1	ST 1	SR 0	SU 0	EL 0	ET 3	ER 0	EU 0	WL 0	WT 4	WR 2	WU 0	TOTAL 12
APPROACH %'s :	0.00%	0.00%	100.00%	0.00%	50.00%	50.00%	0.00%	0.00%	0.00%	100.00%	0.00%	0.00%	0.00%	66.67%	33.33%	0.00%	
PEAK HR :	04:30 PM - 05:30 PM																TOTAL
PEAK HR VOL :	0	0	0	0	1	1	0	0	0	2	0	0	0	3	0	0	7
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.250	0.250	0.000	0.000	0.000	0.500	0.000	0.000	0.000	0.750	0.000	0.000	0.583
					0.500				0.500				0.750				

National Data & Surveying Services Intersection Turning Movement Count

Location: S Hamilton St & Hooker Ave/Montgomery St
City: Poughkeepsie
Control: Signalized

Project ID: 23-380020-003
Date: 7/6/2023

Data - Bikes

NS/EW Streets:	S Hamilton St				S Hamilton St				Hooker Ave/Montgomery St				Hooker Ave/Montgomery St				
AM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	2 WT	0 WR	0 WU	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	1	0	0	2
7:30 AM	0	0	0	0	0	0	1	0	0	1	0	0	0	1	0	0	3
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	2
TOTAL VOLUMES :	NL 0	NT 2	NR 0	NU 0	SL 0	ST 1	SR 1	SU 0	EL 1	ET 1	ER 0	EU 0	WL 0	WT 2	WR 1	WU 0	TOTAL 9
APPROACH %'s :	0.00%	100.00%	0.00%	0.00%	0.00%	50.00%	50.00%	0.00%	50.00%	50.00%	0.00%	0.00%	0.00%	66.67%	33.33%	0.00%	
PEAK HR :	08:00 AM - 09:00 AM																TOTAL
PEAK HR VOL :	0	1	0	0	0	1	0	0	1	0	0	0	0	0	1	0	4
PEAK HR FACTOR :	0.000	0.250	0.000	0.000	0.000	0.250	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.500
	0.250				0.250				0.250				0.250				
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	2 WT	0 WR	0 WU	
4:00 PM	0	0	1	0	0	0	0	0	0	0	0	0	1	1	1	0	4
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	2
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2
5:45 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	2	0	0	3
TOTAL VOLUMES :	NL 0	NT 0	NR 2	NU 0	SL 1	ST 0	SR 3	SU 0	EL 0	ET 1	ER 0	EU 0	WL 1	WT 3	WR 1	WU 0	TOTAL 12
APPROACH %'s :	0.00%	0.00%	100.00%	0.00%	25.00%	0.00%	75.00%	0.00%	0.00%	100.00%	0.00%	0.00%	20.00%	60.00%	20.00%	0.00%	
PEAK HR :	04:30 PM - 05:30 PM																TOTAL
PEAK HR VOL :	0	0	1	0	0	0	1	0	0	1	0	0	0	0	0	0	3
PEAK HR FACTOR :	0.000	0.000	0.250	0.000	0.000	0.000	0.250	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.000	0.375
	0.250				0.250				0.250				0.250				

National Data & Surveying Services

Intersection Turning Movement Count

Location: S Hamilton St & Hooker Ave/Montgomery St
City: Poughkeepsie

Project ID: 23-380020-003
Date: 7/6/2023

Data - Pedestrians (Crosswalks)

NS/EW Streets:	S Hamilton St		S Hamilton St		Hooker Ave/Montgomery St		Hooker Ave/Montgomery St		
AM	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	1	0	0	0	0	0	1
7:30 AM	0	1	0	1	0	0	0	0	2
7:45 AM	0	0	0	0	1	0	0	0	1
8:00 AM	1	0	0	0	1	0	0	0	2
8:15 AM	6	0	0	0	0	2	3	0	11
8:30 AM	5	0	5	1	1	0	2	1	15
8:45 AM	4	4	2	1	0	0	1	0	12
TOTAL VOLUMES :	EB 16	WB 5	EB 8	WB 3	NB 3	SB 2	NB 6	SB 1	TOTAL 44
APPROACH %'s :	76.19%	23.81%	72.73%	27.27%	60.00%	40.00%	85.71%	14.29%	
PEAK HR :	08:00 AM - 09:00 AM								TOTAL 40
PEAK HR VOL :	16	4	7	2	2	2	6	1	0.667
PEAK HR FACTOR :	0.667	0.250	0.350	0.500	0.500	0.250	0.500	0.250	
	0.625		0.375		0.500		0.583		

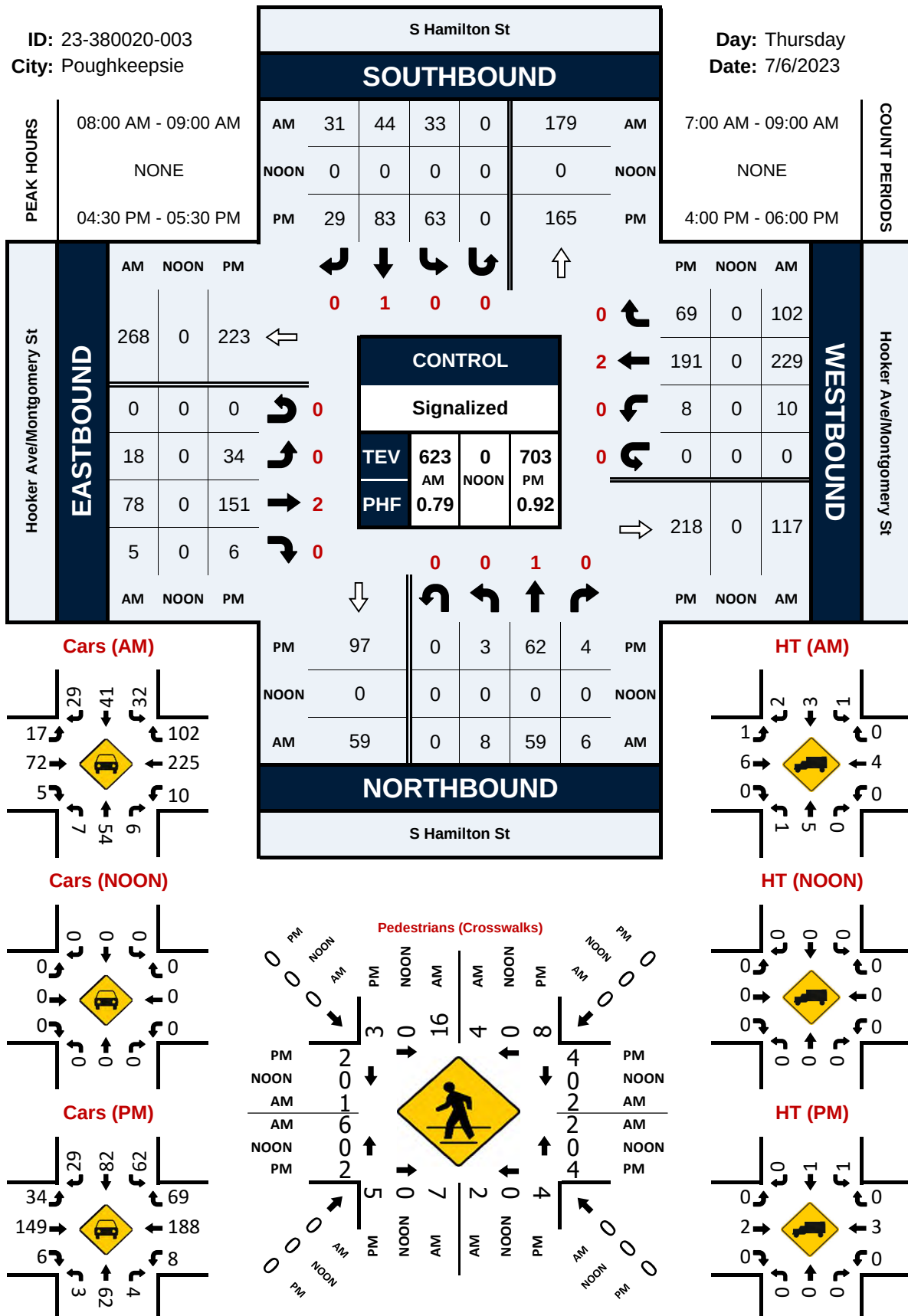
PM	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	
4:00 PM	4	1	0	1	1	2	0	2	11
4:15 PM	1	0	0	0	4	0	0	1	6
4:30 PM	0	2	3	0	1	1	0	0	7
4:45 PM	1	2	2	4	3	3	0	0	15
5:00 PM	2	3	0	0	0	0	2	2	9
5:15 PM	0	1	0	0	0	0	0	0	1
5:30 PM	2	1	0	0	1	0	2	6	12
5:45 PM	1	0	0	0	1	0	0	0	2
TOTAL VOLUMES :	EB 11	WB 10	EB 5	WB 5	NB 11	SB 6	NB 4	SB 11	TOTAL 63
APPROACH %'s :	52.38%	47.62%	50.00%	50.00%	64.71%	35.29%	26.67%	73.33%	
PEAK HR :	04:30 PM - 05:30 PM								TOTAL 32
PEAK HR VOL :	3	8	5	4	4	4	2	2	0.533
PEAK HR FACTOR :	0.375	0.667	0.417	0.250	0.333	0.333	0.250	0.250	
	0.550		0.375		0.333		0.250		

S Hamilton St & Hooker Ave/Montgomery St

Peak Hour Turning Movement Count

ID: 23-380020-003
City: Poughkeepsie

Day: Thursday
Date: 7/6/2023



National Data & Surveying Services Intersection Turning Movement Count

Location: S Hamilton St & Dwight St
City: Poughkeepsie
Control: 1-Way Stop(WB)

Project ID: 23-380020-001
Date: 7/8/2023

Data - Total

NS/EW Streets:	S Hamilton St				S Hamilton St				Dwight St				Dwight St				
NOON	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	8	8	0	1	3	0	0	0	0	0	0	6	0	3	0	29
11:15 AM	0	10	9	0	2	3	0	0	0	0	0	0	7	0	2	0	33
11:30 AM	0	9	14	0	1	7	0	0	0	0	0	0	10	0	2	0	43
11:45 AM	0	14	7	0	0	8	0	0	0	0	0	0	10	0	3	0	42
12:00 PM	0	9	9	0	0	7	0	0	0	0	0	0	5	0	6	0	36
12:15 PM	0	14	5	0	1	9	0	0	0	0	0	0	14	0	0	0	43
12:30 PM	0	9	7	0	1	2	0	0	0	0	0	0	10	0	6	0	35
12:45 PM	0	8	9	0	1	8	0	0	0	0	0	0	3	0	3	0	32
1:00 PM	0	12	4	1	1	12	0	0	0	0	0	0	7	0	3	0	40
1:15 PM	0	11	8	0	0	8	0	0	0	0	0	0	6	0	3	0	36
1:30 PM	0	14	5	0	7	11	0	0	0	0	0	0	6	0	4	0	47
1:45 PM	0	17	12	0	1	7	0	0	0	0	0	0	9	0	8	0	54
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
APPROACH %'s :	0	135	97	1	16	85	0	0	0	0	0	0	93	0	43	0	470
	0.00%	57.94%	41.63%	0.43%	15.84%	84.16%	0.00%	0.00%	0	0	0	0	68.38%	0.00%	31.62%	0.00%	
PEAK HR :	01:00 PM - 02:00 PM																TOTAL
PEAK HR VOL :	0	54	29	1	9	38	0	0	0	0	0	0	28	0	18	0	177
PEAK HR FACTOR :	0.000	0.794	0.604	0.250	0.321	0.792	0.000	0.000	0.000	0.000	0.000	0.000	0.778	0.000	0.563	0.000	0.819
		0.724				0.653								0.676			

National Data & Surveying Services Intersection Turning Movement Count

Location: S Hamilton St & Dwight St
City: Poughkeepsie
Control: 1-Way Stop(WB)

Project ID: 23-380020-001
Date: 7/8/2023

Data - Cars

NS/EW Streets:	S Hamilton St				S Hamilton St				Dwight St				Dwight St				
NOON	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	8	8	0	1	3	0	0	0	0	0	0	6	0	3	0	29
11:15 AM	0	10	8	0	2	3	0	0	0	0	0	0	7	0	2	0	32
11:30 AM	0	7	14	0	1	7	0	0	0	0	0	0	10	0	2	0	41
11:45 AM	0	14	7	0	0	8	0	0	0	0	0	0	10	0	3	0	42
12:00 PM	0	9	8	0	0	7	0	0	0	0	0	0	5	0	5	0	34
12:15 PM	0	13	5	0	1	9	0	0	0	0	0	0	14	0	0	0	42
12:30 PM	0	8	7	0	1	2	0	0	0	0	0	0	10	0	6	0	34
12:45 PM	0	8	9	0	1	8	0	0	0	0	0	0	3	0	3	0	32
1:00 PM	0	12	4	1	1	12	0	0	0	0	0	0	7	0	3	0	40
1:15 PM	0	11	8	0	0	8	0	0	0	0	0	0	6	0	3	0	36
1:30 PM	0	14	5	0	7	11	0	0	0	0	0	0	6	0	4	0	47
1:45 PM	0	17	12	0	1	7	0	0	0	0	0	0	9	0	8	0	54
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
APPROACH %'s :	0	131	95	1	16	85	0	0	0	0	0	0	93	0	42	0	463
	0.00%	57.71%	41.85%	0.44%	15.84%	84.16%	0.00%	0.00%					68.89%	0.00%	31.11%	0.00%	
PEAK HR :	01:00 PM - 02:00 PM																TOTAL
PEAK HR VOL :	0	54	29	1	9	38	0	0	0	0	0	0	28	0	18	0	177
PEAK HR FACTOR :	0.000	0.794	0.604	0.250	0.321	0.792	0.000	0.000	0.000	0.000	0.000	0.000	0.778	0.000	0.563	0.000	0.819
		0.724				0.653								0.676			

National Data & Surveying Services Intersection Turning Movement Count

Location: S Hamilton St & Dwight St
City: Poughkeepsie
Control: 1-Way Stop(WB)

Project ID: 23-380020-001
Date: 7/8/2023

Data - HT

NS/EW Streets:	S Hamilton St				S Hamilton St				Dwight St				Dwight St				
NOON	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1
11:30 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	0	2
12:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
12:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES :	NL 0	NT 4	NR 2	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 1	WU 0	TOTAL 7
APPROACH %'s :	0.00%	66.67%	33.33%	0.00%									0.00%	0.00%	100.00%	0.00%	
PEAK HR :	01:00 PM - 02:00 PM																TOTAL 0
PEAK HR VOL :	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	

National Data & Surveying ServicesIntersection Turning Movement Count

Location: S Hamilton St & Dwight St
City: Poughkeepsie
Control: 1-Way Stop(WB)

Project ID: 23-380020-001
Date: 7/8/2023

Data - Bikes

NS/EW Streets:	S Hamilton St				S Hamilton St				Dwight St				Dwight St				
NOON	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
11:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES :	NL 0	NT 2	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 2
APPROACH %'s :	0.00%	100.00%	0.00%	0.00%													
PEAK HR :	01:00 PM - 02:00 PM																
PEAK HR VOL :	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	

Movement Count

Project ID: 23-380020-001

Date: 7/8/2023

Data - Pedestrians (Crosswalks)

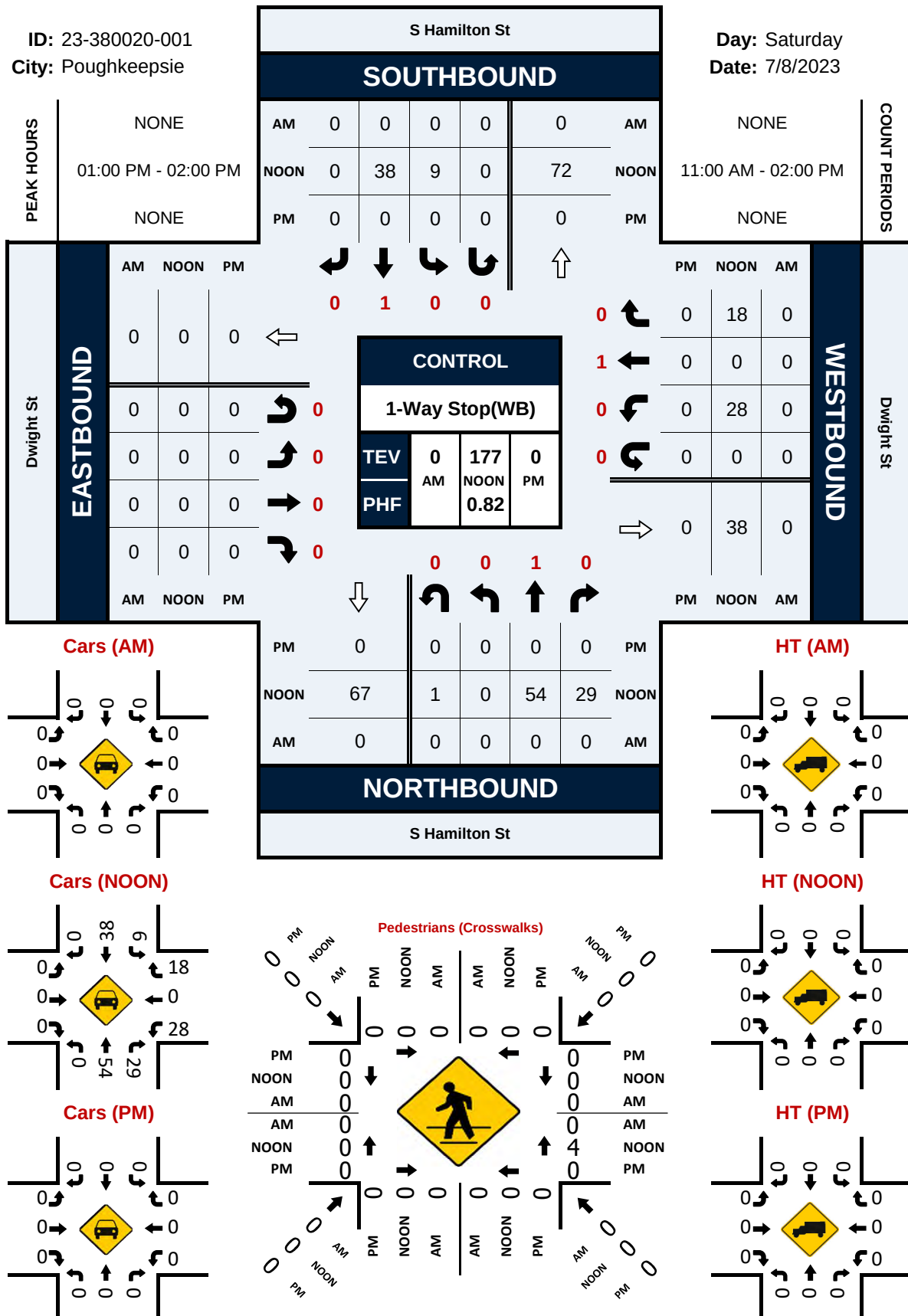
NS/EW Streets:	S Hamilton St		S Hamilton St		Dwight St		Dwight St		
NOON	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	
11:00 AM	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	7	0	0	0	7
12:30 PM	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	1	0	0	0	1
1:00 PM	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	2	0	0	0	2
1:30 PM	0	0	0	0	2	0	0	0	2
1:45 PM	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES : APPROACH %'s :	EB 0	WB 0	EB 0	WB 0	NB 12 100.00%	SB 0 0.00%	NB 0	SB 0	TOTAL 12
PEAK HR :	01:00 PM - 02:00 PM								TOTAL
PEAK HR VOL :	0		0		4 0.500		0		4
PEAK HR FACTOR :					0.500				0.500

S Hamilton St & Dwight St

Peak Hour Turning Movement Count

ID: 23-380020-001
City: Poughkeepsie

Day: Saturday
Date: 7/8/2023



National Data & Surveying ServicesIntersection Turning Movement Count

Location: Hooker Ave/Crescent Rd & Dwight St
City: Poughkeepsie
Control: Signalized

Project ID: 23-380020-002
Date: 7/8/2023

Data - Total

NS/EW Streets:		Hooker Ave/Crescent Rd						Hooker Ave/Crescent Rd						Dwight St						Dwight St						
NOON	NORTHBOUND						SOUTHBOUND						EASTBOUND						WESTBOUND						TOTAL	
	0	1	NT	NR	0	NL2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	0	WL	WT	WR	WU	WL2	N2L2	N2T2		N2U2
	0	1	NT	NR	0	NL2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	0	WL	WT	WR	WU	WL2	N2L2	N2T2		N2U2
11:00 AM	5	28	0	0	0	2	0	19	4	0	0	3	0	4	0	0	2	1	0	0	0	0	0	0	0	68
11:15 AM	4	28	0	0	0	1	0	19	1	0	1	7	0	6	0	0	3	3	4	0	0	0	0	0	2	79
11:30 AM	5	27	0	1	0	0	0	31	3	0	2	8	0	0	0	0	4	4	2	0	0	0	0	0	0	93
11:45 AM	6	30	0	1	1	1	0	31	1	0	3	1	0	0	0	0	1	6	2	0	0	0	0	3	0	87
12:00 PM	7	38	0	2	0	0	0	41	2	0	0	5	0	3	0	0	2	2	2	0	0	0	1	0	0	105
12:15 PM	7	31	0	0	0	0	0	30	6	0	0	5	0	2	0	0	2	2	1	0	1	0	1	1	1	89
12:30 PM	9	24	0	0	0	0	0	23	2	0	0	1	0	6	0	0	1	3	2	0	0	0	0	0	0	71
12:45 PM	2	33	0	0	0	0	0	24	0	0	0	2	7	4	0	0	2	4	4	0	0	0	0	0	0	82
1:00 PM	4	26	0	0	0	0	0	39	6	0	1	1	0	3	0	0	1	1	1	0	0	0	0	3	3	99
1:15 PM	11	31	0	0	0	0	0	38	0	0	3	3	0	6	0	0	8	1	6	0	0	0	0	0	1	108
1:30 PM	5	38	0	0	0	0	0	27	2	1	1	3	0	8	0	0	2	1	2	0	0	0	0	0	1	91
1:45 PM	12	35	0	0	0	0	0	35	4	0	0	6	0	5	0	0	0	1	2	0	1	0	0	0	0	101
TOTAL VOLUMES :	NL	NT	NR	NLU	NLU2		SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	WL	WT	WR	WU	WL2	N2L2	N2T2	N2U2		TOTAL
APPROACH % :	16.96%	36.9	0	4	4		0	367	31	1	10	52	0	58	0	0	28	28	28	0	2	1	1	8		106
PEAK HIR :	66.96%	81.28%	0.00%	0.88%	0.88%		0.00%	89.73%	7.58%	0.24%	2.44%	47.27%	0.00%	52.73%	0.00%	0.00%	32.56%	32.56%	32.56%	0.00%	2.33%	10.00%	10.00%	80.00%		106
PEAK HR VOL :	32	130	0	0	0		0	149	12	5	13	0	22	0	0	0	11	3	11	0	1	0	0	5		395
PEAK HR FACTOR :	0.667	0.855	0.000	0.000	0.000		0.000	0.760	0.500	0.250	0.417	0.542	0.000	0.688	0.000	0.000	0.344	0.750	0.458	0.000	0.250	0.000	0.000	0.417		0.991
									0.746					0.795					0.433							

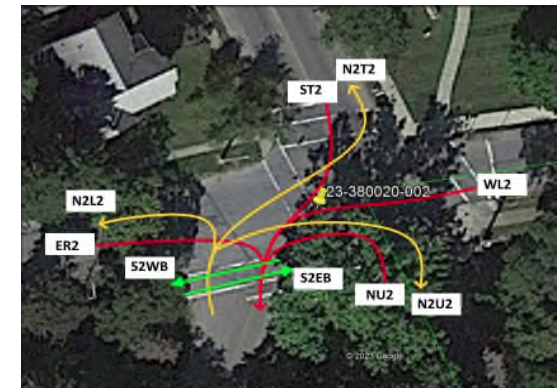
Explanation for extra leg movements

Movements entering the extra leg

ER2	Movements coming from EB on Dwight St entering into the Extra Leg (Crescent Rd)
ST2	Movements coming from SB on Hooker Ave entering into the Extra Leg (Crescent Rd)
WL2	Movements coming from WB on Hanscom Ave entering into the Extra Leg (Crescent Rd)
NU2	Movements coming from NB on Hooker Ave entering into the Extra Leg (Crescent Rd)

Movements exiting the extra leg

N2L2	Movements exiting from Extra Leg (Crescent Rd) entering into Dwight St going WB
N2T2	Movements exiting from Extra Leg (Crescent Rd) entering into Hooker Ave going NB
N2U2	Movements exiting from Extra Leg (Crescent Rd) entering into Hooker Ave going SB



National Data & Surveying ServicesIntersection Turning Movement Count

Location: Hooker Ave/Crescent Rd & Dwight St
City: Poughkeepsie
Control: Signalized

Project ID: 23-380020-002
Date: 7/8/2023

Data - Cars

NS/EW Streets:		Hooker Ave/Crescent Rd					Hooker Ave/Crescent Rd					Dwight St					Dwight St									
NOON	NORTHBOUND					SOUTHBOUND					EASTBOUND					WESTBOUND										
	0	1	0	0	0	0	1	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	TOTAL
	NL	NT	NR	NU	NU2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	WL	WT	WR	WU	WL2	N2L2	N2T2	N2U2			
	5	28	0	0	1	0	17	4	0	0	3	3	4	0	0	2	3	4	0	0	0	0	0	0	66	
	5	28	0	0	1	0	19	1	0	0	6	0	4	0	0	3	3	4	0	0	0	0	0	2	77	
	5	27	0	1	0	0	30	3	0	2	8	0	6	0	0	4	4	2	0	0	0	0	0	0	92	
	11:45 AM	6	30	0	1	1	0	30	1	0	0	3	0	5	0	0	1	6	2	0	0	0	0	0	0	87
	12:00 PM	7	31	0	2	0	38	2	0	0	5	0	2	0	0	1	6	2	0	0	0	0	0	0	100	
	12:15 PM	7	31	0	0	0	29	6	0	0	5	0	2	0	0	2	2	1	0	1	0	1	0	0	88	
	12:30 PM	9	24	0	0	0	22	2	0	0	1	0	6	0	0	1	3	2	0	0	0	0	0	0	70	
12:45 PM	2	32	0	0	0	0	24	0	0	2	7	0	4	0	0	2	4	4	0	0	0	0	0	0	81	
1:00 PM	4	26	0	0	0	0	49	6	0	1	1	0	3	0	0	1	0	1	0	0	0	0	0	3	95	
1:15 PM	11	31	0	0	0	0	38	0	0	3	3	6	6	0	0	8	1	6	0	0	0	0	0	1	108	
1:30 PM	5	38	0	0	0	0	27	2	1	1	3	0	8	0	0	2	1	2	0	0	0	0	0	1	91	
1:45 PM	12	35	0	0	0	0	35	4	0	0	6	0	5	0	0	0	1	2	0	1	0	0	0	0	101	
TOTAL VOLUMES:		NL	NT	NR	NU	NU2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	WL	WT	WR	WU	WL2	N2L2	N2T2	N2U2	TOTAL	
APPROACH %:		77.36%	81.24%	0.00%	0.88%	0.88%	0.00%	89.75%	7.75%	0.25%	2.25%	47.22%	0.00%	52.78%	0.00%	0.00%	32.56%	32.56%	32.56%	0.00%	2.33%	0.00%	11.11%	88.89%	TOTAL	
PEAK HR:		01:00 PM - 02:00 PM																								
PEAK HR VOL:		32	130	0	0	0	0	149	12	1	5	13	0	22	0	0	11	3	11	0	0	0	0	5	395	
PEAK HR FACTOR:		0.667	0.855	0.000	0.000	0.000	0.000	0.760	0.500	0.250	0.417	0.542	0.000	0.688	0.000	0.000	0.344	0.750	0.458	0.000	0.250	0.000	0.000	0.417	0.914	

National Data & Surveying ServicesIntersection Turning Movement Count

Location: Hooker Ave/Crescent Rd & Dwight St
City: Roughkspsie
Control: Signalized

Project ID: 23-380020-002
Date: 7/8/2023

Data - HT

NS/EW Streets:	Hooker Ave/Crescent Rd					Hooker Ave/Crescent Rd					Dwight St					Dwight St										
NOON	NORTHBOUND					SOUTHBOUND					EASTBOUND					WESTBOUND					TOTAL					
	0 NL	1 NT	0 NR	0 NU	0 NU2	0 SL	1 ST	0 SR	0 SU	0 ST2	0 EL	1 ET	0 ER	0 EU	0 ER2	0 WL	1 WT	0 WR	0 WU	0 WL2		0 N2L2	0 N2T2	0 N2U2		
11:00 AM	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2		
11:15 AM	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	2		
11:30 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1		
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
12:00 PM	0	0	0	0	0	0	3	0	0	0	0	0	1	0	0	0	0	0	0	0	1	0	0	5		
12:15 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1		
12:30 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1		
12:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1		
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
TOTAL VOLUMES :	NL	NT	NR	NU	NU2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	WL	WT	WR	WU	WL2	N2L2	N2T2	N2U2	TOTAL		
APPROACH %'s :	0.00%	100.00%	0.00%	0.00%	0.00%	0.00%	88.89%	0.00%	0.00%	11.11%	50.00%	0.00%	50.00%	0.00%	0.00%	0	0	0	0	0	100.00%	0.00%	0.00%	13		
PEAK HR :	01:00 PM - 02:00 PM					0					0					0					0					TOTAL
PEAK HR VOL :	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0		

National Data & Surveying ServicesIntersection Turning Movement Count

Location: Hooker Ave/Crescent Rd & Dwight St
City: Roughkspsie
Control: Signalized

Project ID: 23-380020-002
Date: 7/8/2023

Data - Bikes

NS/EW Streets:	Hooker Ave/Crescent Rd					Hooker Ave/Crescent Rd					Dwight St					Dwight St								
NOON	NORTHBOUND					SOUTHBOUND					EASTBOUND					WESTBOUND					TOTAL			
	0 NL	1 NT	0 NR	0 NU	0 NU2	0 SL	1 ST	0 SR	0 SU	0 ST2	0 EL	1 ET	0 ER	0 EU	0 ER2	0 WL	1 WT	0 WR	0 WU	0 WL2		0 N2L2	0 N2T2	0 N2U2
11:00 AM	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
11:15 AM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
12:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	2
1:30 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES :	NL	NT	NR	NU	NU2	SL	ST	SR	SU	ST2	EL	ET	ER	EU	ER2	WL	WT	WR	WU	WL2	N2L2	N2T2	N2U2	TOTAL
APPROACH %s :	0	3	2	0	1	0	1	0	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	9
PEAK HR :	0.00%	50.00%	33.33%	0.00%	16.67%	0.00%	50.00%	0.00%	0.00%	50.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	100.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
PEAK HR VOL :	0	0	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	3
PEAK HR FACTOR :	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.375

National Data & Surveying Services Intersection Turning Movement Count

Location: Hooker Ave/Crescent Rd & Dwight St
City: Poughkeepsie

Project ID: 23-380020-002
Date: 7/8/2023

Data - Pedestrians (Crosswalks)

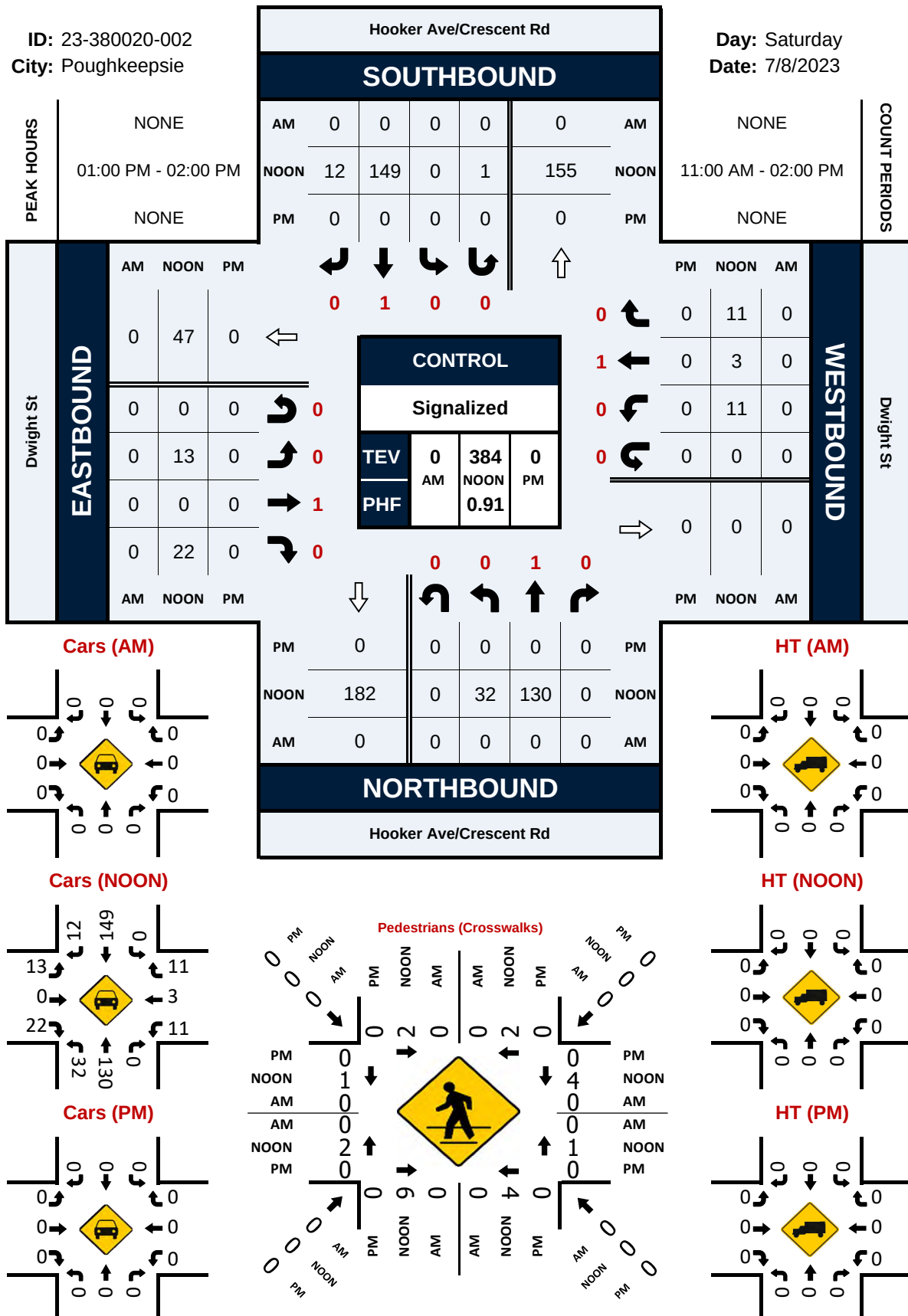
NS/EW Streets:	Hooker Ave/Crescent Rd		Hooker Ave/Crescent Rd		Dwight St		Dwight St			
NOON	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		SOUTH LEG 2	
	EB	WB	EB	WB	NB	SB	NB	SB	EB	WB
11:00 AM	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	2	0	0	2	0	0	1	0	0
11:30 AM	0	0	4	4	3	0	0	0	4	0
11:45 AM	1	0	0	4	1	8	0	0	1	4
12:00 PM	1	0	0	0	0	2	0	0	0	0
12:15 PM	0	0	0	0	1	1	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	1	0	0	0	1	0	0
1:00 PM	1	0	3	0	0	2	0	1	1	0
1:15 PM	0	0	0	3	0	0	2	0	0	0
1:30 PM	1	0	3	1	1	2	0	0	1	0
1:45 PM	0	2	0	0	0	0	0	0	0	0
TOTAL VOLUMES :	EB 4	WB 4	EB 10	WB 13	NB 8	SB 15	NB 2	SB 3	EB 7	WB 4
APPROACH %'s :	50.00%	50.00%	43.48%	56.52%	34.78%	65.22%	40.00%	60.00%	63.64%	36.36%
PEAK HR :	01:00 PM - 02:00 PM									
PEAK HR VOL :	2	2	6	4	1	4	2	1	2	0
PEAK HR FACTOR :	0.500	0.250	0.500	0.333	0.250	0.500	0.250	0.250	0.500	
	0.500		0.625		0.417		0.375		0.500	
										TOTAL
										70
										24
										0.667

Hooker Ave/Crescent Rd & Dwight St

Peak Hour Turning Movement Count

ID: 23-380020-002
City: Poughkeepsie

Day: Saturday
Date: 7/8/2023



National Data & Surveying Services Intersection Turning Movement Count

Location: S Hamilton St & Hooker Ave/Montgomery St
City: Poughkeepsie
Control: Signalized

Project ID: 23-380020-003
Date: 7/8/2023

Data - Total

NS/EW Streets:	S Hamilton St				S Hamilton St				Hooker Ave/Montgomery St				Hooker Ave/Montgomery St				
NOON	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	2 WT	0 WR	0 WU	
11:00 AM	0	6	1	0	9	11	5	0	3	10	0	0	0	35	8	0	88
11:15 AM	5	6	1	0	3	6	2	0	5	10	0	0	2	30	9	0	79
11:30 AM	1	11	2	0	9	10	2	0	3	17	1	0	5	34	12	0	107
11:45 AM	2	15	4	0	7	10	5	0	9	7	3	0	1	33	12	0	108
12:00 PM	0	6	1	0	6	14	10	0	4	17	0	0	1	40	18	0	117
12:15 PM	1	12	0	0	13	9	4	0	4	13	1	0	3	26	12	0	98
12:30 PM	1	5	1	0	9	13	0	0	3	15	1	0	0	33	8	0	89
12:45 PM	2	10	1	0	2	7	7	0	6	20	0	0	6	22	14	0	97
1:00 PM	2	13	1	0	9	12	3	0	1	28	0	0	2	25	7	0	103
1:15 PM	0	7	2	0	13	8	2	0	4	21	0	0	2	36	19	0	114
1:30 PM	3	17	0	0	11	12	2	0	7	17	2	0	3	43	15	0	132
1:45 PM	3	11	1	0	11	7	8	0	5	15	0	0	2	32	14	0	109
TOTAL VOLUMES :	NL 20	NT 119	NR 15	NU 0	SL 102	ST 119	SR 50	SU 0	EL 54	ET 190	ER 8	EU 0	WL 27	WT 389	WR 148	WU 0	TOTAL 1241
APPROACH %'s :	12.99%	77.27%	9.74%	0.00%	37.64%	43.91%	18.45%	0.00%	21.43%	75.40%	3.17%	0.00%	4.79%	68.97%	26.24%	0.00%	
PEAK HR :	01:00 PM - 02:00 PM																
PEAK HR VOL :	8	48	4	0	44	39	15	0	17	81	2	0	9	136	55	0	TOTAL 458
PEAK HR FACTOR :	0.667	0.706	0.500	0.000	0.846	0.813	0.469	0.000	0.607	0.723	0.250	0.000	0.750	0.791	0.724	0.000	0.867
		0.750				0.942				0.862				0.820			

National Data & Surveying Services Intersection Turning Movement Count

Location: S Hamilton St & Hooker Ave/Montgomery St
City: Poughkeepsie
Control: Signalized

Project ID: 23-380020-003
Date: 7/8/2023

Data - Cars

NS/EW Streets:	S Hamilton St				S Hamilton St				Hooker Ave/Montgomery St				Hooker Ave/Montgomery St				
NOON	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	2 WT	0 WR	0 WU	
11:00 AM	0	6	1	0	8	10	4	0	3	9	0	0	0	34	8	0	83
11:15 AM	4	6	1	0	3	6	2	0	5	10	0	0	2	30	9	0	78
11:30 AM	1	9	2	0	9	10	2	0	3	15	1	0	5	33	12	0	102
11:45 AM	2	15	4	0	7	10	4	0	8	7	3	0	1	32	12	0	105
12:00 PM	0	6	1	0	6	14	10	0	4	16	0	0	1	40	18	0	116
12:15 PM	1	11	0	0	13	9	4	0	4	13	1	0	3	26	12	0	97
12:30 PM	1	4	1	0	9	13	0	0	3	13	1	0	0	33	8	0	86
12:45 PM	2	10	1	0	2	7	6	0	6	20	0	0	6	21	14	0	95
1:00 PM	2	13	1	0	9	12	3	0	1	28	0	0	2	25	7	0	103
1:15 PM	0	7	2	0	13	8	2	0	4	21	0	0	2	36	19	0	114
1:30 PM	3	17	0	0	11	12	2	0	7	15	2	0	3	43	15	0	130
1:45 PM	3	11	1	0	11	7	7	0	5	15	0	0	2	32	14	0	108
TOTAL VOLUMES :	NL 19	NT 115	NR 15	NU 0	SL 101	ST 118	SR 46	SU 0	EL 53	ET 182	ER 8	EU 0	WL 27	WT 385	WR 148	WU 0	TOTAL 1217
APPROACH %'s :	12.75%	77.18%	10.07%	0.00%	38.11%	44.53%	17.36%	0.00%	21.81%	74.90%	3.29%	0.00%	4.82%	68.75%	26.43%	0.00%	
PEAK HR :	01:00 PM - 02:00 PM																TOTAL
PEAK HR VOL :	8	48	4	0	44	39	14	0	17	79	2	0	9	136	55	0	455
PEAK HR FACTOR :	0.667	0.706	0.500	0.000	0.846	0.813	0.500	0.000	0.607	0.705	0.250	0.000	0.750	0.791	0.724	0.000	0.875
		0.750				0.970				0.845				0.820			

National Data & Surveying Services Intersection Turning Movement Count

Location: S Hamilton St & Hooker Ave/Montgomery St
City: Poughkeepsie
Control: Signalized

Project ID: 23-380020-003
Date: 7/8/2023

Data - HT

NS/EW Streets:	S Hamilton St				S Hamilton St				Hooker Ave/Montgomery St				Hooker Ave/Montgomery St				
NOON	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	2 WT	0 WR	0 WU	
11:00 AM	0	0	0	0	1	1	1	0	0	1	0	0	0	1	0	0	5
11:15 AM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
11:30 AM	0	2	0	0	0	0	0	0	0	2	0	0	0	1	0	0	5
11:45 AM	0	0	0	0	0	0	1	0	1	0	0	0	0	1	0	0	3
12:00 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
12:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
12:30 PM	0	1	0	0	0	0	0	0	0	2	0	0	0	0	0	0	3
12:45 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	1	0	0	2
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2
1:45 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
TOTAL VOLUMES :	NL 1	NT 4	NR 0	NU 0	SL 1	ST 1	SR 4	SU 0	EL 1	ET 8	ER 0	EU 0	WL 0	WT 4	WR 0	WU 0	TOTAL 24
APPROACH %'s :	20.00%	80.00%	0.00%	0.00%	16.67%	16.67%	66.67%	0.00%	11.11%	88.89%	0.00%	0.00%	0.00%	100.00%	0.00%	0.00%	
PEAK HR :	01:00 PM - 02:00 PM																TOTAL
PEAK HR VOL :	0	0	0	0	0	0	1	0	0	2	0	0	0	0	0	0	3
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.000	0.375

National Data & Surveying Services Intersection Turning Movement Count

Location: S Hamilton St & Hooker Ave/Montgomery St
City: Poughkeepsie
Control: Signalized

Project ID: 23-380020-003
Date: 7/8/2023

Data - Bikes

NS/EW Streets:	S Hamilton St				S Hamilton St				Hooker Ave/Montgomery St				Hooker Ave/Montgomery St				
NOON	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	2 ET	0 ER	0 EU	0 WL	2 WT	0 WR	0 WU	
11:00 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	2
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	1	0	0	2
12:15 PM	0	1	0	0	0	0	0	0	0	1	0	0	0	1	0	0	3
12:30 PM	1	0	0	0	1	1	0	0	0	1	0	0	0	1	0	0	5
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
1:15 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	2	0	3
1:30 PM	0	1	0	0	0	1	0	0	1	0	0	0	0	1	0	0	4
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
TOTAL VOLUMES :	NL 1	NT 2	NR 0	NU 0	SL 3	ST 2	SR 0	SU 0	EL 2	ET 2	ER 0	EU 0	WL 0	WT 5	WR 4	WU 0	TOTAL 21
APPROACH %'s :	33.33%	66.67%	0.00%	0.00%	60.00%	40.00%	0.00%	0.00%	50.00%	50.00%	0.00%	0.00%	0.00%	55.56%	44.44%	0.00%	
PEAK HR :	01:00 PM - 02:00 PM																TOTAL 9
PEAK HR VOL :	0	1	0	0	1	1	0	0	1	0	0	0	0	1	4	0	
PEAK HR FACTOR :	0.000	0.250	0.000	0.000	0.250	0.250	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.250	0.500	0.000	0.563
		0.250				0.500				0.250				0.625			

National Data & Surveying Services Intersection Turning Movement Count

Location: S Hamilton St & Hooker Ave/Montgomery St
City: Poughkeepsie

Project ID: 23-380020-003
Date: 7/8/2023

Data - Pedestrians (Crosswalks)

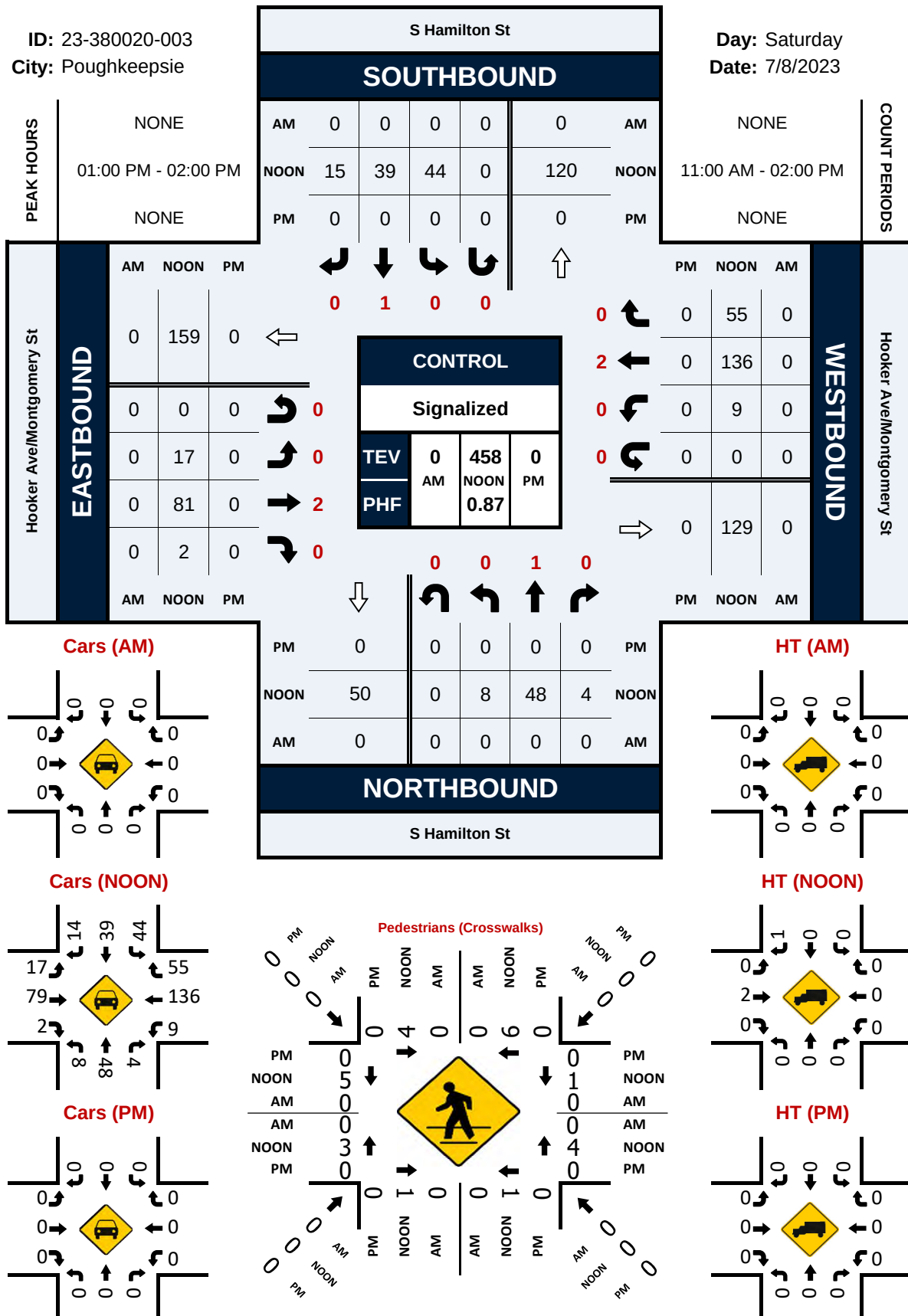
NS/EW Streets:	S Hamilton St		S Hamilton St		Hooker Ave/Montgomery St		Hooker Ave/Montgomery St		
NOON	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	
11:00 AM	5	3	0	0	3	0	0	0	11
11:15 AM	0	0	0	1	1	0	2	2	6
11:30 AM	0	2	0	1	0	0	1	0	4
11:45 AM	1	1	0	1	1	0	0	1	5
12:00 PM	2	2	0	1	2	0	3	4	14
12:15 PM	3	3	0	0	0	0	0	0	6
12:30 PM	1	1	0	0	0	0	0	3	5
12:45 PM	0	0	0	0	0	0	0	1	1
1:00 PM	0	0	0	0	0	1	0	0	1
1:15 PM	1	2	0	0	0	0	2	4	9
1:30 PM	3	1	0	1	3	0	0	0	8
1:45 PM	0	3	1	0	1	0	1	1	7
TOTAL VOLUMES : APPROACH %'s :	EB 16 47.06%	WB 18 52.94%	EB 1 16.67%	WB 5 83.33%	NB 11 91.67%	SB 1 8.33%	NB 9 36.00%	SB 16 64.00%	TOTAL 77
	01:00 PM - 02:00 PM								
PEAK HR VOL : PEAK HR FACTOR :	4 0.333	6 0.500	1 0.250	1 0.250	4 0.333	1 0.250	3 0.375	5 0.313	25 0.694
	0.625		0.500		0.417		0.333		

S Hamilton St & Hooker Ave/Montgomery St

Peak Hour Turning Movement Count

ID: 23-380020-003
City: Poughkeepsie

Day: Saturday
Date: 7/8/2023



ATTACHMENT C
LEVEL OF SERVICE ANALYSIS

City of Poughkeepsie
Dutchess County, New York

HCM Signalized Intersection Capacity Analysis 1: S. Hamilton St & Montgomery St/Hooker Ave

One Key Development - Poughkeepsie

EX 2023_AM Peak Hour

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	18	78	5	10	229	102	8	59	6	33	44	31
Future Volume (vph)	18	78	5	10	229	102	8	59	6	33	44	31
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Lane Util. Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	1.00			1.00	0.97		1.00			0.99	
Flpb, ped/bikes	1.00	1.00			1.00	1.00		1.00			1.00	
Frt	1.00	0.99			1.00	0.85		0.99			0.96	
Flt Protected	0.95	1.00			1.00	1.00		0.99			0.98	
Satd. Flow (prot)	1696	1747			1859	1565		1714			1685	
Flt Permitted	0.58	1.00			0.99	1.00		0.95			0.86	
Satd. Flow (perm)	1027	1747			1838	1565		1635			1476	
Peak-hour factor, PHF	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Adj. Flow (vph)	23	99	6	13	290	129	10	75	8	42	56	39
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	23	105	0	0	303	129	0	93	0	0	137	0
Confl. Peds. (#/hr)	9		20	20		9	4		7	7		4
Heavy Vehicles (%)	6%	8%	0%	0%	2%	0%	13%	9%	0%	3%	7%	7%
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	6	1			5			3			3	
Permitted Phases	1			5		5	3			3		
Actuated Green, G (s)	19.6	19.6			13.9	13.9		7.4			7.4	
Effective Green, g (s)	19.6	19.6			13.9	13.9		7.4			7.4	
Actuated g/C Ratio	0.53	0.53			0.38	0.38		0.20			0.20	
Clearance Time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	556	925			690	587		327			295	
v/s Ratio Prot	0.00	c0.06										
v/s Ratio Perm	0.02				c0.16	0.08		0.06			c0.09	
v/c Ratio	0.04	0.11			0.44	0.22		0.28			0.46	
Uniform Delay, d1	4.2	4.4			8.6	7.9		12.6			13.1	
Progression Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.0	0.1			0.4	0.2		0.5			1.2	
Delay (s)	4.2	4.4			9.1	8.0		13.0			14.2	
Level of Service	A	A			A	A		B			B	
Approach Delay (s)		4.4			8.8			13.0			14.2	
Approach LOS		A			A			B			B	
Intersection Summary												
HCM 2000 Control Delay			9.5									
HCM 2000 Volume to Capacity ratio			0.45									
Actuated Cycle Length (s)			37.0									
Intersection Capacity Utilization			40.4%									
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

EX 2023_AM Peak Hour

Movement	EBL	EBT	EBR	WBL2	WBL	WBT	WBR	NBL	NBT	SBT	SBR	SBR2
Lane Configurations		↔			↔	↔			↔	↔		
Traffic Volume (vph)	14	0	25	4	0	8	4	39	208	119	15	2
Future Volume (vph)	14	0	25	4	0	8	4	39	208	119	15	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0			5.0	5.0			5.0	5.0		
Lane Util. Factor		1.00			1.00	1.00			1.00	1.00		
Frpb, ped/bikes		0.97			1.00	0.99			1.00	1.00		
Flpb, ped/bikes		0.99			0.94	1.00			1.00	1.00		
Frt		0.91			1.00	0.95			1.00	0.98		
Flt Protected		0.98			0.95	1.00			0.99	1.00		
Satd. Flow (prot)		1557			1693	1653			1868	1825		
Flt Permitted		1.00			1.00	1.00			0.93	1.00		
Satd. Flow (perm)		1585			1782	1653			1749	1825		
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	15	0	27	4	0	9	4	43	229	131	16	2
RTOR Reduction (vph)	0	0	0	0	0	4	0	0	0	1	0	0
Lane Group Flow (vph)	0	42	0	0	4	9	0	0	272	148	0	0
Confl. Peds. (#/hr)	3		5	5	5		3	5			5	5
Heavy Vehicles (%)	7%	0%	4%	0%	0%	12%	0%	0%	1%	2%	2%	2%
Turn Type	Perm	NA		Perm	Perm	NA		Perm	NA	NA		
Protected Phases		1				1			3	3		
Permitted Phases	1			1	1			3				
Actuated Green, G (s)		2.2			2.2	2.2			11.1	11.1		
Effective Green, g (s)		2.2			2.2	2.2			11.1	11.1		
Actuated g/C Ratio		0.08			0.08	0.08			0.38	0.38		
Clearance Time (s)		5.0			5.0	5.0			5.0	5.0		
Vehicle Extension (s)		3.0			3.0	3.0			3.0	3.0		
Lane Grp Cap (vph)		120			135	125			671	700		
v/s Ratio Prot						0.01				0.08		
v/s Ratio Perm		c0.03			0.00				c0.16			
v/c Ratio		0.35			0.03	0.07			0.41	0.21		
Uniform Delay, d1		12.7			12.4	12.4			6.5	6.0		
Progression Factor		1.00			1.00	1.00			1.00	1.00		
Incremental Delay, d2		1.8			0.1	0.3			0.4	0.2		
Delay (s)		14.4			12.5	12.7			6.9	6.1		
Level of Service		B			B	B			A	A		
Approach Delay (s)		14.4				12.6			6.9	6.1		
Approach LOS		B				B			A	A		
Intersection Summary												
HCM 2000 Control Delay		7.6			HCM 2000 Level of Service				A			
HCM 2000 Volume to Capacity ratio		0.38										
Actuated Cycle Length (s)		28.9			Sum of lost time (s)				15.0			
Intersection Capacity Utilization		49.8%			ICU Level of Service				A			
Analysis Period (min)		15										

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

EX 2023_AM Peak Hour



Movement	NEL	NER2
Lane Configurations		
Traffic Volume (vph)	2	1
Future Volume (vph)	2	1
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	5.0	
Lane Util. Factor	1.00	
Frpb, ped/bikes	1.00	
Flpb, ped/bikes	0.95	
Frt	0.95	
Flt Protected	0.97	
Satd. Flow (prot)	1639	
Flt Permitted	0.97	
Satd. Flow (perm)	1639	
Peak-hour factor, PHF	0.91	0.91
Adj. Flow (vph)	2	1
RTOR Reduction (vph)	3	0
Lane Group Flow (vph)	0	0
Confl. Peds. (#/hr)	5	
Heavy Vehicles (%)	2%	2%
Turn Type	Perm	
Protected Phases		
Permitted Phases	4	
Actuated Green, G (s)	0.6	
Effective Green, g (s)	0.6	
Actuated g/C Ratio	0.02	
Clearance Time (s)	5.0	
Vehicle Extension (s)	3.0	
Lane Grp Cap (vph)	34	
v/s Ratio Prot		
v/s Ratio Perm	c0.00	
v/c Ratio	0.00	
Uniform Delay, d1	13.9	
Progression Factor	1.00	
Incremental Delay, d2	0.0	
Delay (s)	13.9	
Level of Service	B	
Approach Delay (s)	13.9	
Approach LOS	B	
Intersection Summary		

Intersection						
Int Delay, s/veh	3.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	43	21	51	25	12	40
Future Vol, veh/h	43	21	51	25	12	40
Conflicting Peds, #/hr	2	0	0	2	2	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	86	86	86	86	86	86
Heavy Vehicles, %	7	10	6	8	0	3
Mvmt Flow	50	24	59	29	14	47
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	153	76	0	0	90	0
Stage 1	76	-	-	-	-	-
Stage 2	77	-	-	-	-	-
Critical Hdwy	6.47	6.3	-	-	4.1	-
Critical Hdwy Stg 1	5.47	-	-	-	-	-
Critical Hdwy Stg 2	5.47	-	-	-	-	-
Follow-up Hdwy	3.563	3.39	-	-	2.2	-
Pot Cap-1 Maneuver	827	963	-	-	1518	-
Stage 1	934	-	-	-	-	-
Stage 2	934	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	816	961	-	-	1515	-
Mov Cap-2 Maneuver	816	-	-	-	-	-
Stage 1	932	-	-	-	-	-
Stage 2	924	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	9.6	0		1.7		
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBL	SBT	
Capacity (veh/h)	-	- 859		1515	-	
HCM Lane V/C Ratio	-	- 0.087		0.009	-	
HCM Control Delay (s)	-	- 9.6		7.4	0	
HCM Lane LOS	-	- A		A	A	
HCM 95th %tile Q(veh)	-	- 0.3		0	-	

HCM Signalized Intersection Capacity Analysis 1: S. Hamilton St & Montgomery St/Hooker Ave

One Key Development - Poughkeepsie

EX 2023_PM Peak Hour

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	34	151	6	8	191	69	3	62	4	63	83	29
Future Volume (vph)	34	151	6	8	191	69	3	62	4	63	83	29
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Lane Util. Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	1.00			1.00	0.97		1.00			1.00	
Flpb, ped/bikes	0.99	1.00			1.00	1.00		1.00			1.00	
Frt	1.00	0.99			1.00	0.85		0.99			0.98	
Flt Protected	0.95	1.00			1.00	1.00		1.00			0.98	
Satd. Flow (prot)	1795	1868			1842	1563		1879			1798	
Flt Permitted	0.62	1.00			0.98	1.00		0.98			0.85	
Satd. Flow (perm)	1176	1868			1810	1563		1847			1558	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	37	164	7	9	208	75	3	67	4	68	90	32
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	37	171	0	0	217	75	0	74	0	0	190	0
Confl. Peds. (#/hr)	11		9	9		11	8		4	4		8
Heavy Vehicles (%)	0%	1%	0%	0%	3%	0%	0%	0%	0%	1%	1%	0%
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	6	1			5			3			3	
Permitted Phases	1			5		5	3			3		
Actuated Green, G (s)	15.2	15.2			8.6	8.6		7.9			7.9	
Effective Green, g (s)	15.2	15.2			8.6	8.6		7.9			7.9	
Actuated g/C Ratio	0.46	0.46			0.26	0.26		0.24			0.24	
Clearance Time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	569	857			470	406		440			371	
v/s Ratio Prot	0.00	c0.09										
v/s Ratio Perm	0.03				c0.12	0.05		0.04			c0.12	
v/c Ratio	0.07	0.20			0.46	0.18		0.17			0.51	
Uniform Delay, d1	5.0	5.3			10.3	9.5		10.0			10.9	
Progression Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.0	0.1			0.7	0.2		0.2			1.2	
Delay (s)	5.0	5.4			11.0	9.7		10.2			12.1	
Level of Service	A	A			B	A		B			B	
Approach Delay (s)		5.4			10.7			10.2			12.1	
Approach LOS		A			B			B			B	
Intersection Summary												
HCM 2000 Control Delay			9.6									
HCM 2000 Volume to Capacity ratio			0.49									
Actuated Cycle Length (s)			33.1									
Intersection Capacity Utilization			49.5%									
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

EX 2023_PM Peak Hour

												
Movement	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	NBL2	NBL	NBT	SBT
Lane Configurations												
Traffic Volume (vph)	25	0	34	1	2	13	11	6	3	30	188	260
Future Volume (vph)	25	0	34	1	2	13	11	6	3	30	188	260
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0				5.0	5.0				5.0	5.0
Lane Util. Factor		1.00				1.00	1.00				1.00	1.00
Frpb, ped/bikes		0.98				1.00	0.99				1.00	1.00
Flpb, ped/bikes		1.00				0.98	1.00				1.00	1.00
Frt		0.92				1.00	0.95				1.00	0.99
Flt Protected		0.98				0.95	1.00				0.99	1.00
Satd. Flow (prot)		1672				1777	1788				1838	1852
Flt Permitted		0.86				1.00	1.00				0.91	1.00
Satd. Flow (perm)		1463				1871	1788				1687	1852
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	27	0	37	1	2	14	12	6	3	32	202	280
RTOR Reduction (vph)	0	57	0	0	0	0	5	0	0	0	0	2
Lane Group Flow (vph)	0	8	0	0	0	16	13	0	0	0	237	310
Confl. Peds. (#/hr)	2		2	2	2	2		2	2	2		
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	3%	1%
Turn Type	Perm	NA			Perm	Perm	NA		Perm	Perm	NA	NA
Protected Phases		1					1				3	3
Permitted Phases	1				1	1			3	3		
Actuated Green, G (s)		3.7				3.7	3.7				11.9	11.9
Effective Green, g (s)		3.7				3.7	3.7				11.9	11.9
Actuated g/C Ratio		0.12				0.12	0.12				0.38	0.38
Clearance Time (s)		5.0				5.0	5.0				5.0	5.0
Vehicle Extension (s)		3.0				3.0	3.0				3.0	3.0
Lane Grp Cap (vph)		172				221	211				641	704
v/s Ratio Prot							0.01					c0.17
v/s Ratio Perm		0.01				c0.01					0.14	
v/c Ratio		0.04				0.07	0.06				0.37	0.44
Uniform Delay, d1		12.2				12.3	12.3				7.0	7.2
Progression Factor		1.00				1.00	1.00				1.00	1.00
Incremental Delay, d2		0.1				0.1	0.1				0.4	0.4
Delay (s)		12.3				12.4	12.4				7.4	7.7
Level of Service		B				B	B				A	A
Approach Delay (s)		12.3					12.4				7.4	7.7
Approach LOS		B					B				A	A
Intersection Summary												
HCM 2000 Control Delay		8.3				HCM 2000 Level of Service				A		
HCM 2000 Volume to Capacity ratio		0.34										
Actuated Cycle Length (s)		31.3				Sum of lost time (s)				15.0		
Intersection Capacity Utilization		57.6%				ICU Level of Service				B		
Analysis Period (min)		15										
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

EX 2023_PM Peak Hour



Movement	SBR	SBR2	NEL2	NEL	NER2
Lane Configurations					
Traffic Volume (vph)	18	12	1	3	3
Future Volume (vph)	18	12	1	3	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900
Total Lost time (s)				5.0	
Lane Util. Factor				1.00	
Frpb, ped/bikes				0.98	
Flpb, ped/bikes				0.98	
Frt				0.94	
Flt Protected				0.97	
Satd. Flow (prot)				1672	
Flt Permitted				0.97	
Satd. Flow (perm)				1672	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	19	13	1	3	3
RTOR Reduction (vph)	0	0	0	7	0
Lane Group Flow (vph)	0	0	0	0	0
Confl. Peds. (#/hr)	2	2		2	2
Heavy Vehicles (%)	0%	0%	0%	0%	0%
Turn Type			Perm	Perm	
Protected Phases					
Permitted Phases			4	4	
Actuated Green, G (s)				0.7	
Effective Green, g (s)				0.7	
Actuated g/C Ratio				0.02	
Clearance Time (s)				5.0	
Vehicle Extension (s)				3.0	
Lane Grp Cap (vph)				37	
v/s Ratio Prot					
v/s Ratio Perm				0.00	
v/c Ratio				0.00	
Uniform Delay, d1				15.0	
Progression Factor				1.00	
Incremental Delay, d2				0.0	
Delay (s)				15.0	
Level of Service				B	
Approach Delay (s)				15.0	
Approach LOS				B	
Intersection Summary					

Intersection						
Int Delay, s/veh	2.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	33	25	65	53	8	68
Future Vol, veh/h	33	25	65	53	8	68
Conflicting Peds, #/hr	1	0	0	3	3	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	86	86	86	86	86	86
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	38	29	76	62	9	79
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	208	110	0	0	141	0
Stage 1	110	-	-	-	-	-
Stage 2	98	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	785	949	-	-	1455	-
Stage 1	920	-	-	-	-	-
Stage 2	931	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	777	946	-	-	1451	-
Mov Cap-2 Maneuver	777	-	-	-	-	-
Stage 1	917	-	-	-	-	-
Stage 2	924	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	9.6	0		0.8		
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	842	1451	-	
HCM Lane V/C Ratio	-	-	0.08	0.006	-	
HCM Control Delay (s)	-	-	9.6	7.5	0	
HCM Lane LOS	-	-	A	A	A	
HCM 95th %tile Q(veh)	-	-	0.3	0	-	

HCM Signalized Intersection Capacity Analysis 1: S. Hamilton St & Montgomery St/Hooker Ave

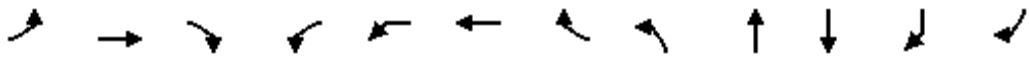
One Key Development - Poughkeepsie

EX 2023_SAT Midday Peak Hour

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	17	81	2	9	136	55	8	48	4	44	39	15
Future Volume (vph)	17	81	2	9	136	55	8	48	4	44	39	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Lane Util. Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	1.00			1.00	0.97		1.00			1.00	
Flpb, ped/bikes	0.99	1.00			1.00	1.00		1.00			1.00	
Frt	1.00	1.00			1.00	0.85		0.99			0.98	
Flt Protected	0.95	1.00			1.00	1.00		0.99			0.98	
Satd. Flow (prot)	1795	1839			1893	1565		1864			1791	
Flt Permitted	0.65	1.00			0.98	1.00		0.94			0.82	
Satd. Flow (perm)	1232	1839			1853	1565		1757			1505	
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	20	93	2	10	156	63	9	55	5	51	45	17
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	20	95	0	0	166	63	0	69	0	0	113	0
Confl. Peds. (#/hr)	11		9	9		11	8		4	4		8
Heavy Vehicles (%)	0%	3%	0%	0%	0%	0%	0%	0%	0%	0%	0%	7%
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	6	1			5			3			3	
Permitted Phases	1			5		5	3			3		
Actuated Green, G (s)	13.7	13.7			7.9	7.9		6.3			6.3	
Effective Green, g (s)	13.7	13.7			7.9	7.9		6.3			6.3	
Actuated g/C Ratio	0.46	0.46			0.26	0.26		0.21			0.21	
Clearance Time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	577	839			487	412		368			316	
v/s Ratio Prot	0.00	c0.05										
v/s Ratio Perm	0.01				c0.09	0.04		0.04			c0.08	
v/c Ratio	0.03	0.11			0.34	0.15		0.19			0.36	
Uniform Delay, d1	4.5	4.7			8.9	8.5		9.7			10.1	
Progression Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.0	0.1			0.4	0.2		0.2			0.7	
Delay (s)	4.5	4.7			9.4	8.7		10.0			10.8	
Level of Service	A	A			A	A		A			B	
Approach Delay (s)		4.7			9.2			10.0			10.8	
Approach LOS		A			A			A			B	
Intersection Summary												
HCM 2000 Control Delay			8.7				HCM 2000 Level of Service			A		
HCM 2000 Volume to Capacity ratio			0.35									
Actuated Cycle Length (s)			30.0				Sum of lost time (s)			15.0		
Intersection Capacity Utilization			36.7%				ICU Level of Service			A		
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

EX 2023_SAT Midday Peak Hour

												
Movement	EBL	EBT	EBR	WBL2	WBL	WBT	WBR	NBL	NBT	SBT	SBR	SBR2
Lane Configurations		↔			↔	↔			↔	↔		
Traffic Volume (vph)	13	0	22	1	11	3	11	32	130	149	12	5
Future Volume (vph)	13	0	22	1	11	3	11	32	130	149	12	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0			5.0	5.0			5.0	5.0		
Lane Util. Factor		1.00			1.00	1.00			1.00	1.00		
Frpb, ped/bikes		0.95			1.00	0.97			1.00	1.00		
Flpb, ped/bikes		0.99			0.90	1.00			1.00	1.00		
Frt		0.91			1.00	0.88			1.00	0.99		
Flt Protected		0.98			0.95	1.00			0.99	1.00		
Satd. Flow (prot)		1614			1616	1616			1834	1850		
Flt Permitted		1.00			1.00	1.00			0.88	1.00		
Satd. Flow (perm)		1643			1701	1616			1638	1850		
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	14	0	24	1	12	3	12	35	143	164	13	5
RTOR Reduction (vph)	0	0	0	0	0	11	0	0	0	2	0	0
Lane Group Flow (vph)	0	38	0	0	13	4	0	0	178	180	0	0
Confl. Peds. (#/hr)	4		10	10	10		4	5			5	5
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	3%	1%	0%	0%
Turn Type	Perm	NA		Perm	Perm	NA		Perm	NA	NA		
Protected Phases		1				1			3	3		
Permitted Phases	1			1	1			3				
Actuated Green, G (s)		2.1			2.1	2.1			5.5	5.5		
Effective Green, g (s)		2.1			2.1	2.1			5.5	5.5		
Actuated g/C Ratio		0.09			0.09	0.09			0.24	0.24		
Clearance Time (s)		5.0			5.0	5.0			5.0	5.0		
Vehicle Extension (s)		3.0			3.0	3.0			3.0	3.0		
Lane Grp Cap (vph)		148			153	146			388	438		
v/s Ratio Prot						0.00				0.10		
v/s Ratio Perm		c0.02			0.01				c0.11			
v/c Ratio		0.26			0.08	0.03			0.46	0.41		
Uniform Delay, d1		9.8			9.7	9.6			7.6	7.5		
Progression Factor		1.00			1.00	1.00			1.00	1.00		
Incremental Delay, d2		0.9			0.2	0.1			0.9	0.6		
Delay (s)		10.7			9.9	9.7			8.4	8.1		
Level of Service		B			A	A			A	A		
Approach Delay (s)		10.7				9.8			8.4	8.1		
Approach LOS		B				A			A	A		
Intersection Summary												
HCM 2000 Control Delay		8.6			HCM 2000 Level of Service				A			
HCM 2000 Volume to Capacity ratio		0.37										
Actuated Cycle Length (s)		23.2			Sum of lost time (s)				15.0			
Intersection Capacity Utilization		46.9%			ICU Level of Service				A			
Analysis Period (min)		15										
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

EX 2023_SAT Midday Peak Hour



Movement	NEL	NER2
Lane Configurations		
Traffic Volume (vph)	0	5
Future Volume (vph)	0	5
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	5.0	
Lane Util. Factor	1.00	
Frpb, ped/bikes	0.87	
Flpb, ped/bikes	1.00	
Frt	0.86	
Flt Protected	1.00	
Satd. Flow (prot)	1424	
Flt Permitted	1.00	
Satd. Flow (perm)	1424	
Peak-hour factor, PHF	0.91	0.91
Adj. Flow (vph)	0	5
RTOR Reduction (vph)	5	0
Lane Group Flow (vph)	0	0
Confl. Peds. (#/hr)	4	10
Heavy Vehicles (%)	0%	0%
Turn Type	Perm	
Protected Phases		
Permitted Phases	4	
Actuated Green, G (s)	0.6	
Effective Green, g (s)	0.6	
Actuated g/C Ratio	0.03	
Clearance Time (s)	5.0	
Vehicle Extension (s)	3.0	
Lane Grp Cap (vph)	36	
v/s Ratio Prot		
v/s Ratio Perm	c0.00	
v/c Ratio	0.00	
Uniform Delay, d1	11.0	
Progression Factor	1.00	
Incremental Delay, d2	0.0	
Delay (s)	11.0	
Level of Service	B	
Approach Delay (s)	11.0	
Approach LOS	B	
Intersection Summary		

Intersection						
Int Delay, s/veh	2.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	28	18	54	29	9	38
Future Vol, veh/h	28	18	54	29	9	38
Conflicting Peds, #/hr	0	0	0	4	4	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	34	22	66	35	11	46
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	156	88	0	0	105	0
Stage 1	88	-	-	-	-	-
Stage 2	68	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	840	976	-	-	1499	-
Stage 1	940	-	-	-	-	-
Stage 2	960	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	830	972	-	-	1493	-
Mov Cap-2 Maneuver	830	-	-	-	-	-
Stage 1	936	-	-	-	-	-
Stage 2	952	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	9.4	0		1.4		
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	880	1493	-	
HCM Lane V/C Ratio	-	-	0.064	0.007	-	
HCM Control Delay (s)	-	-	9.4	7.4	0	
HCM Lane LOS	-	-	A	A	A	
HCM 95th %tile Q(veh)	-	-	0.2	0	-	

HCM Signalized Intersection Capacity Analysis 1: S. Hamilton St & Montgomery St/Hooker Ave

One Key Development - Poughkeepsie

No-Build 2026_AM Peak Hour

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	18	79	5	10	232	104	8	60	6	33	45	31
Future Volume (vph)	18	79	5	10	232	104	8	60	6	33	45	31
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Lane Util. Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	1.00			1.00	0.97		1.00			0.99	
Flpb, ped/bikes	1.00	1.00			1.00	1.00		1.00			1.00	
Frt	1.00	0.99			1.00	0.85		0.99			0.96	
Flt Protected	0.95	1.00			1.00	1.00		0.99			0.99	
Satd. Flow (prot)	1696	1747			1859	1565		1714			1686	
Flt Permitted	0.57	1.00			0.99	1.00		0.95			0.86	
Satd. Flow (perm)	1024	1747			1839	1565		1636			1478	
Peak-hour factor, PHF	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Adj. Flow (vph)	23	100	6	13	294	132	10	76	8	42	57	39
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	23	106	0	0	307	132	0	94	0	0	138	0
Confl. Peds. (#/hr)	9		20	20		9	4		7	7		4
Heavy Vehicles (%)	6%	8%	0%	0%	2%	0%	13%	9%	0%	3%	7%	7%
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	6	1			5			3			3	
Permitted Phases	1			5		5	3			3		
Actuated Green, G (s)	19.7	19.7			14.0	14.0		7.4			7.4	
Effective Green, g (s)	19.7	19.7			14.0	14.0		7.4			7.4	
Actuated g/C Ratio	0.53	0.53			0.38	0.38		0.20			0.20	
Clearance Time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	556	927			693	590		326			294	
v/s Ratio Prot	0.00	c0.06										
v/s Ratio Perm	0.02				c0.17	0.08		0.06			c0.09	
v/c Ratio	0.04	0.11			0.44	0.22		0.29			0.47	
Uniform Delay, d1	4.2	4.3			8.6	7.9		12.6			13.1	
Progression Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.0	0.1			0.5	0.2		0.5			1.2	
Delay (s)	4.2	4.4			9.1	8.0		13.1			14.3	
Level of Service	A	A			A	A		B			B	
Approach Delay (s)		4.4			8.8			13.1			14.3	
Approach LOS		A			A			B			B	
Intersection Summary												
HCM 2000 Control Delay			9.5									
HCM 2000 Volume to Capacity ratio			0.45									
Actuated Cycle Length (s)			37.1									
Intersection Capacity Utilization			40.6%									
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

No-Build 2026_AM Peak Hour

												
Movement	EBL	EBT	EBR	WBL2	WBL	WBT	WBR	NBL	NBT	SBT	SBR	SBR2
Lane Configurations												
Traffic Volume (vph)	14	0	25	4	0	8	4	40	211	121	15	2
Future Volume (vph)	14	0	25	4	0	8	4	40	211	121	15	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0			5.0	5.0			5.0	5.0		
Lane Util. Factor		1.00			1.00	1.00			1.00	1.00		
Frpb, ped/bikes		0.97			1.00	0.99			1.00	1.00		
Flpb, ped/bikes		0.99			0.94	1.00			1.00	1.00		
Frt		0.91			1.00	0.95			1.00	0.98		
Flt Protected		0.98			0.95	1.00			0.99	1.00		
Satd. Flow (prot)		1557			1693	1653			1868	1826		
Flt Permitted		1.00			1.00	1.00			0.93	1.00		
Satd. Flow (perm)		1585			1782	1653			1747	1826		
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	15	0	27	4	0	9	4	44	232	133	16	2
RTOR Reduction (vph)	0	0	0	0	0	4	0	0	0	1	0	0
Lane Group Flow (vph)	0	42	0	0	4	9	0	0	276	150	0	0
Confl. Peds. (#/hr)	3		5	5	5		3	5			5	5
Heavy Vehicles (%)	7%	0%	4%	0%	0%	12%	0%	0%	1%	2%	2%	2%
Turn Type	Perm	NA		Perm	Perm	NA		Perm	NA	NA		
Protected Phases		1				1			3	3		
Permitted Phases	1			1	1			3				
Actuated Green, G (s)		2.2			2.2	2.2			11.2	11.2		
Effective Green, g (s)		2.2			2.2	2.2			11.2	11.2		
Actuated g/C Ratio		0.08			0.08	0.08			0.39	0.39		
Clearance Time (s)		5.0			5.0	5.0			5.0	5.0		
Vehicle Extension (s)		3.0			3.0	3.0			3.0	3.0		
Lane Grp Cap (vph)		120			135	125			674	705		
v/s Ratio Prot						0.01				0.08		
v/s Ratio Perm		c0.03			0.00				c0.16			
v/c Ratio		0.35			0.03	0.07			0.41	0.21		
Uniform Delay, d1		12.7			12.4	12.5			6.5	6.0		
Progression Factor		1.00			1.00	1.00			1.00	1.00		
Incremental Delay, d2		1.8			0.1	0.3			0.4	0.2		
Delay (s)		14.5			12.5	12.7			6.9	6.1		
Level of Service		B			B	B			A	A		
Approach Delay (s)		14.5				12.7			6.9	6.1		
Approach LOS		B				B			A	A		
Intersection Summary												
HCM 2000 Control Delay		7.5			HCM 2000 Level of Service				A			
HCM 2000 Volume to Capacity ratio		0.38										
Actuated Cycle Length (s)		29.0			Sum of lost time (s)				15.0			
Intersection Capacity Utilization		50.1%			ICU Level of Service				A			
Analysis Period (min)		15										
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

No-Build 2026_AM Peak Hour



Movement	NEL	NER2
Lane Configurations	↩	
Traffic Volume (vph)	2	1
Future Volume (vph)	2	1
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	5.0	
Lane Util. Factor	1.00	
Frpb, ped/bikes	1.00	
Flpb, ped/bikes	0.95	
Frt	0.95	
Flt Protected	0.97	
Satd. Flow (prot)	1638	
Flt Permitted	0.97	
Satd. Flow (perm)	1638	
Peak-hour factor, PHF	0.91	0.91
Adj. Flow (vph)	2	1
RTOR Reduction (vph)	3	0
Lane Group Flow (vph)	0	0
Confl. Peds. (#/hr)	5	
Heavy Vehicles (%)	2%	2%
Turn Type	Perm	
Protected Phases		
Permitted Phases	4	
Actuated Green, G (s)	0.6	
Effective Green, g (s)	0.6	
Actuated g/C Ratio	0.02	
Clearance Time (s)	5.0	
Vehicle Extension (s)	3.0	
Lane Grp Cap (vph)	33	
v/s Ratio Prot		
v/s Ratio Perm	c0.00	
v/c Ratio	0.00	
Uniform Delay, d1	13.9	
Progression Factor	1.00	
Incremental Delay, d2	0.0	
Delay (s)	13.9	
Level of Service	B	
Approach Delay (s)	13.9	
Approach LOS	B	
Intersection Summary		

Intersection						
Int Delay, s/veh	3.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	44	21	52	25	12	41
Future Vol, veh/h	44	21	52	25	12	41
Conflicting Peds, #/hr	2	0	0	2	2	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	86	86	86	86	86	86
Heavy Vehicles, %	7	10	6	8	0	3
Mvmt Flow	51	24	60	29	14	48
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	155	77	0	0	91	0
Stage 1	77	-	-	-	-	-
Stage 2	78	-	-	-	-	-
Critical Hdwy	6.47	6.3	-	-	4.1	-
Critical Hdwy Stg 1	5.47	-	-	-	-	-
Critical Hdwy Stg 2	5.47	-	-	-	-	-
Follow-up Hdwy	3.563	3.39	-	-	2.2	-
Pot Cap-1 Maneuver	825	962	-	-	1517	-
Stage 1	934	-	-	-	-	-
Stage 2	933	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	814	960	-	-	1514	-
Mov Cap-2 Maneuver	814	-	-	-	-	-
Stage 1	932	-	-	-	-	-
Stage 2	923	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	9.6	0		1.7		
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBL	SBT	
Capacity (veh/h)	-	-		856	1514	
HCM Lane V/C Ratio	-	-		0.088	0.009	
HCM Control Delay (s)	-	-		9.6	7.4	
HCM Lane LOS	-	-		A	A	
HCM 95th %tile Q(veh)	-	-		0.3	0	

HCM Signalized Intersection Capacity Analysis 1: S. Hamilton St & Montgomery St/Hooker Ave

One Key Development - Poughkeepsie

No-Build 2026_PM Peak Hour

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	35	153	6	8	194	70	3	63	4	64	84	29
Future Volume (vph)	35	153	6	8	194	70	3	63	4	64	84	29
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Lane Util. Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	1.00			1.00	0.97		1.00			1.00	
Flpb, ped/bikes	0.99	1.00			1.00	1.00		1.00			1.00	
Frt	1.00	0.99			1.00	0.85		0.99			0.98	
Flt Protected	0.95	1.00			1.00	1.00		1.00			0.98	
Satd. Flow (prot)	1795	1868			1843	1563		1880			1798	
Flt Permitted	0.62	1.00			0.98	1.00		0.98			0.85	
Satd. Flow (perm)	1173	1868			1811	1563		1847			1554	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	38	166	7	9	211	76	3	68	4	70	91	32
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	38	173	0	0	220	76	0	75	0	0	193	0
Confl. Peds. (#/hr)	11		9	9		11	8		4	4		8
Heavy Vehicles (%)	0%	1%	0%	0%	3%	0%	0%	0%	0%	1%	1%	0%
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	6	1			5			3			3	
Permitted Phases	1			5		5	3			3		
Actuated Green, G (s)	15.3	15.3			8.7	8.7		7.9			7.9	
Effective Green, g (s)	15.3	15.3			8.7	8.7		7.9			7.9	
Actuated g/C Ratio	0.46	0.46			0.26	0.26		0.24			0.24	
Clearance Time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	570	860			474	409		439			369	
v/s Ratio Prot	0.00	c0.09										
v/s Ratio Perm	0.03				c0.12	0.05		0.04			c0.12	
v/c Ratio	0.07	0.20			0.46	0.19		0.17			0.52	
Uniform Delay, d1	5.0	5.3			10.3	9.5		10.0			11.0	
Progression Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.0	0.1			0.7	0.2		0.2			1.3	
Delay (s)	5.0	5.4			11.0	9.7		10.2			12.3	
Level of Service	A	A			B	A		B			B	
Approach Delay (s)		5.4			10.7			10.2			12.3	
Approach LOS		A			B			B			B	
Intersection Summary												
HCM 2000 Control Delay			9.6									
HCM 2000 Volume to Capacity ratio			0.49									
Actuated Cycle Length (s)			33.2									
Intersection Capacity Utilization			49.8%									
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

No-Build 2026_PM Peak Hour

Movement	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	NBL2	NBL	NBT	SBT
Lane Configurations		↔				↔	↔				↔	↔
Traffic Volume (vph)	25	0	35	1	2	13	11	6	3	30	191	264
Future Volume (vph)	25	0	35	1	2	13	11	6	3	30	191	264
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0				5.0	5.0				5.0	5.0
Lane Util. Factor		1.00				1.00	1.00				1.00	1.00
Frpb, ped/bikes		0.98				1.00	0.99				1.00	1.00
Flpb, ped/bikes		1.00				0.98	1.00				1.00	1.00
Frt		0.92				1.00	0.95				1.00	0.99
Flt Protected		0.98				0.95	1.00				0.99	1.00
Satd. Flow (prot)		1671				1777	1788				1838	1853
Flt Permitted		0.86				1.00	1.00				0.91	1.00
Satd. Flow (perm)		1464				1870	1788				1689	1853
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	27	0	38	1	2	14	12	6	3	32	205	284
RTOR Reduction (vph)	0	58	0	0	0	0	5	0	0	0	0	2
Lane Group Flow (vph)	0	8	0	0	0	16	13	0	0	0	240	314
Confl. Peds. (#/hr)	2		2	2	2	2		2	2	2		
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	3%	1%
Turn Type	Perm	NA			Perm	Perm	NA		Perm	Perm	NA	NA
Protected Phases		1					1				3	3
Permitted Phases	1				1	1			3	3		
Actuated Green, G (s)		3.7				3.7	3.7				12.1	12.1
Effective Green, g (s)		3.7				3.7	3.7				12.1	12.1
Actuated g/C Ratio		0.12				0.12	0.12				0.38	0.38
Clearance Time (s)		5.0				5.0	5.0				5.0	5.0
Vehicle Extension (s)		3.0				3.0	3.0				3.0	3.0
Lane Grp Cap (vph)		171				219	210				648	711
v/s Ratio Prot							0.01					c0.17
v/s Ratio Perm		0.01				c0.01					0.14	
v/c Ratio		0.05				0.07	0.06				0.37	0.44
Uniform Delay, d1		12.3				12.4	12.4				7.0	7.2
Progression Factor		1.00				1.00	1.00				1.00	1.00
Incremental Delay, d2		0.1				0.1	0.1				0.4	0.4
Delay (s)		12.4				12.5	12.5				7.3	7.6
Level of Service		B				B	B				A	A
Approach Delay (s)		12.4					12.5				7.3	7.6
Approach LOS		B					B				A	A
Intersection Summary												
HCM 2000 Control Delay		8.3				HCM 2000 Level of Service				A		
HCM 2000 Volume to Capacity ratio		0.34										
Actuated Cycle Length (s)		31.5				Sum of lost time (s)				15.0		
Intersection Capacity Utilization		58.0%				ICU Level of Service				B		
Analysis Period (min)		15										

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

No-Build 2026_PM Peak Hour



Movement	SBR	SBR2	NEL2	NEL	NER2
Lane Configurations					
Traffic Volume (vph)	18	12	1	3	3
Future Volume (vph)	18	12	1	3	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900
Total Lost time (s)				5.0	
Lane Util. Factor				1.00	
Frpb, ped/bikes				0.98	
Flpb, ped/bikes				0.98	
Frt				0.94	
Flt Protected				0.97	
Satd. Flow (prot)				1672	
Flt Permitted				0.97	
Satd. Flow (perm)				1672	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	19	13	1	3	3
RTOR Reduction (vph)	0	0	0	7	0
Lane Group Flow (vph)	0	0	0	0	0
Confl. Peds. (#/hr)	2	2		2	2
Heavy Vehicles (%)	0%	0%	0%	0%	0%
Turn Type			Perm	Perm	
Protected Phases					
Permitted Phases			4	4	
Actuated Green, G (s)				0.7	
Effective Green, g (s)				0.7	
Actuated g/C Ratio				0.02	
Clearance Time (s)				5.0	
Vehicle Extension (s)				3.0	
Lane Grp Cap (vph)				37	
v/s Ratio Prot					
v/s Ratio Perm				0.00	
v/c Ratio				0.00	
Uniform Delay, d1				15.1	
Progression Factor				1.00	
Incremental Delay, d2				0.0	
Delay (s)				15.1	
Level of Service				B	
Approach Delay (s)				15.1	
Approach LOS				B	
Intersection Summary					

Intersection						
Int Delay, s/veh	2.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	33	25	66	54	8	69
Future Vol, veh/h	33	25	66	54	8	69
Conflicting Peds, #/hr	1	0	0	3	3	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	86	86	86	86	86	86
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	38	29	77	63	9	80
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	211	112	0	0	143	0
Stage 1	112	-	-	-	-	-
Stage 2	99	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	782	947	-	-	1452	-
Stage 1	918	-	-	-	-	-
Stage 2	930	-	-	-	-	-
Platoon blocked, %			-	-	-	-
Mov Cap-1 Maneuver	773	944	-	-	1448	-
Mov Cap-2 Maneuver	773	-	-	-	-	-
Stage 1	915	-	-	-	-	-
Stage 2	923	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	9.7	0	0.8			
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	838	1448	-	
HCM Lane V/C Ratio	-	-	0.08	0.006	-	
HCM Control Delay (s)	-	-	9.7	7.5	0	
HCM Lane LOS	-	-	A	A	A	
HCM 95th %tile Q(veh)	-	-	0.3	0	-	

HCM Signalized Intersection Capacity Analysis 1: S. Hamilton St & Montgomery St/Hooker Ave

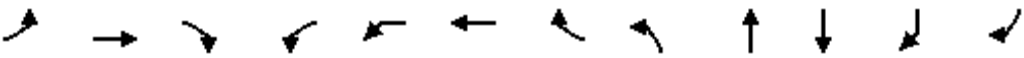
One Key Development - Poughkeepsie

No-Build 2026_SAT Midday Peak Hour

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	17	82	2	9	138	56	8	49	4	45	40	15
Future Volume (vph)	17	82	2	9	138	56	8	49	4	45	40	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Lane Util. Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	1.00			1.00	0.97		1.00			1.00	
Flpb, ped/bikes	0.99	1.00			1.00	1.00		1.00			1.00	
Frt	1.00	1.00			1.00	0.85		0.99			0.98	
Flt Protected	0.95	1.00			1.00	1.00		0.99			0.98	
Satd. Flow (prot)	1795	1839			1894	1565		1865			1792	
Flt Permitted	0.65	1.00			0.98	1.00		0.94			0.82	
Satd. Flow (perm)	1229	1839			1854	1565		1758			1505	
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	20	94	2	10	159	64	9	56	5	52	46	17
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	20	96	0	0	169	64	0	70	0	0	115	0
Confl. Peds. (#/hr)	11		9	9		11	8		4	4		8
Heavy Vehicles (%)	0%	3%	0%	0%	0%	0%	0%	0%	0%	0%	0%	7%
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	6	1			5			3			3	
Permitted Phases	1			5		5	3			3		
Actuated Green, G (s)	13.7	13.7			7.9	7.9		6.3			6.3	
Effective Green, g (s)	13.7	13.7			7.9	7.9		6.3			6.3	
Actuated g/C Ratio	0.46	0.46			0.26	0.26		0.21			0.21	
Clearance Time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	576	839			488	412		369			316	
v/s Ratio Prot	0.00	c0.05										
v/s Ratio Perm	0.01				c0.09	0.04		0.04			c0.08	
v/c Ratio	0.03	0.11			0.35	0.16		0.19			0.36	
Uniform Delay, d1	4.5	4.7			9.0	8.5		9.7			10.1	
Progression Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.0	0.1			0.4	0.2		0.3			0.7	
Delay (s)	4.5	4.7			9.4	8.7		10.0			10.9	
Level of Service	A	A			A	A		B			B	
Approach Delay (s)		4.7			9.2			10.0			10.9	
Approach LOS		A			A			B			B	
Intersection Summary												
HCM 2000 Control Delay			8.7									
HCM 2000 Volume to Capacity ratio			0.36									
Actuated Cycle Length (s)			30.0									
Intersection Capacity Utilization			36.7%									
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

No-Build 2026_SAT Midday Peak Hour

												
Movement	EBL	EBT	EBR	WBL2	WBL	WBT	WBR	NBL	NBT	SBT	SBR	SBR2
Lane Configurations		↔			↔	↔			↔	↔		
Traffic Volume (vph)	13	0	22	1	11	3	11	32	132	151	12	5
Future Volume (vph)	13	0	22	1	11	3	11	32	132	151	12	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0			5.0	5.0			5.0	5.0		
Lane Util. Factor		1.00			1.00	1.00			1.00	1.00		
Frpb, ped/bikes		0.95			1.00	0.97			1.00	1.00		
Flpb, ped/bikes		0.99			0.89	1.00			1.00	1.00		
Frt		0.91			1.00	0.88			1.00	0.99		
Flt Protected		0.98			0.95	1.00			0.99	1.00		
Satd. Flow (prot)		1613			1614	1616			1834	1851		
Flt Permitted		1.00			1.00	1.00			0.88	1.00		
Satd. Flow (perm)		1643			1699	1616			1639	1851		
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	14	0	24	1	12	3	12	35	145	166	13	5
RTOR Reduction (vph)	0	0	0	0	0	11	0	0	0	2	0	0
Lane Group Flow (vph)	0	38	0	0	13	4	0	0	180	182	0	0
Confl. Peds. (#/hr)	4		10	10	10		4	5			5	5
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	3%	1%	0%	0%
Turn Type	Perm	NA		Perm	Perm	NA		Perm	NA	NA		
Protected Phases		1				1			3	3		
Permitted Phases	1			1	1			3				
Actuated Green, G (s)		2.1			2.1	2.1			5.7	5.7		
Effective Green, g (s)		2.1			2.1	2.1			5.7	5.7		
Actuated g/C Ratio		0.09			0.09	0.09			0.24	0.24		
Clearance Time (s)		5.0			5.0	5.0			5.0	5.0		
Vehicle Extension (s)		3.0			3.0	3.0			3.0	3.0		
Lane Grp Cap (vph)		147			152	145			399	450		
v/s Ratio Prot						0.00				0.10		
v/s Ratio Perm		c0.02			0.01				c0.11			
v/c Ratio		0.26			0.09	0.03			0.45	0.41		
Uniform Delay, d1		9.9			9.8	9.7			7.5	7.4		
Progression Factor		1.00			1.00	1.00			1.00	1.00		
Incremental Delay, d2		0.9			0.2	0.1			0.8	0.6		
Delay (s)		10.9			10.0	9.8			8.3	8.0		
Level of Service		B			B	A			A	A		
Approach Delay (s)		10.9				9.9			8.3	8.0		
Approach LOS		B				A			A	A		
Intersection Summary												
HCM 2000 Control Delay		8.6			HCM 2000 Level of Service				A			
HCM 2000 Volume to Capacity ratio		0.37										
Actuated Cycle Length (s)		23.4			Sum of lost time (s)				15.0			
Intersection Capacity Utilization		47.1%			ICU Level of Service				A			
Analysis Period (min)		15										

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

No-Build 2026_SAT Midday Peak Hour



Movement	NEL	NER2
Lane Configurations		
Traffic Volume (vph)	0	5
Future Volume (vph)	0	5
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	5.0	
Lane Util. Factor	1.00	
Frpb, ped/bikes	0.87	
Flpb, ped/bikes	1.00	
Frt	0.86	
Flt Protected	1.00	
Satd. Flow (prot)	1422	
Flt Permitted	1.00	
Satd. Flow (perm)	1422	
Peak-hour factor, PHF	0.91	0.91
Adj. Flow (vph)	0	5
RTOR Reduction (vph)	5	0
Lane Group Flow (vph)	0	0
Confl. Peds. (#/hr)	4	10
Heavy Vehicles (%)	0%	0%
Turn Type	Perm	
Protected Phases		
Permitted Phases	4	
Actuated Green, G (s)	0.6	
Effective Green, g (s)	0.6	
Actuated g/C Ratio	0.03	
Clearance Time (s)	5.0	
Vehicle Extension (s)	3.0	
Lane Grp Cap (vph)	36	
v/s Ratio Prot		
v/s Ratio Perm	c0.00	
v/c Ratio	0.00	
Uniform Delay, d1	11.1	
Progression Factor	1.00	
Incremental Delay, d2	0.0	
Delay (s)	11.1	
Level of Service	B	
Approach Delay (s)	11.1	
Approach LOS	B	
Intersection Summary		

Intersection						
Int Delay, s/veh	2.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	28	18	55	29	9	39
Future Vol, veh/h	28	18	55	29	9	39
Conflicting Peds, #/hr	0	0	0	4	4	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	34	22	67	35	11	48
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	159	89	0	0	106	0
Stage 1	89	-	-	-	-	-
Stage 2	70	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	837	975	-	-	1498	-
Stage 1	940	-	-	-	-	-
Stage 2	958	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	827	971	-	-	1492	-
Mov Cap-2 Maneuver	827	-	-	-	-	-
Stage 1	936	-	-	-	-	-
Stage 2	950	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	9.4	0		1.4		
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBL	SBT	
Capacity (veh/h)	-	- 878		1492	-	
HCM Lane V/C Ratio	-	- 0.064		0.007	-	
HCM Control Delay (s)	-	- 9.4		7.4	0	
HCM Lane LOS	-	- A		A	A	
HCM 95th %tile Q(veh)	-	- 0.2		0	-	

HCM Signalized Intersection Capacity Analysis 1: S. Hamilton St & Montgomery St/Hooker Ave

One Key Development - Poughkeepsie

Build 2026_AM Peak Hour

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	18	79	7	10	232	104	14	69	6	33	48	31
Future Volume (vph)	18	79	7	10	232	104	14	69	6	33	48	31
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Lane Util. Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	1.00			1.00	0.97		1.00			0.99	
Flpb, ped/bikes	1.00	1.00			1.00	1.00		1.00			1.00	
Frt	1.00	0.99			1.00	0.85		0.99			0.96	
Flt Protected	0.95	1.00			1.00	1.00		0.99			0.99	
Satd. Flow (prot)	1696	1742			1859	1565		1708			1688	
Flt Permitted	0.57	1.00			0.99	1.00		0.92			0.86	
Satd. Flow (perm)	1024	1742			1839	1565		1583			1473	
Peak-hour factor, PHF	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Adj. Flow (vph)	23	100	9	13	294	132	18	87	8	42	61	39
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	23	109	0	0	307	132	0	113	0	0	142	0
Confl. Peds. (#/hr)	9		20	20		9	4		7	7		4
Heavy Vehicles (%)	6%	8%	0%	0%	2%	0%	13%	9%	0%	3%	7%	7%
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	6	1			5			3			3	
Permitted Phases	1			5		5	3			3		
Actuated Green, G (s)	19.8	19.8			14.1	14.1		7.5			7.5	
Effective Green, g (s)	19.8	19.8			14.1	14.1		7.5			7.5	
Actuated g/C Ratio	0.53	0.53			0.38	0.38		0.20			0.20	
Clearance Time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	556	924			695	591		318			296	
v/s Ratio Prot	0.00	c0.06										
v/s Ratio Perm	0.02				c0.17	0.08		0.07			c0.10	
v/c Ratio	0.04	0.12			0.44	0.22		0.36			0.48	
Uniform Delay, d1	4.2	4.4			8.7	7.9		12.8			13.2	
Progression Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.0	0.1			0.5	0.2		0.7			1.2	
Delay (s)	4.2	4.4			9.1	8.1		13.5			14.4	
Level of Service	A	A			A	A		B			B	
Approach Delay (s)		4.4			8.8			13.5			14.4	
Approach LOS		A			A			B			B	
Intersection Summary												
HCM 2000 Control Delay			9.7									
HCM 2000 Volume to Capacity ratio			0.46									
Actuated Cycle Length (s)			37.3									
Intersection Capacity Utilization			39.0%									
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

Build 2026_AM Peak Hour

Movement	EBL	EBT	EBR	WBL2	WBL	WBT	WBR	NBL	NBT	SBT	SBR	SBR2
Lane Configurations		↔			↔	↔			↔	↔		
Traffic Volume (vph)	14	0	27	4	0	8	4	46	211	121	15	2
Future Volume (vph)	14	0	27	4	0	8	4	46	211	121	15	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0			5.0	5.0			5.0	5.0		
Lane Util. Factor		1.00			1.00	1.00			1.00	1.00		
Frpb, ped/bikes		0.96			1.00	0.99			1.00	1.00		
Flpb, ped/bikes		0.99			0.94	1.00			1.00	1.00		
Frt		0.91			1.00	0.95			1.00	0.98		
Flt Protected		0.98			0.95	1.00			0.99	1.00		
Satd. Flow (prot)		1553			1692	1653			1866	1826		
Flt Permitted		1.00			1.00	1.00			0.92	1.00		
Satd. Flow (perm)		1578			1781	1653			1728	1826		
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	15	0	30	4	0	9	4	51	232	133	16	2
RTOR Reduction (vph)	0	0	0	0	0	4	0	0	0	1	0	0
Lane Group Flow (vph)	0	45	0	0	4	9	0	0	283	150	0	0
Confl. Peds. (#/hr)	3		5	5	5		3	5			5	5
Heavy Vehicles (%)	7%	0%	4%	0%	0%	12%	0%	0%	1%	2%	2%	2%
Turn Type	Perm	NA		Perm	Perm	NA		Perm	NA	NA		
Protected Phases		1				1			3	3		
Permitted Phases	1			1	1			3				
Actuated Green, G (s)		2.2			2.2	2.2			11.5	11.5		
Effective Green, g (s)		2.2			2.2	2.2			11.5	11.5		
Actuated g/C Ratio		0.08			0.08	0.08			0.39	0.39		
Clearance Time (s)		5.0			5.0	5.0			5.0	5.0		
Vehicle Extension (s)		3.0			3.0	3.0			3.0	3.0		
Lane Grp Cap (vph)		118			133	124			678	716		
v/s Ratio Prot						0.01				0.08		
v/s Ratio Perm		c0.03			0.00				c0.16			
v/c Ratio		0.38			0.03	0.08			0.42	0.21		
Uniform Delay, d1		12.9			12.6	12.6			6.5	5.9		
Progression Factor		1.00			1.00	1.00			1.00	1.00		
Incremental Delay, d2		2.1			0.1	0.3			0.4	0.1		
Delay (s)		15.0			12.7	12.9			6.9	6.0		
Level of Service		B			B	B			A	A		
Approach Delay (s)		15.0				12.8			6.9	6.0		
Approach LOS		B				B			A	A		
Intersection Summary												
HCM 2000 Control Delay			7.6									
HCM 2000 Volume to Capacity ratio			0.39									
Actuated Cycle Length (s)			29.3									
Intersection Capacity Utilization			50.6%									
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

Build 2026_AM Peak Hour



Movement	NEL	NER2
Lane Configurations	↩	
Traffic Volume (vph)	2	1
Future Volume (vph)	2	1
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	5.0	
Lane Util. Factor	1.00	
Frpb, ped/bikes	1.00	
Flpb, ped/bikes	0.95	
Frt	0.95	
Flt Protected	0.97	
Satd. Flow (prot)	1637	
Flt Permitted	0.97	
Satd. Flow (perm)	1637	
Peak-hour factor, PHF	0.91	0.91
Adj. Flow (vph)	2	1
RTOR Reduction (vph)	3	0
Lane Group Flow (vph)	0	0
Confl. Peds. (#/hr)	5	
Heavy Vehicles (%)	2%	2%
Turn Type	Perm	
Protected Phases		
Permitted Phases	4	
Actuated Green, G (s)	0.6	
Effective Green, g (s)	0.6	
Actuated g/C Ratio	0.02	
Clearance Time (s)	5.0	
Vehicle Extension (s)	3.0	
Lane Grp Cap (vph)	33	
v/s Ratio Prot		
v/s Ratio Perm	c0.00	
v/c Ratio	0.00	
Uniform Delay, d1	14.1	
Progression Factor	1.00	
Incremental Delay, d2	0.0	
Delay (s)	14.1	
Level of Service	B	
Approach Delay (s)	14.1	
Approach LOS	B	
Intersection Summary		

Intersection						
Int Delay, s/veh	3.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	44	23	55	25	18	51
Future Vol, veh/h	44	23	55	25	18	51
Conflicting Peds, #/hr	2	0	0	2	2	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	86	86	86	86	86	86
Heavy Vehicles, %	7	10	6	8	0	3
Mvmt Flow	51	27	64	29	21	59
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	184	81	0	0	95	0
Stage 1	81	-	-	-	-	-
Stage 2	103	-	-	-	-	-
Critical Hdwy	6.47	6.3	-	-	4.1	-
Critical Hdwy Stg 1	5.47	-	-	-	-	-
Critical Hdwy Stg 2	5.47	-	-	-	-	-
Follow-up Hdwy	3.563	3.39	-	-	2.2	-
Pot Cap-1 Maneuver	794	957	-	-	1512	-
Stage 1	930	-	-	-	-	-
Stage 2	909	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	780	955	-	-	1509	-
Mov Cap-2 Maneuver	780	-	-	-	-	-
Stage 1	928	-	-	-	-	-
Stage 2	894	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	9.8	0		1.9		
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBL	SBT	
Capacity (veh/h)	-	- 832		1509	-	
HCM Lane V/C Ratio	-	- 0.094		0.014	-	
HCM Control Delay (s)	-	- 9.8		7.4	0	
HCM Lane LOS	-	- A		A	A	
HCM 95th %tile Q(veh)	-	- 0.3		0	-	

Intersection						
Int Delay, s/veh	1.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	16	15	73	5	5	53
Future Vol, veh/h	16	15	73	5	5	53
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	7	0	0	2
Mvmt Flow	17	16	79	5	5	58
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	150	82	0	0	84	0
Stage 1	82	-	-	-	-	-
Stage 2	68	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	847	983	-	-	1526	-
Stage 1	946	-	-	-	-	-
Stage 2	960	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	844	983	-	-	1526	-
Mov Cap-2 Maneuver	844	-	-	-	-	-
Stage 1	946	-	-	-	-	-
Stage 2	957	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	9.1	0		0.6		
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBL	SBT	
Capacity (veh/h)	-	- 906		1526	-	
HCM Lane V/C Ratio	-	- 0.037		0.004	-	
HCM Control Delay (s)	-	- 9.1		7.4	0	
HCM Lane LOS	-	- A		A	A	
HCM 95th %tile Q(veh)	-	- 0.1		0	-	

HCM Signalized Intersection Capacity Analysis 1: S. Hamilton St & Montgomery St/Hooker Ave

One Key Development - Poughkeepsie

Build 2026_PM Peak Hour

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	35	153	11	8	194	70	6	68	4	64	93	29
Future Volume (vph)	35	153	11	8	194	70	6	68	4	64	93	29
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Lane Util. Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	1.00			1.00	0.97		1.00			1.00	
Flpb, ped/bikes	0.99	1.00			1.00	1.00		1.00			1.00	
Frt	1.00	0.99			1.00	0.85		0.99			0.98	
Flt Protected	0.95	1.00			1.00	1.00		1.00			0.98	
Satd. Flow (prot)	1795	1860			1842	1563		1877			1802	
Flt Permitted	0.62	1.00			0.98	1.00		0.96			0.85	
Satd. Flow (perm)	1173	1860			1811	1563		1808			1561	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	38	166	12	9	211	76	7	74	4	70	101	32
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	38	178	0	0	220	76	0	85	0	0	203	0
Confl. Peds. (#/hr)	11		9	9		11	8		4	4		8
Heavy Vehicles (%)	0%	1%	0%	0%	3%	0%	0%	0%	0%	1%	1%	0%
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	6	1			5			3			3	
Permitted Phases	1			5		5	3			3		
Actuated Green, G (s)	15.4	15.4			8.8	8.8		8.3			8.3	
Effective Green, g (s)	15.4	15.4			8.8	8.8		8.3			8.3	
Actuated g/C Ratio	0.46	0.46			0.26	0.26		0.25			0.25	
Clearance Time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	565	849			472	408		445			384	
v/s Ratio Prot	0.00	c0.10										
v/s Ratio Perm	0.03				c0.12	0.05		0.05			c0.13	
v/c Ratio	0.07	0.21			0.47	0.19		0.19			0.53	
Uniform Delay, d1	5.1	5.5			10.5	9.7		10.0			11.0	
Progression Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.1	0.1			0.7	0.2		0.2			1.3	
Delay (s)	5.2	5.6			11.2	9.9		10.3			12.3	
Level of Service	A	A			B	A		B			B	
Approach Delay (s)		5.5			10.9			10.3			12.3	
Approach LOS		A			B			B			B	
Intersection Summary												
HCM 2000 Control Delay			9.7									
HCM 2000 Volume to Capacity ratio			0.50									
Actuated Cycle Length (s)			33.7									
Intersection Capacity Utilization			50.5%									
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

Build 2026_PM Peak Hour

Movement	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	NBL2	NBL	NBT	SBT
Lane Configurations		↔				↔	↔				↔	↔
Traffic Volume (vph)	25	0	41	1	2	13	11	6	3	33	191	264
Future Volume (vph)	25	0	41	1	2	13	11	6	3	33	191	264
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0				5.0	5.0				5.0	5.0
Lane Util. Factor		1.00				1.00	1.00				1.00	1.00
Frpb, ped/bikes		0.98				1.00	0.99				1.00	1.00
Flpb, ped/bikes		1.00				0.98	1.00				1.00	1.00
Frt		0.92				1.00	0.95				1.00	0.99
Flt Protected		0.98				0.95	1.00				0.99	1.00
Satd. Flow (prot)		1663				1777	1788				1838	1853
Flt Permitted		0.87				1.00	1.00				0.91	1.00
Satd. Flow (perm)		1472				1870	1788				1679	1853
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	27	0	44	1	2	14	12	6	3	35	205	284
RTOR Reduction (vph)	0	64	0	0	0	0	5	0	0	0	0	2
Lane Group Flow (vph)	0	8	0	0	0	16	13	0	0	0	243	314
Confl. Peds. (#/hr)	2		2	2	2	2		2	2	2		
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	3%	1%
Turn Type	Perm	NA			Perm	Perm	NA		Perm	Perm	NA	NA
Protected Phases		1					1				3	3
Permitted Phases	1				1	1			3	3		
Actuated Green, G (s)		3.7				3.7	3.7				12.6	12.6
Effective Green, g (s)		3.7				3.7	3.7				12.6	12.6
Actuated g/C Ratio		0.12				0.12	0.12				0.39	0.39
Clearance Time (s)		5.0				5.0	5.0				5.0	5.0
Vehicle Extension (s)		3.0				3.0	3.0				3.0	3.0
Lane Grp Cap (vph)		170				216	206				661	729
v/s Ratio Prot							0.01					c0.17
v/s Ratio Perm		0.01				c0.01					0.14	
v/c Ratio		0.05				0.07	0.06				0.37	0.43
Uniform Delay, d1		12.6				12.6	12.6				6.9	7.1
Progression Factor		1.00				1.00	1.00				1.00	1.00
Incremental Delay, d2		0.1				0.1	0.1				0.3	0.4
Delay (s)		12.7				12.8	12.7				7.2	7.5
Level of Service		B				B	B				A	A
Approach Delay (s)		12.7					12.7				7.2	7.5
Approach LOS		B					B				A	A
Intersection Summary												
HCM 2000 Control Delay		8.3				HCM 2000 Level of Service				A		
HCM 2000 Volume to Capacity ratio		0.34										
Actuated Cycle Length (s)		32.0				Sum of lost time (s)				15.0		
Intersection Capacity Utilization		58.5%				ICU Level of Service				B		
Analysis Period (min)		15										

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

Build 2026_PM Peak Hour



Movement	SBR	SBR2	NEL2	NEL	NER2
Lane Configurations					
Traffic Volume (vph)	18	12	1	3	3
Future Volume (vph)	18	12	1	3	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900
Total Lost time (s)				5.0	
Lane Util. Factor				1.00	
Frpb, ped/bikes				0.98	
Flpb, ped/bikes				0.98	
Frt				0.94	
Flt Protected				0.97	
Satd. Flow (prot)				1671	
Flt Permitted				0.97	
Satd. Flow (perm)				1671	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	19	13	1	3	3
RTOR Reduction (vph)	0	0	0	7	0
Lane Group Flow (vph)	0	0	0	0	0
Confl. Peds. (#/hr)	2	2		2	2
Heavy Vehicles (%)	0%	0%	0%	0%	0%
Turn Type			Perm	Perm	
Protected Phases					
Permitted Phases			4	4	
Actuated Green, G (s)				0.7	
Effective Green, g (s)				0.7	
Actuated g/C Ratio				0.02	
Clearance Time (s)				5.0	
Vehicle Extension (s)				3.0	
Lane Grp Cap (vph)				36	
v/s Ratio Prot					
v/s Ratio Perm				0.00	
v/c Ratio				0.00	
Uniform Delay, d1				15.3	
Progression Factor				1.00	
Incremental Delay, d2				0.0	
Delay (s)				15.4	
Level of Service				B	
Approach Delay (s)				15.4	
Approach LOS				B	
Intersection Summary					

Intersection						
Int Delay, s/veh	2.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	33	31	75	54	11	74
Future Vol, veh/h	33	31	75	54	11	74
Conflicting Peds, #/hr	1	0	0	3	3	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	86	86	86	86	86	86
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	38	36	87	63	13	86
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	235	122	0	0	153	0
Stage 1	122	-	-	-	-	-
Stage 2	113	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	758	935	-	-	1440	-
Stage 1	908	-	-	-	-	-
Stage 2	917	-	-	-	-	-
Platoon blocked, %			-	-	-	-
Mov Cap-1 Maneuver	747	932	-	-	1436	-
Mov Cap-2 Maneuver	747	-	-	-	-	-
Stage 1	905	-	-	-	-	-
Stage 2	907	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	9.8	0	1			
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	826	1436	-	
HCM Lane V/C Ratio	-	-	0.09	0.009	-	
HCM Control Delay (s)	-	-	9.8	7.5	0	
HCM Lane LOS	-	-	A	A	A	
HCM 95th %tile Q(veh)	-	-	0.3	0	-	

Intersection						
Int Delay, s/veh	1.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	8	8	91	15	14	77
Future Vol, veh/h	8	8	91	15	14	77
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	9	9	99	16	15	84
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	221	107	0	0	115	0
Stage 1	107	-	-	-	-	-
Stage 2	114	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	772	953	-	-	1487	-
Stage 1	922	-	-	-	-	-
Stage 2	916	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	764	953	-	-	1487	-
Mov Cap-2 Maneuver	764	-	-	-	-	-
Stage 1	922	-	-	-	-	-
Stage 2	906	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	9.3	0		1.1		
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBL	SBT	
Capacity (veh/h)	-	- 848		1487	-	
HCM Lane V/C Ratio	-	- 0.021		0.01	-	
HCM Control Delay (s)	-	- 9.3		7.4	0	
HCM Lane LOS	-	- A		A	A	
HCM 95th %tile Q(veh)	-	- 0.1		0	-	

HCM Signalized Intersection Capacity Analysis 1: S. Hamilton St & Montgomery St/Hooker Ave

One Key Development - Poughkeepsie

Build 2026_SAT Midday Peak Hour

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	17	82	4	9	138	56	10	51	4	45	45	15
Future Volume (vph)	17	82	4	9	138	56	10	51	4	45	45	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Lane Util. Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	1.00			1.00	0.97		1.00			1.00	
Flpb, ped/bikes	0.99	1.00			1.00	1.00		1.00			1.00	
Frt	1.00	0.99			1.00	0.85		0.99			0.98	
Flt Protected	0.95	1.00			1.00	1.00		0.99			0.98	
Satd. Flow (prot)	1795	1831			1894	1565		1864			1797	
Flt Permitted	0.65	1.00			0.98	1.00		0.93			0.83	
Satd. Flow (perm)	1229	1831			1853	1565		1741			1516	
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	20	94	5	10	159	64	11	59	5	52	52	17
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	20	99	0	0	169	64	0	75	0	0	121	0
Confl. Peds. (#/hr)	11		9	9		11	8		4	4		8
Heavy Vehicles (%)	0%	3%	0%	0%	0%	0%	0%	0%	0%	0%	0%	7%
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	6	1			5			3			3	
Permitted Phases	1			5		5	3			3		
Actuated Green, G (s)	13.7	13.7			7.9	7.9		6.4			6.4	
Effective Green, g (s)	13.7	13.7			7.9	7.9		6.4			6.4	
Actuated g/C Ratio	0.46	0.46			0.26	0.26		0.21			0.21	
Clearance Time (s)	5.0	5.0			5.0	5.0		5.0			5.0	
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	574	833			486	410		370			322	
v/s Ratio Prot	0.00	c0.05										
v/s Ratio Perm	0.01				c0.09	0.04		0.04			c0.08	
v/c Ratio	0.03	0.12			0.35	0.16		0.20			0.38	
Uniform Delay, d1	4.5	4.7			9.0	8.5		9.8			10.1	
Progression Factor	1.00	1.00			1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.0	0.1			0.4	0.2		0.3			0.7	
Delay (s)	4.6	4.8			9.4	8.7		10.0			10.9	
Level of Service	A	A			A	A		B			B	
Approach Delay (s)		4.8			9.2			10.0			10.9	
Approach LOS		A			A			B			B	
Intersection Summary												
HCM 2000 Control Delay		8.7			HCM 2000 Level of Service			A				
HCM 2000 Volume to Capacity ratio		0.37										
Actuated Cycle Length (s)		30.1			Sum of lost time (s)			15.0				
Intersection Capacity Utilization		36.7%			ICU Level of Service			A				
Analysis Period (min)		15										
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

Build 2026_SAT Midday Peak Hour

												
Movement	EBL	EBT	EBR	WBL2	WBL	WBT	WBR	NBL	NBT	SBT	SBR	SBR2
Lane Configurations												
Traffic Volume (vph)	13	0	25	1	11	3	11	34	132	151	12	5
Future Volume (vph)	13	0	25	1	11	3	11	34	132	151	12	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0			5.0	5.0			5.0	5.0		
Lane Util. Factor		1.00			1.00	1.00			1.00	1.00		
Frpb, ped/bikes		0.95			1.00	0.97			1.00	1.00		
Flpb, ped/bikes		0.99			0.89	1.00			1.00	1.00		
Frt		0.91			1.00	0.88			1.00	0.99		
Flt Protected		0.98			0.95	1.00			0.99	1.00		
Satd. Flow (prot)		1606			1615	1616			1834	1851		
Flt Permitted		1.00			1.00	1.00			0.88	1.00		
Satd. Flow (perm)		1634			1700	1616			1630	1851		
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	14	0	27	1	12	3	12	37	145	166	13	5
RTOR Reduction (vph)	0	0	0	0	0	11	0	0	0	2	0	0
Lane Group Flow (vph)	0	41	0	0	13	4	0	0	182	182	0	0
Confl. Peds. (#/hr)	4		10	10	10		4	5			5	5
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	3%	1%	0%	0%
Turn Type	Perm	NA		Perm	Perm	NA		Perm	NA	NA		
Protected Phases		1				1			3	3		
Permitted Phases	1			1	1			3				
Actuated Green, G (s)		2.1			2.1	2.1			5.7	5.7		
Effective Green, g (s)		2.1			2.1	2.1			5.7	5.7		
Actuated g/C Ratio		0.09			0.09	0.09			0.24	0.24		
Clearance Time (s)		5.0			5.0	5.0			5.0	5.0		
Vehicle Extension (s)		3.0			3.0	3.0			3.0	3.0		
Lane Grp Cap (vph)		146			152	145			397	450		
v/s Ratio Prot						0.00				0.10		
v/s Ratio Perm		c0.03			0.01				c0.11			
v/c Ratio		0.28			0.09	0.03			0.46	0.41		
Uniform Delay, d1		9.9			9.8	9.7			7.5	7.4		
Progression Factor		1.00			1.00	1.00			1.00	1.00		
Incremental Delay, d2		1.1			0.2	0.1			0.8	0.6		
Delay (s)		11.0			10.0	9.8			8.4	8.0		
Level of Service		B			B	A			A	A		
Approach Delay (s)		11.0				9.9			8.4	8.0		
Approach LOS		B				A			A	A		
Intersection Summary												
HCM 2000 Control Delay		8.6			HCM 2000 Level of Service				A			
HCM 2000 Volume to Capacity ratio		0.38										
Actuated Cycle Length (s)		23.4			Sum of lost time (s)				15.0			
Intersection Capacity Utilization		47.5%			ICU Level of Service				A			
Analysis Period (min)		15										
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis One Key Development - Poughkeepsie

Build 2026_SAT Midday Peak Hour



Movement	NEL	NER2
Lane Configurations	↰↱	
Traffic Volume (vph)	0	5
Future Volume (vph)	0	5
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	5.0	
Lane Util. Factor	1.00	
Frpb, ped/bikes	0.87	
Flpb, ped/bikes	1.00	
Frt	0.86	
Flt Protected	1.00	
Satd. Flow (prot)	1422	
Flt Permitted	1.00	
Satd. Flow (perm)	1422	
Peak-hour factor, PHF	0.91	0.91
Adj. Flow (vph)	0	5
RTOR Reduction (vph)	5	0
Lane Group Flow (vph)	0	0
Confl. Peds. (#/hr)	4	10
Heavy Vehicles (%)	0%	0%
Turn Type	Perm	
Protected Phases		
Permitted Phases	4	
Actuated Green, G (s)	0.6	
Effective Green, g (s)	0.6	
Actuated g/C Ratio	0.03	
Clearance Time (s)	5.0	
Vehicle Extension (s)	3.0	
Lane Grp Cap (vph)	36	
v/s Ratio Prot		
v/s Ratio Perm	c0.00	
v/c Ratio	0.00	
Uniform Delay, d1	11.1	
Progression Factor	1.00	
Incremental Delay, d2	0.0	
Delay (s)	11.1	
Level of Service	B	
Approach Delay (s)	11.1	
Approach LOS	B	
Intersection Summary		

Intersection						
Int Delay, s/veh	2.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	28	21	60	29	11	42
Future Vol, veh/h	28	21	60	29	11	42
Conflicting Peds, #/hr	0	0	0	4	4	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	34	26	73	35	13	51
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	172	95	0	0	112	0
Stage 1	95	-	-	-	-	-
Stage 2	77	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	823	967	-	-	1490	-
Stage 1	934	-	-	-	-	-
Stage 2	951	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	812	963	-	-	1484	-
Mov Cap-2 Maneuver	812	-	-	-	-	-
Stage 1	930	-	-	-	-	-
Stage 2	942	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	9.4	0		1.5		
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBL	SBT	
Capacity (veh/h)	-	- 870		1484	-	
HCM Lane V/C Ratio	-	- 0.069		0.009	-	
HCM Control Delay (s)	-	- 9.4		7.4	0	
HCM Lane LOS	-	- A		A	A	
HCM 95th %tile Q(veh)	-	- 0.2		0	-	

Intersection						
Int Delay, s/veh	0.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	5	4	73	8	7	49
Future Vol, veh/h	5	4	73	8	7	49
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	5	4	79	9	8	53
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	153	84	0	0	88	0
Stage 1	84	-	-	-	-	-
Stage 2	69	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	843	981	-	-	1520	-
Stage 1	944	-	-	-	-	-
Stage 2	959	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	839	981	-	-	1520	-
Mov Cap-2 Maneuver	839	-	-	-	-	-
Stage 1	944	-	-	-	-	-
Stage 2	954	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	9.1	0		0.9		
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	897	1520	-	
HCM Lane V/C Ratio	-	-	0.011	0.005	-	
HCM Control Delay (s)	-	-	9.1	7.4	0	
HCM Lane LOS	-	-	A	A	A	
HCM 95th %tile Q(veh)	-	-	0	0	-	

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1.0 INTRODUCTION

1.1 Project Summary

The O'Neill Group Hamilton (hereinafter referred to as "The Applicant") is seeking site plan approval from the City of Poughkeepsie, to develop a 58-unit, mid-rise multifamily residential development with surface-level parking (hereinafter referred to as the "Proposed Action"). The Proposed Action is located on an approximately 3.378-acre parcel of vacant land on the west side of South Hamilton Street (Tax Lot 13130-6161-30-151783), (hereinafter referred to as the "Site"). The Proposed Action includes construction of a three-story building with 29 one-bedroom, and 29 two-bedroom apartments supported by 102 off-street parking spaces, stormwater management facilities, utility infrastructure, landscaping and associated site improvements. The perimeter of the Site is wooded, and interior contains predominantly small brush.

The Site is located within the R-4, Medium High Density Zoning District. Multi-family residential development is a permitted principal use within the R-4 zone. Infrastructural improvements include a looped municipal water connection from South Hamilton Street to Dwight Street and a tie-in to the exiting 8" sanitary sewer main on South Hamilton Street. The proposed ground disturbance is approximately 3.00 acres. Since more than 1 acre of ground disturbance will occur, the Proposed Action will also require a SPDES General Permit for stormwater.

1.2 Background

The Site is comprised of approximately 3.378 acres and is zoned to accommodate the Proposed Action. The Site is comprised of one parcel, identified as follows:

- Taxparcel:131300-6161-30-151783 (South Hamilton Street)

Multi-family residential development is classified as a permitted use within the R-4 zone, subject to the applicable requirements. The proposed project will comply with all of the lot, bulk, and design requirements and standards included in ZS 19-3.15(4)(c), ZS 19-3.15(4)(i), ZS 19-4.11(1)(b), ZS 19-4.3(1) and, ZS 19-4.3(5)(a).

1.3 Site and Surrounding Area Description

The Site is currently undeveloped and wooded. The land uses located within 1,000 feet of the project site are characterized by single and multi-family dwellings and a mix of neighborhood retail and small commercial offices.

The Site is not located within a floodway, 100-year floodplain, or 500-year floodplain. The Site is also not located over an aquifer.

1.4 Approvals, Consultations and Referrals

Table 1 provides a list of the approvals/permits that are anticipated for the Proposed Action.

Table 1	
Anticipated Approvals and Permits	
City of Poughkeepsie Planning Board	Site Plan Approval
City of Poughkeepsie Department of Public Works	Water and Sewer Hook-Ups
Dutchess County Department of Health	Water
NYS Department of Environmental Conservation	SPDES General Permit GP-0-020-001

2.0 CONSERVATION ANALYSIS

The Proposed Action has been prepared in accordance with §19.64 of the City Code – *Natural Resources*. The Code requires the preparation of a conservation analysis based on the data and information contained in the 2019 Natural Resources Inventory (NRI). The format of this analysis is based on the eight chapters of the NRI; Geology and Soils, Water Resources, Biological Communities and Habitats, Land Use, Recreational Resources, Historical Resources and Climate Change.

2.1 Geology and Soils

Figure 1¹ presents the bedrock geology of the Site, which has been identified as Graywacke, shale (Oag), which is a coarse-grained sandstone.

The Site's surficial geology consists of glacial till deposited over shallow bedrock (Figure 2). Soils on the Site are classified as Dutchess Cardigan Urban Land Complex (Figure 3). This soil unit consists of very deep, well drained loamy soils formed in glacial till. Folded shale bedrock occurs within 20 - 40 inches of the surface. Slope, frost action and variable depth to bedrock are the main limitations of this soil unit.

Table 2 Soil Characteristics						
% of Site	Symbol	Soil Type	Erosion Hazard	Permeability	Depth to Water Table	Depth to Bedrock
100%	DxB	Dutchess-Cardigan-Urban Land Complex	Moderate	Moderate	>6'	20" – 60"

The Soil Survey² reports that the water table is typically at a depth of six feet or more year-round where these soils are present. Any groundwater encountered during the construction will be pumped from the open excavations and discharged to approved sediment trapping devices in accordance with NYSDEC guidelines.

2.2 Water Resources

As documented in Figure 4 (Surface Water and Watersheds), the Site is located within two watersheds. The majority of the Site is located within the Hudson River Direct Drainage E Watershed, while a small portion of the center of the Site is located within the Fall Kill Watershed. No surface water features are present in or around the Site, and it does not sit atop an aquifer.

Figure 5 (Wetlands) confirms that no wetlands are present on the Site. During the geotechnical investigation of the Site on June 14, 2023, no standing water was observed, which is indicative of the

¹ All maps in this report are reproduced from the Natural resources Inventory City of Poughkeepsie, 2019.

² Soil Survey of Dutchess County, New York, US Dept. of Agriculture, Natural Resources Conservation Service.

well-drained on-site soils and topography of the Site. Also, there was no evidence of wetland vegetation in the slight depressions located on the Site.

Figure 6 (Flood Hazard Areas and Riparian Buffer Areas) indicates that no floodplains, flood hazard areas or riparian buffer areas are located on the Site. And finally, Figure 7 (Potential and Known Sources of Pollution) documents that the Site is not constrained by existing sources of water pollution.

The Proposed Action will involve greater than one acre of ground disturbance; therefore, coverage under the State Pollutant Discharge Elimination System (SPDES) General Permit for Stormwater Discharges from Construction Activities is required. A Stormwater Pollution Prevention Plan (SWPPP) has been prepared in conformance with the most current New York State Stormwater Management Design Manual and New York State Standards and Specifications for Erosion and Sediment Control. With the implementation of the SWPPP, no significant adverse impacts to wetlands and watercourses located off site will occur.

2.3 Biological Communities and Habitat

As depicted on Figure 8 (Significant Habitats), the Site was identified as supporting the Upland Hardwood Forest cover type. The majority of this cover type was removed by a prior owner of the property. Today, the majority of the Site supports cleared meadow.

According to the NYSDEC Environmental Resource Mapper (Figure 9 – Rare and Endangered Animals and Plants), there are known occurrences of the bald eagle in the vicinity of the Site. According to Habitat Assessments conducted by Ecological Solutions, LLC in 2020 and 2023, no habitats that would support bald eagles exist on the Site.

An inquiry to Region 3 NYSDEC Bureau of Wildlife was made on September 28, 2023. The Agency confirmed that the Proposed action would have no impact on bald eagle habitat. The locus for the bald eagle occurrence records is in the vicinity of the Poughkeepsie Rural Cemetery, which is separated from the Site by NYS Route 9 and the intervening development.

Figure 10 (Habitat Envelopes and Potential Corridors) identifies the Site as a “significant habitat area.” This designation reflects the characteristics of the Site prior to the clearance of the Upland Hardwood Forest. Absent this cover type, the Site no longer provides significant habitat.

2.4 Land Use

The Site is located within the R-4, Medium Density Zoning District. As set forth in §19-3.15 (1), the purpose of this district is:

“...to provide areas for a broad range of housing types with opportunity for appropriate nonresidential uses at a fairly high density commensurate with the scale and characteristics of the city’s older neighborhoods; these are areas with access to major

transportation arteries and a range of community services.”

The Proposed Action fully complies with the applicable R-4 zoning district bulk, area, height and dimensional requirements (Table 3).

Table 3		
R-4 Medium Density Zoning District – Project Compliance		
Zoning Provision	Required	Proposed
Minimum Lot Area	145,000 SqFt	147,145.7 SqFt
Minimum Lot Width	50 Ft	346.3 Ft
Minimum Lot Frontage	30 Ft	346.3 Ft
Front Yard Setback	35 Ft	40.9 Ft
Side Yard Setback	20 Ft	20 Ft (Left Side) 65 Ft (Right Side)
Rear Yard Setback	20 Ft	240 Ft
Usable Open Space	40%	52.2%
Max. Building Height	40 Ft	40 Ft
Lot Coverage	40%	15.4%

As illustrated on Figure 11, the land use surrounding the Site is predominately residential, with the notable exception of a community service use (New Horizons Resources) that is immediately adjacent to the Site.

The Proposed Action’s full compliance with the R-4 zoning district requirements, and its conformity with the existing character of the land uses surrounding the Site demonstrates that the Proposed Action is consistent with, and will strengthen and reinforce the City of Poughkeepsie’s land use pattern.

2.5 Recreational Resources

While the Site is currently vacant and undeveloped, it is privately owned, and does not currently afford any public recreational opportunities.

The Proposed Action will result in new residents that will result in a proportional increase in the demand for recreational resources. The Proposed Action incorporates an array of new recreational amenities including open spaces, a walking trail around the entire perimeter of the Site, play and sitting areas, totaling 26,700 square feet. These amenities will off-set the demand on public recreational resources.

In addition, the Site is very near to Bartett Park, and is a short walk to other public recreational amenities such as Eastman Park, the Springside Historic Landmark, Hulme Park, among others (Figure 12).

Finally, the Proposed Action will result in a significant increase in taxes, which can be used by the City to support recreational programs and activities.

2.6 Historic Resources

As noted in Figure 13, the Site substantially contiguous to the Dwight Street-Hooker Avenue Historic District. This district was created in 1982 to preserve the architectural integrity of 17 contributing residential buildings that were built between 1895 and 1915, and reflect a variety of revival styles.

The Proposed Action will not physically impact the Historic District, and upon completion, limited views of the new building would be possible from the homes in the northwest corner of the District.

The project site is not located within a known archaeologically sensitive area. A consultation request has been submitted to NYSOPRHP.

2.7 Climate Change

The Proposed Action will convert a vacant formerly wooded site, into a well-designed, fully compliant multi-family development. To mitigate the Proposed Action's impact on climate change, a number of measures have been incorporated. To restore the tree canopy that was removed by the prior owner of the property, a very robust landscaping plan is proposed consisting of the following plantings:

Shade Trees

- Red Maple
- Columnar Hornbeam
- Honey Locust
- London Plane Tree
- Japanese Zelkova

Ornamental Trees

- Shadblow Serviceberry
- River Birch
- Eastern Redbud
- Winter King Hawthorne

Shrubs

- Summersweet
- Ink Berry
- Sea Green Juniper
- Bar Harbor Juniper
- Blue Pfitzer Juniper
- Dwarf Mugo Pine
- Anthony waterer Spirea

The proposed stormwater management plan that has been developed for the Proposed Action, includes several green practices as recommended in the New York State Stormwater Management Design Manual. The building will be constructed to comply with current stringent building code requirements, and

will include measures such as utilizing water efficient fixtures and appliances, such as *EnergyStar*, the utilization of high efficiency heating and air conditioning equipment, the installation of high performing building materials, insulation and windows, and the use of recycled materials when possible. These measures taken collectively, will minimize the Proposed Action's impact on Climate Change.

3.0 CONCLUSION

The Site is identified in the Natural Resource Inventory as a property that supports an upland hardwood forest. This forest patch was substantially cleared by the previous owner, and no longer provides the ecological benefits of that cover type.

The proposed project involves the development of a 3.378-acre formerly wooded and undeveloped site, into a 58-unit, 3-story mid-rise multifamily residential development with surface-level parking. The site is zoned for the proposed use and has been modified from its natural condition by the construction of the adjoining uses and roadways and historic district. The Site does not contain wetlands or watercourses and the NYSDEC has been consulted with regard to the bald eagle concern. A consultation with NYSOPRHP is underway. For these reasons, the Proposed Action is consistent with the policy objectives of the Natural Resources Inventory for the City of Poughkeepsie.

FIGURES

Figure 1

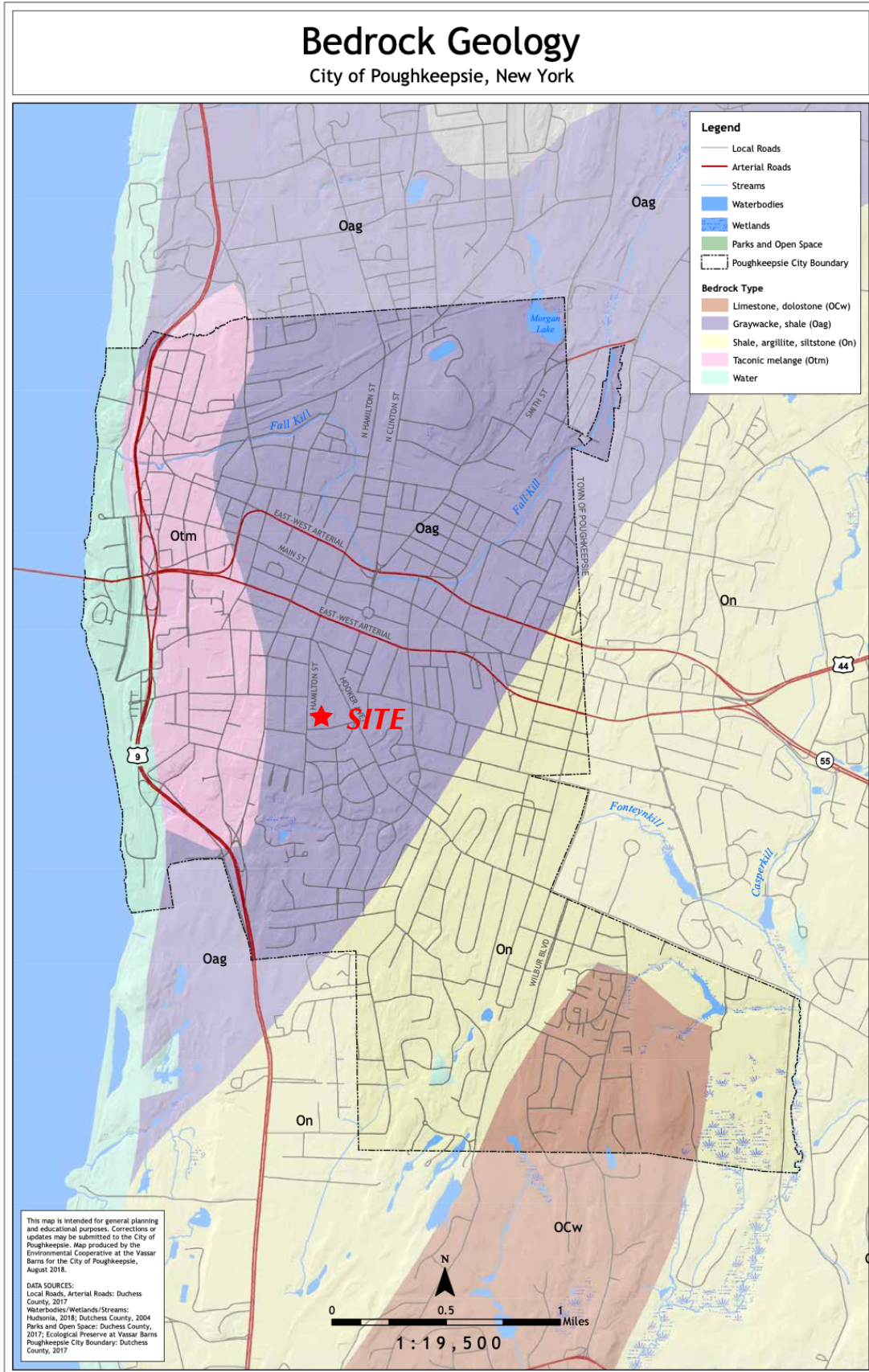


Figure 2

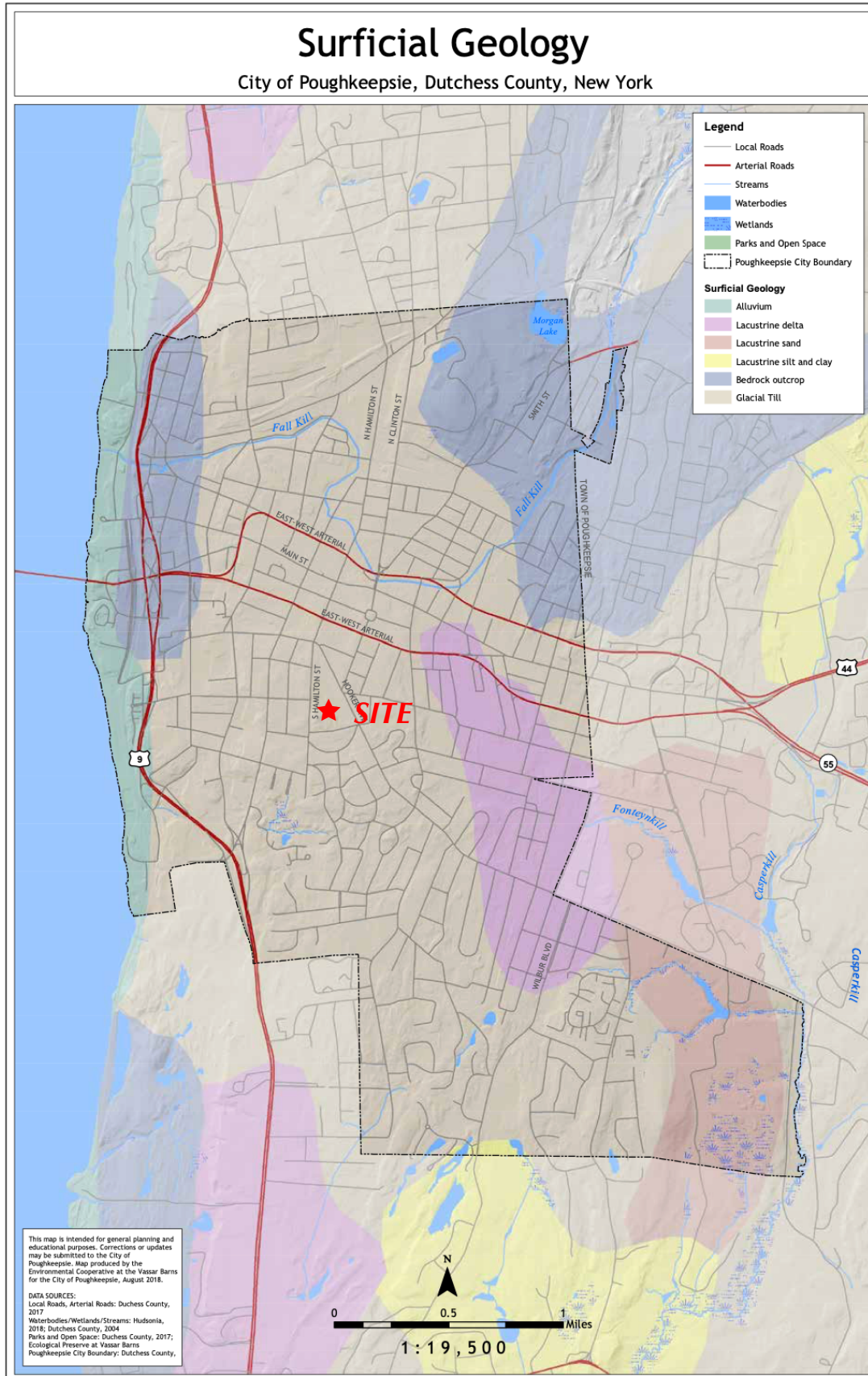


Figure 3

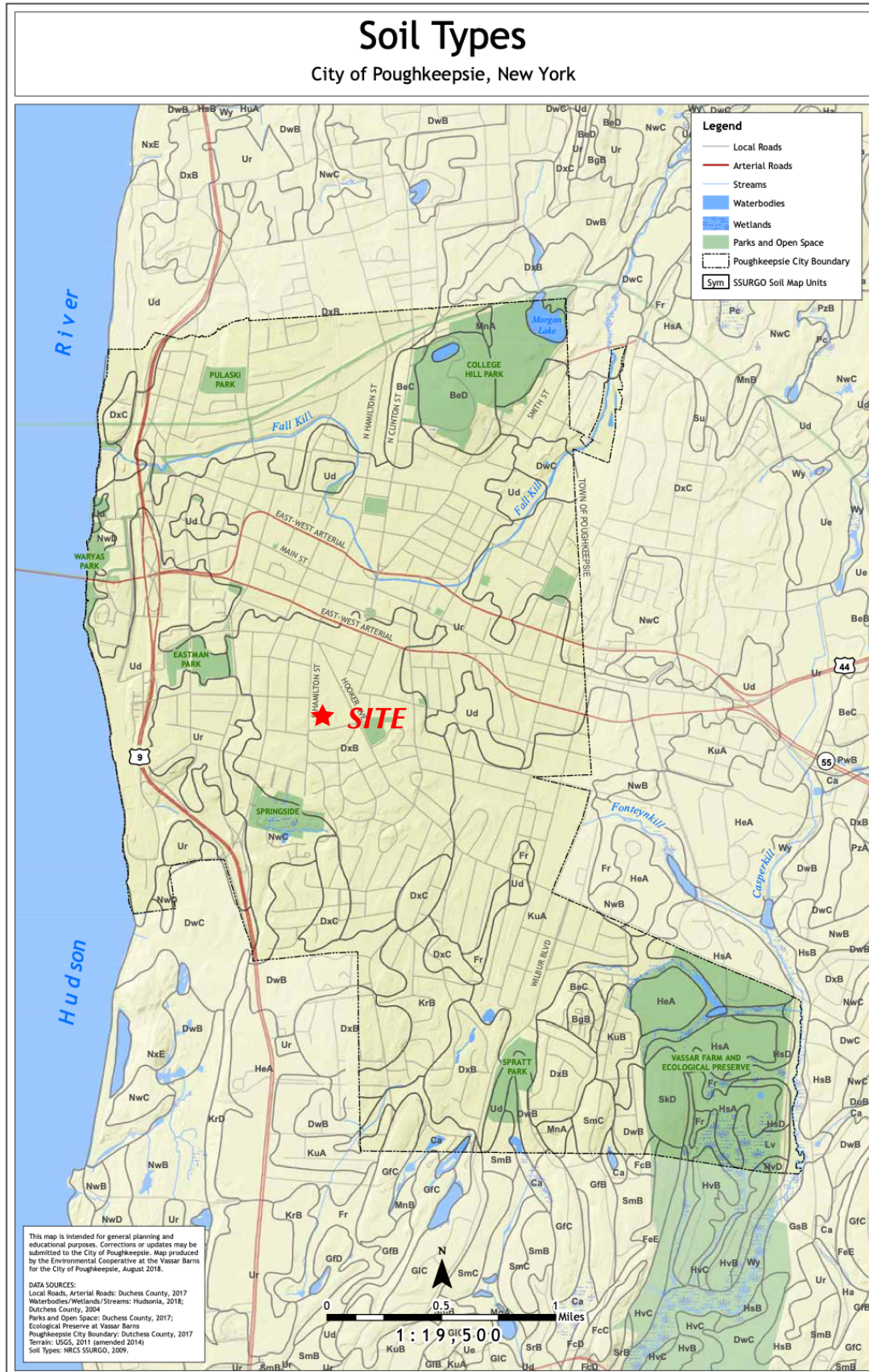


Figure 4

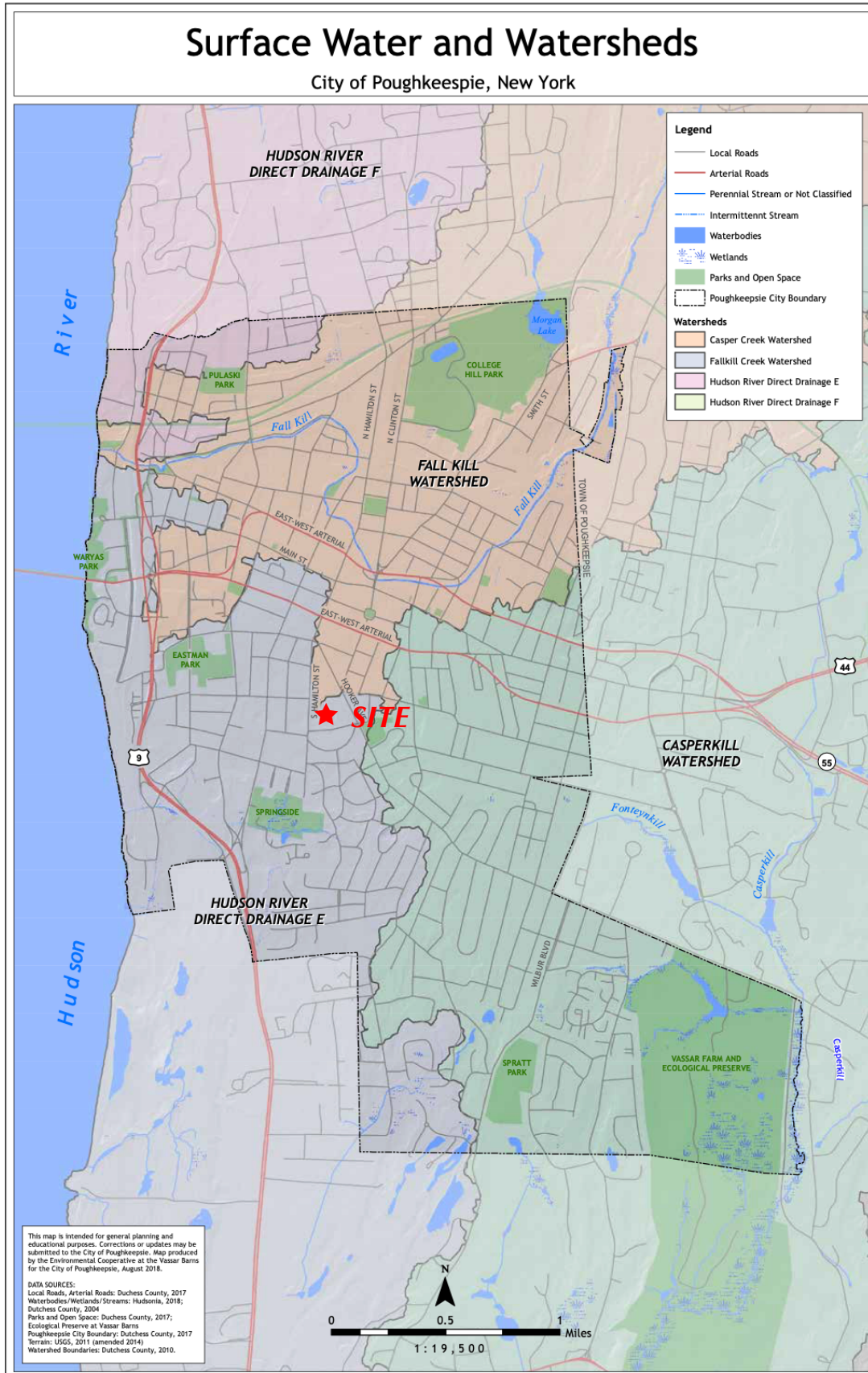


Figure 5

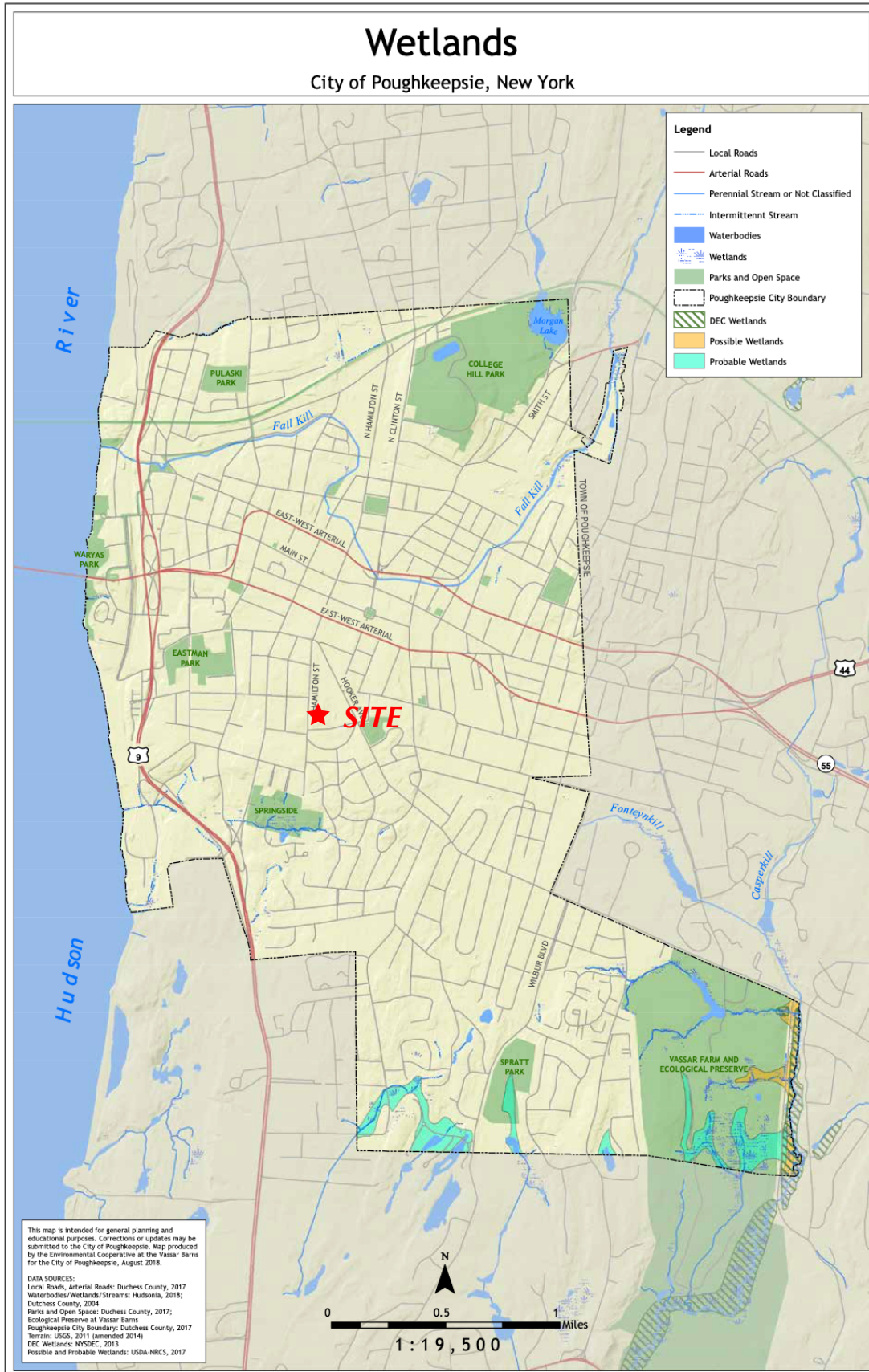


Figure 6

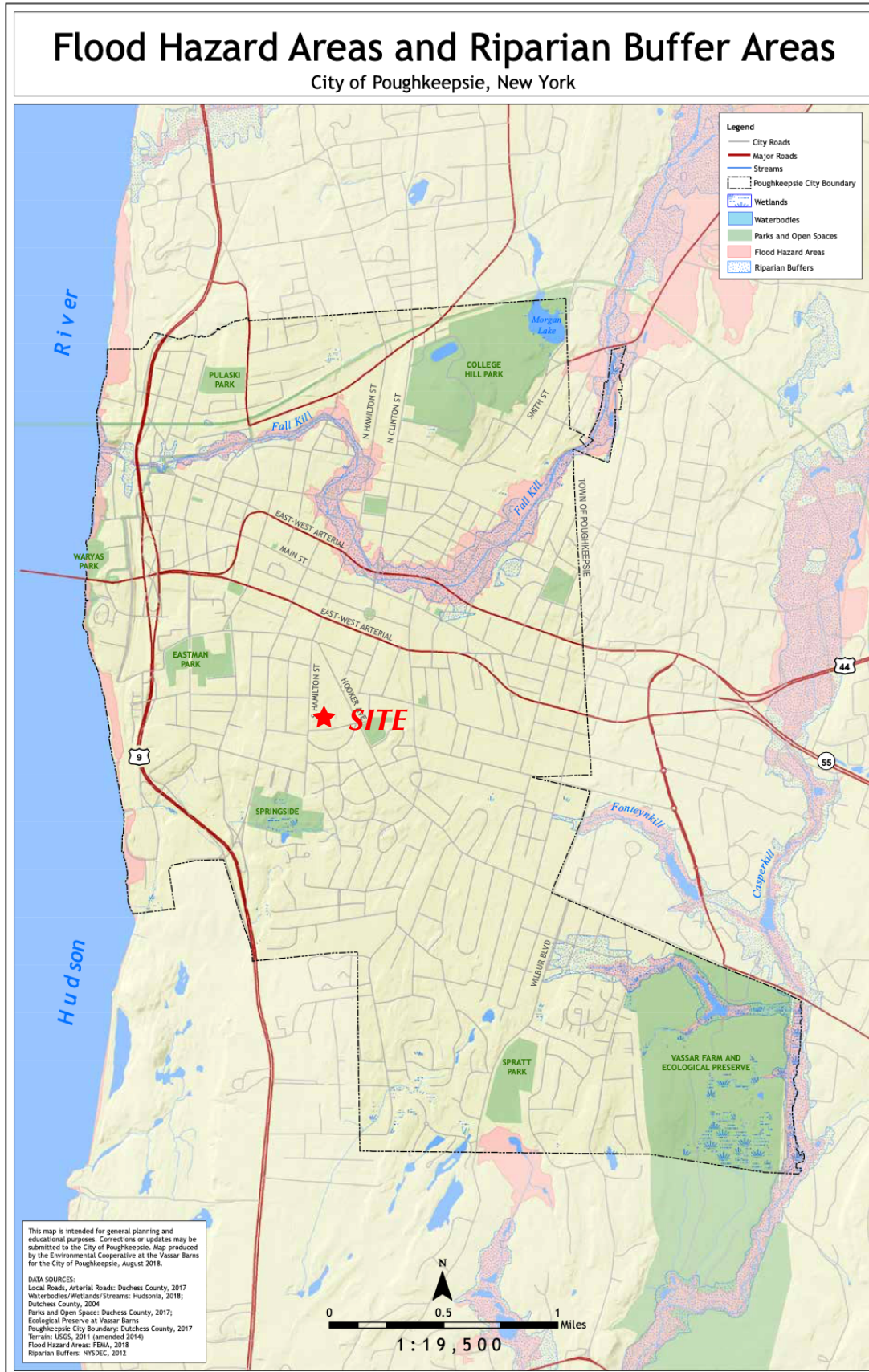


Figure 7

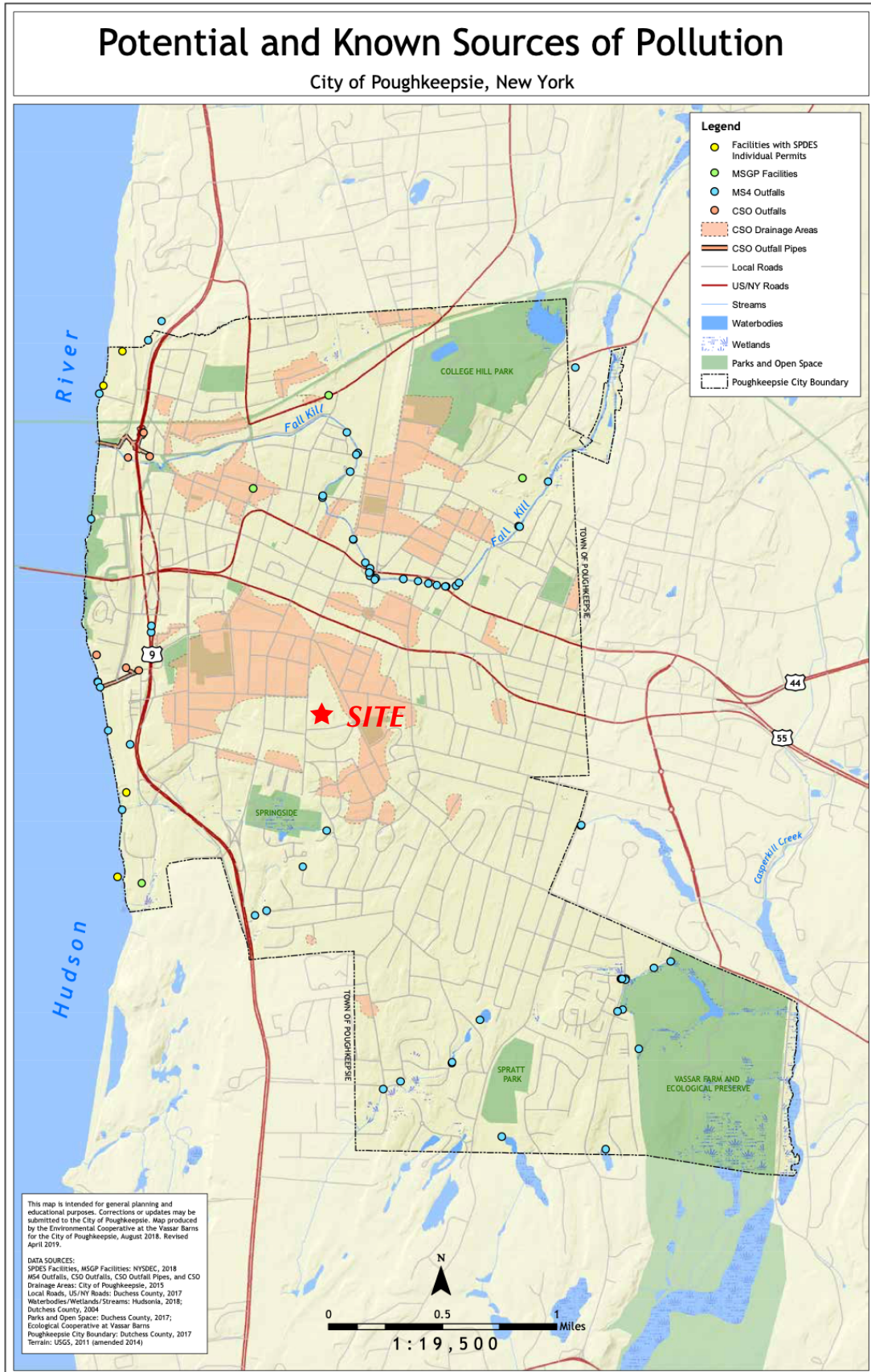


Figure 8

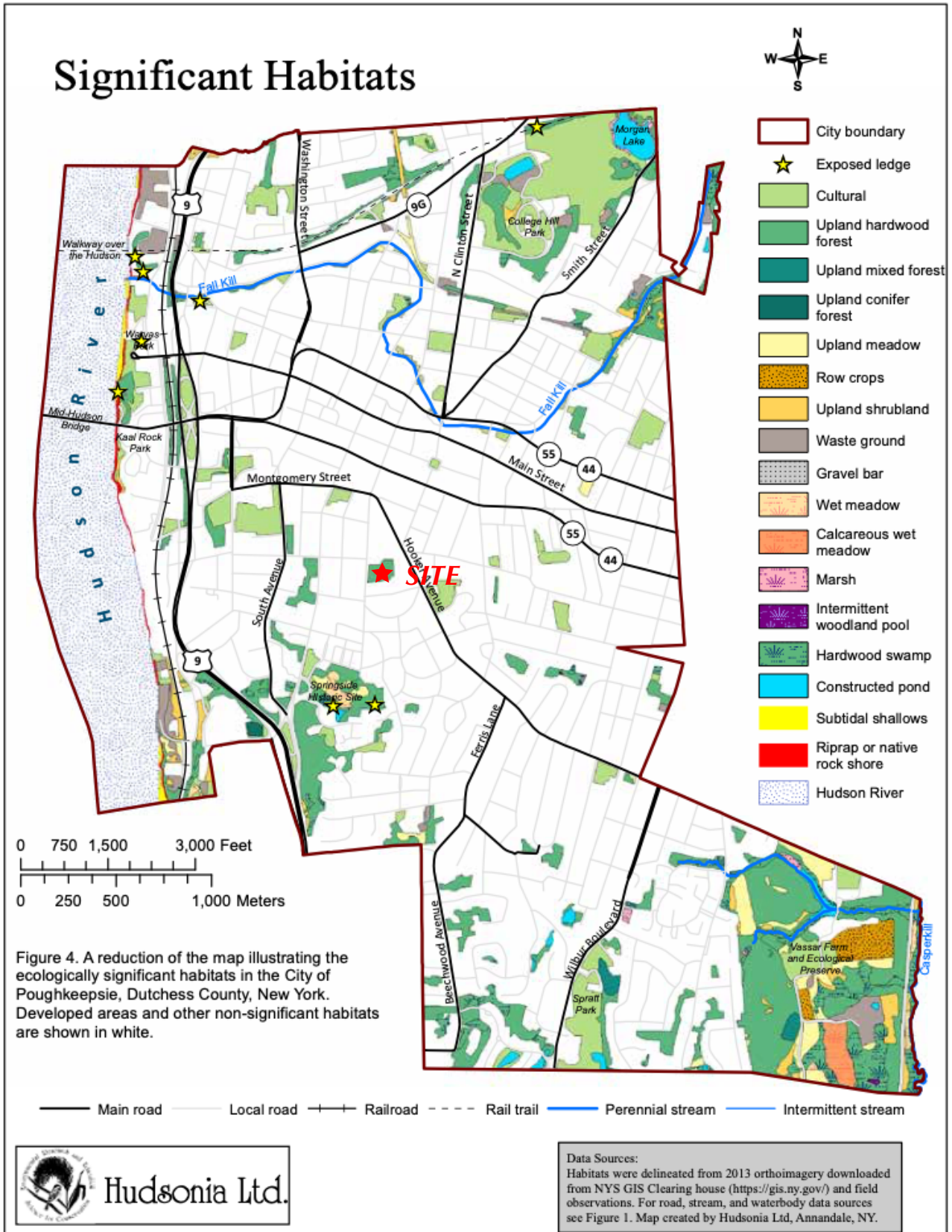
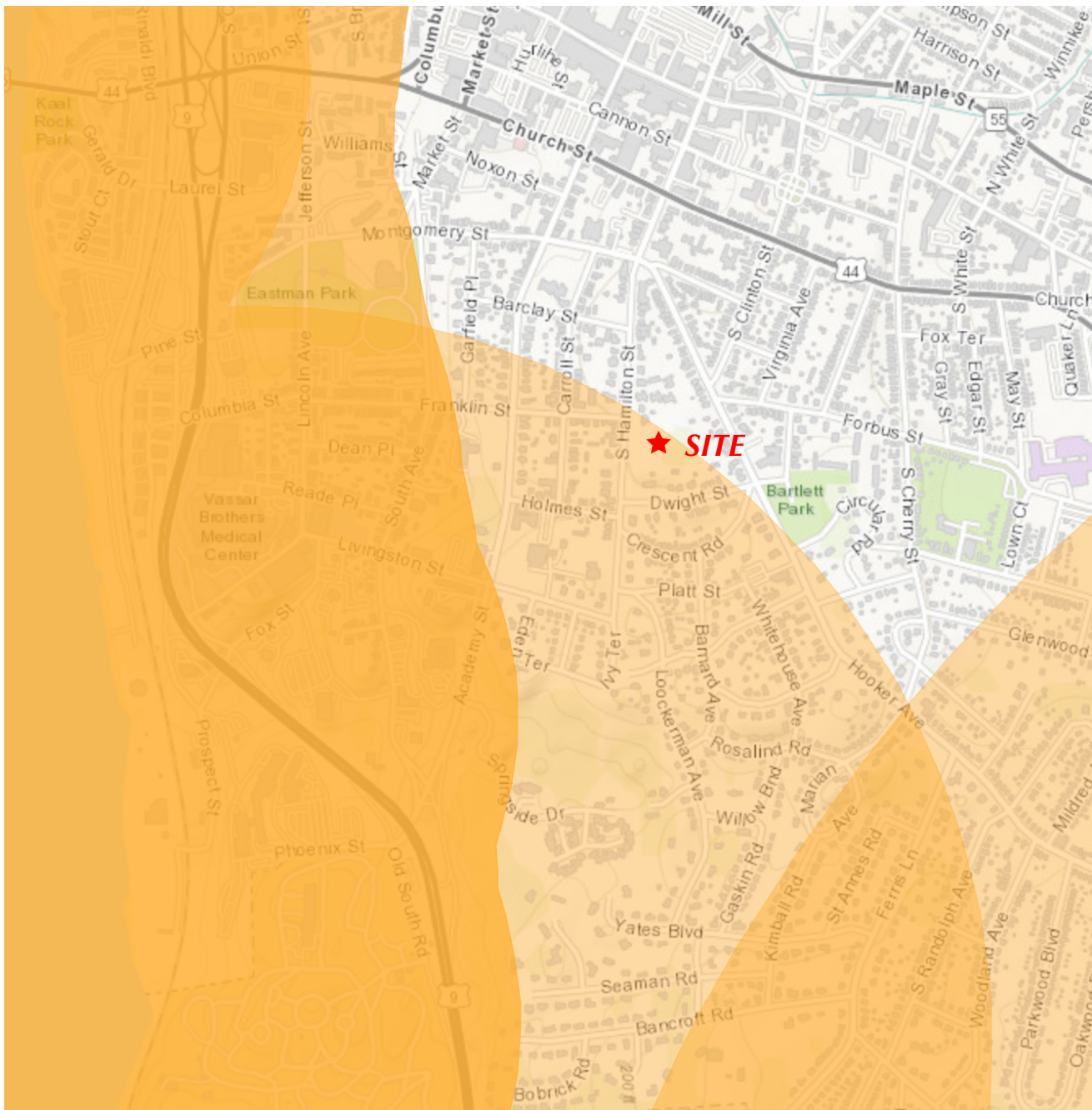


Figure 9
Rare Plants and Animals



Source: NYSDEC Environmental Resource Mapper

Figure 10

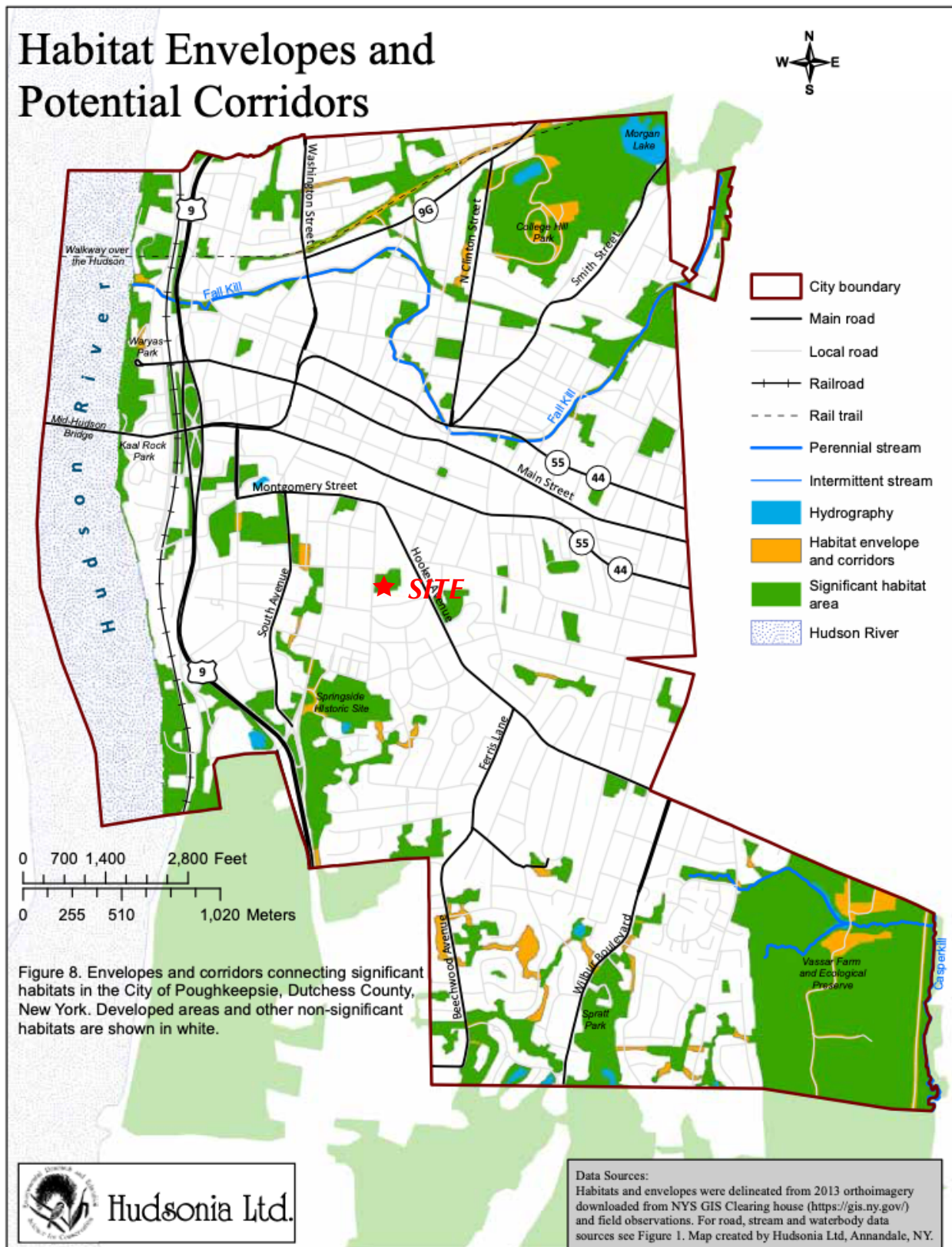


Figure 11

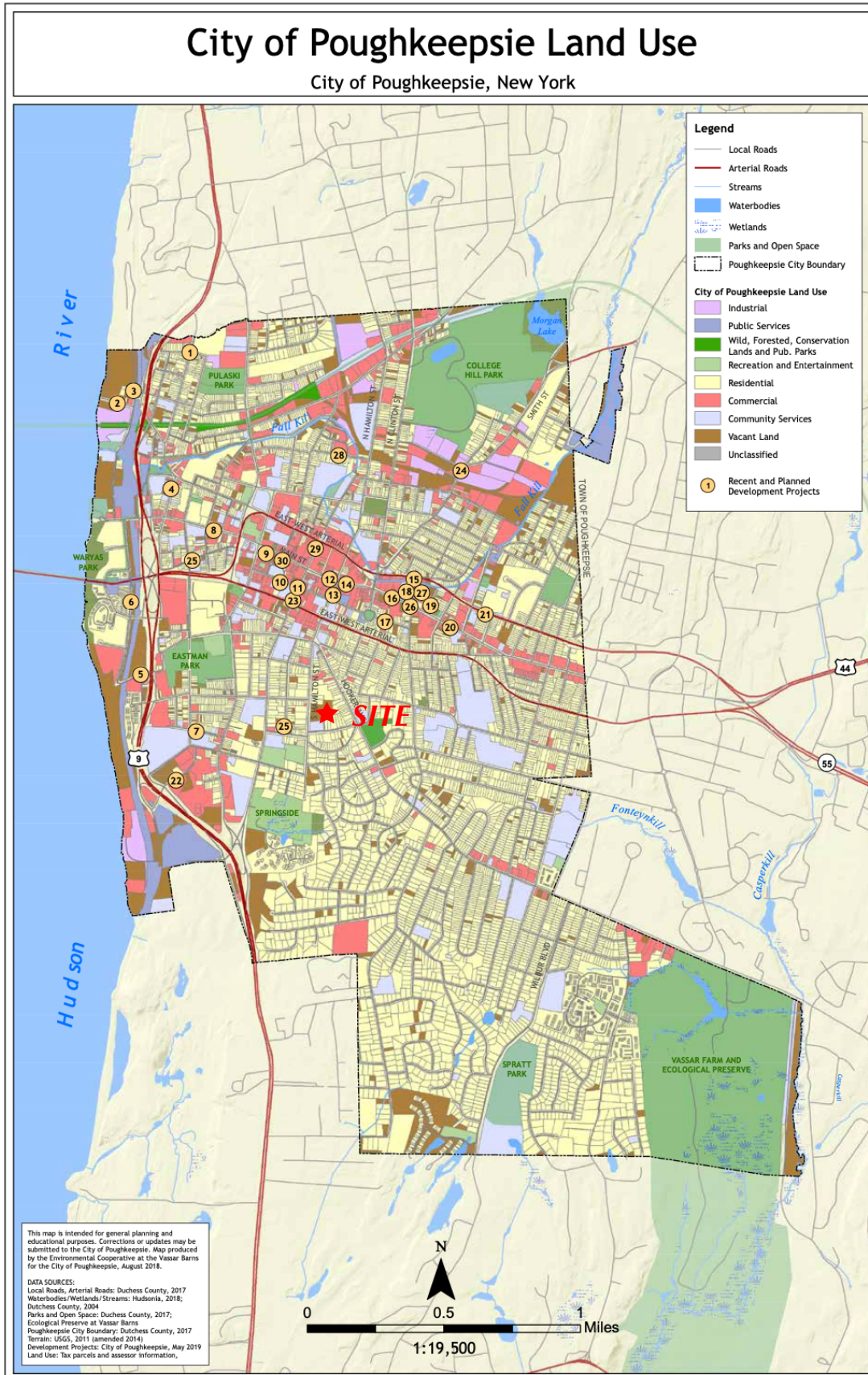


Figure 12

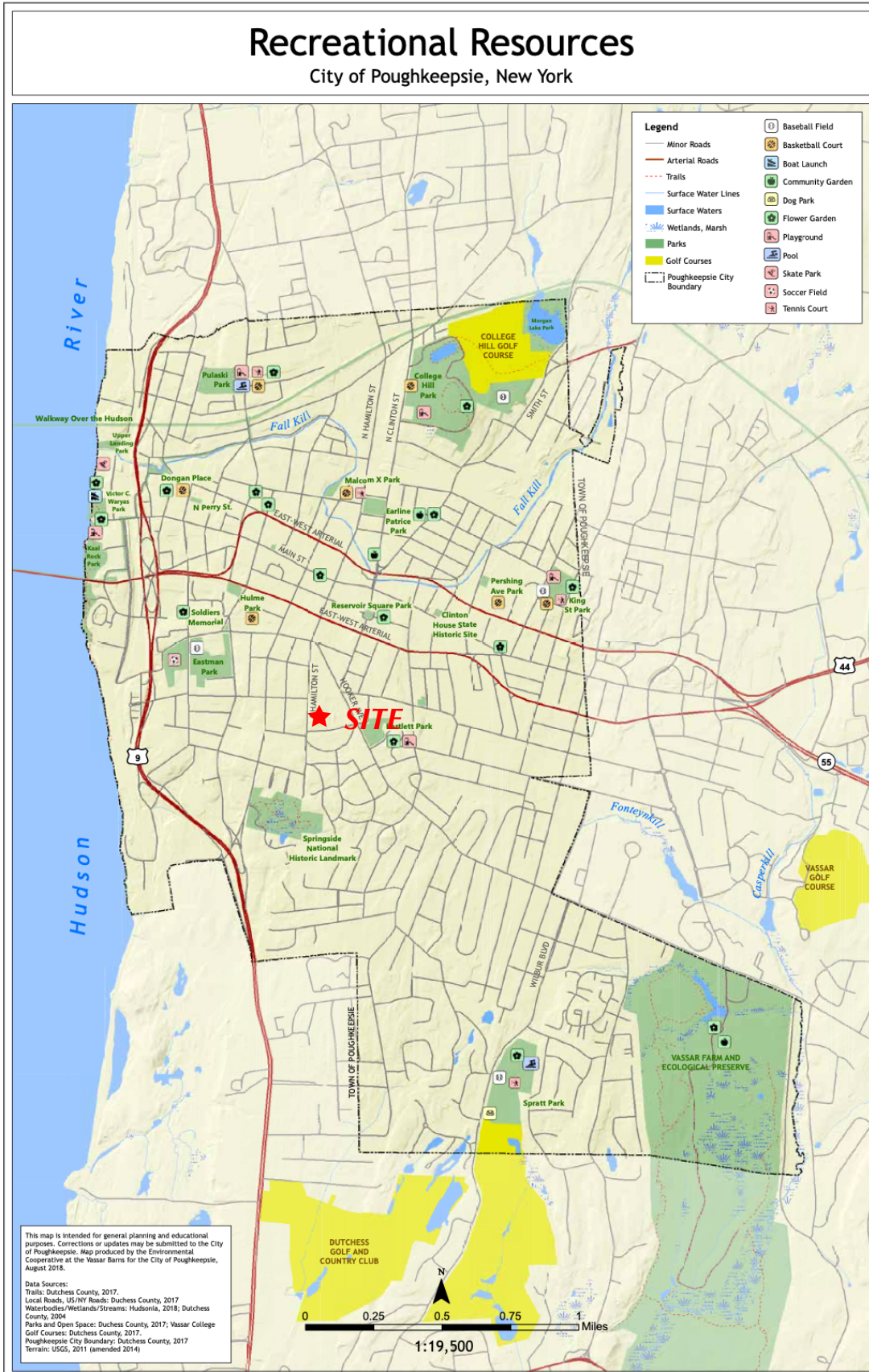
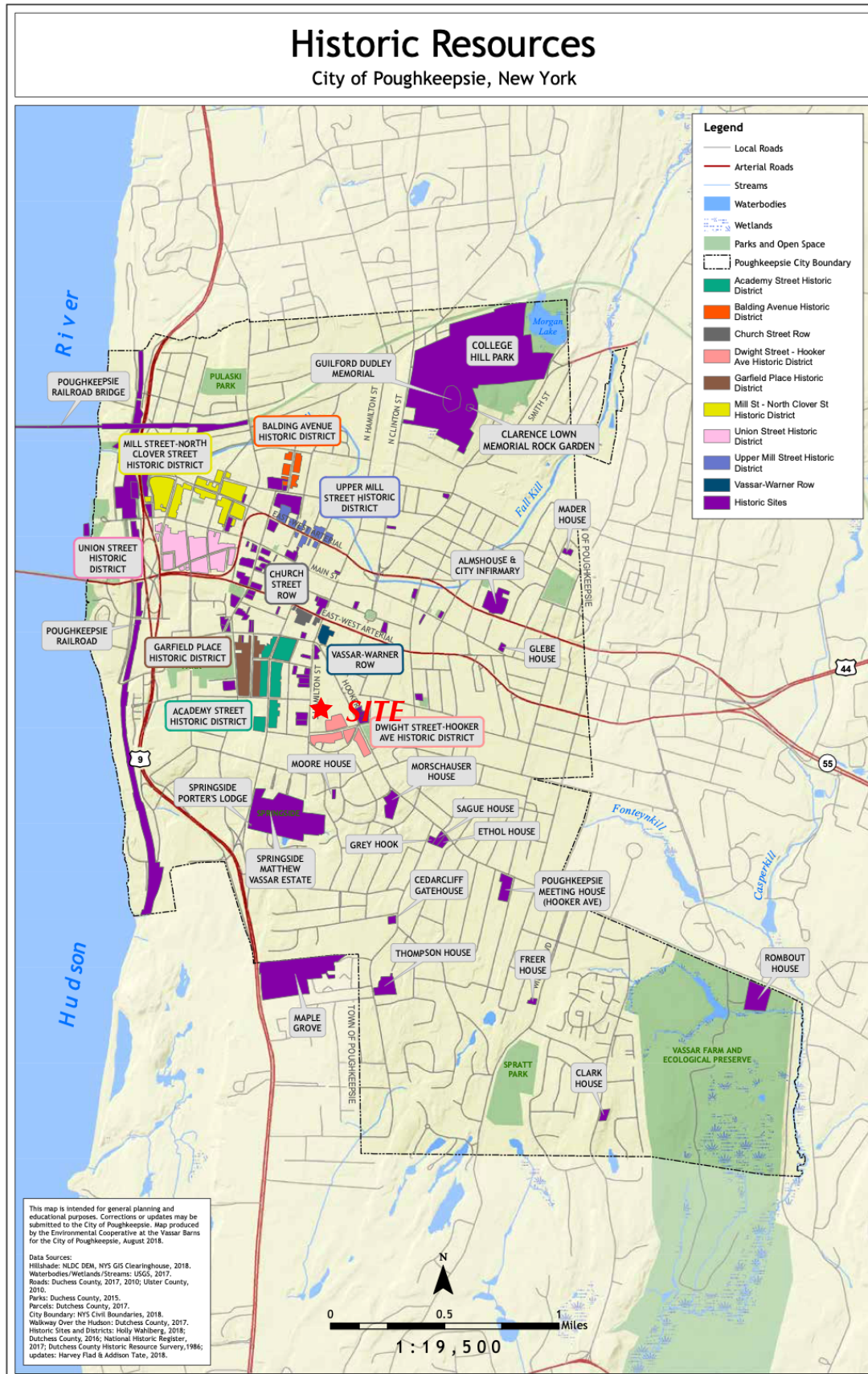


Figure 13





~~October 5, 2023~~

Revised October 9, 2023

City of Poughkeepsie Planning Board
62 Civic Center Plaza, 2nd Floor.
Poughkeepsie, NY 12601
Attention: Lori Garcia

Re: S. Hamilton Street (151783) proposed construction of a 58-unit multifamily building.

Dear Ms. Garcia:

Enclosed please find a thumb drive containing the following:

1. Site Plan Set prepared by Arico Associates Engineering revised October 3, 2023. In response to Board comments, a Boundary and Topographic Survey prepared by Vincent Ausfeld, PLS dated September 28, 2023, which highlights existing conditions is added to the plan set. A Landscape Plan prepared by The Michael Boice Collaboration dated September 27, 2023, is included. Two gazebo gathering spaces and four benches have been added to the landscape plan in response to comments from the Board. Landscape and Lighting plans are separate plan sheets at the request of the board. A Lighting and Photometric Plan is added to the plan set in response to comments. In addition, the Grading and Drainage plan is now a separate sheet as is the Water and Sewer Utility plan. An Erosion Control Plan is added to the plan set. Several sheets of construction details and notes are added to the plan set in response to Board comments.
2. Stormwater Pollution Prevention Plan (SWPPP) prepared by Arico Associates Engineering dated October 3, 2023.

3. Floor Plans & Elevations by Mija Architecture revised October 3, 2023. The entrance lobby was shifted to the center of the two building wings. The stairway columns were relocated to the front of the building and the access doors that were previously located in the rear of the building now open to the front. Line of site perspectives are added to the drawing set in response to comments.
4. Color Renderings prepared by Mija Architecture and dated October 3, 2023. Brick is proposed in place of brick that was shown on previous rendering. In response to comments balcony partitions and railings are revised and proposed landscaping is depicted in the renderings.
5. Subsurface Investigation Report prepared by Kevin L. Patton, P.E. dated July 6, 2023.
6. Traffic Impact Study prepared by Creighton Manning Engineering LLP Dated July 6, 2023. The study concludes that the Build condition of the study intersections will operate at the same levels of service consistent with the No-Build conditions.
7. Poughkeepsie Natural Resource Inventory Conservation Analysis Prepared by Cleary Consulting dated October 5, 2023. Based on the Analysis, the Action is consistent with the policy objectives of the Natural Resources Inventory for the City of Poughkeepsie

We look forward to our appearance before the Board on November 21st.

Sincerely,



George Brandt
gbrandt@onekeyllc.com
(201) 488-4455 ext. 122

KEVIN L. PATTON, P.E.
36 PATTON ROAD
NEWBURGH, N.Y. 12550
845 275-7732 PATTONGEOTECH.COM

CLIENT:	One Key Development 241 Hudson Street Hackensack, NJ 07601	PROJECT:	South Hamilton Street Site Poughkeepsie, N.Y.
		Project No.	23216
		Date:	July 6, 2023

SUBSURFACE INVESTIGATION REPORT

PROJECT DESCRIPTION

This geotechnical investigation report was prepared for use in the design and construction of a new three-story multi-family apartment building on a previously-developed lot at 100 South Hamilton Street, as shown on the attached plan. The building will have an L-shaped footprint of approximately 22,800 square feet, with the main section aligned generally north-south, along South Hamilton Street, with a width of approximately seventy feet and a length of about 235 feet. A wing seventy feet in width will extend approximately one hundred feet east from the south part of the building. A stormwater control area is proposed along the east side of the site.

The proposed building is expected to have a first floor elevation of 213 feet, with no basement level. It will straddle the top of a small hill which peaks at about 223 feet in the middle of the building area, most of which is at elevations of 210 to 223 feet, with low points of 206 feet at the northwest corner, 208 feet at the southwest corner and 202 feet at the northeast corner. The existing elevations in the proposed stormwater control area are approximately 197 to 198 feet. Aerial photos indicate that a large residence on the site was demolished approximately forty to fifty years ago, and that since then the site has been idle, and was covered with trees prior to starting the project. Remnants of the driveway are still present, near the northeast corner of the site, minor demolition debris was encountered in the former house area, and some fill materials were noted in the proposed stormwater control area.

The Surficial Geologic Map of New York State (N.Y. State Museum, 1989,) indicates that the site is in a large area of glacial till over shallow bedrock. The USDA Soil Survey indicates that the native topsoil type in the project area is Dutchess-Cardigan-Urban land complex. Dutchess soils typically form over deposits of glacial till composed of sandy silty gravel with few flat (flaggy) cobbles, and are typically more than seven feet thick over bedrock. Cardigan soils typically form in till deposits composed of silt and sand with traces gravel and trace flaggy cobbles, with bedrock at about thirty to 34 inches depth. The soil survey reports that the water table is typically at a depth of six feet or more year-round where these soils are present. The local bedrock is thinly-layered gray siltstone, shale and sandstone of the Ordovician-age Austin Glen formation. Bedrock was encountered in several of the test pits. The soils and rock encountered in the test pits were generally consistent with the reference data. Some minor areas of fill were also noted.

SOIL INVESTIGATION AND TEST RESULTS

Eight test pits were excavated on June 14, 2023, six in the proposed building area and two in the proposed stormwater control area. The two test pits at the north end of the building were offset to the north of the building due to the site conditions. Only the north half of the stormwater area was accessible. The test pits were excavated using a tracked mini-excavator by One Key LLC. The subsurface investigation was supervised and witnessed by Kevin Patton, P.E.

Soils and bedrock were observed and sampled, and laboratory testing was performed on representative soil samples, for moisture content, particle size distribution and Atterberg Limits. USCS classifications of the soil, per ASTM D2487 and D2488, are provided on the logs and on the subsurface profile drawing.

TEST PIT SUMMARY								
	Building Area						Stormwater	
Test Pit No.	TP1	TP2	TP3	TP4	TP5	TP6	TP7	TP8
Elevation, approx.	212	205	214	222	222	209	197.5	197
Depth, feet	12	6	7.5	8	5	4.9	6	8.5
Depth to Rock, feet	-	5.2	6.5	7.5	4.2	4.9	-	-



PHOTO 1. Excavating test pit TP4. All photos were taken on June 14, 2023.

Laboratory Data

Natural Moisture Content, Percent						
Depth, feet	TP1	TP2	TP3	TP4	TP7	TP8
2					16.5	
3		14.1	13.7			
4				13.6		20.6
5					13.5	
8	13.6					
11	14.4					

SOIL TEXTURE					
Particle Size Analysis					
Sample		TP1	TP3	TP7	TP8
Depth		8 feet	3 feet	2 feet	4 feet
Type					
USCS Class		SC-SM	ML	CL	ML
Sieve Size	mm	Percent Passing by Weight			
¾"	19.0	86	100	100	100
#4	4.75	74	92	98	100
#10	2.00	66	88	97	99
#40	0.425	53	82	94	96
#200	0.075	35	54	67	53
Hydrometer	0.050	28	42	54	33
	0.005	5	11	17	1
	0.002	1	5	9	0
Atterberg Limits					
Liquid Limit		22	-	28	-
Plastic Limit		16	-	17	-
Plasticity Index		6	Non-plastic	11	Non-plastic

The moisture content test results were consistent with the observed moist to very moist or wet conditions in well-consolidated soils with negligible organic content. The particle size analyses and Atterberg Limits tests represent some of the typical native soils in the project area. The first sample was composed of 26% gravel, 39% sand and 35% silty clay. The second sample consisted of 8% gravel, 38% sand and 54% silt, the third was 2% gravel, 31% sand and 67% clay, and the fourth sample contained 47% sand and 53% silt, with no gravel. Occasional cobbles and few boulders were present in the soil, as noted in the logs, but were not included in the tests. The Atterberg Limits tests indicate that the fine fraction of the more highly-plastic soils is silty clay or lean clay, making them somewhat moisture-sensitive, but indicating a low potential for detrimental shrink-swell (expansive) behavior. The Atterberg Limits were not tested for the two silty samples; the sample from TP3 had low cohesion; it might have a plastic limit but does not have a liquid limit. The sample from TP8 was fine-sandy cohesionless silt, without an LL or a PL.

SUBSURFACE PROFILE AND SUMMARY OF SOIL CONDITIONS

The observed subsurface conditions are described in the test pit logs and are summarized in the drawing attached to this report. Most of the native soils and most of the existing fill consisted of fine-sandy silt with traces gravel, few cobbles and boulders. Some of the native soils in test pits TP3 and TP4 were identified as glacial till, but most of the native material appears to be till that has been significantly reworked by frost, soil creep and similar factors. Well-graded granular soil (gravelly silty clayey sand) was only encountered in test pit TP1, below elevation 205; this was also the only soil in the building area that was noted as being 'very moist,' with moist conditions observed elsewhere.

Bedrock was encountered in five of the six building area test pits. It was not found in test pit TP1, near the northeast building corner, which stopped at twelve feet depth, approximately elevation 200. Near the top of the hill, bedrock was encountered at 7.5 feet depth, approximately elevation 214.5 in test pit TP4, and in TP5 it was at 50 inches depth, elevation 218. Test pit TP2, near the proposed northwest building corner, encountered rock at five feet depth, roughly elevation 200. Test pit TP6, in the low southeast corner area, stopped on rock at about five feet depth, at an approximate elevation of 204 feet. Near the southwest corner, TP3 encountered bedrock at 6.5 feet depth, approximately 207 feet elevation. The bedrock was thin-bedded shale, with near-vertical beds striking northeast observed in test pit TP2. The first six to twelve inches of the bedrock could be ripped easily with the small excavator. The other three test pits in the building area ended in soil at elevation of 200 to 207 feet. The other test pits indicate rock near elevation 200 at the northwest corner (TP2,) becoming shallower toward the south and east (± 207 ft southwest, TP3; 214 ft north-center, TP4; 218 feet center, TP5,) then dropping off at the southeast corner (TP6, ± 204 feet.)

The soils in test pit TP8, near the north end of the proposed stormwater control area, consisted of approximately thirty inches of moist fill (gravelly silt and sand) over silt and fine sand, which was very moist to sixty inches depth, where it became wet. Test pit TP7, near the center of the proposed practice, encountered two inches of organics over eight inches of silty sandy fill over clay, which was moist from ten inches to 32 inches depth, where it became wet. A rock outcrop projects above grade off-site, just east of the property line in this area.

Fill materials were observed in the two stormwater area test pits as noted above, and in three of the six building area test pits. The fill materials in the stormwater control area were mostly soil, but included minor amounts of debris such as steel, concrete, cinders and glass. In the building area, twelve to eighteen inches of sandy silt fill was noted in test pits TP1 and TP4, with a couple small pipes at the latter location. Test pit TP5, near the location of the former residence, encountered forty inches of sandy silty fill over a ten-inch layer of bricks with soil, over bedrock.

SUBGRADE PREPARATION

The investigation indicates that the subgrade conditions throughout the proposed building area are suitable for the use of shallow spread footing foundations and slabs-on-grade, subject to performing the required subgrade preparation operations, as described below.

The test pits indicate that the top of rock is typically about five feet below existing grade in the building area, deeper near the northeast corner. Using five feet as an average, approximately five-eighths of the slab is expected to be supported on a bedrock subgrade at elevation 212 feet. At the north end and at the southwest and southeast corners of the building the subgrade will transition from rock to native soil, then to fill. Most of the footings will also bear on rock, transitioning to native soil at the north end and at the two south corners, however at the south end the footings may be made deeper as the grade drops off; with close proximity to the property line, this appears to be more practical than building up a fill pad to the same elevation as the rest of the foundation.

Remove all existing pavement, topsoil, soft subsoil, stumps and large roots from the subgrade surface, in all foundation and slab areas. Remove existing slabs and foundations, if encountered, in their entirety from below new foundation areas, and to at least twenty-four inches below bottom-of-slab in slab-on-grade areas, and as needed to prevent interference with the new construction. Remove uncontrolled fill in its entirety from below new foundation areas and from below slab-on-grade areas unless otherwise approved by the Engineer.

Excavate to the design subgrade elevation, or deeper if required to reach suitable subgrade, which shall be bedrock (hard or weathered) or undisturbed native soils, free from topsoil, fine organics and root masses. Where the subgrade material is soil it shall have a stiff/ medium-dense or harder consistency. Dewatering and draining of the excavation may be required to some extent during excavation, with storms expected to be the main source of water.

Trim soil to the required subgrade elevation using excavation methods that minimize disturbance of the final soil surface. Compact the surface as needed to consolidate any soil that was loosened during excavation. Remove any pockets or small zones of unsuitable materials that are encountered, and replace them with controlled compacted fill. Contact the Engineer prior to performing any significant extra excavation. Where old foundations, stumps or boulders are removed, or where other over-excavation work is performed to prepare subgrade areas, the sides of the excavation shall be trimmed back to stable soil as each lift of backfill is placed. Where deficient soil is removed from below footing locations, the remediated area shall extend at least one foot out from the footing per foot of depth (1 to 1 splay.)

Where bedrock is present and excavation is performed by ripping, hammering or blasting, remove the rock to an approximately level and uniform elevation, with a slope of ten percent or less in areas below footings and slabs. If the rock subgrade surface has open fractures, level and seat the surface by tracking back-and-forth over it with a bulldozer or excavator and/or spade it with the excavator bucket, then compact the surface with several passes of a vibratory trench roller or a single-drum soil roller.

Footings may bear directly on the prepared soil or rock subgrade, or on controlled compacted fill placed over the subgrade. Where bedrock is present, over-excavation of the rock by six to twelve inches is

recommended, with the rock replaced by compacted granular site-borrow soil or Structural Fill, to act as a cushioning layer and as a leveling course for the concrete forms. Where fine-grained native soil is present at the bearing elevation, a layer up to four inches thick of Structural Fill or equivalent site-borrow soil may be placed in the footing bottom to protect the soil surface, after properly preparing the surface to a level and stable condition. This layer shall be thoroughly compacted with a vibratory plate tamper or roller, and its surface shall not extend above the design bearing elevation. Footing bearing surfaces shall be free from frost, mud and loose soil or standing water, when concrete is placed. Rock surfaces should be thoroughly moistened prior to placing concrete.

Protect the prepared subgrade surface from erosion, from frost and from construction damage. Traffic from dump trucks and similar heavy vehicles should be minimized on the exposed surface of the subgrade and on compacted fine-grained fills.

EXCAVATION

The existing fill and native soils may be excavated using conventional heavy equipment, such as tracked excavators and bulldozers. Backhoes and mini-excavators should be suitable for the excavation of shallow trenches in soil, but may require assistance if boulders are encountered. The investigation indicates that the soils which will be encountered in the building excavations are likely to predominately be OSHA Type B, requiring a minimum slope of 1-to-1 in shallow excavations, with benching permitted. Soil types and excavation requirements must be confirmed by a qualified representative of the Contractor during construction.

Rock excavation is expected to be needed in most of the building area. A large bulldozer with a ripper tooth should be able to excavate the shallow rock, provided it can 'bite' into the bedrock surface, which may be smooth on top of the hill. A large excavator with a hydraulic hoe-ram will likely be required to excavate to depths of more than two or three feet into the bedrock. Excavation work in the building area should be practical using conventional methods, without blasting. Blasting could, however, be used to facilitate the removal of bedrock. The project area is sufficiently distant from existing structures, roads and other public areas to perform shallow blasting without causing adverse impacts to them. There is sufficient overburden present to use small, lightly-loaded blast holes to fracture the bedrock, so that it can be more easily removed by conventional excavation, although some ripping and hammering could still be required.

Little or no shoring of excavations should be required for building construction, as there appears to be sufficient distance from the property line to the estimated limits of the foundation work area to allow the use of conventional excavation slopes. Trench boxes or other temporary shoring will be required for work in deep trenches, other than trenches in rock, and may be required when excavating trenches in wet areas, regardless of depth. The design of any necessary shoring or other support-of-excavation is the responsibility of the Contractor and is not included in this report.

Groundwater seepage rates in the building excavations are expected to be slow, but may be persistent during the wet season. Groundwater seepage and stormwater should be removed promptly from the excavations, and the groundwater elevation should be maintained at a depth of at least one foot below the soil surface in active construction areas. When dewatering excavations, the water level should be drawn

down at a controlled rate to minimize sloughing, allowing the water to drain from the soil in the sides of the excavation. Gravity drainage and conventional shallow sump pits installed in the excavation should be adequate for the control of groundwater seepage and stormwater.

FILL MATERIALS

All fill placed below foundations and slabs shall consist of Structural Fill or Structural Fill HD, as described below, or shall be suitable site-borrow soil or rock, as described below, or shall be other imported fill of a quality at least equal to that of the site-borrow fill.

Soils excavated from the site are expected to be of suitable quality for re-use as fill and backfill for foundations, slabs and pavement areas. Exclude topsoil and wet, organic otherwise deficient soils from the excavated soil, which should be blended as much as practical, to produce a uniformly compactible fill material. The site-borrow soil will likely require some require drying prior to compaction. Boulders and large cobbles must be removed from the borrow fill. Clumps of clayey soil should be excluded from the fill, but if included they must be thoroughly broken up and mixed in.

Rock excavated from the site may also be used as fill in building and pavement areas. If the rock is excavated by ripping or hammering, it should be processed by crushing and/or screening to approximately the grading of Structural Fill, below. Blasted rock will likely be suitable for use as fill after removing the oversize pieces, without further processing.

Open-graded gravel or crushed stone should not be placed directly against the silty native soils for most applications, as the soil may erode into the stone. The gravel should be separated from the soil by a geotextile layer or by a layer of well-graded soil. Pipe bedding in utility trenches may also act as a drainage path for groundwater seepage. If coarse granular bedding is used in sloping trenches, it should be interrupted periodically by sections of well-graded bedding, to regulate the flow of this seepage.

Structural Fill, if imported for use below foundations and slabs, shall be good-quality bank-run sand and gravel or crushed stone, and should be a well-graded product complying with the gradation limits below. Structural Fill may also be used as foundation backfill. Structural Fill HD (Heavy Duty) should be used in areas to be protected from heavy construction traffic and where subgrade stabilization and/or enhanced drainage is needed and is suitable for use as fill during winter conditions.

Sieve size		Structural Fill	Structural Fill HD
Inch	mm	Percent Passing by Weight	
4"	100	100	100
1½"	37.5	60-100	50-95
#4	4.75	20-85	20-50
#40	0.425	5-50	5-25
#200	0.075	0-30	0-10
Plasticity Index		12 max.	4 max.

All fill materials shall be composed of sound, durable particles, shall be free from frost or snow, garbage, construction debris or other deleterious material, and shall be substantially free from organic matter and roots. Recycled crushed concrete and masonry from a registered source may be acceptable for some applications above the water table, subject to approval by the Designer of Record. Fill shall not be placed over frozen or unstable soil, unless approved by the Engineer.

CLSM (Controlled Low-Strength Material, aka flowable fill) may be used under footings and foundations when specifically approved by the Engineer, and may also be used to backfill trenches or other excavations, typically where rapid fill placement is required, fill areas are narrow, or the use of conventional compaction methods is not practical. For support of footings, a CLSM mix consisting of sand, cement and water, with a 56-day compressive strength of 75 to 200 psi, is appropriate. CLSM may produce high fluid pressures during placement, and caution must be used for placements against foundation walls, near unbraced cuts, etc. Pipes or tanks can also float if not properly restrained during placement. CLSM should not be placed against unprotected aluminum; CLSM containing flyash should not be used in contact with cast iron or ductile iron. Hardened CLSM masses may also adversely affect groundwater flow, possibly causing erosion under or along the CLSM, particularly in sloping trenches.

Other Fill Materials:

- Crushed stone base course for slabs-on-grade should consist of ASTM C33 #56 or #57 stone ($\frac{3}{8}$ - to $\frac{3}{4}$ -inch size,) or as required by the slab system design.
- Crushed stone or gravel for footing drains should consist of ASTM C33 #5, #56 or #57 stone ($\frac{3}{4}$ -inch or $\frac{3}{8}$ - $\frac{3}{4}$ -inch size.)
- Well-graded granular subbase material (NYSDOT Item 733-04 'Item 4' or similar) should be used under sidewalks and exterior slabs.

FILL PLACEMENT AND COMPACTION

Soil surfaces, including the surface of the subgrade and of previously-placed fill materials, shall be prepared to a stiff and essentially unyielding condition prior to placing each new lift of fill. Bedrock surfaces to receive fill shall be free from voids or loose areas and the rock surface shall be free from large loose pieces of stone. Use mid-size equipment to compact the site-borrow fill or similar materials. Vibratory trench rollers, and single-drum soil rollers with a nominal size of three to five tons, should be appropriate for the observed soil conditions. Larger rollers may be used when compacting well-graded granular fill over essentially unyielding surfaces. In areas with limited access, vibratory plate tampers or jumping-jack tampers may be used. Avoid over-compacting the shallow soils in landscaped areas.

Soil fill and processed rock fill shall be placed in controlled lifts, with each lift compacted to the required density at a moisture content close to optimum moisture, as determined by the ASTM D1557 Modified Proctor Method. When the moisture content of fill which will support footings or slabs is within two percent of optimum, fill may be placed in lifts with compacted thicknesses of up to eight inches. If the moisture content is two to 2.5 percent from optimum, reduce the maximum thickness to six inches, and if it is more than 2.5 percent from optimum, discontinue compaction. In roadway, embankment and other non-structural locations, fill may be placed in lifts with a compacted thickness of up to twelve inches when the moisture content of the fill is within two percent of optimum, and up to eight inches thick when it is

two to 2.5 percent from optimum. Use a reduced lift thickness if required to obtain the specified percent compaction and when using small compaction equipment.

When placing fill during freezing temperatures, use Structural Fill HD or similar fill material containing less than ten percent non-plastic fines. Fill produced from natural sand and gravel is preferable to crushed stone or concrete fill for winter work, for ease of compaction. Compact the fill with a maximum lift thickness of six inches and do not compact fill whose moisture content is more than two percent above or below optimum. Do not place or compact fill when the air temperature is less than 25°F. Do not place frozen fill materials.

Mass rock fills may be constructed using well-graded shot-rock from blasting. The design lift thicknesses should be approximately twelve to eighteen inches, depending on the dimensions of the fill, the fill placement method and the compaction method, and may require adjustment based on the actual size and grading of the rock after it is blasted. Rock fill should be placed in a manner that minimizes segregation and which creates an initially-dense state. Allowing the rock fill to free-fall into place from a belt or bucket can be an effective method, but dust, noise and flying particles are of concern. Lifts of rock fill may be acceptably placed with a bulldozer, but a moving pile of fill should be maintained in front of the blade, rather than running the pile out completely. Re-work any boulder pockets to a dense, well-graded condition, and level the surface of the fill prior to compaction. The rock fill should be compacted in a wet condition when practical, but may also be compacted when dry or moist. Rock fills should be placed in conformance with an engineered design which includes any necessary requirements for drainage; for geotextiles and/or aggregate filter zones between the rock fill and the subgrade, adjacent and/or overlying soils; the clearance depth for utilities, etc.

Where fill will be placed against slopes, bench the fill into the slope to create a stair-step interface, for improved stability and groundwater control. Lightly scarify the surface of the existing soil prior to placing the fill, and key the fill into the subgrade at the toe of the slope. When the fill is more than five feet high against a slope of twenty percent or more, the key should be at least two feet deep and ten feet wide.

Backfill placed against foundation walls should be compacted with trench rollers or with similar equipment which will not produce damaging stresses on the wall. Place backfill equally on opposite sides of the foundation unless otherwise indicated by the specifications or drawings. Foundation walls acting as retaining walls should be braced (i.e. by installation of the floor framing) before placing backfill, unless otherwise indicated on the drawings or approved by the Engineer.

Open-graded stone base course material for slabs-on-grade should be graded level and seated with one or more compaction passes, to help resist displacement during slab area preparation and concrete placement.

Compaction Requirements

Compact each lift of soil fill supporting slabs or foundations with at least six one-way compaction passes, even if the required compaction percentage is obtained with fewer passes. Each compaction pass shall be made at a slow walking speed (less than four feet per second,) with the equipment passing completely over all areas of the fill. Fill materials shall be compacted to at least the following percentage of the ASTM D1557

maximum dry density. For coarse-graded fill materials with more than thirty percent retained on the 3/4-inch sieve, the ASTM D4253 Maximum Index Density test may be substituted for the D1557 test.

Location	Percent Compaction
Below Footings, Foundations, Slabs and Pavements	95% minimum
Exterior Foundation Backfill in Landscaped Areas	90% minimum

Mass rock fills must be compacted with the largest vibratory single-drum soil roller that is available and appropriate for the site, typically in the ten-ton to sixteen-ton size range. Compact each lift with at least six vibratory roller passes, with the passes applied in two perpendicular directions, or in multiple directions, when possible. Vary the vibration frequency to maximize compaction, using the high-frequency mode for the final passes, for increased compaction depth with less surface disturbance. Continue compaction until the fill surface is in an unyielding condition.

TESTING

The prepared subgrade shall be inspected to verify that it has been prepared in conformance with the requirements of this report, prior to placing fill or foundations. Recommended test procedures and frequencies are provided below.

PROOF-ROLLING: Proof-rolling of the prepared subgrade soil is not required, but may be performed to determine the limits of a soft area. Use an appropriately-sized vehicle, to avoid damaging wet and/or fine-grained, but otherwise acceptable soils. Observe the effects of the moving vehicle; if the soil exhibits excessive deflection, rutting or cracking, additional excavation or drying of the subgrade may be required.

BEARING CAPACITY: The prepared subgrade surface throughout the foundation and slab areas area shall be probed thoroughly to check for soft spots. The foundation subgrade shall be in a dense and unyielding condition, substantially free from soft areas and/or loose material. If these conditions are not encountered at the bearing elevation, testing shall be performed with a Static Cone Penetrometer or equivalent device in foundation bearing areas. If the design bearing capacity is not indicated within three inches of the bearing surface, the soil conditions shall be corrected and/or, if approved by the Engineer, the footing depth may be increased to reach acceptable soil. The slab subgrade areas shall be densely consolidated and sufficiently stiff to prevent rutting or displacement during slab base course and concrete placement operations. Where fractured rock or bedrock is present at the subgrade surface, it shall be visually inspected to verify that it is generally level and free from accumulations of loose material, and that in areas where it is fractured, the fractured mass is in a dense and stable condition, free from large open voids.

COMPACTION TESTING: Compaction testing shall be performed for each lift of soil fill supporting foundations, with testing performed while the work is in progress. Testing is also recommended for the fill supporting slabs or pavement, exterior backfill and for fill placed in embankments. Compaction testing is recommended for all retaining wall construction, with requirements to be determined by the wall designer. Compaction tests of fill and backfill supporting foundations and slabs should be performed in at least one location per 50 linear feet or per 1000 square feet of fill surface, per lift. At least one test per 100 linear feet

or per 2500 square feet should be performed for each lift of fill in embankment, roadway and other non-structural areas. Compaction tests should be performed with a nuclear moisture-density gauge, per ASTM Test Method D6938, unless otherwise approved. Required percent compaction values are provided above. Rock fill placement shall be visually monitored for fill quality and grading and for conformance with the requirements for lift thickness and placement methods and for application of the specified compaction effort.

CLSM: When flowable fill is used to support footings or foundations, at least one set of three 6x12-inch test cylinders shall be cast from each day's placement, per ASTM D4832. Test the cylinders for unit weight and for compliance with the specified strength requirements. Cast additional cylinders if early tests are needed.

GEOSYNTHETIC MATERIALS

Geosynthetic materials are expected to be used for reinforcement and drainage applications at the site on an as-needed basis, or where required by Code, such as for footing drains. Geosynthetic materials shall be installed over a smooth and evenly shaped subgrade, to avoid 'tenting' of the material over voids or high points. The geosynthetic shall be installed substantially free from wrinkles, and fill material shall be placed and spread in a manner which pushes the wrinkles out but which does not otherwise displace the geosynthetic material. Vehicles shall not drive on the exposed geosynthetics. The following material types are recommended.

Drainage Separation: For footing drains and similar applications, a woven drainage geotextile with at least 4% open area, with an apparent opening size of 0.21mm (#70) or smaller, should be installed around the open-graded drainage stone. Non-woven geotextiles are not recommended for use in this application, due to the presence of fine particles in the native soil that will tend to clog the fabric. If stone drains are installed in areas of fine sand or cohesionless silt, provide a zone of clean sand at least three inches thick between the geotextile and the soil.

Subgrade Reinforcement: Typically, a woven reinforcing geotextile with a grab tensile strength of at least 200 pounds should be used where needed to improve the stability of soft subgrade soils. Geogrids may be used instead of woven geotextiles, especially if free drainage is desired. A minimum of twelve inches of granular fill cover is typically required to fully mobilize the strength of the reinforcing geosynthetic.

Subgrade Separation: Where fines from the subgrade may infiltrate into an overlying granular layer, and strengthening of the subgrade and some vertical drainage is desirable, a non-woven geotextile should be used. A four-ounce nominal weight fabric is suitable for light- to moderate-duty applications; heavier fabric should be used where it will be subjected to heavy traffic, especially when it is placed in contact with sharp or very coarse-graded materials.

Woven reinforcement geotextiles will usually act as an infiltration barrier when installed in a continuous horizontal layer, and non-woven separation geotextile may also work as a barrier; this may or may not be desirable, depending on the installation location and conditions, and should be considered when specifying or using these products.

ENGINEERING DESIGN VALUES AND RECOMMENDATIONS

Soil engineering properties and recommendations for design are provided in this section; additional important design considerations are also discussed in the other sections of this report. The design values assume that the building will be supported by a conventional spread footing foundation with slab-on-grade floor, as described in the previous sections, and will be provided with proper drainage.

Bearing Capacity and Soil Pressure

Allowable Bearing Capacity, q_a	
Footings on Soil, bearing at least 42 inches below finished grade, with a minimum width of 12 inches	4000 psf
Footings on Soil, bearing at least 24 inches below finished grade, with a minimum width of 36 inches	4000 psf
Minor Footings on Soil, bearing at least 12 inches below finished grade, with a minimum width of 12 inches	2000 psf
Footings on Fractured and/or Weathered Rock, bearing at least 12 inches below finished grade, with a minimum width of 12 inches	4000 psf

Soil Properties	Native Soils	Rock, fractured
Moist Density, γ , lbs/cu ft	130	160
Effective Internal Angle of Friction, ϕ	32°	42°
Coefficient of Active Earth Pressure, k_a	0.31	0.20
Coefficient of Passive Earth Pressure, k_p	3.25	5.0
Coefficient of At-Rest Earth Pressure, k_o	0.47	0.33
Lateral Bearing Capacity (psf per ft below grade)	210	400
Coefficient of Friction (vs. concrete)	0.35	0.60
Modulus of Subgrade Reaction, k , psi per inch	175	550

Footings subject to frost shall bear at least 42 inches below finished grade, or shall be otherwise protected from frost. Footings not exposed to frost, or bearing on non-frost susceptible subgrade material (e.g. fractured bedrock) shall bear at least one foot below finished grade. Bearing elevations of footings shall be established such that a line drawn between the bottoms of two adjacent footings is not steeper than 30 degrees between the closest points on the footings. (Slope of 1 vertical to 1.75 horizontal.)

Settlement of the foundation elements over the course of construction, due to normal elastic compression of the underlying soils, is expected to be one half inch or less in the well-consolidated soils at this site. Up to one inch of settlement and 3/4-inch of differential settlement should be assumed for design purposes. No measurable settlement is expected where foundations bear on rock. If fill is placed below footings, an additional quarter inch of settlement should be expected per five feet of fill thickness.

If encountered above the design bearing depth, undisturbed bedrock and rock that has been fractured by excavation are non-frost-susceptible subgrade materials, and frost-protected shallow footings, as specified

in ASCE 32, may bear on rock at depths as shallow as twelve inches when the required vertical foundation wall insulation is also provided. Where footing depths are reduced, pinning to rock may be required if the footing will be subjected to significant uplift or lateral forces. These installation conditions must be reviewed and approved on a case-by-case basis.

Control of Groundwater and Soil Gases

Minor groundwater seepage should be expected in excavations and below-grade areas during and after construction. It is understood that the building will not have a basement and the first floor slab will be entirely above grade. Adequate drainage must be provided to maintain the groundwater at an elevation below the slab base course; footing drains may be helpful, but are not required by Code for the above-grade slab condition. Construction dewatering is discussed elsewhere in this report. For the completed construction, the elevations shown on the plans indicate that a foundation drain, if used, should be able to discharge by gravity.

Soil gases that could normally be expected to impact the structure are water vapor and radon. The interior spaces should be adequately protected from these gases by providing drainage and moisture protection measures as described above, and by sealing all wall-to-slab joints, concrete cracks, pipe penetrations, drainage sumps, etc. If an open-graded base course is used under the slab, a passive or powered vapor mitigation system using small-diameter PVC pipes can be included or can be added retroactively. The potential for these gases to adversely impact the use of the building is estimated to be low, if the above recommended practices are used, and normal interior ventilation is provided.

Seismic and Expansive Properties

Seismic Design Values: The Seismic Site Class and Seismic Design Category for the proposed construction were determined per section 1613 of the New York State Building Code and ASCE 7-16. Seismic values for the site were obtained from the current database maintained by the Applied Technology Council, Redwood City, Cal., and are consistent with the published maps in the Building Code. Values were as follow.

Occupancy Category	I/II/III	
Seismic Site Class	C- Very Dense Soil and Soft Rock D – Stiff Soil	
IBC Seismic Design Category	SDC - B	
Maximum Acceleration	0.2 sec S_S 1.0 sec S_1	0.195 g 0.055 g
Maximum Spectral Response Acceleration	0.2 sec S_{MS} 1.0 sec S_{M1}	0.254 g 0.082 g
5% Damped Spectral Response	0.2 sec S_{DS} 1.0 sec S_{D1}	0.169 g 0.055 g

The seismic design values are based on the “risk adjusted maximum probable earthquake.” These are not the maximum values that *could* occur, they are values that are not likely to be exceeded during the service life of a typical structure.

Liquefaction Potential: The soils encountered in the investigation have very low liquefaction susceptibility. Liquefaction is typically associated with thick deposits of loose poorly-graded sand situated below the water table, and may also occur in loose, poorly-graded cohesionless silt. A similar phenomenon, cyclic softening, can occur in saturated soft clays. Those soils are also prone to shifting or spreading during seismic events when they are situated near a slope, even if the slope is gentle. The soils encountered in the test pits had textures, drainage conditions, locations and/or sufficient consolidation (density) to resist liquefaction or unusual movement during the design seismic event. No special mitigation measures are required.

Expansive Soils and Frost Heave: The soils encountered in the investigation have a very low potential for expansion due to shrinking and swelling resulting from moisture changes. This behavior is typically associated with high-plasticity silt and clay soils. Physical testing and qualitative examination indicate that the soil properties do not meet the criteria for potentially expansive soils as defined in section 1803.5.3 of the Code. No mitigation measures are required. The on-site soils are moderately to highly susceptible to frost heave. Frost heave can be minimized by providing good drainage and by thoroughly compacting the soil. Well-graded granular fill should be used in areas where frost heave could result in damage.



PHOTO 2. The slope along the west side of the site, overlooking South Hamilton Street.



PHOTO 3. The proposed stormwater control area on the east side of the site. A rock outcrop forms a long knob on the far side of the fence.

NOTES AND LIMITATIONS

Please see the attached pages for additional information. Subsurface conditions encountered during construction shall be compared to the test pit logs and to this report; any significant variations from anticipated conditions must be evaluated for their effect on the design. This report summarizes the results of a limited investigation and does not purport to predict every variation in subsurface conditions. Elevations, slopes, contours, project layout and similar or related data provided in this report were interpreted from the drawings, from field data or from other information which was provided, unless otherwise noted.

This geotechnical investigation was conducted to evaluate the engineering properties of the soils at the site, to aid in the design and construction of the proposed work. The investigation did not include evaluation of the potential effects of the proposed construction on other properties, nor did it include inspection of, or sampling for, items of environmental concern such as the presence of soil contaminants or of regulated wetlands, and did not include review of local zoning regulations, codes, floodplain boundaries or similar matters, unless specifically referenced in the report. This investigation was conducted solely for the use of the Client, the Client's Project Designers and Agents and the Authorities Having Jurisdiction; this report should not be used by others, nor for any use other than its stated purpose, without contacting the Engineer. Any such use is solely at the user's risk.



Prepared by Kevin L. Patton, P.E.

Attachments:

- Soil Technical Notes
- Test Pit Location Plan
- Test Pit Logs
- Laboratory Test Reports
- Bedrock Geologic Map
- Surficial Geologic Map
- USDA Soil Survey Data

Soil Technical Notes:

Soil Classifications, Descriptions and Properties

The USCS (Unified Soil Classification System) was used to classify the soils in this report. The USCS is described in ASTM D2487 (laboratory test method) and D2488 (visual-manual method.) The USCS classification gives a 'Group Symbol' and 'Group Name' based on particle size distribution (gradation,) clay properties (Atterberg Limits) and basic composition (mineral or organic.)

USCS Soil Classes

Soils with less than 5% passing the #200 sieve:

GW, GP, SW, SP – Well-graded gravel, Poorly-graded gravel, Well-graded sand, Poorly-graded sand.

Soils with 12% to 50% passing the #200 sieve:

GC, GM, GC-GM, SC, SM, SC-SM – Clayey gravel, Silty gravel, Silty clayey gravel, Clayey sand, Silty sand, Silty clayey sand.

Soils with 5% to 12% passing the #200 sieve use a dual symbol, such as SW-SC (Clayey well-graded sand.)

Soils with more than 50% passing the #200 sieve:

CL-ML, ML, CL, MH, CH, OL, OH – Silty clay, Silt, Lean clay, Elastic silt, Fat clay, Organic silt, Organic clay.

Highly organic soils:

PT – Peat.

The soil group name is modified with the term 'with sand' or 'with gravel' added if the soil contains more than 15% of these materials; clays and silts with 30% or more plus-#200 material are described as 'sandy' or 'gravelly' (whichever is predominate.) Examples – GM, Silty gravel with sand; CL, Gravelly lean clay.

Particle size	Fine- and Coarse-grained Soils	Atterberg Limits
>12" (300mm) Boulders 12" to 3" (300-75mm) Cobbles 3" to #4 (75-4.75mm) Gravel #4 to #200 (4.75-0.075mm) Sand <#200 (0.075mm) Silt & Clay	The USCS classification applies to the material smaller than the 3-inch sieve. 'Fine-Grained Soils' (silts and clays) have more than 50% passing the #200 sieve and are classified by their Atterberg Limits. 'Coarse-Grained Soils' (sands and gravels) have less than 50% passing the #200 sieve. When more than 50% of the plus-200 material is retained on the #4 sieve the general soil type is gravel, and if more than 50% is finer than the #4 sieve, it is sand. Clean coarse-grained soils are classified as well-graded (Classes GW, SW) or poorly-graded (GP, SP.) Well-graded soils have a wider range of sizes and are typically more stable. Poorly-graded soils are usually more permeable.	Test is performed on the clay, silt and fine sand fraction of the soil: Liquid Limit (LL) – moisture content (%) at which soil becomes very soft. Plastic Limit (PL) – moisture content at which soil crumbles. Plasticity Index (PI) = LL minus PL Higher PI values may indicate reduced permeability and increased drying shrinkage. LL > 50 indicates soil with a higher potential to shrink and swell due to changing moisture content. Silts have lower PI values, and behave like very fine sand; most silts also contain some clay. Behavior of clays is partly controlled by electrochemical forces and varies among the several clay minerals.
Organic Soils		
Highly organic soils such as peat are visually classified. Partly organic soils, with a mix of organic and mineral matter, are classified visually and by Atterberg Limits tests.		
Moisture Content		
Moisture is visually estimated and samples are usually tested. Soil moisture capacity varies with texture and compaction. Typical examples: GW, moist at 3%, saturated at 9% SP, moist at 6%, saturated at 20%. CL, moist at 12%, saturated at 33%.		
Color	Relative Quantities	USDA Soil Classification
Soil color sometimes indicates groundwater conditions, with subdued colors below the water table and mottled (mixed) colors in the zone of seasonal water table fluctuation. Color changes tend to be more prominent in fine-grained soils.	Estimated percentages in descriptions: <5% - Trace 5-10% - Traces 10-25% - Little 25-35% - Some 'And' - Approx. equal amounts 'Few' - <10% (cobbles and boulders)	USDA classifications are based on the relative amounts of sand, silt and clay in the soil fraction passing the #10 (2mm) sieve. 'Gravelly' indicates more than 15% of #10 to 3" size. 'Channery' indicates 15 to 35% thin flat pieces up to 6" long.

GENERALIZED SUBSURFACE PROFILE

NO HORIZONTAL SCALE.
USCS SOIL CLASSIFICATIONS ARE IN BRACKETS.

IN GENERAL, RED PATTERNS INDICATE RELATIVELY CLEAN SANDY OR GRAVELLY SOILS,
PURPLE PATTERNS INDICATE SOILS WITH SIGNIFICANT CLAY CONTENT AND ORANGE
PATTERNS INDICATE SOILS WITH A SIGNIFICANT SILT CONTENT.

THESE SECTIONS ARE GENERALIZED REPRESENTATIONS OF THE SUBSURFACE PROFILE,
BASED ON THE SUBSURFACE EXPLORATION DATA, OBSERVATIONS, RESEARCH, AND OTHER
RELEVANT INFORMATION. THE SOILS INFORMATION PRESENTED HEREIN SHOULD BE
INTERPRETED IN CONJUNCTION WITH THE INFORMATION FROM THE TEST PIT LOGS AND
THE GEOTECHNICAL INVESTIGATION REPORT. SITE CONDITIONS MAY DIFFER FROM THOSE
ENCOUNTERED AT THE TEST PIT LOCATIONS.



			SOUTH HAMILTON STREET	KEVIN L. PATTON, P.E. 36 PATTON ROAD NEWBURGH, NY 12550 845 275-7732 PATTONGEOTECH.COM
			100 SOUTH HAMILTON ST., POUGHKEEPSIE, N.Y.	
0	7/5/2023	KLP	SUBSURFACE PROFILE	
REV.	DATE	BY		

KEVIN L. PATTON, P.E. 36 PATTON ROAD NEWBURGH, NY 12550 PATTONGEOTECH.COM 845 275-7732	CLIENT:	One Key LLC		
	PROJECT:	South Hamilton Street, Poughkeepsie, N.Y.		
	DATE:	6/14/2023	Project No.:	23216
	WEATHER:	Cloudy, 65F		

TEST PIT LOG					
EXCAVATING COMPANY:	One Key LLC	LOCATION:	50 ft NE from NE corner of proposed building	TEST PIT NUMBER	TP1
OPERATOR:	Garrett	ELEVATION:	212+/-		
EQUIPMENT:	Takeuchi TB180FR Mini-Excavator	WATER DEPTH:			
INSPECTOR:	Kevin Patton				

Depth	USCS Class	Moisture	Description	Notes
0-12"	ML	Slightly moist	Fill. Loose. Grayish brown sandy silt with trace brick, glass, few small roots.	
12-24"	ML	Moist	Native soil. Loose to medium-dense. Mottled brown silt loam.	
24-42"	ML	Moist	Dense. Slightly mottled brown silt with little sand, trace gravel and cobbles.	
42" to 7ft	ML	Moist	Dense. Mottled pale brown silt with little sand, trace gravel and cobbles, few boulders to 24" size.	
7ft - 12ft	SC-SM	Very Moist	Dense. Mottled pale brown sand and silty clay with little gravel, few cobbles and boulders to 24" size.	
12 ft	-	-	Stopped at 12 feet in soil.	



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	PROJECT:	South Hamilton Street, Poughkeepsie, N.Y.		
	DATE:	6/14/2023	Project No.:	23216
	WEATHER:	Cloudy, 65F		

TEST PIT LOG					
EXCAVATING COMPANY:	One Key LLC	LOCATION:	30 ft N from NW corner of proposed building	TEST PIT NUMBER	TP2
OPERATOR:	Garrett	ELEVATION:	205+/-		
EQUIPMENT:	Takeuchi TB180FR Mini-Excavator	WATER DEPTH:			
INSPECTOR:	Kevin Patton				

Depth	USCS Class	Moisture	Description	Notes
0-6"	ML	Slightly moist.	Loose. Grayish brown silt loam with some small roots.	
6-24"	ML	Moist	Medium-dense. Pale orangeish brown silt with some sand, traces gravel, few cobbles.	
24-62"	ML	Moist	Dense. Slightly mottled olive brown silt with some sand, little gravel and few cobbles.	
62"-72"	Rock	-	Top of shale bedrock. Beds nearly vertical, strike northeast. Ripped 12 inches into rock, which was still somewhat soft at this depth.	
72"	-	-	Stopped in rock at 6 feet.	



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	PROJECT:	South Hamilton Street, Poughkeepsie, N.Y.		
	DATE:	6/14/2023	Project No.:	23216
	WEATHER:	Cloudy, 65F		

TEST PIT LOG				
EXCAVATING COMPANY:	One Key LLC	LOCATION:	50 ft N from SW corner of proposed building	TEST PIT NUMBER TP3
OPERATOR:	Garrett	ELEVATION:	214+/-	
EQUIPMENT:	Takeuchi TB180FR Mini-Excavator	WATER DEPTH:		
INSPECTOR:	Kevin Patton			

Depth	USCS Class	Moisture	Description	Notes
0-4"	ML	Moist	Loose. Topsoil. Brown silt loam with fine roots.	
4"-22"	ML	Moist	Medium-dense. Pale orangeish brown silt with some fine-graded sand, trace gravel and cobbles.	
22"-48"	ML	Moist	Dense. Pale orangeish brown silt and fine-graded sand, trace gravel and cobbles, with few small boulders.	
48"-6.5ft	ML	Moist	Very dense. Till. Mottled olive brown silt with little sand, little gravel, few cobbles and boulders.	
6.5-7.5 ft	Rock	-	Top of shale bedrock. Ripped 12 inches into rock with ease.	
7.5 ft	-	-	Stopped in rock.	



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	PROJECT:	South Hamilton Street, Poughkeepsie, N.Y.		
	DATE:	6/14/2023	Project No.:	23216
	WEATHER:	Cloudy, 65F		

TEST PIT LOG				
EXCAVATING COMPANY:	One Key LLC	LOCATION:	North wing of proposed building, S from center	TEST PIT NUMBER TP4
OPERATOR:	Garrett	ELEVATION:	222+/-	
EQUIPMENT:	Takeuchi TB180FR Mini-Excavator	WATER DEPTH:		
INSPECTOR:	Kevin Patton			

Depth	USCS Class	Moisture	Description	Notes
0-18"	ML	Moist	Loose. Fill. Silt with some sand, trace gravel and cobbles, few small boulders. Buried pipes (+/- 1.5" diameter steel, 2" lead.)	
18"-48"	ML	Moist	Dense. Till. Olive brown silt with little sand, little gravel, few cobbles and small boulders.	
48"-7.5ft	ML	Moist	Same, Very dense.	
7.5ft - 8ft	Rock	-	Shale bedrock. Ripped six inches into the surface, with effort.	
8 ft	-	-	Stopped in rock.	



KEVIN L. PATTON, P.E. 36 PATTON ROAD NEWBURGH, NY 12550 PATTONGEOTECH.COM 845 275-7732	CLIENT:	One Key LLC		
	PROJECT:	South Hamilton Street, Poughkeepsie, N.Y.		
	DATE:	6/14/2023	Project No.:	23216
	WEATHER:	Cloudy, 65F		

TEST PIT LOG				
EXCAVATING COMPANY:	One Key LLC	LOCATION:	Inside corner of proposed building	TEST PIT NUMBER TP5
OPERATOR:	Garrett	ELEVATION:	222+/-	
EQUIPMENT:	Takeuchi TB180FR Mini-Excavator	WATER DEPTH:		
INSPECTOR:	Kevin Patton			

Depth	USCS Class	Moisture	Description	Notes
0-40"	ML	Moist	Loose. Fill. Grayish brown silt with some sand, little gravel, few cobbles, bricks and shale fragments.	
40-50"	Bricks	Moist	Loose. Fill. Mostly bricks with little soil mixed in.	
50-60"	Rock	-	Shale bedrock. Ripped to 12", becoming hard.	
60"	-	-	Stopped in rock.	



KEVIN L. PATTON, P.E. 36 PATTON ROAD NEWBURGH, NY 12550 PATTONGEOTECH.COM 845 275-7732	CLIENT:	One Key LLC		
	PROJECT:	South Hamilton Street, Poughkeepsie, N.Y.		
	DATE:	6/14/2023	Project No.:	23216
	WEATHER:	Cloudy, 65F		

TEST PIT LOG				
EXCAVATING COMPANY:	One Key LLC	LOCATION:	East wing of proposed building, near east end	TEST PIT NUMBER TP6
OPERATOR:	Garrett	ELEVATION:	209+/-	
EQUIPMENT:	Takeuchi TB180FR Mini-Excavator	WATER DEPTH:		
INSPECTOR:	Kevin Patton			

Depth	USCS Class	Moisture	Description	Notes
0-10"	ML	Moist	Loose. Grayish brown silt with little sand, trace gravel, few cobbles. Few roots.	
10"-36"	ML	Moist	Medium-dense. Pale orangeish brown silt with little sand, traces gravel, few cobbles, few small boulders.	
36"-58"	ML	Moist	Dense. Pale yellowish brown silt with little sand, trace gravel, few cobbles.	
58"	Rock	-	Top of bedrock. Flat, hard.	
58"	-	-	Stopped on top of rock.	



KEVIN L. PATTON, P.E. 36 PATTON ROAD NEWBURGH, NY 12550 PATTONGEOTECH.COM 845 275-7732	CLIENT: One Key LLC	
	PROJECT: South Hamilton Street, Poughkeepsie, N.Y.	
	DATE: 6/14/2023	Project No.: 23216
	WEATHER: Cloudy, 65F	

TEST PIT LOG					
EXCAVATING COMPANY:	One Key LLC	LOCATION:	Proposed stormwater control area, center	TEST PIT NUMBER	TP7
OPERATOR:	Garrett	ELEVATION:	197.5+/-		
EQUIPMENT:	Takeuchi TB180FR Mini-Excavator	WATER DEPTH:			
INSPECTOR:	Kevin Patton				

Depth	USCS Class	Moisture	Description	Notes
0-2"	PT	Moist	Decomposed organics and leaf-litter. Dusky brown.	
2"-10"	SM	Moist	Loose. Fill. Brownish gray sand with some silt, little gravel, trace glass, small pocket of coal cinders.	
10"-32"	CL	Moist	Native Soil. Medium-dense. Pale yellowish brown, mottled orange. Lean Clay with some fine-graded sand, trace gravel. Massive structure, no significant joints or pores.	
32"-6ft	CL	Wet	Dense. Mottled olive brown-orange-gray Lean Clay with trace gravel, traces sand, trace roots. Dense.	
6 ft	-	-	Stopped in soil.	

No photo available.

KEVIN L. PATTON, P.E. 36 PATTON ROAD NEWBURGH, NY 12550 PATTONGEOTECH.COM 845 275-7732	CLIENT:	One Key LLC		
	PROJECT:	South Hamilton Street, Poughkeepsie, N.Y.		
	DATE:	6/14/2023	Project No.:	23216
	WEATHER:	Cloudy, 65F		

TEST PIT LOG				
EXCAVATING COMPANY:	One Key LLC	LOCATION:	Proposed stormwater control area, north end	TEST PIT NUMBER TP8
OPERATOR:	Garrett	ELEVATION:	197+/-	
EQUIPMENT:	Takeuchi TB180FR Mini-Excavator	WATER DEPTH:		
INSPECTOR:	Kevin Patton			

Depth	USCS Class	Moisture	Description	Notes
0-30"	ML	Moist	Loose. Fill. Dusky brown silt and sand with little gravel, trace concrete, two streetcar rails.	
30"-60"	ML	Very Moist	Native soil. Loose to Medium-dense. Mottled olive brown and gray silt and fine sand. Massive structure. Trace concretions (cemented sand,) 3/8" size, noted in lab sample.	
60"-8.5ft	ML	Wet	Same, wet.	
8.5 ft	-	-	Stopped in soil.	



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CLIENT:	One Key LLC		
PROJECT:	South Hamilton Street, Poughkeepsie, N.Y.		
PROJECT No.:	23216	SAMPLE LOT No.:	230614-1
DATE SAMPLED:	6/14/2023	DATE TESTED:	6/20/2023
SAMPLED BY:	Kevin Patton	TESTED BY:	Wyeth Patton

MOISTURE CONTENT OF SOIL
TEST METHOD: ASTM D2216

Location	DEPTH, FT.	% MOISTURE
TP1	8'	13.6
TP1	11'	14.4
TP2	3'	14.1
TP3	3'	13.7
TP4	4'	13.6
TP7	2'	16.5
TP7	5'	13.5
TP8	4'	20.6

Moisture content is expressed as a percent of the dry mass of the soil.

Reviewed by: *Kevin Patton*

Form NMC

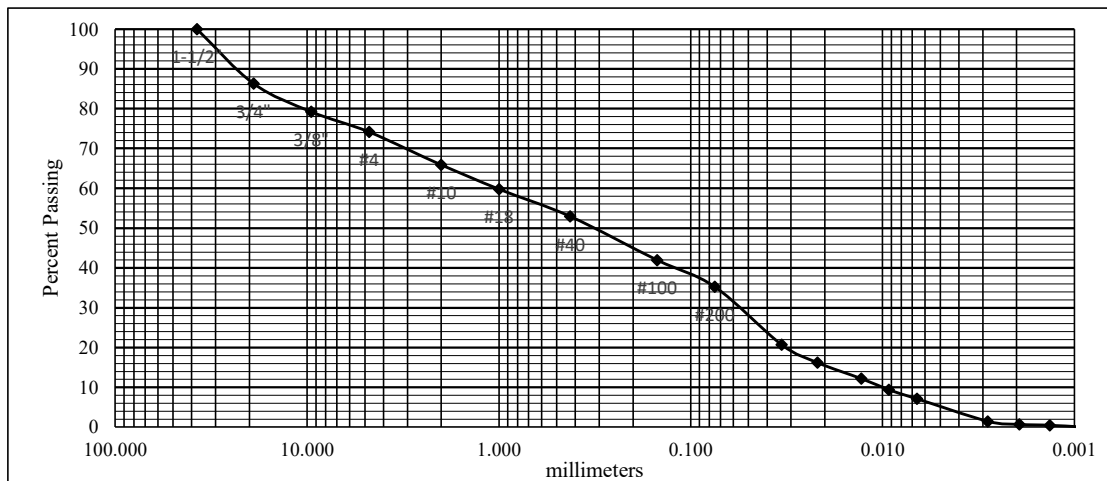
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CLIENT:	One Key LLC		
PROJECT:	South Hamilton Street, Poughkeepsie, N.Y.		
PROJECT No.:	23216	SAMPLE LOT No.:	230614-1
DATE SAMPLED:	6/14/2023	DATE TESTED:	6/20/2023
SAMPLED BY:	Kevin Patton	TESTED BY:	Wyeth Patton

SIEVE-AND-HYDROMETER ANALYSIS TEST REPORT
TEST METHOD(S): ASTM D422, AASHTO T88

Sample Location	TP1
Depth	8 feet

Sieve Size		Percent Retained	Percent Passing	Specification
inches	mm			
1-1/2"	37.5	0	100	
3/4"	19.0	14	86	
3/8"	9.5	7	79	
#4	4.75	5	74	
#10	2.00	8	66	
#18	1.00	6	60	
#40	0.425	7	53	
#100	0.150	11	42	
#200	0.075	7	35	
Hydrometer Analysis	0.050	7	28	
	0.020	13	15	
	0.010	5	10	
	0.005	5	5	
	0.002	4	1	
	0.001	1	0	



USDA Particle Size Classification:	USDA Textural Class: Gravelly Sandy Loam
Gravel, 3" to 2.00mm: 34	USCS Classification (ASTM D2487/D2488): SC-SM, Silty Clayey Sand with Gravel
Sand, 2.00 to 0.050mm: 38	
Silt, 0.050 to 0.002mm: 27	Atterberg Limits were determined by: Test
Clay, <0.002mm: 1	
Total: 100	

Reviewed by: Kevin Patton

Form HYD

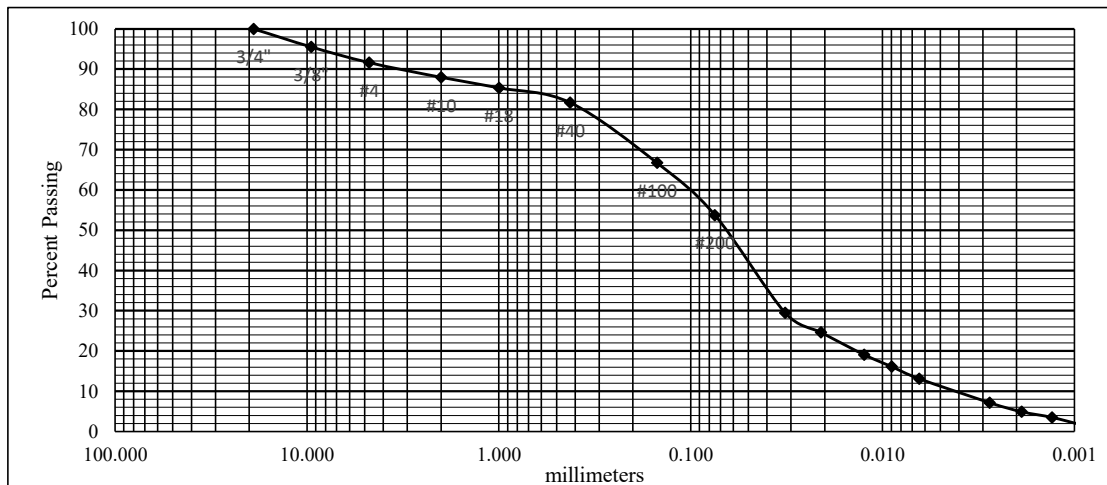
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CLIENT:	One Key LLC		
PROJECT:	South Hamilton Street, Poughkeepsie, N.Y.		
PROJECT No.:	23216	SAMPLE LOT No.:	230614-1
DATE SAMPLED:	6/14/2023	DATE TESTED:	6/20/2023
SAMPLED BY:	Kevin Patton	TESTED BY:	Wyeth Patton

SIEVE-AND-HYDROMETER ANALYSIS TEST REPORT
TEST METHOD(S): ASTM D422, AASHTO T88

Sample Location	TP3
Depth	3 feet

Sieve Size		Percent Retained	Percent Passing	Specification
inches	mm			
3/4"	19.0	0	100	
3/8"	9.5	5	95	
#4	4.75	3	92	
#10	2.00	4	88	
#18	1.00	3	85	
#40	0.425	3	82	
#100	0.150	15	67	
#200	0.075	13	54	
Hydrometer Analysis	0.050	12	42	
	0.020	18	24	
	0.010	7	17	
	0.005	6	11	
	0.002	6	5	
	0.001	3	2	



USDA Particle Size Classification:		USDA Textural Class:	Sandy Loam
Gravel, 3" to 2.00mm	12	USCS Classification (ASTM D2487/D2488): ML, Sandy Silt	
Sand, 2.00 to 0.050mm:	46		
Silt, 0.050 to 0.002mm:	37		
Clay, <0.002mm	5		
Total	100		
		Atterberg Limits were determined by: Estimated (ASTM D2488)	

Reviewed by: *Kevin Patton*

Form HYD

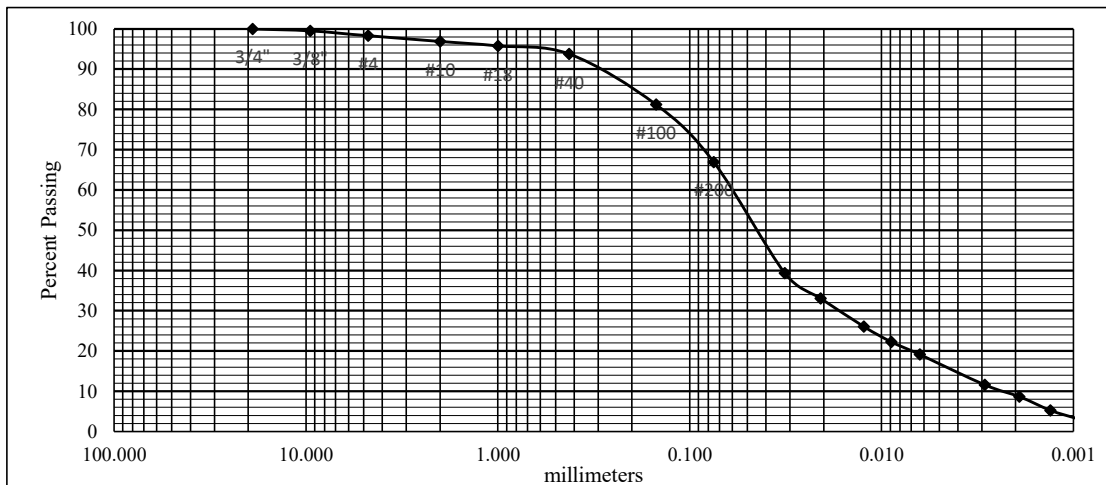
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CLIENT:	One Key LLC		
PROJECT:	South Hamilton Street, Poughkeepsie, N.Y.		
PROJECT No.:	23216	SAMPLE LOT No.:	230614-1
DATE SAMPLED:	6/14/2023	DATE TESTED:	6/20/2023
SAMPLED BY:	Kevin Patton	TESTED BY:	Wyeth Patton

SIEVE-AND-HYDROMETER ANALYSIS TEST REPORT
TEST METHOD(S): ASTM D422, AASHTO T88

Sample Location	TP7
Depth	2 feet

Sieve Size		Percent Retained	Percent Passing	Specification
inches	mm			
3/4"	19.0	0	100	
3/8"	9.5	1	99	
#4	4.75	1	98	
#10	2.00	1	97	
#18	1.00	1	96	
#40	0.425	2	94	
#100	0.150	13	81	
#200	0.075	14	67	
Hydrometer Analysis	0.050	13	54	
	0.020	22	32	
	0.010	9	23	
	0.005	6	17	
	0.002	8	9	
	0.001	6	3	



USDA Particle Size Classification:		USDA Textural Class: Loam
Gravel, 3" to 2.00mm	3	USCS Classification (ASTM D2487/D2488): CL, Sandy Clay
Sand, 2.00 to 0.050mm:	43	
Silt, 0.050 to 0.002mm:	45	
Clay, <0.002mm	9	
Total	100	Atterberg Limits were determined by: Test

Reviewed by: Kevin Patton

Form HYD

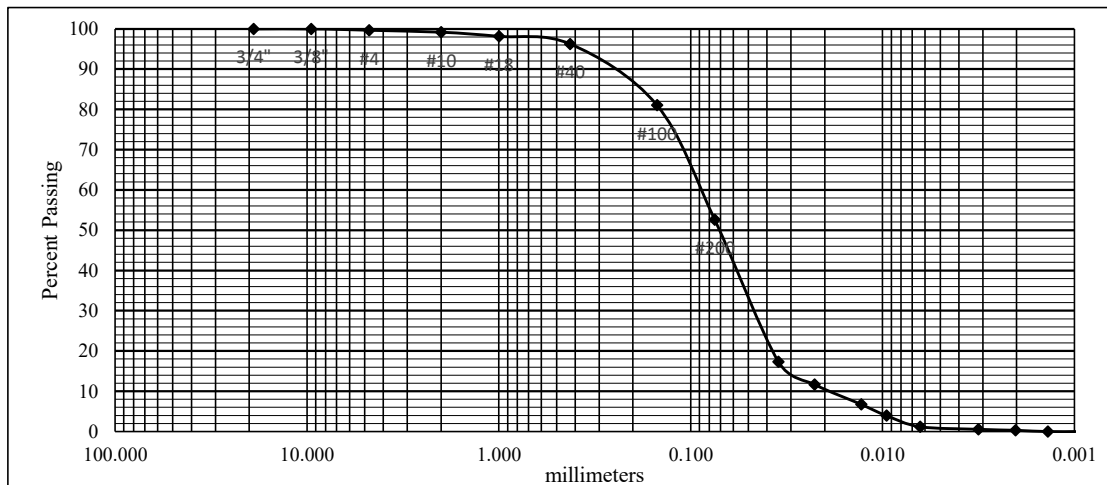
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CLIENT:	One Key LLC		
PROJECT:	South Hamilton Street, Poughkeepsie, N.Y.		
PROJECT No.:	23216	SAMPLE LOT No.:	230614-1
DATE SAMPLED:	6/14/2023	DATE TESTED:	6/20/2023
SAMPLED BY:	Kevin Patton	TESTED BY:	Wyeth Patton

SIEVE-AND-HYDROMETER ANALYSIS TEST REPORT
TEST METHOD(S): ASTM D422, AASHTO T88

Sample Location	TP8
Depth	4 feet

Sieve Size		Percent Retained	Percent Passing	Specification
inches	mm			
3/4"	19.0	0	100	
3/8"	9.5	0	100	
#4	4.75	0	100	
#10	2.00	1	99	
#18	1.00	1	98	
#40	0.425	2	96	
#100	0.150	15	81	
#200	0.075	28	53	
Hydrometer Analysis	0.050	20	33	
	0.020	23	10	
	0.010	6	4	
	0.005	3	1	
	0.002	1	0	
	0.001	0	0	



USDA Particle Size Classification:		USDA Textural Class:	Sandy Loam
Gravel, 3" to 2.00mm	1	USCS Classification (ASTM D2487/D2488):	ML, Sandy Silt
Sand, 2.00 to 0.050mm:	66		
Silt, 0.050 to 0.002mm:	33		
Clay, <0.002mm	0		
Total	100	Atterberg Limits were determined by: Estimated (ASTM D2488)	

Reviewed by: Kevin Patton

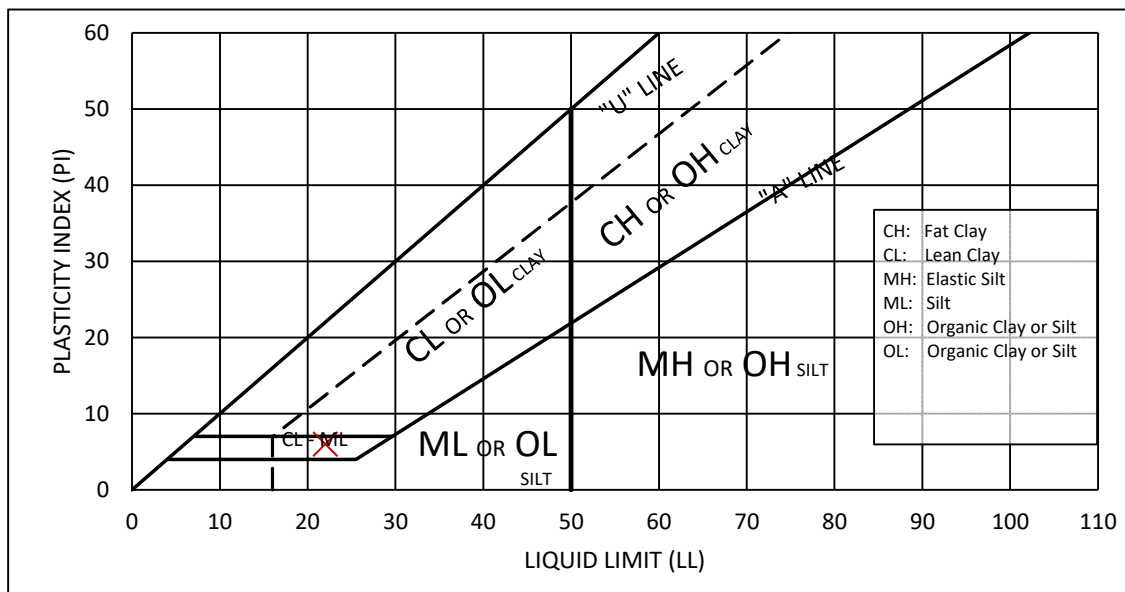
Form HYD

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NEWBURGH, NY 12550
845 275-7732 PATTONGEOTECH.COM

CLIENT:	One Key LLC		
PROJECT:	South Hamilton Street, Poughkeepsie, N.Y.		
PROJECT No.:	23216	SAMPLE LOT No.:	230614-1
DATE SAMPLED:	6/14/2023	DATE TESTED:	6/20/2023
SAMPLED BY:	Kevin Patton	TESTED BY:	Wyeth Patton

ATTERBERG LIMITS TEST
TEST METHODS: ASTM D4318/ AASHTO T89, T90

Sample Location	TP1
Depth	8 feet
Percent Passing #40	53
Liquid Limit (LL)	22
Plastic Limit (PL)	16
Plasticity Index (PI)	6
USCS Class of -#40	CL-ML, Silty Clay



LL, PL and PI values are percent moisture of the soil by dry mass.

Test is performed on the 'matrix' fraction of the soil, finer than the #40 (0.425mm) sieve.

The Liquid Limit is the moisture content at which the matrix fraction of the soil changes from a stiff to a flowing consistency. The plastic limit is the moisture content at which it changes from cohesive to crumbly. The Plasticity Index is the Liquid Limit minus the Plastic Limit.

Reviewed by: *Kevin Patton*

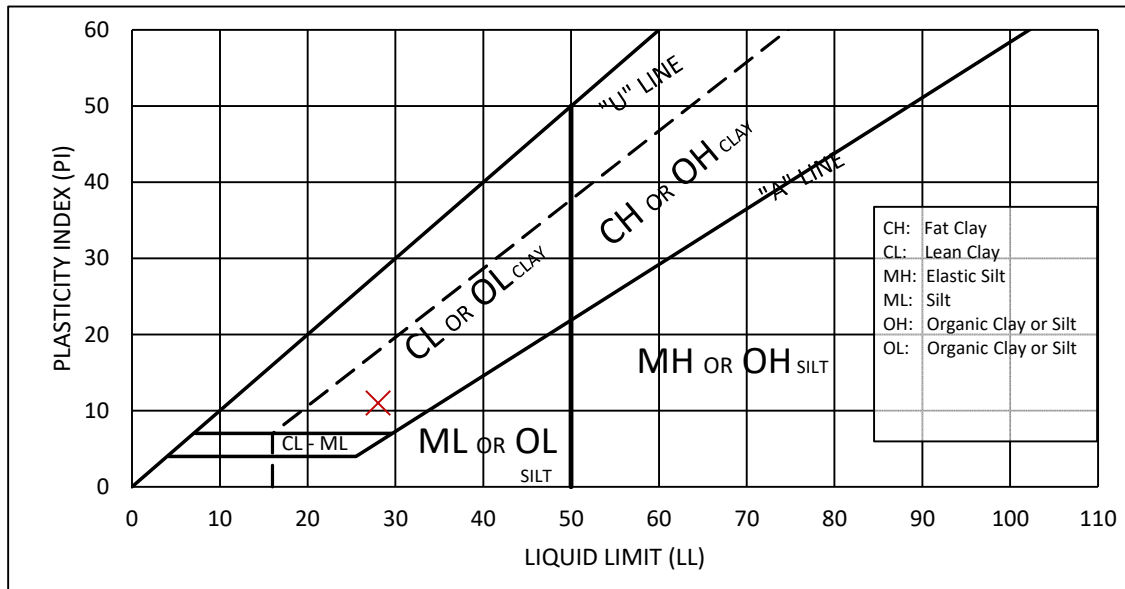
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KEVIN L. PATTON, P.E.
36 PATTON ROAD
NEWBURGH, NY 12550
845 275-7732 PATTONGEOTECH.COM

CLIENT:	One Key LLC		
PROJECT:	South Hamilton Street, Poughkeepsie, N.Y.		
PROJECT No.:	23216	SAMPLE LOT No.:	230614-1
DATE SAMPLED:	6/14/2023	DATE TESTED:	6/20/2023
SAMPLED BY:	Kevin Patton	TESTED BY:	Wyeth Patton

ATTERBERG LIMITS TEST
TEST METHODS: ASTM D4318/ AASHTO T89, T90

Sample Location	TP7
Depth	2 feet
Percent Passing #40	94
Liquid Limit (LL)	28
Plastic Limit (PL)	17
Plasticity Index (PI)	11
USCS Class of -#40	CL, Lean Clay



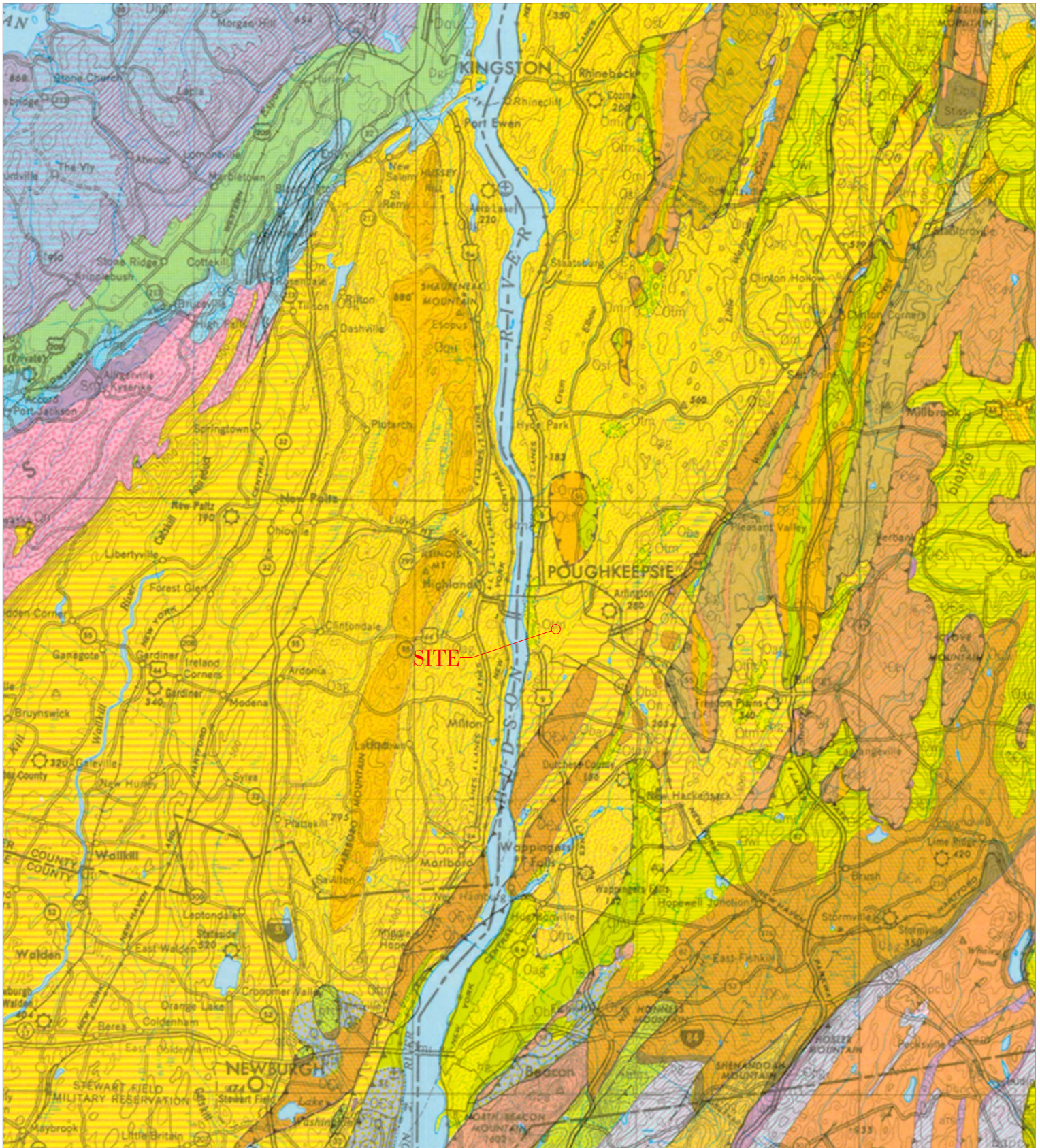
LL, PL and PI values are percent moisture of the soil by dry mass.

Test is performed on the 'matrix' fraction of the soil, finer than the #40 (0.425mm) sieve.

The Liquid Limit is the moisture content at which the matrix fraction of the soil changes from a stiff to a flowing consistency. The plastic limit is the moisture content at which it changes from cohesive to crumbly. The Plasticity Index is the Liquid Limit minus the Plastic Limit.

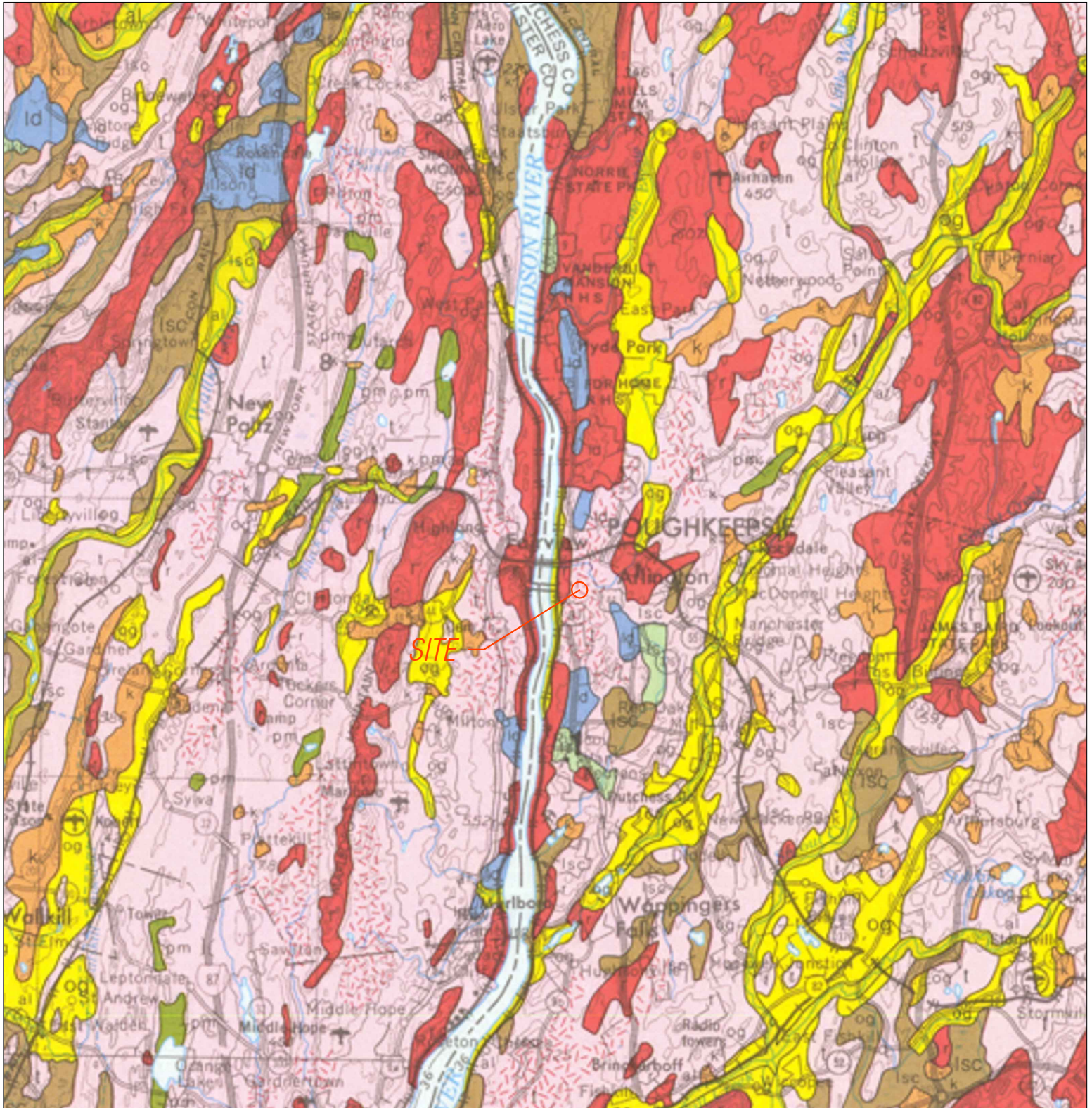
Reviewed by: *Kevin Patton*

Form ABL



Approximate location of the project site, shown on part of the Bedrock Geologic Map of New York (1970.) The site is in an area of Ordovician-age sedimentary bedrock consisting mostly of gray siltstone, shale and greywacke sandstone. The bedrock at the site is shown as Austin Glen Formation (mottled green, symbol Oag,) which is generally bounded to the east and west by the somewhat older Normanskill shale (yellow with red stripes, symbol On,) and is overlain by the younger Quassaic quartzite in the high areas (orange, Oqu.) Taconic melange (green, Otm,) is indicated along the Hudson River in Poughkeepsie, to the west of the site; it is composed of slaty, sheared Austin Glen bedrock. To the southeast, the older Wappinger dolostone (brown, OCw,) is down-faulted against the Normanskill Formation bedrock.

SURFICIAL GEOLOGIC MAP



Approximate location of the project site, shown on a partial copy of the Surficial Geologic Map of New York (N.Y. State Museum, 1989.) The map indicates that the soils at and near the site are glacial till, shown in pink with the symbol 't'. A red stipple pattern is also shown, indicating that bedrock is generally within three to ten feet of the surface. Areas where bedrock is mostly very shallow or is exposed are indicated by red. These map units are generalized; alluvium, outwash and lacustrine deposits have all been encountered in parts of the City of Poughkeepsie where till is indicated by the map.

EXPLANATION

	al — Recent deposits Generally confined to floodplains within a valley, oxidized, non-calcareous, fine sand to gravel, in larger valleys may be overlain by silt, subject to frequent flooding, thickness 1-10 meters.		fg — Fluvial sand and gravel Deposits of sand and gravel, occasional laterally continuous lenses of silt, deposition farther from glacier, age uncertain.
	alf — Alluvial fan Fan shaped accumulations, poorly stratified silt, sand and boulders, at the foot of steep slopes, generally permeable.		k — Kame deposits Includes kames, eskers, kame terraces, kame deltas, coarse to fine gravel and/or sand, deposition adjacent to ice, lateral variability in sorting, coarseness and thickness, locally firmly cemented with calcareous cement, thickness variable (10-30 meters).
	b — Beach Sand and gravel deposit at marine shoreline, thickness variable.		km — Kame moraine Variable texture (size and sorting) from boulders to sand, deposition at an ice margin during deglaciation, positive constructional relief, locally cemented with calcareous cement, thickness variable (10-30 meters).
	bi — Barrier island Sand and gravel deposit as barrier island, south shore of Long Island, may have associated dunes, thickness variable.		tm — Till moraine More variably sorted than till, generally more permeable than till, deposition adjacent to ice, more variably drained, may include ablation till, thickness variable (10-30 meters).
	pm — Swamp deposits Peat-muck, organic silt and sand in poorly drained areas, un-oxidized, may be overlying marl and lake silts, potential land instability, thickness generally 2-20 meters.		t — Till Variable texture (e.g. clay, silt-clay, boulder clay), usually poorly sorted diamict, deposition beneath glacier ice, relatively impermeable (loamy matrix), variable clast content — ranging from abundant well-rounded diverse lithologies in valley tills to relatively angular, more limited lithologies in upland tills, tends to be sandy in areas underlain by gneiss or sandstone, potential land instability on steep slopes, thickness variable (1-50 meters).
	ld — Lacustrine delta Coarse to fine gravel and sand, stratified, generally well sorted, deposited at a lake shoreline, thickness variable (3-15 meters).		af — Artificial fill
	lsc — Lacustrine silt and clay Generally laminated silt and clay, deposited in proglacial lakes, generally calcareous, potential land instability, thickness variable (up to 100 meters).		r — Bedrock Exposed or generally within 1 meter of surface.
	ls — Lacustrine sand Sand deposits associated with large bodies of water, generally a near-shore deposit or near a sand source, well sorted, stratified, generally quartz sand, thickness variable (2-20 meters).		Bedrock stipple overprint Bedrock may be within 1-3 meters of surface, may sporadically crop out, variable mantle of rock debris and glacial till.
	og — Outwash sand and gravel Coarse to fine gravel with sand, proglacial fluvial deposition, well rounded and stratified, generally finer texture away from ice border, thickness variable (2-20 meters).		

MAP SYMBOLS



Contact

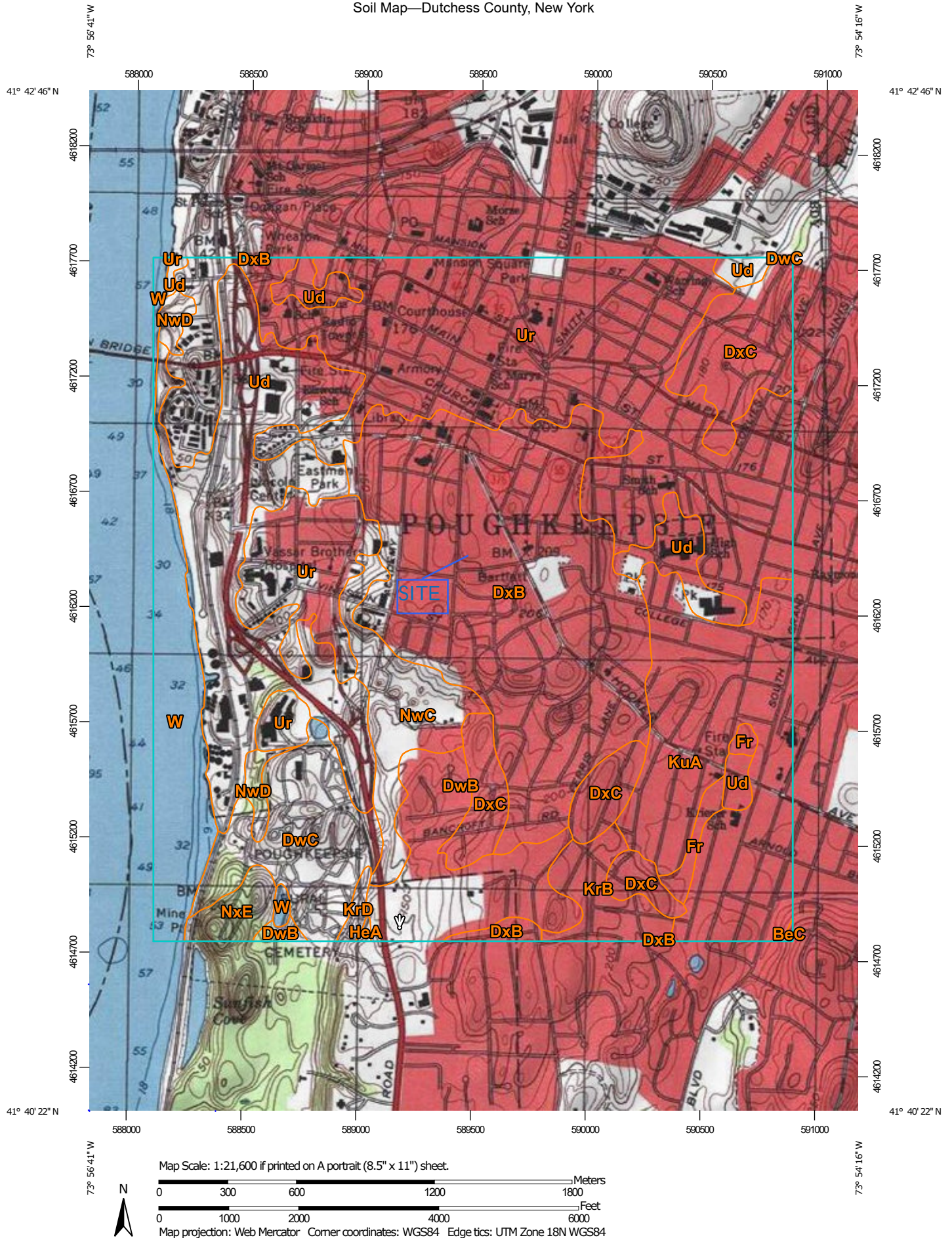
.3

Dated radiocarbon locality

MAP KEY NEW YORK STATE SURFICIAL GEOLOGIC MAP

KEVIN L. PATTON, P.E.

Soil Map—Dutchess County, New York



Natural Resources
Conservation Service

Web Soil Survey
National Cooperative Soil Survey

4/21/2023
Page 1 of 3
Page 650 of 752

MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features



Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow



Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot



Spoil Area



Stony Spot



Very Stony Spot



Wet Spot



Other



Special Line Features

Background



Topographic Map

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Dutchess County, New York

Survey Area Data: Version 19, Sep 10, 2022

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
BeC	Bernardston silt loam, 8 to 15 percent slopes	0.6	0.0%
DwB	Dutchess-Cardigan complex, undulating, rocky	102.3	5.0%
DwC	Dutchess-Cardigan complex, rolling, rocky	96.6	4.7%
DxB	Dutchess-Cardigan-Urban land complex, undulating, rocky	399.6	19.5%
DxC	Dutchess-Cardigan-Urban land complex, rolling, rocky	123.4	6.0%
Fr	Fredon silt loam	12.3	0.6%
HeA	Haven loam, nearly level	1.0	0.1%
KrB	Knickerbocker fine sandy loam, undulating	36.0	1.8%
KrD	Knickerbocker fine sandy loam, hilly	5.2	0.3%
KuA	Knickerbocker-Urban land complex, nearly level	214.3	10.5%
NwC	Nassau-Cardigan complex, rolling, very rocky	68.8	3.4%
NwD	Nassau-Cardigan complex, hilly, very rocky	14.7	0.7%
NxE	Nassau-Rock outcrop complex, steep	19.6	1.0%
Ud	Udorthents, smoothed	286.3	14.0%
Ur	Urban land	565.5	27.6%
W	Water	104.2	5.1%
Totals for Area of Interest		2,050.6	100.0%

Dutchess County, New York

DxB—Dutchess-Cardigan-Urban land complex, undulating, rocky

Map Unit Setting

National map unit symbol: 9rfr

Elevation: 20 to 1,000 feet

Mean annual precipitation: 41 to 47 inches

Mean annual air temperature: 45 to 50 degrees F

Frost-free period: 115 to 195 days

Farmland classification: Not prime farmland

Map Unit Composition

Dutchess and similar soils: 30 percent

Cardigan and similar soils: 25 percent

Urban land: 20 percent

Minor components: 25 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Dutchess

Setting

Landform: Ridges, hills

Landform position (two-dimensional): Summit

Landform position (three-dimensional): Crest

Down-slope shape: Convex

Across-slope shape: Convex

Parent material: Loamy till derived mainly from phyllite, slate, schist, and shale

Typical profile

H1 - 0 to 8 inches: silt loam

H2 - 8 to 28 inches: silt loam

H3 - 28 to 86 inches: channery silt loam

Properties and qualities

Slope: 1 to 6 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Capacity of the most limiting layer to transmit water

(Ksat): Moderately high to high (0.57 to 1.98 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Available water supply, 0 to 60 inches: High (about 9.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: B

Ecological site: F144AY034CT - Well Drained Till Uplands
Hydric soil rating: No

Description of Cardigan

Setting

Landform: Ridges, hills
Landform position (two-dimensional): Summit
Landform position (three-dimensional): Crest
Down-slope shape: Convex
Across-slope shape: Convex
Parent material: Loamy till or colluvium derived from phyllite, slate, shale, and schist

Typical profile

H1 - 0 to 8 inches: channery silt loam
H2 - 8 to 20 inches: channery loam
H3 - 20 to 30 inches: channery silt loam
H4 - 30 to 34 inches: unweathered bedrock

Properties and qualities

Slope: 1 to 6 percent
Depth to restrictive feature: 20 to 40 inches to lithic bedrock
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Low to moderately low (0.00 to 0.06 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Low (about 4.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 2e
Hydrologic Soil Group: C
Ecological site: F144AY034CT - Well Drained Till Uplands
Hydric soil rating: No

Description of Urban Land

Typical profile

H1 - 0 to 6 inches: variable

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 8s
Hydric soil rating: Unranked

Minor Components

Udorthents

Percent of map unit: 10 percent
Hydric soil rating: No

Georgia

Percent of map unit: 5 percent

Hydric soil rating: No

Nassau

Percent of map unit: 5 percent

Hydric soil rating: No

Massena

Percent of map unit: 4 percent

Hydric soil rating: No

Sun

Percent of map unit: 1 percent

Landform: Depressions

Hydric soil rating: Yes

Data Source Information

Soil Survey Area: Dutchess County, New York

Survey Area Data: Version 19, Sep 10, 2022

Engineering Properties

This table gives the engineering classifications and the range of engineering properties for the layers of each soil in the survey area.

Hydrologic soil group is a group of soils having similar runoff potential under similar storm and cover conditions. The criteria for determining Hydrologic soil group is found in the National Engineering Handbook, Chapter 7 issued May 2007(<http://directives.sc.egov.usda.gov/OpenNonWebContent.aspx?content=17757.wba>). Listing HSGs by soil map unit component and not by soil series is a new concept for the engineers. Past engineering references contained lists of HSGs by soil series. Soil series are continually being defined and redefined, and the list of soil series names changes so frequently as to make the task of maintaining a single national list virtually impossible. Therefore, the criteria is now used to calculate the HSG using the component soil properties and no such national series lists will be maintained. All such references are obsolete and their use should be discontinued. Soil properties that influence runoff potential are those that influence the minimum rate of infiltration for a bare soil after prolonged wetting and when not frozen. These properties are depth to a seasonal high water table, saturated hydraulic conductivity after prolonged wetting, and depth to a layer with a very slow water transmission rate. Changes in soil properties caused by land management or climate changes also cause the hydrologic soil group to change. The influence of ground cover is treated independently. There are four hydrologic soil groups, A, B, C, and D, and three dual groups, A/D, B/D, and C/D. In the dual groups, the first letter is for drained areas and the second letter is for undrained areas.

The four hydrologic soil groups are described in the following paragraphs:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

Depth to the upper and lower boundaries of each layer is indicated.

Texture is given in the standard terms used by the U.S. Department of Agriculture. These terms are defined according to percentages of sand, silt, and clay in the fraction of the soil that is less than 2 millimeters in diameter. "Loam," for example, is soil that is 7 to 27 percent clay, 28 to 50 percent silt, and less than 52 percent sand. If the content of particles coarser than sand is 15 percent or more, an appropriate modifier is added, for example, "gravelly."

Classification of the soils is determined according to the Unified soil classification system (ASTM, 2005) and the system adopted by the American Association of State Highway and Transportation Officials (AASHTO, 2004).

The Unified system classifies soils according to properties that affect their use as construction material. Soils are classified according to particle-size distribution of the fraction less than 3 inches in diameter and according to plasticity index, liquid limit, and organic matter content. Sandy and gravelly soils are identified as GW, GP, GM, GC, SW, SP, SM, and SC; silty and clayey soils as ML, CL, OL, MH, CH, and OH; and highly organic soils as PT. Soils exhibiting engineering properties of two groups can have a dual classification, for example, CL-ML.

The AASHTO system classifies soils according to those properties that affect roadway construction and maintenance. In this system, the fraction of a mineral soil that is less than 3 inches in diameter is classified in one of seven groups from A-1 through A-7 on the basis of particle-size distribution, liquid limit, and plasticity index. Soils in group A-1 are coarse grained and low in content of fines (silt and clay). At the other extreme, soils in group A-7 are fine grained. Highly organic soils are classified in group A-8 on the basis of visual inspection.

If laboratory data are available, the A-1, A-2, and A-7 groups are further classified as A-1-a, A-1-b, A-2-4, A-2-5, A-2-6, A-2-7, A-7-5, or A-7-6. As an additional refinement, the suitability of a soil as subgrade material can be indicated by a group index number. Group index numbers range from 0 for the best subgrade material to 20 or higher for the poorest.

Percentage of rock fragments larger than 10 inches in diameter and 3 to 10 inches in diameter are indicated as a percentage of the total soil on a dry-weight basis. The percentages are estimates determined mainly by converting volume percentage in the field to weight percentage. Three values are provided to identify the expected Low (L), Representative Value (R), and High (H).

Percentage (of soil particles) passing designated sieves is the percentage of the soil fraction less than 3 inches in diameter based on an oven-dry weight. The sieves, numbers 4, 10, 40, and 200 (USA Standard Series), have openings of 4.76, 2.00, 0.420, and 0.074 millimeters, respectively. Estimates are based on laboratory tests of soils sampled in the survey area and in nearby areas and on estimates made in the field. Three values are provided to identify the expected Low (L), Representative Value (R), and High (H).

Liquid limit and plasticity index (Atterberg limits) indicate the plasticity characteristics of a soil. The estimates are based on test data from the survey area or from nearby areas and on field examination. Three values are provided to identify the expected Low (L), Representative Value (R), and High (H).

References:

American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.

American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.

Report—Engineering Properties

Absence of an entry indicates that the data were not estimated. The asterisk "*" denotes the representative texture; other possible textures follow the dash. The criteria for determining the hydrologic soil group for individual soil components is found in the National Engineering Handbook, Chapter 7 issued May 2007(<http://directives.sc.egov.usda.gov/OpenNonWebContent.aspx?content=17757.wba>). Three values are provided to identify the expected Low (L), Representative Value (R), and High (H).

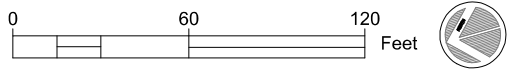
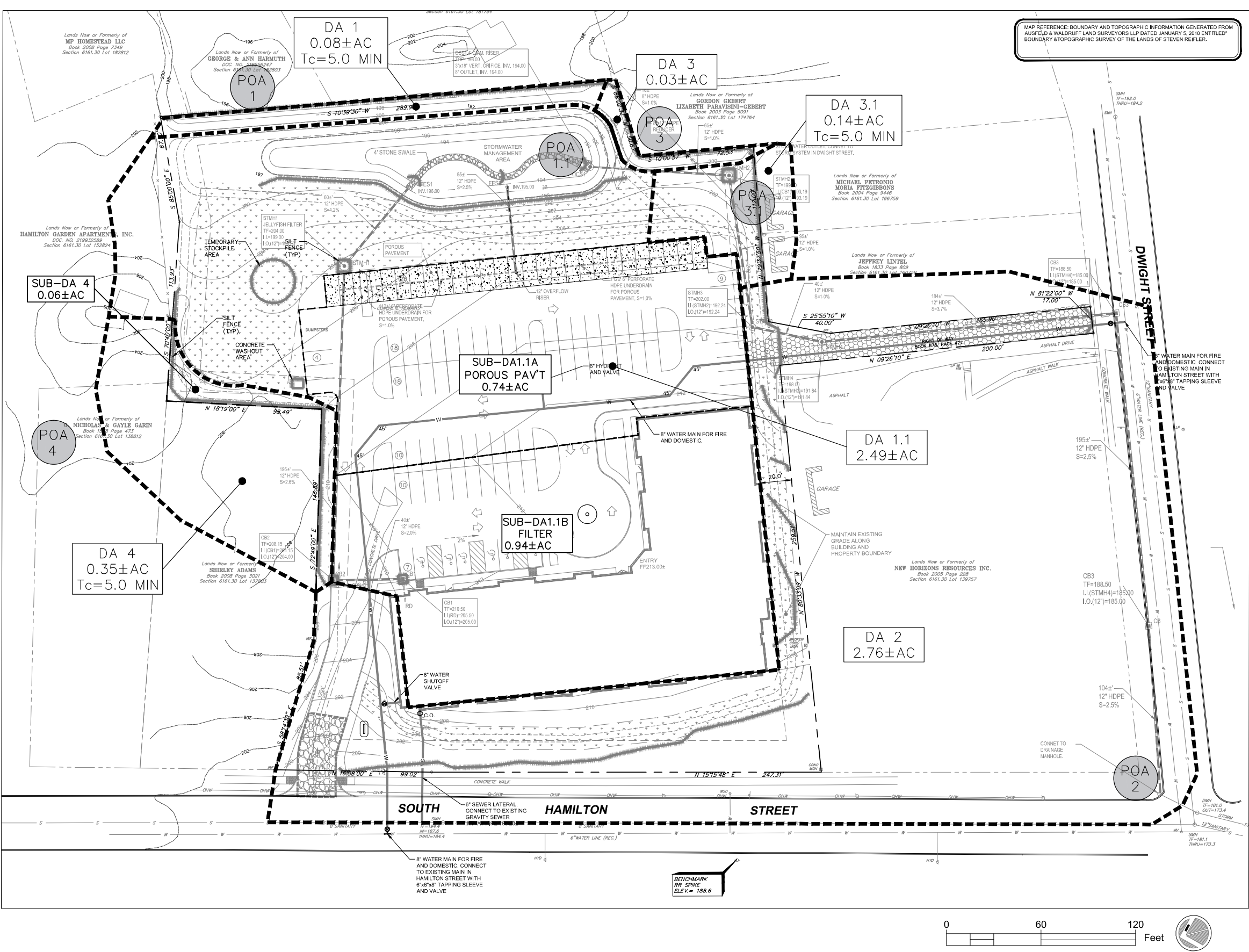


Engineering Properties--Dutchess County, New York														
Map unit symbol and soil name	Pct. of map unit	Hydrologic group	Depth	USDA texture	Classification		Pct Fragments		Percentage passing sieve number--				Liquid limit	Plasticity index
					Unified	AASHTO	>10 inches	3-10 inches	4	10	40	200		
			<i>In</i>				<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>
DxB--Dutchess-Cardigan-Urban land complex, undulating, rocky														
Dutchess	30	B	0-8	Silt loam	ML	A-4	0- 0- 0	0- 8- 15	95-98-100	90-93-95	80-88-95	55-70-85	15-24-33	NP-2 -4
			8-28	Silt loam, channery silt loam, gravelly loam	GM, ML, SM	A-4	0- 0- 0	0- 0- 20	70-92-95	65-90-95	55-80-95	40-70-85	15-24-33	NP-2 -4
			28-86	Channery silt loam, very channery fine sandy loam, very gravelly sandy loam	GM, ML, SM	A-1-b, A-4, A-2-4	0- 0- 0	5-15- 25	55-68-80	50-63-75	30-53-75	15-40-65	15-20-25	NP-2 -4
Cardigan	25	C	0-8	Channery silt loam	ML, SM	A-4	0- 0- 0	0- 3- 5	80-88-95	70-83-95	60-68-75	35-53-70	15-24-33	NP-3 -5
			8-20	Silt loam, channery silt loam, channery loam	ML, SM	A-4	0- 0- 0	0- 5- 10	80-88-95	70-83-95	60-68-75	35-53-70	15-24-33	NP-3 -5
			20-30	Silt loam, loam, channery silt loam	ML, SM	A-1, A-4, A-2	0- 0- 0	0- 5- 10	80-88-95	60-75-90	40-58-75	20-45-70	15-24-33	NP-3 -5
			30-34	Unweathered bedrock	—	—	0- 0- 0	0- 0- 0	—	—	—	—	—	—
Urban land	20		0-6	Variable	—	—	0- 0- 0	0- 0- 0	—	—	—	—	—	—

Data Source Information

Soil Survey Area: Dutchess County, New York
 Survey Area Data: Version 19, Sep 10, 2022





ISSUED FOR REVIEW

NOT FOR CONSTRUCTION

RECORD PLANS

DATE SUBMITTED: 05/13/23

REVISIONS

NO.	PER CITY COMMENTS	DESCRIPTION
1		

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ENGINEERS LAND PLANNERS & CONSULTANTS

1407 ROUTE 30, Box 2 Suite 6, Clifton Park, New York 12065 518.267.3685 aricoassociates@arico.com

CITY OF POUGHKEEPSIE COUNTY OF DUTCHESS

POST-DEVELOPMENT DRAINAGE SOUTH HAMILTON STREET

LOCATED AT 100 SOUTH HAMILTON STREET

TAX MAP: 6161-30-15163

DATE: AUGUST 2023

JOB NO. 22.10

SCALE: AS NOTED

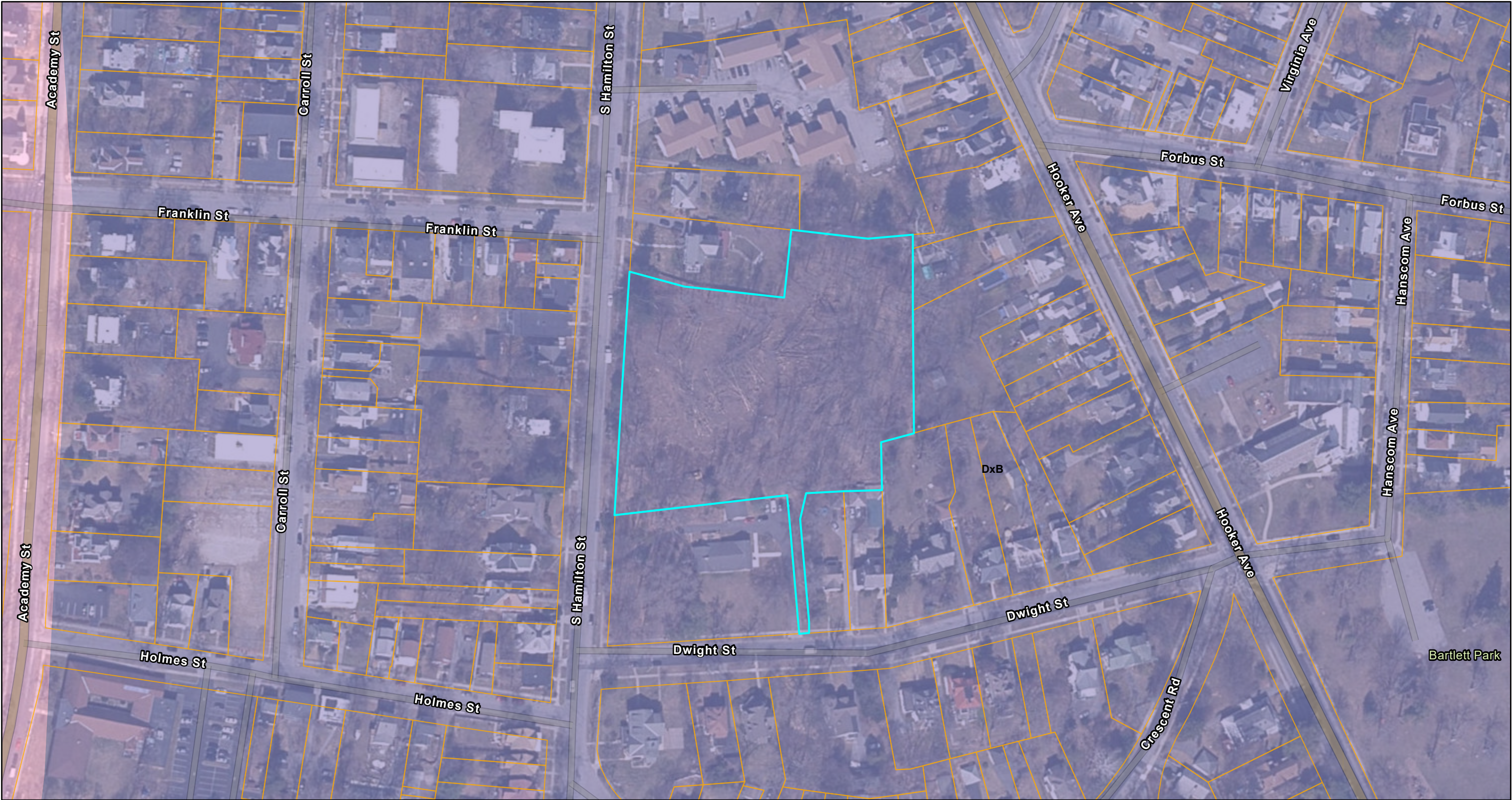
DRAWN BY: DFA

CHECKED BY: DFA

DRAWING NO. SWPPP 2.0

SHEET 1 OF 1

Poughkeepsie NRI Map: Geology and Soils



6/20/2023, 9:58:45 AM

City of Poughkeepsie Boundary

Parcel Boundaries

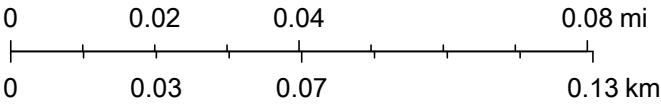
Soils

Bedrock Geology

Graywacke, shale (Oag)

Taconic melange (Otm)

1:2,257



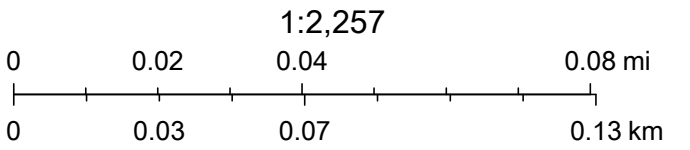
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Poughkeepsie NRI Map: Habitats



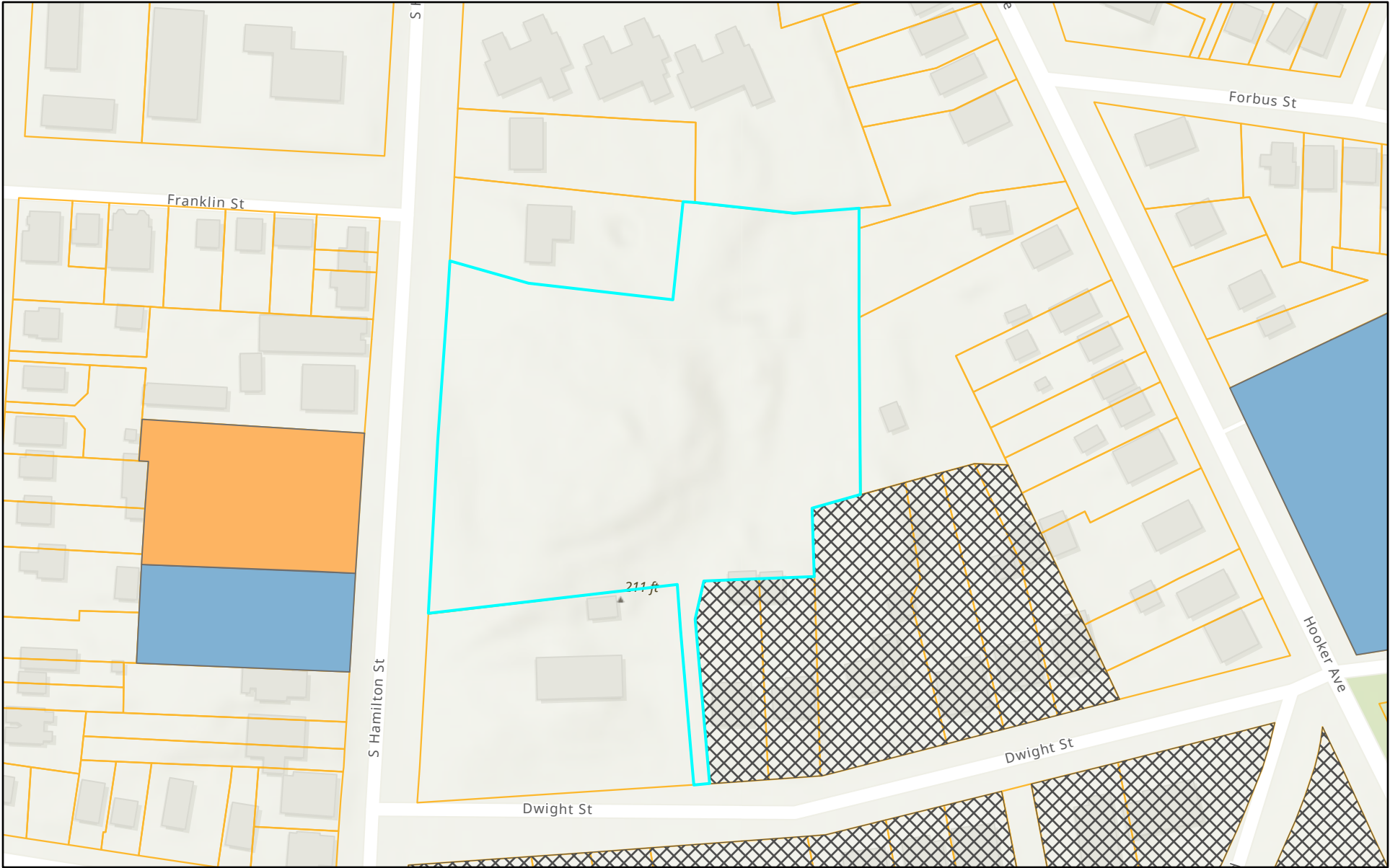
6/20/2023, 9:34:54 AM

- Parcel Boundaries
- City of Poughkeepsie Boundary
- Hudsonia Habitats
- Developed
- Cultural
- Upland hardwood forest



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NRI Map: Historic Resources



6/20/2023, 9:47:30 AM

Parcel Boundaries

Historic Sites (2021)

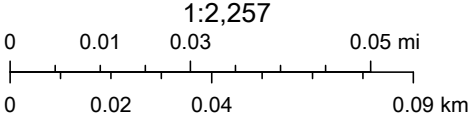
City of Poughkeepsie Boundary

Historic Districts (2021)

Local, State, and National

Local, State and National

State and National



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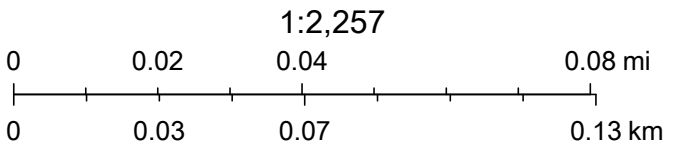
ArcGIS Web AppBuilder

Poughkeepsie NRI Map: Recreation Resources



6/20/2023, 9:49:52 AM

-  Parcel Boundaries
-  City of Poughkeepsie Boundary
-  Parks and Open Spaces



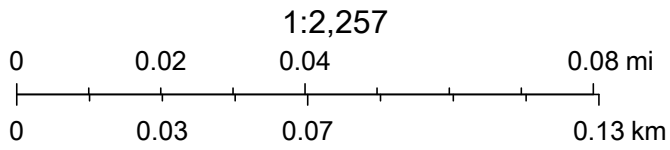
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Poughkeepsie NRI Map: Water Resources



6/20/2023, 9:41:56 AM

-  Parcel Boundaries
-  City of Poughkeepsie Boundary



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APPLICATION TO THE PLANNING BOARD
CITY OF POUGHKEEPSIE, NEW YORK

Application for Site Plan, Special Permit & Facade Approval

I. **PROPERTY ADDRESS:** South Avenue, City of Poughkeepsie, Dutchess Co., New York;
Tax Parcel ID 6061-52-937583

II. **PROPERTY OWNER:** South Ave Real Est Assoc LLC
ADDRESS: 510 Haight Avenue,
Poughkeepsie, NY 12603

New York, 12603 (state and zip code)

PHONE NUMBER: 914-494-2610 (include area code)

III. **NAME OF APPLICANT:** ARGO Development (Contact: Mr. Michael Abuladze, CEO)

ADDRESS: 95 Chestnut Ridge Road,
Montvale, NJ 07645

New Jersey, 07045 (state and zip code)

PHONE NUMBER: 914-494-2610 (include area code)

EMAIL ADDRESS: mike@argodev.com

(If applicant is NOT the owner, proof of owner's consent to the application MUST be provided.)

IV. **CONSULTANT:** Christopher Lapine, P.E., LEED AP

ADDRESS: 21 Fox Street,
Poughkeepsie, NY 12601

New York, 12601 (state and zip code)

PHONE NUMBER: 845-486-1478 (include area code)

V. **PROPOSED USE or FAÇADE CHANGE** (Summarize proposed use or uses): _____

87-unit, 5-story mid-rise multifamily residential development with underground and surface-level parking.

VI. **ZONING DISTRICT:** R-D, Research and Development District

OFFICE USE ONLY

CODE: PLSF

ID # _____

FEE: _____

Total, from:
\$500, plus
\$ _____
(\$150 per 1000
square feet of floor
area), plus
\$ _____
(\$35 per number of
required parking
spaces), plus
\$ _____
(\$500 for special
permit, if applicable)

(Proceed to XVII for special permit & façade approval only)

PROJECT INFORMATION

New construction: X (yes) (no)

Change of use: (yes) X (no)

Expansion/addition: (yes) X (no)

Alteration: (yes) X (no)

Cost of Construction: \$ 9.3 million

LOT AND BULK INFORMATION:

Lot area: 2.76 (acres) ~120,226 (square feet)

Building Footprint: 22,640 (proposed square feet for new construction)

 NA (existing square feet for change of use/expansion)

 NA (additional proposed square feet for expansion)

 22,640 (total building footprint, square feet)

Lot Coverage: ~19% (percentage, building coverage only)

Gross Floor Area: 113,200 (proposed square feet for new construction)

 NA (existing square feet for change of use/expansion)

 NA (additional proposed square feet for expansion)

 113,200 (total gross floor area, square feet)

Building Height: 5 stories of MF residential units over one level of underground parking. (stories) 70 (feet)

Floor Area Ratio: 0.94 (ratio of gross floor area divided by lot area)

VIII. PROJECT PARKING INFORMATION

A) Number of Off-Street Parking Spaces required: 143.5

B) Number of Off-Street Parking Spaces provided: 144

C) Waiver Requested: X (Yes) (No)

Is municipal parking available within 600 feet? (Yes) X (No)

If yes, name all such municipal facilities: NA

(If a waiver is requested based on proximity to municipal facilities, a written request for such waiver, addressed to the City Planning Staff and the Chairman of the Planning Board, must be submitted with the application.)

If a private parking facility is to be utilized, is such facility within 600 feet?

_____ (Yes) X (No)

If yes, provide the address of the facility, name of the property owner, number of spaces in the lot, number of spaces available for lease, and length of lease. Attach copy of lease. Additional information may be required by the Planning Board):

NA

IX. RESIDENTIAL PROPOSALS

A) Type of Development: Multifamily residential development with underground and surface-level parking.
(Townhouse, condominium, multiple residence rental, etc.)

B) Unit Breakdown:

Number of efficiency or "studio" units:	<u>0</u>
Number of one (1) bedroom units:	<u>87</u>
Number of two (2) bedroom unit's	<u>0</u>
Number with three or more bedrooms:	<u>0</u>
Total number of units:	<u>87</u>

C) Is funding public? _____ Yes X No

If yes, describe funding source: ARGO Development

NA **X. OFFICE PROPOSALS** (_____ Medical/Dental _____ Professional _____ Business)

A) Number of Employees: _____

B) Number of doctors/dentists/medical practitioners: _____

C) Days/Hours of Operation: _____

NA **XI. SERVICE BUSINESS PROPOSALS**

- A) Specify Business: _____
(e.g., laundromat, drycleaner, beauty parlor, travel agency, banks)
- B) Number of Employees: _____
- C) Number of Washing Machines (for laundromat): _____
- D) Days/Hours of Operation: _____

NA **XII. MERCANTILE PROPOSALS** (Retail _____ Wholesale _____)

- A) Type of Mercantile (specify): _____
- B) Number of Employees: _____
- C) Days/Hours of Operation: _____

NA **XIII. STANDARD RESTAURANT/FAST FOOD RESTAURANT/COFFEE SHOP/DONUT SHOP/NIGHTCLUB/DISCOTHEQUE/BAR PROPOSALS**

- A) Specify Use: _____
- B) Number of seats (excluding bar stools and outdoor seating): _____
- C) Meals served (i.e., breakfast, lunch, dinner): _____
- D) Type of Menu: _____
- E) Days/Hours of Operation: _____
- F) Entertainment (live and/or recorded): _____
- G) Will the restaurant contain a bar/lounge?: _____ Yes _____ No
- H) If yes, how many seats at the bar? _____
- I) Is a drive-through window proposed? _____
- J) Is a walk-up window proposed? _____
- K) Is an outdoor seating area proposed? _____
- L) If yes, how many seats are proposed? _____
- M) If yes, is outdoor cooking proposed? _____

NA **XIV. INDUSTRIAL/COMMERCIAL PROPOSALS** (Automobile repair, taxi, motor vehicle sales, warehouse, manufacturing, animal hospital, research facilities, etc.)

- A) Specify Use: _____
- B) Number of shifts (if any): _____
- C) Number of employees: _____
(If shifts are proposed, list number of employees per shift)
- E) Number of work bays (if automobile repair): _____
- F) Days/Hours of operation: _____

NA **XV. EDUCATIONAL/INSTITUTIONAL/RECREATIONAL PROPOSALS** (e.g., Museums, Theatres, Conference Centers, Hotels, Motels, Hospitals, Nursing Homes, Assisted Living Facilities, Nursery Schools, Private Schools, Day Care Centers, Places of Worship, Marinas, Membership Clubs, etc.)

A) Specify: _____

B) Number of shifts (if any): _____

C) Number of employees: _____
(If shifts are proposed, list number of employees per shift)

D) Number of beds (hospitals, nursing homes, etc): _____

E) Number of seats in largest assembly space: _____

NA

F) Number of classrooms (schools): _____

G) Number of rental units (hotels, motels, etc.) _____

H) Number of boat slips/courts (e.g., tennis, handball, etc.), alleys: _____

I) Days/Hours of Operation: _____

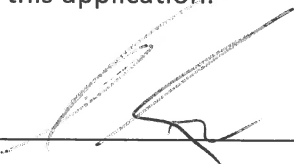
XVI. PROPOSALS NOT MENTIONED ABOVE (Please be specific):

NA

XVII. APPLICANT CERTIFICATION

I, Mr. Michael Abuladze, CEO of ARGO Development, certify that the this application has addressed the objectives outlined in the Zoning Ordinance with consideration given to the public health, safety and welfare; the comfort and convenience of the public in general or the residents or users of the proposed development and of the immediate neighborhood.

The applicant certifies that all information contained herein is accurate and complete as of the date of this application.



Signature of Applicant

11/29/22

Date

November 14, 2023

Ms. Anne Saylor, Chairperson
City of Poughkeepsie Planning Board
63 Civic Center Plaza
Poughkeepsie, NY 12601

Re: **ARGO Development – South Avenue Multi-Family Housing**
South Avenue, Poughkeepsie, NY, 12601
Tax Lot 937583

Dear Chair Saylor:

ARGO Development, obtained conditional site plan approval for an 87-unit, mid-rise multifamily residential development with underground and surface-level parking. The proposed development is located on an approximately 2.76-acre parcel on the west side of South Avenue (Tax Lot 937583) in the City of Poughkeepsie. The parcel is located within the R-D, Research and Development District. Residential development is a permitted use within the R-D zone, subject to the requirements of the High-Density R-5 District Permitted Uses. The proposed action included the construction of a five-story building with 87, one-bedroom apartments located over one level of underground parking (50 spaces), and 55 off-street surface parking spaces (105 total parking spaces). The Zoning Board of Appeals granted a variance for the relief of 26 parking spaces. Primary access is to be provided via South Avenue.

Since obtaining conditional site plan approval, the Applicant has continued to seek the approval of the DCDOH (Water and sewer) and NYSDOT (drainage connection) to address the remaining site plan issues. Additionally, the Applicant has been progressing the development of construction plans associated with the building and site. As a result, there have been a few amendments to the approved architectural elevations and site plans for which, the Applicant wishes to obtain an approval from the Planning Board for. The following modifications were made to the approved elevations and site plans.

- Per the Applicant's request, the roof terrace have been enlarged. Due to Building Code regulations, this required an additional means of egress as well as ADA Compliant access and access to restrooms. Stair 1 bulkhead was approved on the roof under the initial application, but this amendment shows the extension of Stair 2 to satisfy the secondary means of egress requirement. To satisfy the ADA accessibility requirement, the building elevators have been extended to the roof level. It has been designed to open into a glass vestibule which also provides the shelter for the Building Code required bathrooms. These Applicant and building code-driven changes requires an amendment to the bulk.
- To satisfy MEP requirements, a 14" louver was added to a window at the southern corner of the building on the first floor. This provides fresh air to the fitness room which is accessible to the building tenants.
- The South Avenue entrance has been redesigned due to constructability issues. Since the South Avenue lobby is one and a half stories tall, the originally designed punched windows would interrupt the structural slab. The redesigned storefront now proposes as a curtain wall surrounded by the originally approved zinc façade with the same door and mullions as the previously approved central courtyard entrance.



- The approved Equitone fiber cement panels have been discontinued, and therefore the Applicant is proposing to replace it with Nichiha fiber cement panels.
- Due to the final design of the columns in the parking garage, two spaces were required to be relocated to the exterior resulting in 48 underground parking spaces and 57 exterior parking spaces. No changes in overall parking have occurred. The site plan, landscaping plans, and truck maneuvering plan were updated accordingly.
- Due to the cost of relocating the existing utility pole with transformer located at the entry way, the Applicant has incorporated an island at the point of ingress/egress to preserve the utility pole. The site plan, landscaping plans, and truck maneuvering plan were updated accordingly.

Please find enclosed:

- 1 copy of the site plan
- 1 copy of Sheet C130 – Site Plan, last revised 11/14/2023
- 1 copy of Sheet C131 – Truck Maneuvering Plan, last revised 11/14/2023
- 1 copy of Sheet C180 – Landscape Plan, last revised 11/14/2023
- 1 copy of Sheet A300 – Exterior Elevation I, last revised November 1, 2023
- 1 copy of Sheet A301 – Exterior Elevation II, last revised November 1, 2023
- Check for \$500 Amendment Fee

At this time the Applicant wishes to be placed on the next available Planning Board agenda to discuss the changes specified herein for an amended approval. If you should have any questions or comments during your review of the submitted documents, feel free to reach out to me directly at (845) 486-1478.

Sincerely,

Christopher Lapine, PE
LaBella Associates | Senior Civil Engineer
Regional Marketing Leader

cc: Michael Abuladze
Devora Schwart, AIA
Mathew Tether, AIA

It is a violation of New York Education Law Article 145 Sec.7209, for any person, unless acting under the direction of a licensed architect, professional engineer, or land surveyor, to alter an item in any way. If an item bearing the seal of an architect, engineer, or land surveyor is altered; the altering architect, engineer, or land surveyor shall affix to the item their seal and notation "altered by" followed by their signature and date of such alteration, and a specific description of the alteration.

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APPLICANT NAME & ADDRESS:
ARGO DEVELOPMENT
95 CHESTNUT RIDGE ROAD
MONTVALE, NJ 07645

PROJECT NAME & LOCATION:
**SOUTH AVE
MULTI-FAMILY DEVELOPMENT**
SOUTH AVENUE
CITY OF POUGHKEEPSIE, NY 12601

7	11/14/23	REVISED PER BUILDING PLANS
6	06/13/23	REVISED PER UPDATED STORMWATER DESIGN
5	05/22/23	REVISED PER ARCHITECT
4	04/18/23	REVISED PER CITY COMMENTS
3	04/04/23	REVISIONS PER INTERNAL COORDINATION
2	03/07/23	REVISED PER PLANNING BOARD COMMENTS
1	01/27/23	REVISED PER CITY COMMENTS
NO.	DATE:	DESCRIPTION:

PROJECT NUMBER: 2222201

DRAWN BY:

REVIEWED BY: CPL

ISSUED FOR: PLANNING BOARD REVIEW

DATE: 11/28/2022

DRAWING NAME:

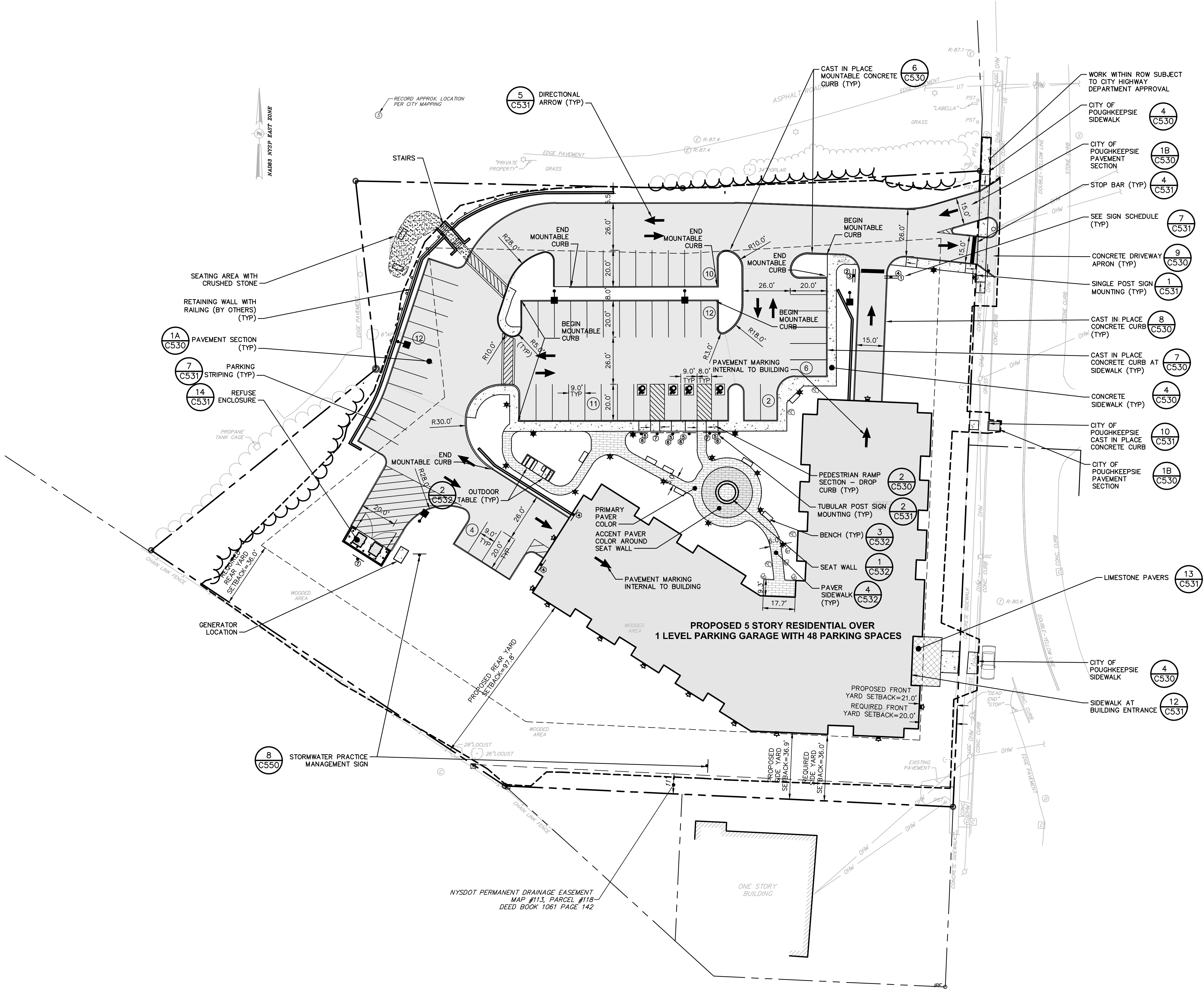
SITE PLAN

DRAWING NUMBER:

C130

SITE LEGEND:

- PROPERTY LINE
- SETBACK LINE
- BUILDING
- PAVEMENT
- ROAD CENTER LINE
- SIDEWALK
- CURB
- WORK LIMITS
- CONCRETE SURFACE
- ROAD LINING & STRIPING
- SIGN
- 2 POSTED SIGN
- BOLLARD
- MAILBOX
- PARKING COUNT



1 SITE PLAN
C-130 SCALE: 1" = 30'

SOUTH AVENUE MULTI-FAMILY DEVELOPMENT

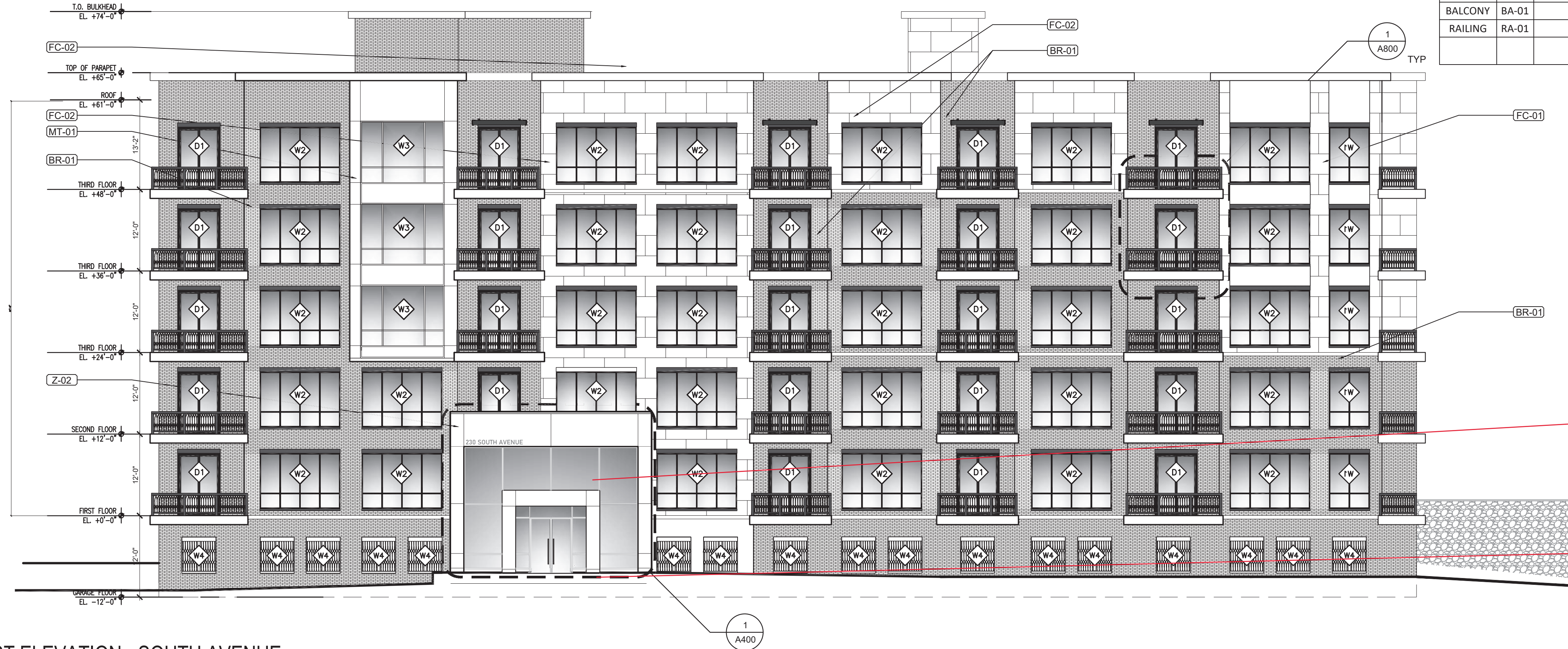


8/1/2023 10:27:30 AM



1 SOUTH ELEVATION

SCALE: 3/32" = 1'-0"



2 EAST ELEVATION - SOUTH AVENUE

SCALE: 3/32" = 1'-0"

NOTES

1. FOR WINDOW TYPES AND SCHEDULES, SEE SHEET A920

ORIGINAL APPROVED ENTRANCE



90% CD SUBMISSION	11/01/2023
DD SUBMISSION	09/27/2023
SD SUBMISSION	08/31/2023
No. REVISION/SUBMISSION	DATE

CIVIL ENGINEER



FA/FP ENGINEER



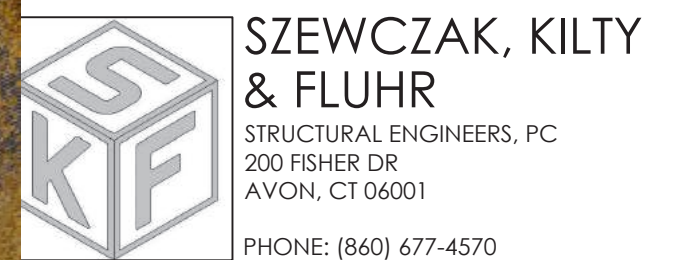
WHITMAN
ENGINEERING P.C.
2600 SOUTH RD. STE 44-260
POUGHKEEPSIE, NY 12601
PHONE: (845) 471-6036

MEP ENGINEER



181 CHURCH STREET
POUGHKEEPSIE, NY 12601
PHONE: +38 044 332 0322
MOBILE: +38 093 332 0322

STRUCTURAL ENGINEER



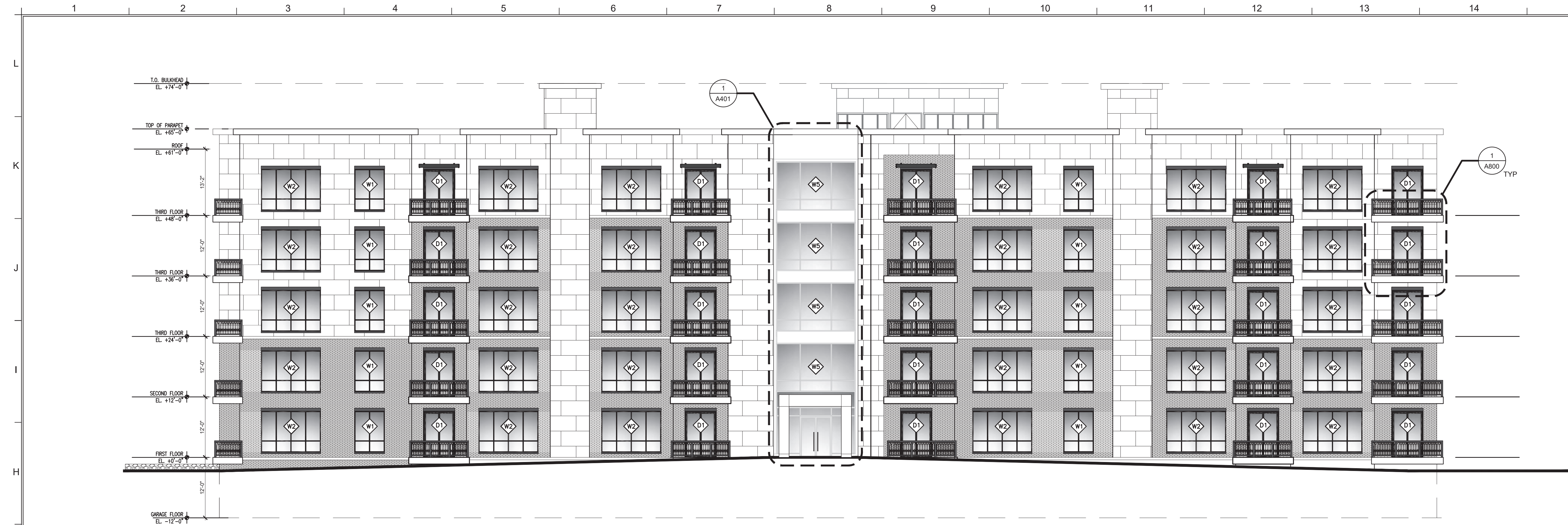
181 CHURCH STREET
POUGHKEEPSIE, NEW YORK 12601
PHONE 845-452-2268 FAX 845-452-3752

ARCHITECTURE
POUGHKEEPSIE MID-RISE MULTIPLE
Y OF POUGHKEEPSIE, NY

EXTERIOR ELEVATIONS I

SEAL	DRAWN BY CQ/DS	PROJECT NUMBER 22054
	CHECKED BY MM	DATE 9-27-2023
	DRAWING NUMBER	

A300



1 WEST ELEVATION - INTERIOR COURTYARD

SCALE: 3/32" = 1'-0"

2 NORTHWEST ELEVATION - INTERIOR COURTYARD

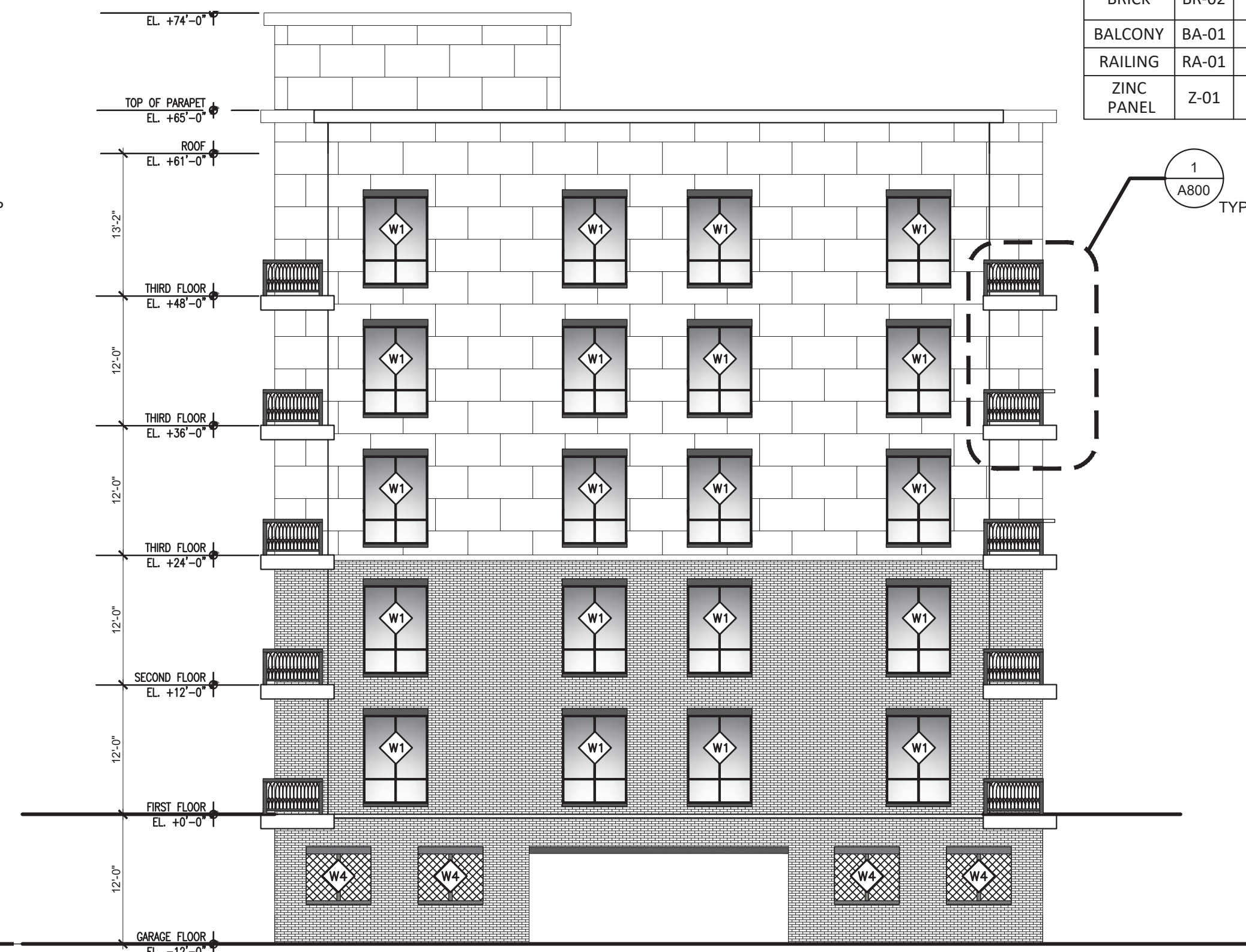
SCALE: 3/32" = 1'-0"

EXTERIOR FINISHES					
TYPE	TAG	PHOTO	BRAND	MODEL	LOCATION
METAL PANEL	MT-01		FAIRVIEW ARCHITECTURAL	SMOKE SILVER METALIC FV9134	
	MT-02		FAIRVIEW ARCHITECTURAL	ANODIC SATIN BLACK FV5292	
FIBRE CEMENT	FC-01		NICHIHA	TBD	
	FC-02		NICHIHA	TBD	
BRICK	BR-02		BELDEN BRICK COMPANY	MORA CERAMICAS - NERO SMOOTH	
BALCONY	BA-01				
RAILING	RA-01			WROUGHT IRON	
ZINC PANEL	Z-01		RHEINZINK	GRANUM BASALTE	SOUTH AVE LOBBY ENTRANCE



3 NORTH ELEVATION

SCALE: 3/32" = 1'-0"



4 NORTHEAST ELEVATION

SCALE: 3/32" = 1'-0"

NOTES

1. FOR WINDOW TYPES AND SCHEDULES, SEE SHEET A920

90% CD SUBMISSION	11/01/2023
DD SUBMISSION	09/27/2023
SD SUBMISSION	08/31/2023
No. REVISION/SUBMISSION	DATE

CIVIL ENGINEER



FA/FP ENGINEER



WHITMAN
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PHONE: (845) 471-6036

MEP ENGINEER



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MOBILE: +38 093 332 0322

STRUCTURAL ENGINEER



SZEWCAK, KILTY
& FLUHR
STRUCTURAL ENGINEERS, PC
200 FISHER DR
AVON, CT 06001
PHONE: (860) 677-4570



181 CHURCH STREET
POUGHKEEPSIE, NEW YORK 12601
PHONE 845-452-2268 FAX 845-452-3752

PROJECT
SOUTH AVENUE PARCEL
POUGHKEEPSIE MID-RISE MULTIPLE
CITY OF POUGHKEEPSIE, NY

DRAWING

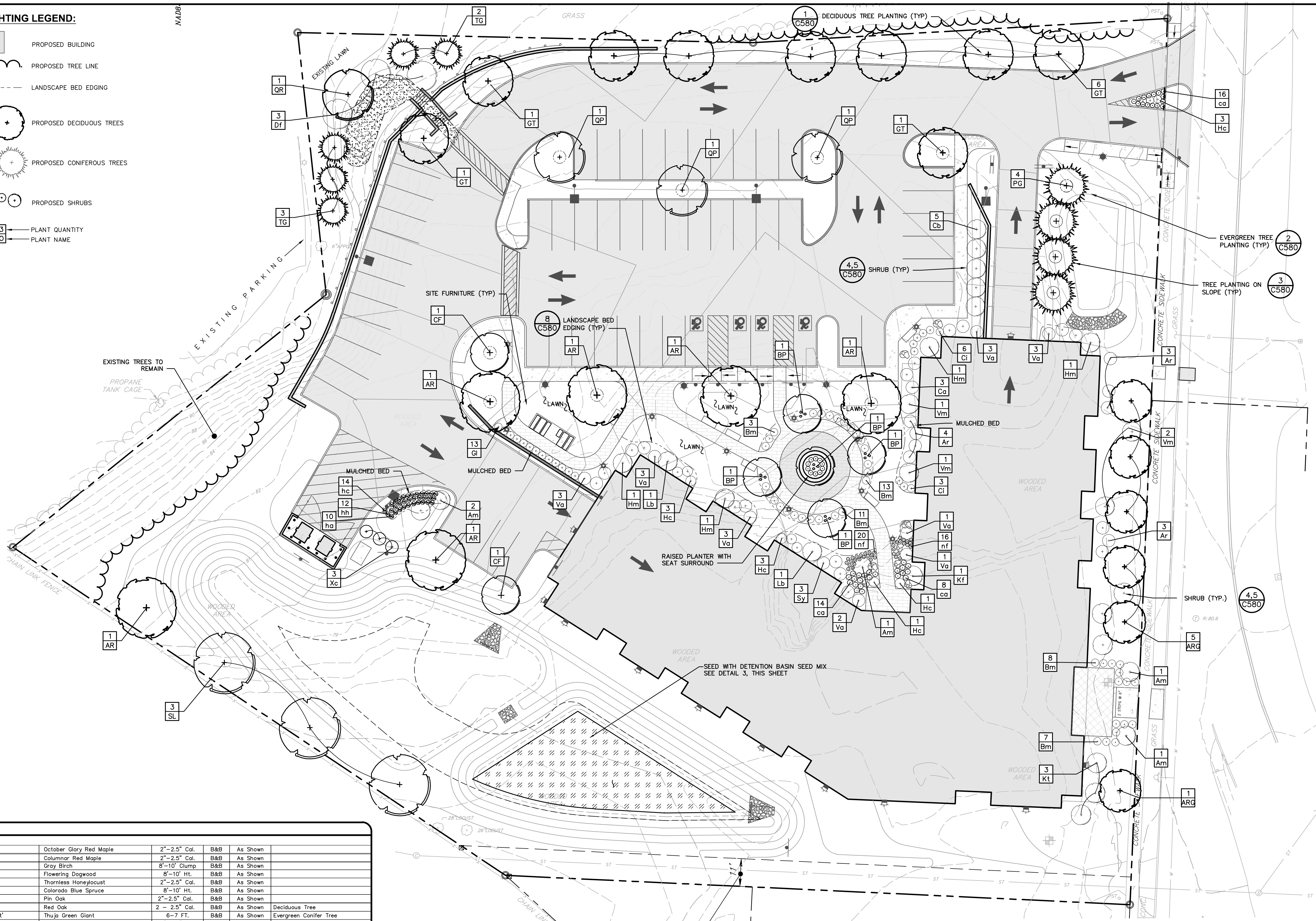
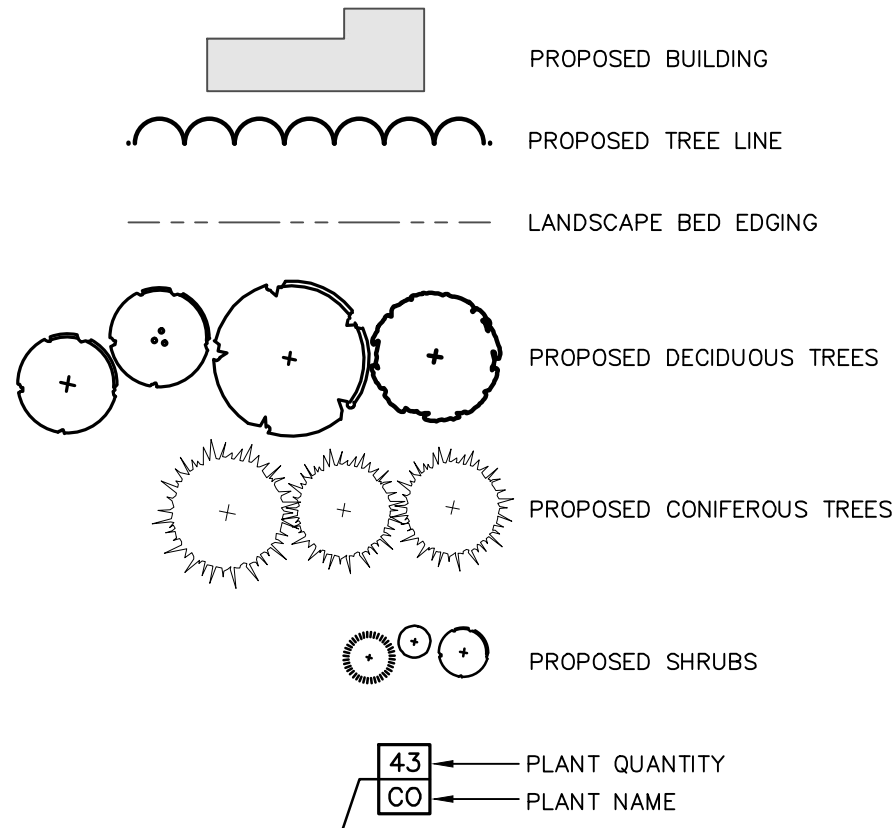
EXTERIOR ELEVATIONS II

SEAL	DRAWN BY CQ/DS	PROJECT NUMBER 22054
	CHECKED BY MM	DATE 9-27-2023
	DRAWING NUMBER	

A301

FILE: C:\Users\pouhkeepsie\OneDrive\Documents\22054\22054-01\22054-01.dwg

LANDSCAPING & LIGHTING LEGEND:



PLANT LIST

QTY	ABRV						
TREES							
6	AR	Acer rubrum 'Red Sunset'	October Glory Red Maple	2"-2.5" Cal.	B&B	As Shown	
6	ARG	Acer rubrum 'Columnar Red Maple'	Columnar Red Maple	2"-2.5" Cal.	B&B	As Shown	
5	BP	Betula populifolia	Gray Birch	8"-10" Clump	B&B	As Shown	
2	CF	Cornus florida	Flowering Dogwood	8"-10" Ht.	B&B	As Shown	
9	GT	Gleditsia triacanthos inermis	Thornless Honeylocust	2"-2.5" Cal.	B&B	As Shown	
4	PG	Picea pungens glauca	Colorado Blue Spruce	8"-10" Ht.	B&B	As Shown	
3	QP	Quercus palustris	Pin Oak	2"-2.5" Cal.	B&B	As Shown	
1	QR	Quercus rubra	Red Oak	2" - 2.5" Cal.	B&B	As Shown	Deciduous Tree
5	TG	Thuja plicata x standishii 'Green Giant'	Thuja Green Giant	6-7 FT.	B&B	As Shown	Evergreen Conifer Tree
3	SL	Salix Lutea	Yellow Willow	2"-2.5" Cal.	B&B	As Shown	
SHRUBS							
5	Am	Amelanchier canadensis	Serviceberry	42"-48" Ht.	B&B	As Shown	
10	Ar	Aronia arbutifolia 'Brilliantissima'	Brilliant Red Chokeberry	30"-36" Ht.	Cont.	As Shown	
41	Bm	Buxus x 'Green Mountain'	'Green Mountain' Boxwood	24"-30" Ht.	Cont.	3" O.C.	
3	Ca	Clethra alnifolia 'Hummingbird'	Hummingbird Summersweet	30"-36" Ht.	Cont.	As Shown	
9	Cl	Comptonia peregrina	Sweet Fern	24"-30" Ht.	Cont.	As Shown	
5	Cb	Cornus sericea baileyi	Bailey Red Twig Dogwood	42"-48" Ht.	B&B	As Shown	
13	Gl	Gonolobus virginicus	Banana Plant	24"-30" Ht.	Cont.	As Shown	
4	Hm	Hamamelis virginiana	Witch Hazel	42"-48" Ht.	B&B	As Shown	
11	Hc	Hydrangea macrophylla 'Mini Penny'	Mini Penny Hydrangea	30"-36" Ht.	Cont.	4" O.C.	
1	Kf	Kalmia latifolia	Mountain Laurel	42"-48" Ht.	B&B	As Shown	
2	Lb	Lindera benzoin	Spicebush	42"-48" Ht.	B&B	As Shown	
3	Sy	Symphoricarpos x doorenbosii 'Marleen'	Marleen Coral Berry	30"-36" Ht.	Cont.	As Shown	
19	Va	Viburnum acerifolium	Mapleleaf Viburnum	30"-36" Ht.	Cont.	As Shown	
4	Vm	Viburnum latatum 'Mohican'	Mohican Viburnum	42"-48" Ht.	B&B	As Shown	
3	DF	Viburnum plicatum f. tomentosum 'Marisii'	Doublefile Viburnum, marisii	5-6 FT.	B&B	As Shown	Deciduous Shrub
3	Xc	Ilex crenata 'Stedds'	Stedds Upright Holly	6 FT.	B&B	As Shown	
PERNNIALS/GRASSES							
36	nm	Adiantum pedatum	Northern Maidenhair Fern	#3	Cont.	24" O.C.	mulched bed
36	ca	Calamagrostis x acutiflora 'Karl Foerster'	Foerster's Feather Reed Grass	#3	Cont.	As Shown	
13	hc	Hemerocallis 'Happy Returns'	Happy Returns Dwarf Daylily	#3	Cont.	18" O.C.	White & Purple Flowering Variety
12	hh	Hemerocallis 'Hyperion'	Hyperion Daylily	#3	Cont.	24" O.C.	Canary yellow, mid / late summer
11	ha	Hemerocallis 'Autumn Accent'	Autumn Accent Daylily	#3	Cont.	24" O.C.	crimson, late summer bloom
10	pa	Pennisetum alopecuroides 'Hameln'	Hameln Fountain Grass	#3	Cont.	24" O.C.	

2 LANDSCAPE PLAN
SCALE: 1" = 20'

1 PLANT LIST
SCALE: NTS

* ALL DISTURBED AREAS NOT SPECIFICALLY CALLED OUT FOR NEW IMPROVEMENTS SHALL BE TOPSOILED, SEEDED WITH LAWN SEED MIX, & MULCHED (TYP)

DETENTION BASIN - PLANTING LIST

Retention Basin Wildlife Mix
Item Number: ERNMX-127

Grasses & Grass-like Species, Herbaceous Flowering Species, Stormwater Management, Wildlife Habitat & Food Plots
Life Cycle:
Perennial
Typical Height:
4.5 ft

20 lbs per acre, or 0.5-1 lb/1,000 sq ft with a cover crop. For a cover crop use one of the following: grain rye (1 Sep to 30 Apr; 30 lbs/acre), Japanese millet (1 May to 31 Aug; 10 lbs/acre), or barnyard grass (1 May to 31 Aug; 10 lbs/acre).

3 DETENTION POND - PLANT LIST
SCALE: NTS

It is a violation of New York Education Law Article 145 Sec.7209, for any person, unless acting under the direction of a licensed architect, professional engineer, or land surveyor, to alter an item in any way. If an item bearing the seal of an architect, engineer, or land surveyor is altered; the altering architect, engineer, or land surveyor shall affix to the item their seal and notation "altered by" followed by their signature and date of such alteration, and a specific description of the alteration.

APPLICANT NAME & ADDRESS:

ARGO DEVELOPMENT
95 CHESTNUT RIDGE ROAD
MONTVALE, NJ 07645

PROJECT NAME & LOCATION:

**SOUTH AVE
MULTI-FAMILY DEVELOPMENT**
SOUTH AVENUE
CITY OF POUGHKEEPSIE, NY 12601

NO.	DATE:	DESCRIPTION:
5	11/14/23	REVISED PER BUILDING PLANS
4	06/13/23	REVISED PER UPDATED STORMWATER DESIGN
3	04/18/23	REVISED PER PLANNING COMMENTS
2	03/07/23	REVISED PER PLANNING BOARD COMMENTS
1	01/27/23	REVISED PER CITY COMMENTS

Revisions

PROJECT NUMBER: 2222201

DRAWN BY:

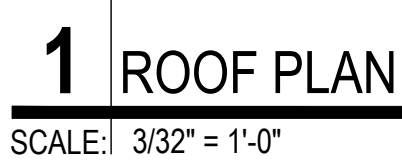
REVIEWED BY: CPL

ISSUED FOR: PLANNING BOARD REVIEW

DATE: 11/28/2022

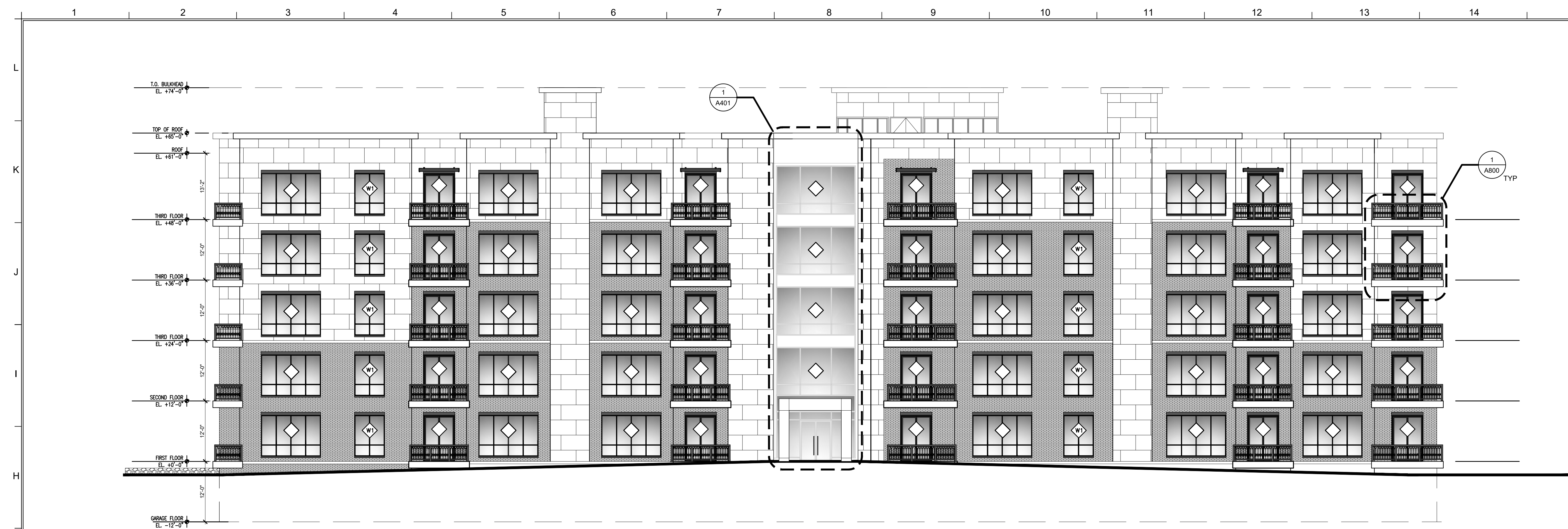
DRAWING NUMBER:

LANDSCAPE PLAN



SEAL	DRAWN BY	PROJECT NUMBER
	CQ/DS	22054
	CHECKED BY	DATE
	MM	9-27-2023
	DRAWING NUMBER	
	A106	

FILE: F:\20220504 - POLYMERIZED METHACRYLATES\050401 - REDUCED CRYSTAL FOR ROCK PLACEMENT



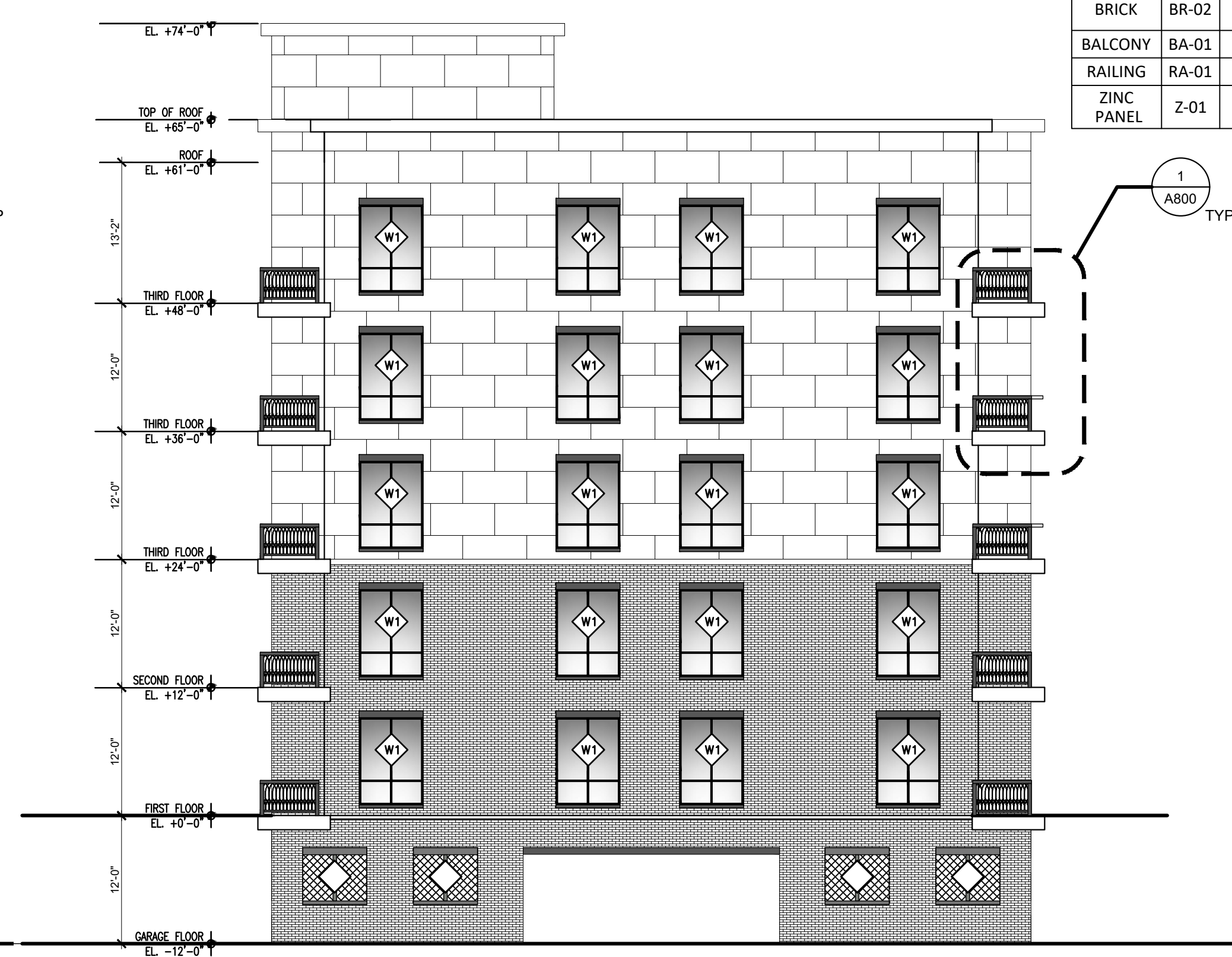
1 WEST ELEVATION - INTERIOR COURTYARD
SCALE: 3/32" = 1'-0"

2 NORTHWEST ELEVATION - INTERIOR COURTYARD
SCALE: 3/32" = 1'-0"

EXTERIOR FINISHES					
TYPE	TAG	PHOTO	BRAND	MODEL	LOCATION
METAL PANEL	MT-01		FAIRVIEW ARCHITECTURAL	SMOKE SILVER METALIC FV9134	
	MT-02		FAIRVIEW ARCHITECTURAL	ANODIC SATIN BLACK FV5292	
FIBRE CEMENT	FC-01		EQUITONE	MA400 COLOR: N073 CHARCOAL	
	FC-02		EQUITONE	[natura] COLOR: N163 SOFT GREY	
BRICK	BR-02		BELDEN BRICK COMPANY	MORA CERAMICAS - NERO SMOOTH	
BALCONY	BA-01				
RAILING	RA-01			WROUGHT IRON	
ZINC PANEL	Z-01		RHEINZINK	GRANUM BASALTE	SOUTH AVE LOBBY ENTRANCE



3 NORTH ELEVATION
SCALE: 3/32" = 1'-0"



4 NORTHEAST ELEVATION
SCALE: 3/32" = 1'-0"

NOTES

1. FOR WINDOW TYPES AND SCHEDULES, SEE SHEET A920

90% CD SUBMISSION

DD SUBMISSION

SD SUBMISSION

No. REVISION/SUBMISSION

11/01/2023

09/27/2023

08/31/2023

DATE

CIVIL ENGINEER



Powered by partnership.

FA/FP ENGINEER



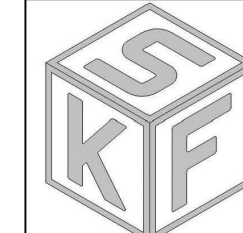
WHITMAN
ENGINEERING P.C.
2600 SOUTH RD. STE 44-260
POUGHKEEPSIE, NY 12601
PHONE: (845) 471-6036

MEP ENGINEER



excellent engineering systems
KILTSEVA RD. 22a
KYIV, UKRAINE 02000
PHONE: +38 044 332 0322
MOBILE: +38 093 332 0322

STRUCTURAL ENGINEER



SZEWCAK, KILTY
& FLUHR
STRUCTURAL ENGINEERS, PC
200 FISHER DR
AVON, CT 06001
PHONE: (860) 677-4570



181 CHURCH STREET
POUGHKEEPSIE, NEW YORK 12601
PHONE 845-452-2268 FAX 845-452-3752

PROJECT
SOUTH AVENUE PARCEL
POUGHKEEPSIE MID-RISE MULTIPLE
CITY OF POUGHKEEPSIE, NY

DRAWING
EXTERIOR ELEVATIONS II

SEAL

CQ/DS
CHECKED BY
MM

PROJECT NUMBER
22054
DATE
9-27-2023
DRAWING NUMBER

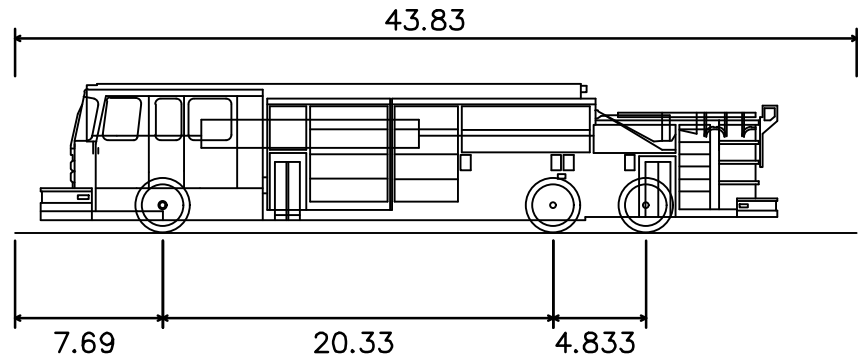
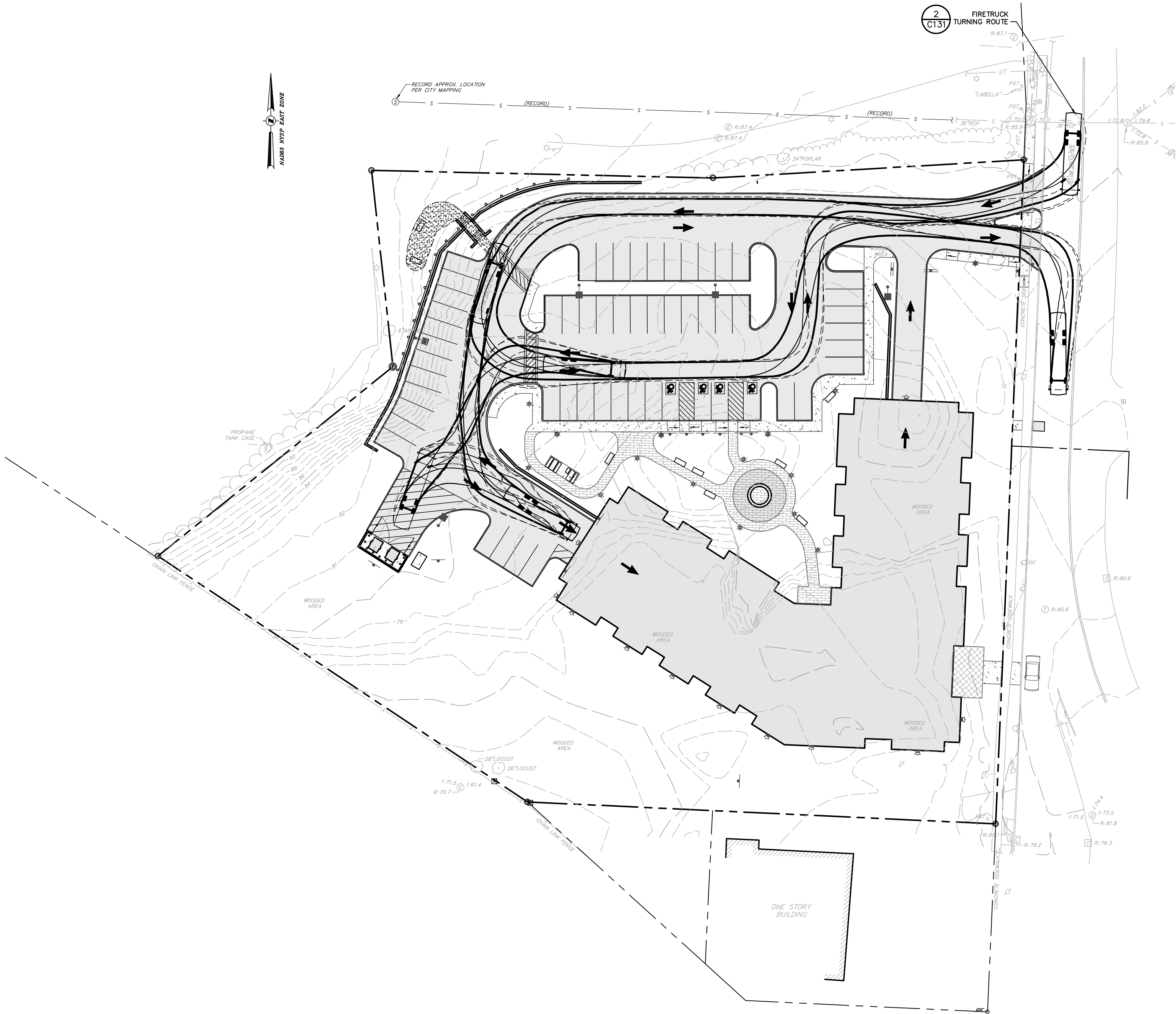
A301

FILE

NOTES		
1. FOR WINDOW TYPES AND SCHEDULES, SEE SHEET A920		
	90% CD SUBMISSION	11/01/2023
	DD SUBMISSION	09/27/2023
	SD SUBMISSION	08/31/2023
No.	REVISION/SUBMISSION	DATE
CIVIL ENGINEER		
 LaBella Powered by partnership.		
FA/FP ENGINEER		
 WHITMAN ENGINEERING P.C. 2600 SOUTH RD, STE 44-260 POUGHKEEPSIE, NY 12601 PHONE: (845) 471-6036		
MEP ENGINEER		
 EXSYS excellent engineering systems		
KILTSEVA RD. 22c KYIV, UKRAINE 02000 PHONE: +38 044 332 0322 MOBILE: +38 093 332 0322		
STRUCTURAL ENGINEER		
 SZEWCZAK, KILTY & FLUHR STRUCTURAL ENGINEERS, PC 200 FISHER DR AVON, CT 06001 PHONE: (860) 677-4570		
 Liscum McCormack VanVoorhis 181 CHURCH STREET POUGHKEEPSIE, NEW YORK 12601 PHONE 845-452-2268 FAX 845-452-3752		
ARCHITECTURE PLANNING INTERIORS		
PROJECT SOUTH AVENUE PARCEL POUGHKEEPSIE MID-RISE MULTIPLE CITY OF POUGHKEEPSIE, NY		
DRAWING EXTERIOR ELEVATIONS III		
SEAL	DRAWN BY CQ/DS	PROJECT NUMBER 22054
	CHECKED BY MM	DATE 9-27-2023
	DRAWING NUMBER A302	

11/14/2023 2:56:01 PM
\\ashlab\l\Projects\ARGO\2222201 - South Ave Multi Family Dev\06_Drawings\Civil\01_C131_2222201_TRUCK.dwg

Drawing Name: \\ashlab\l\Projects\ARGO\2222201 - South Ave Multi Family Dev\06_Drawings\Civil\01_C131_2222201_TRUCK.dwg
Xref's Attached: XENG_2222201; XLAYOUT_2222201; XBORDER_2222201; XBASE-ENG_2222201
Date Printed: Nov 14, 2023, 2:56pm



100 ft Tower Ladder 2—City Pok NY
Overall Length 43.830ft
Overall Width 8.167ft
Overall Body Height 7.745ft
Min Body Ground Clearance 0.656ft
Track Width 8.167ft
Lock-to-lock time 6.00s
Curb to Curb Turning Radius 35.440ft

2 CITY OF POUGHKEEPSIE LADDER TRUCK
C-131 SCALE: NTS

1 TRUCK MANEUVERING PLAN
C-131 SCALE: 1" = 30'

21 Fox Street
Poughkeepsie, NY 12601
(845) 454-3980
labellapc.com

It is a violation of New York Education Law Article 145 Sec.7209, for any person, unless acting under the direction of a licensed architect, professional engineer, or land surveyor, to alter an item in any way. If an item bearing the seal of an architect, engineer, or land surveyor is altered; the altering architect, engineer, or land surveyor shall affix to the item their seal and notation "altered by" followed by their signature and date of such alteration, and a specific description of the alteration.

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APPLICANT NAME & ADDRESS:
ARGO DEVELOPMENT
95 CHESTNUT RIDGE ROAD
MONTVALE, NJ 07645

PROJECT NAME & LOCATION:
**SOUTH AVE
MULTI-FAMILY DEVELOPMENT**
SOUTH AVENUE
CITY OF POUGHKEEPSIE, NY 12601

3	11/14/23	REVISED PER BUILDING PLANS
2	06/13/23	REVISED PER UPDATED STORMWATER DESIGN
1	01/27/23	REVISED PER CITY COMMENTS
NO.	DATE:	DESCRIPTION:
Revisions		

PROJECT NUMBER: 2222201
DRAWN BY:
REVIEWED BY: CPL
ISSUED FOR: PLANNING BOARD REVIEW
DATE: 11/28/2022
DRAWING NAME:

TRUCK MANEUVERING PLAN

DRAWING NUMBER:

C131

APPLICATION TO THE PLANNING BOARD
CITY OF POUGHKEEPSIE, NEW YORK

Application for Site Plan, Special Permit & Facade Approval

- I. **PROPERTY ADDRESS:** 149 North Hamilton Street
- II. **PROPERTY OWNER:** RC Home & Office Solutions, Inc.
ADDRESS: 46 Morningside Drive
Patterson
NY 12563 (state and zip code)
PHONE NUMBER: (914) 837-5690 (include area code)
- III. **NAME OF APPLICANT:** Mr. Roy Campbell
ADDRESS: (Owner of RC Home & Office Solutions Inc.)
same as above
(state and zip code)
PHONE NUMBER: (include area code)
EMAIL ADDRESS: royc8159@gmail.com
- (If applicant is NOT the owner, proof of owner's consent to the application MUST be provided.)
- IV. **CONSULTANT:** Paul Tirums, P.E.
ADDRESS: 16 Friendship Lane
Poughkeepsie
NY 12601 (state and zip code)
PHONE NUMBER: (914) 490-3269 (include area code)
- V. **PROPOSED USE or FAÇADE CHANGE** (Summarize proposed use or uses): None

OFFICE USE ONLY

CODE: PLSF

ID #

FEE:

Total, from:
\$500, plus
\$
(\$150 per 1000
square feet of floor
area), plus
\$
(\$35 per number of
required parking
spaces), plus
\$
(\$500 for special
permit, if applicable)

- VI. **ZONING DISTRICT:** R-3A

(Proceed to XVII for special permit & façade approval only)

PROJECT INFORMATION

New construction: _____ (yes) ^{XX}_____ (no)
Change of use: _____ (yes) ^{XX}_____ (no)
Expansion/addition: _____ (yes) ^{XX}_____ (no)
Alteration: ^{XX}_____ (yes) _____ (no)
Cost of Construction: \$ ^{New Asphalt = \$12,000}_____

LOT AND BULK INFORMATION:

Lot area: 0.234 (acres) 10,193 (square feet)
Building Footprint: 0 (proposed square feet for new construction)
0 (existing square feet for change of use/expansion)
0 (additional proposed square feet for expansion)
Exist.= (total building footprint, square feet)
Lot Coverage: 10.64% (percentage, building coverage only)
Gross Floor Area: 0 (proposed square feet for new construction)
0 (existing square feet for change of use/expansion)
0 (additional proposed square feet for expansion)
2,536 SF (total gross floor area, square feet)
Building Height: 3 (stories) 35 (feet)
Floor Area Ratio: 2,536/10,193=0.25 (ratio of gross floor area divided by lot area)

VIII. PROJECT PARKING INFORMATION

A) Number of Off-Street Parking Spaces required: 7.5
B) Number of Off-Street Parking Spaces provided: 9
C) Waiver Requested: _____ (Yes) No (No)
Is municipal parking available within 600 feet? _____ (Yes) _____ (No)

If yes, name all such municipal facilities: _____

(If a waiver is requested based on proximity to municipal facilities, a written request for such waiver, addressed to the City Planning Staff and the Chairman of the Planning Board, must be submitted with the application.)

If a private parking facility is to be utilized, is such facility within 600 feet?

_____ (Yes) _____ (No)

If yes, provide the address of the facility, name of the property owner, number of spaces in the lot, number of spaces available for lease, and length of lease. Attach copy of lease. Additional information may be required by the Planning Board):

IX. RESIDENTIAL PROPOSALS

A) Type of Development: Paved Parking Area & Driveway for Exist. (4) Unit Building
(Townhouse, condominium, multiple residence rental, etc.)

B) Unit Breakdown:

Number of efficiency or "studio" units: _____

Number of one (1) bedroom units: Existing Bldg. = (1)

Number of two (2) bedroom unit's Existing Bldg. = (1)

Number with three or more bedrooms: Existing Bldg. = (2)

Total number of units: 4

C) Is funding public? _____ Yes XX No

If yes, describe funding source: _____

X. OFFICE PROPOSALS (_____ Medical/Dental _____ Professional _____ Business)

A) Number of Employees: _____

B) Number of doctors/dentists/medical practitioners: _____

C) Days/Hours of Operation: _____

XI. SERVICE BUSINESS PROPOSALS

- A) Specify Business: _____
(e.g., laundromat, drycleaner, beauty parlor, travel agency, banks)
- B) Number of Employees: _____
- C) Number of Washing Machines (for laundromat): _____
- D) Days/Hours of Operation: _____

XII. MERCANTILE PROPOSALS (Retail _____ Wholesale _____)

- A) Type of Mercantile (specify): _____
- B) Number of Employees: _____
- C) Days/Hours of Operation: _____

XIII. STANDARD RESTAURANT/FAST FOOD RESTAURANT/COFFEE SHOP/DONUT SHOP/NIGHTCLUB/DISCOTHEQUE/BAR PROPOSALS

- A) Specify Use: _____
- B) Number of seats (excluding bar stools and outdoor seating): _____
- C) Meals served (i.e., breakfast, lunch, dinner): _____
- D) Type of Menu: _____
- E) Days/Hours of Operation: _____
- F) Entertainment (live and/or recorded): _____
- G) Will the restaurant contain a bar/lounge?: _____ Yes _____ No
- H) If yes, how many seats at the bar? _____
- I) Is a drive-through window proposed? _____
- J) Is a walk-up window proposed? _____
- K) Is an outdoor seating area proposed? _____
- L) If yes, how many seats are proposed? _____
- M) If yes, is outdoor cooking proposed? _____

XIV. INDUSTRIAL/COMMERCIAL PROPOSALS (Automobile repair, taxi, motor vehicle sales, warehouse, manufacturing, animal hospital, research facilities, etc.)

- A) Specify Use: _____
- B) Number of shifts (if any): _____
- C) Number of employees: _____
(If shifts are proposed, list number of employees per shift)
- E) Number of work bays (if automobile repair): _____
- F) Days/Hours of operation: _____

XV. EDUCATIONAL/INSTITUTIONAL/RECREATIONAL PROPOSALS (e.g., Museums, Theatres, Conference Centers, Hotels, Motels, Hospitals, Nursing Homes, Assisted Living Facilities, Nursery Schools, Private Schools, Day Care Centers, Places of Worship, Marinas, Membership Clubs, etc.)

A) Specify: _____

B) Number of shifts (if any): _____

C) Number of employees: _____
(If shifts are proposed, list number of employees per shift)

D) Number of beds (hospitals, nursing homes, etc): _____

E) Number of seats in largest assembly space: _____

F) Number of classrooms (schools): _____

G) Number of rental units (hotels, motels, etc.) _____

H) Number of boat slips/courts (e.g., tennis, handball, etc.), alleys: _____

I) Days/Hours of Operation: _____

XVI. PROPOSALS NOT MENTIONED ABOVE (Please be specific):

The purpose of this Application is for the asphalt paving performed recently for the

driveway and rear parking area for the residential building located at

149 North Hamilton Street.

XVII. APPLICANT CERTIFICATION

I, Roy Campbell, certify that the this application has addressed the objectives outlined in the Zoning Ordinance with consideration given to the public health, safety and welfare; the comfort and convenience of the public in general or the residents or users of the proposed development and of the immediate neighborhood.

The applicant certifies that all information contained herein is accurate and complete as of the date of this application.

Signature of Applicant

Date

Paul Tirums, P.E. Consultant Engineer

16 Friendship Lane
Poughkeepsie, N.Y. 12601

Cell (914) 490-3269

August 29, 2023

The City of Poughkeepsie Building, Planning & Zoning Department
ATTN: Ms. Judith Knauss, Deputy Zoning Administrator
62 Civic Center Plaza
Poughkeepsie, N.Y. 12601

Project: Campbell – Asphalt Paved Driveway

RC Home & Office Solutions, Inc.
Mr. Roy Campbell
149 North Hamilton Street
Poughkeepsie, N.Y. 12603

RE: Site Plan Submittal (Revised)

Dear Judith,

This transmittal is for the City's review of the application package that has been revised based on our initial appearance in front of the Planning Board on Tuesday, August 16th. Mr. Campbell was required to submit a Site Plan for the fact that he placed new asphalt pavement for his property's driveway and rear parking area.

Attached is a Site Plan submission for the City's review and approval of the asphalt driveway and rear parking area for his existing (4) unit apartment building at Mr. Campbell's property at 149 North Hamilton Street. Included in this submission are:

1. "Updated" Site Plan Sheet
2. Short Environmental Assessment Form
3. Site Plan Application
4. Statement from Mr. Campbell saying that I can represent him in this application
5. Current Site photos

In this updated submittal we addressed the comments received from the Planning Board:

- The Parking Spaces were increased from 18' in length to 20' in length.
- Added dimensions for the driveway widths were added to the Site Plan.
- When Mr. Campbell repainted and newly sided the Building a couple of years ago he removed the Bail Bondsman sign. Since he became Owner in 2020 there has never been a Bail Bondsman renter within the Building.
- The curb cut for the driveway was pre-existing as indicated in the photos provided.
- We will be submitting a Variance Application for the September ZBA Meeting based on Judith Knauss's comments.

Please place us on the September Planning Board Meeting. If there are any questions please contact me.

Sincerely,

Paul Tirums, P.E.

North per deed
Doc ID# 02-2022-50176

found 1/2" rebar
S 75°-30'-34" E 0.17'

house clears
0.1'±

1 story frame
house clears
2.3'±

3-Story Brick
(4) Unit Apart. Bldg.
(2,536 GSF)

43.25' long

concrete

porch
(8' x 20')

paved asphalt
driveway limits
18.8'

house clears
1.5'±

found 1/2" rebar
N 09°-44'-58" E 0.51'

N/F Green
Doc. ID No. 02-2012-748

(9) Total Parking spaces
indicated (9' x 20' each) Typical

Tax Map # 131300-6162-63-275350
= 0.234± Acres.

N/F Rogers
L 1706 P 852

TAX MAP IDENTIFICATION

North Hamilton Street	# 240389	# 282392	# 302384
		# 279377	# 301378
		# 279374	# 301373
		# 279372	# 300366
		# 278369	# 299362
		# 278366	# 298358
		# 277361	# 298354
		# 276355	# 297351
		# 275350	# 296348
		# 274345	# 296344
		# 274344	# 294339
		# 274340	# 294333
		# 273336	# 293329
		# 271325	# 292324

TAX MAP NUMBER	ADDRESS	OWNER
# 240389	N. Hamilton Street	County of Dutchess
# 238330	150 N. Hamilton Street	County of Dutchess
# 292392	183 N. Hamilton Street	Urban/Gerlach Realty LLC
# 279377	165 N. Hamilton Street	City of Poughkeepsie
# 279374	163 N. Hamilton Street	Westney, Saleesha
# 279372	161 N. Hamilton Street	Hooper, Alexis D.
# 278369	159 N. Hamilton Street	White, Joyce E.
# 278366	157 N. Hamilton Street	Walden, D W.
# 277361	155 N. Hamilton Street	Green, Roberta J.
# 276355	151 N. Hamilton Street	Green, William Earl Sr.
# 274345	147 N. Hamilton Street	Rogers, Judith
# 274344	145 N. Hamilton Street	Rojas, Sabas
# 274340	143 N. Hamilton Street	Williams, Jannette D.
# 273336	141 N. Hamilton Street	Margulies, Ira Joseph
# 272344	139 N. Hamilton Street	Klein, Tzy A.
# 271325	135 N. Hamilton Street	North Hamilton LLC
# 302384	172-174 N. Clinton Street	Walkway Properties LLC
# 301378	168 N. Clinton Street	168 N Clinton LLC
# 301373	166 N. Clinton Street	Shelli, Curtis
# 300366	162 N. Clinton Street	Gray, Lloyd E.
# 299362	160 N. Clinton Street	Greene, Sheldon R.
# 298358	158 N. Clinton Street	Diffus, Daniel
# 298354	156 N. Clinton Street	Maresco, Anthony J.
# 297351	154 N. Clinton Street	Maresco, Anthony J.
# 296348	152 N. Clinton Street	Dugger, John W. Jr
# 296344	150 N. Clinton Street	Reid, Jaime
# 294339	148 N. Clinton Street	Stubbs, Erik R.
# 294333	146 N. Clinton Street	Rodriguez, Ariel
# 293329	144 N. Clinton Street	Ulutas, Ahmet
# 292324	140 N. Clinton Street	Kimon, Eon L.

LEGEND:

- recovered 1/2" rebar
- recovered iron pipe
- utility pole
- light post
- chain link fence
- water valve
- bollard



LOCATION MAP

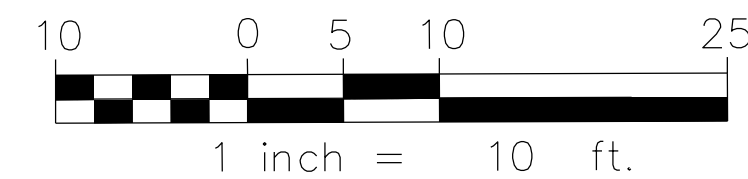
N.T.S.

found 1/2" rebar
N 28°-03'-54" E 0.31'

N/F Dugger
L 1900 P 95

N/F Reid
Doc. ID No. 02-2007-502

GRAPHIC SCALE



CITY PLANNING BOARD
CITY OF POUGHKEEPSIE, NEW YORK
SITE PLAN APPROVAL
FILE NO:
PROJECT NAME:
PROJECT LOCATION:
APPLICANT:
OFFICIALLY APPROVED at the Planning Board
meeting held the ___ day of _____ 202_
PLANNING CHAIR:
DESIGNEE:
DATE SIGNED:
CONDITION(S):

Boundary Survey by:
MICHAEL J. PARRELLA, PLS
Licensed Land Surveyor
NY License No. 050162
MAY 2023
168 SCHULTZ HILL ROAD
STAATSBURG, NY 12580
(845) 876-4163

OWNER INFORMATION:
RC Home & Office Solutions Inc.
Mr. Roy Campbell
46 Morningside Drive
Patterson, N.Y. 12563
(914) 837-5690

SITE PLAN
DRIVEWAY IMPROVEMENTS
PREPARED FOR
RC HOME & OFFICE SOLUTIONS INC.
MR. ROY CAMPBELL

149 N. HAMILTON STREET
CITY OF POUGHKEEPSIE
COUNTY OF DUTCHESS, N.Y.

BY: PAUL TIRUMS, P.E.
16 FRIENDSHIP LANE
POUGHKEEPSIE, NY 12601

SCALE: 1" = 30'
DATE: 08/24/2023

PAGE 1 OF 1

Short Environmental Assessment Form

Part 1 - Project Information

Instructions for Completing

Part 1 – Project Information. The applicant or project sponsor is responsible for the completion of Part 1. Responses become part of the application for approval or funding, are subject to public review, and may be subject to further verification. Complete Part 1 based on information currently available. If additional research or investigation would be needed to fully respond to any item, please answer as thoroughly as possible based on current information.

Complete all items in Part 1. You may also provide any additional information which you believe will be needed by or useful to the lead agency; attach additional pages as necessary to supplement any item.

Part 1 – Project and Sponsor Information			
RC Home & Office Solutions, Inc.; Mr. Roy Campbell			
Name of Action or Project: Asphalt Paving of Driveway @ 149 N. Hamilton St.			
Project Location (describe, and attach a location map): 149 N. Hamilton St., Poughkeepsie, NY 12603			
Brief Description of Proposed Action: Owner had driveway newly asphalt paved & parking area behind the building. The building is an existing (4) unit Apartment building.			
Name of Applicant or Sponsor: Mr. Roy Campbell		Telephone: (914) 837-5690	
		E-Mail: royc8159@gmail.com	
Address: 46 Morningside Drive			
City/PO: Patterson		State: NY	Zip Code: 12563
1. Does the proposed action only involve the legislative adoption of a plan, local law, ordinance, administrative rule, or regulation?			NO ☐
If Yes, attach a narrative description of the intent of the proposed action and the environmental resources that may be affected in the municipality and proceed to Part 2. If no, continue to question 2.			YES ☒
2. Does the proposed action require a permit, approval or funding from any other government Agency?			NO ☐
If Yes, list agency(s) name and permit or approval: City of Poughkeepsie Planning Board			YES ☒
3. a. Total acreage of the site of the proposed action?		0.234 acres	
b. Total acreage to be physically disturbed?		0.08 acres	
c. Total acreage (project site and any contiguous properties) owned or controlled by the applicant or project sponsor?		0.234 acres	
4. Check all land uses that occur on, are adjoining or near the proposed action:			
5. ☒ Urban ☐ Rural (non-agriculture) ☐ Industrial ☒ Commercial ☐ Residential (suburban)			
☐ Forest ☐ Agriculture ☐ Aquatic ☐ Other(Specify):			
☐ Parkland			

5. Is the proposed action,	NO	YES	N/A
a. A permitted use under the zoning regulations?	☐	☒	☐
b. Consistent with the adopted comprehensive plan?	☐	☒	☐
6. Is the proposed action consistent with the predominant character of the existing built or natural landscape?	NO	YES	
	☐	☒	
7. Is the site of the proposed action located in, or does it adjoin, a state listed Critical Environmental Area?	NO	YES	
If Yes, identify: _____	☒	☐	
8. a. Will the proposed action result in a substantial increase in traffic above present levels?	NO	YES	
	☒	☐	
b. Are public transportation services available at or near the site of the proposed action?	☐	☒	
c. Are any pedestrian accommodations or bicycle routes available on or near the site of the proposed action?	☐	☒	
9. Does the proposed action meet or exceed the state energy code requirements?	NO	YES	
If the proposed action will exceed requirements, describe design features and technologies: _____ _____	☐	☒	
10. Will the proposed action connect to an existing public/private water supply?	NO	YES	
If No, describe method for providing potable water: <u>(connection already exists)</u>	☐	☒	
11. Will the proposed action connect to existing wastewater utilities?	NO	YES	
If No, describe method for providing wastewater treatment: <u>(connection already exists)</u>	☐	☒	
12. a. Does the project site contain, or is it substantially contiguous to, a building, archaeological site, or district which is listed on the National or State Register of Historic Places, or that has been determined by the Commissioner of the NYS Office of Parks, Recreation and Historic Preservation to be eligible for listing on the State Register of Historic Places?	NO	YES	
	☒	☐	
b. Is the project site, or any portion of it, located in or adjacent to an area designated as sensitive for archaeological sites on the NY State Historic Preservation Office (SHPO) archaeological site inventory?	☒	☐	
13. a. Does any portion of the site of the proposed action, or lands adjoining the proposed action, contain wetlands or other waterbodies regulated by a federal, state or local agency?	NO	YES	
	☒	☐	
b. Would the proposed action physically alter, or encroach into, any existing wetland or waterbody?	☒	☐	
If Yes, identify the wetland or waterbody and extent of alterations in square feet or acres: _____ _____ _____			

14. Identify the typical habitat types that occur on, or are likely to be found on the project site. Check all that apply: ☐ Shoreline ☐ Forest ☐ Agricultural/grasslands ☐ Early mid-successional ☐ Wetland ☒ Urban ☐ Suburban		
15. Does the site of the proposed action contain any species of animal, or associated habitats, listed by the State or Federal government as threatened or endangered?	NO	YES
	☒	☐
16. Is the project site located in the 100-year flood plan?	NO	YES
	☒	☐
17. Will the proposed action create storm water discharge, either from point or non-point sources? If Yes,	NO	YES
a. Will storm water discharges flow to adjacent properties?	☒	☐
b. Will storm water discharges be directed to established conveyance systems (runoff and storm drains)?	☒	☐
If Yes, briefly describe: _____ _____		
18. Does the proposed action include construction or other activities that would result in the impoundment of water or other liquids (e.g., retention pond, waste lagoon, dam)? If Yes, explain the purpose and size of the impoundment: _____ _____	NO	YES
	☒	☐
19. Has the site of the proposed action or an adjoining property been the location of an active or closed solid waste management facility? If Yes, describe: _____ _____	NO	YES
	☒	☐
20. Has the site of the proposed action or an adjoining property been the subject of remediation (ongoing or completed) for hazardous waste? If Yes, describe: _____ _____	NO	YES
	☒	☐
I CERTIFY THAT THE INFORMATION PROVIDED ABOVE IS TRUE AND ACCURATE TO THE BEST OF MY KNOWLEDGE Applicant/sponsor/name: _____ Date: _____ Signature: _____ Title: _____		



**THE CITY OF POUGHKEEPSIE
NEW YORK**

DEVELOPMENT DEPARTMENT

62 CIVIC CENTER PLAZA, 2ND FLOOR

POUGHKEEPSIE, NY 12601

Phone: (845) 451-4263 Fax: (845) 451-4006

MEMORANDUM

To: Roy Campbell

From: Tyler Maegerle, Junior Planner

Date: 8/14/2023

Re: 149 North Hamilton Street

Upon review of project materials provided by the applicant, including Short EAF Part 1 and site plan application, it is my understanding that the proposed action to be undertaken includes paving of a parking area behind a four-unit residence.

Based on the above description it is the determination of the planning division staff that the proposed action is classified as an Unlisted Action with regards to SEQRA, and therefore is subject to an uncoordinated or coordinated (optional) environmental review. This determination was made due to the fact that the proposed action was not expressly listed in 6 NYCRR Section 617.4 (Type I Actions) or 617.5 (Type II Actions).













BUILDING DEPARTMENT

CITY OF POUGHKEEPSIE

62 CIVIC CENTER PLAZA, 2ND FLOOR

POUGHKEEPSIE, NY 12601

Phone: (845) 451-4007 Fax: (845) 451-4006

Eric Philipp
Building Inspector

SITE PLAN REVIEW

DATE: 10-2-23

PROJECT ADDRESS: 149 N. Hamilton St.

PROJECT NAME: Parking area

OWNER: RC Home & Office Solutions

ENGINEER/ARCHITECT: Paul Tirums, P.E.

☐

The site plan is acceptable and will be signed with no conditions.

☒

The site plan cannot be approved as submitted. The following comments must be addressed or conditions satisfied:

The section of sidewalk that has the macadam should be changed to concrete


Eric Philipp
Building Inspector

Dutchess County Department of Planning and Development		Fax Info Only	To	Date	#pgs
			Co./Dept.	From	
			Fax #	Phone #	

239 Planning/Zoning Referral - Exemption Communities

Municipality: **City of Poughkeepsie**

Referring Agency: **Planning Board**

Tax Parcel Number(s): **2753500000**

Project Name: **Driveway**

Applicant: **Roy Campbell**

Address of Property: **149 Hamilton St, Poughkeepsie, NY 12601**

Exempt Actions:*
239 Review is NOT Required

- Administrative Amendments (fees, procedures, penalties, etc.)
- Special Permits for residential uses (accessory apts, home occupations, etc.)
- Use Variances for residential uses
- Area Variances for residential uses
- Renewals/Extension of Site Plans or Special Permits that have no changes from previous approvals

No Authority to review these Actions

- Subdivisions / Lot Line Adjustments
- Interpretations

☐ Exempt Action submitted for informal review

Actions Requiring 239 Review

- ☐ Comprehensive/Master Plans
- ☐ Zoning Amendments (standards, uses, definitions, district regulations, etc.)
- ☐ Other Local Laws associated with zoning (wetlands, historic preservation, affordable housing, architectural review, etc.)
- ☐ Rezoning involving all map changes
- ☐ Architectural Review
- ☒ Site Plans (all)
- ☐ Special Permits for all non-residential uses
- ☐ Use Variances for all non-residential uses
- ☐ Area Variances for all non-residential uses
- ☐ Other (Describe):

Parcels within 500 feet of:

- ☐ State Road:
- ☐ County Road:
- ☐ State Property (with recreation area or public building)
- ☒ County Property (with recreation area or public building)
- ☐ Municipal Boundary
- ☐ Farm operation in an Agricultural District

Date Response Requested: **9/15/2023**

Entered By: **Garcia, Lori**

These actions are only exempt in municipalities that signed an intermunicipal agreement with Dutchess County to that effect.

For County Office Use Only			
Response From Dutchess County Department of Planning and Development			
No Comments: ☒ Matter of Local Concern ☐ No Jurisdiction ☐ No Authority ☐ Withdrawn ☐ Incomplete - municipality must resubmit to County ☐ Exempt from 239 Review ☐ None		Comments Attached: ☐ Local Concern with Comments ☐ Conditional ☐ Denial ☐ Incomplete with Comments- municipality must resubmit to County ☐ Informal Comments Only (Action Exempt from 239 Review)	
Date Submitted: 9/13/2023	Notes:	☐ Major Project	
Date Received: 9/13/2023		Referral #: ZR23-267	
Date Requested: 9/15/2023			
Date Required: 10/12/2023	☐ Also mailed hard copy	Reviewer:	
Date Transmitted: 9/14/2023			



CITY OF POUGHKEEPSIE POLICE DEPARTMENT

SITE PLAN REVIEW

DATE 09/15/2023

PROJECT NAME: 149 N. Hamilton Street

PROJECT ADDRESS: 149 N. Hamilton Street

☒ **THE SITE PLAN IS ACCEPTABLE AND WILL BE SIGNED WITH NO CONDITIONS**

☐ **THE SITE PLAN IS UNACCEPTABLE.**

CONDITIONS:

APPROVED BY CHIEF THOMAS PAPE OR AGENT:

DATE:

Sgt E Acken #112

SGT. E. ACKEN #112

9/15/2023



THE CITY OF POUGHKEEPSIE

DEPARTMENT OF PUBLIC WORKS & ENGINEERING

SITE PLAN REVIEW

Form #1

1. Date: September 15, 2023
2. Name of Site Plan: Driveway Improvements
3. Address or Location: 149 North Hamilton Street
4. Owner: RC Home & Office Solutions, Inc.
5. Engineer/ Architect: Paul Tirums, P.E.
6. Yes ☐ No ☐ The site plan as forwarded by the Planning Department is acceptable and will be signed off on with no condition(s).
7. Yes ☐ No ☐ The site plan as forwarded by the Planning Department is acceptable and has been signed off on with no condition(s).
8. Yes ☒ No ☐ The applicant should be notified that the Dept. of Public Works/Engineering requires the following comment(s) be addressed:
 1. Prepare brief stormwater report. The report should demonstrate how post development stormwater rates will be limited to less than predevelopment runoff rates. On-site attenuation may be required; there is not a storm sewer available for connection. Green infrastructure practices are encouraged. A maintenance schedule and plan will be required for all post construction stormwater management practices. Maintenance records for practices are to be submitted to the engineering department on a biannual basis.
 2. Show contours or spot elevations at a minimum.


Joseph A. Chenier, Jr. CPESC, CPMSM
Assistant Civil Engineer

~~2023~~ 2023-053

APPLICATION TO THE PLANNING BOARD
CITY OF POUGHKEEPSIE, NEW YORK

Application for Site Plan, Special Permit & Facade Approval

- I. **PROPERTY ADDRESS:** 4 South Clinton Street
- II. **PROPERTY OWNER:** 4 S. Clinton, LLC
ADDRESS: 11 Garden St., Suite 101
Poughkeepsie
NY 12601 (state and zip code)
PHONE NUMBER: 845-471-1047 (include area code)
- III. **NAME OF APPLICANT:** 4 S. Clinton, LLC
ADDRESS: 11 Garden St., Suite 101
Poughkeepsie
NY 12601 (state and zip code)
PHONE NUMBER: 845-471-1047 (include area code)
EMAIL ADDRESS: Eric@BaxterBuilt.com
- (If applicant is NOT the owner, proof of owner's consent to the application MUST be provided.)
- IV. **CONSULTANT:** Natalie Quinn, KARC Consulting
ADDRESS: PO Box 924
Poughkeepsie
NY, 12601 (state and zip code)
PHONE NUMBER: 845.874.7166 (include area code)
Email Address: natalie@karcpc.com
- V. **PROPOSED USE or FAÇADE CHANGE** (Summarize proposed use or uses): Legalize the use of the existing accessory structure (carriage house) for commercial use as artist studios
- VI. **ZONING DISTRICT:** C-2

OFFICE USE ONLY

CODE: PLSF

ID #

FEE: \$754.00

Total, from:
\$500, plus
\$254
(\$150 per 1000
square feet of floor
area), plus
\$
(\$35 per number of
required parking
spaces), plus
\$
(\$500 for special
permit, if applicable)

PAID
7/17/23
9/13/23

(Proceed to XVII for special permit & façade approval only)

PROJECT INFORMATION

New construction: _____ (yes) X (no)

Change of use: X (yes) _____ (no)

Expansion/addition: _____ (yes) X (no)

Alteration: _____ (yes) X (no)

Cost of Construction: \$ NA

LOT AND BULK INFORMATION:

Lot area: 0.14 (acres) 6,098 (square feet)

Building Footprint: 0 (proposed square feet for new construction)

1,683 SF (existing square feet for change of
use/expansion) Carriage House Only

0 (additional proposed square feet for expansion)

3,937 SF (total building footprint, square feet)

Lot Coverage: 65% (percentage, building coverage only)

Gross Floor Area: 0 SF (proposed square feet for new construction)

3,175 SF (existing square feet for change of
use/expansion)

_____ (additional proposed square feet for expansion)

10,619 SF (total gross floor area, square feet)

Building Height: 2/3 (stories) _____ (feet) Carriage/primary structure

Floor Area Ratio: 1.74 (ratio of gross floor area divided by lot area)

VIII. PROJECT PARKING INFORMATION

A) Number of Off-Street Parking Spaces required: 0 spaces See cover letter for calculation

B) Number of Off-Street Parking Spaces provided: 0 spaces

C) Waiver Requested: _____ (Yes) X (No)

Is municipal parking available within 600 feet? X (Yes) _____ (No)

If yes, name all such municipal facilities: _____
Little Smith Street Parking Lot (350 feet)

(If a waiver is requested based on proximity to municipal facilities, a written request for such waiver, addressed to the City Planning Staff and the Chairman of the Planning Board, must be submitted with the application.)

If a private parking facility is to be utilized, is such facility within 600 feet?

_____ (Yes) X (No)

If yes, provide the address of the facility, name of the property owner, number of spaces in the lot, number of spaces available for lease, and length of lease. Attach copy of lease. Additional information may be required by the Planning Board):

IX. RESIDENTIAL PROPOSALS

Existing Multi-Family Residential in Primary Building
A) Type of Development: Excluded from Site Plan Review
(Townhouse, condominium, multiple residence rental, etc.)

B) Unit Breakdown:

Number of efficiency or "studio" units: 1

Number of one (1) bedroom units: 6

Number of two (2) bedroom unit's _____

Number with three or more bedrooms: _____

Total number of units: 7

C) Is funding public? _____ Yes X No

If yes, describe funding source: _____

X. OFFICE PROPOSALS (N/A Medical/Dental _____ Professional _____ Business)

A) Number of Employees: _____

B) Number of doctors/dentists/medical practitioners: _____

C) Days/Hours of Operation: _____

XI. SERVICE BUSINESS PROPOSALS N/A

- A) Specify Business: _____
(e.g., laundromat, drycleaner, beauty parlor, travel agency, banks)
- B) Number of Employees: _____
- C) Number of Washing Machines (for laundromat): _____
- D) Days/Hours of Operation: _____

XII. MERCANTILE PROPOSALS (Retail _____ Wholesale _____) N/A

- A) Type of Mercantile (specify): _____
- B) Number of Employees: _____
- C) Days/Hours of Operation: _____

XIII. STANDARD RESTAURANT/FAST FOOD RESTAURANT/COFFEE SHOP/DONUT SHOP/NIGHTCLUB/DISCOTHEQUE/BAR PROPOSALS N/A

- A) Specify Use: _____
- B) Number of seats (excluding bar stools and outdoor seating): _____
- C) Meals served (i.e., breakfast, lunch, dinner): _____
- D) Type of Menu: _____
- E) Days/Hours of Operation: _____
- F) Entertainment (live and/or recorded): _____
- G) Will the restaurant contain a bar/lounge?: _____ Yes _____ No
- H) If yes, how many seats at the bar? _____
- I) Is a drive-through window proposed? _____
- J) Is a walk-up window proposed? _____
- K) Is an outdoor seating area proposed? _____
- L) If yes, how many seats are proposed? _____
- M) If yes, is outdoor cooking proposed? _____

XIV. INDUSTRIAL/COMMERCIAL PROPOSALS (Automobile repair, taxi, motor vehicle sales, warehouse, manufacturing, animal hospital, research facilities, etc.) N/A

- A) Specify Use: _____
- B) Number of shifts (if any): _____
- C) Number of employees: _____
(If shifts are proposed, list number of employees per shift)
- E) Number of work bays (if automobile repair): _____
- F) Days/Hours of operation: _____

XV. EDUCATIONAL/INSTITUTIONAL/RECREATIONAL PROPOSALS (e.g., Museums, Theatres, Conference Centers, Hotels, Motels, Hospitals, Nursing Homes, Assisted Living Facilities, Nursery Schools, Private Schools, Day Care Centers, Places of Worship, Marinas, Membership Clubs, etc.) N/A

A) Specify: _____

B) Number of shifts (if any): _____

C) Number of employees: _____
(If shifts are proposed, list number of employees per shift)

D) Number of beds (hospitals, nursing homes, etc): _____

E) Number of seats in largest assembly space: _____

F) Number of classrooms (schools): _____

G) Number of rental units (hotels, motels, etc.) _____

H) Number of boat slips/courts (e.g., tennis, handball, etc.), alleys: _____

I) Days/Hours of Operation: _____

XVI. PROPOSALS NOT MENTIONED ABOVE (Please be specific):

Two (2) artist studio spaces

Excluded from Site Plan Review - Approximately 2,000 SF of existing art
gallery and art studios on ground floor of existing primary building

XVII. APPLICANT CERTIFICATION

I, ERIC BAXTER, certify that the this application has addressed the objectives outlined in the Zoning Ordinance with consideration given to the public health, safety and welfare; the comfort and convenience of the public in general or the residents or users of the proposed development and of the immediate neighborhood.

The applicant certifies that all information contained herein is accurate and complete as of the date of this application.



Signature of Applicant

11/7/2023

Date

November 7, 2023

Lori Garcia, Board Administrative Assistant
City of Poughkeepsie
62 Civic Center Plaza
Poughkeepsie, NY 12601

Judith Knauss, Deputy Zoning Administrator
City of Poughkeepsie
62 Civic Center Plaza
Poughkeepsie, NY 12601

**Re: 4 S Clinton – Carriage House
Application for Site Plan / Change of Use**

Dear Ms. Garcia and Ms. Knauss:

Enclosed please find the following:

1. Cover letter with responses to deficiencies noted on Site Plan Check Sheet (attached, dated September 25, 2023, received October 31, 2023)
2. Revised Site Plan Application Form, dated November 7, 2023
3. Site Plan Notes Sheet SPN-1, dated November 7, 2023
4. Site Plan Notes Sheet SPN-2, dated November 7, 2023
5. Additional photographs of front façade of primary structure, courtyard, and accessway
6. Utilities Letter from LRC Engineering Group, dated November 7, 2023

Below please find responses to deficiencies noted on the site plan check sheet forwarded by Judith Knauss on October 31, 2023.

I) Application Requirements and General Site Plan Information:

1. *Properly completed application form, including accurate contact information and application signature.*

City Comment: No. All uses in both buildings to be detailed.

Response:

The application form has been updated to reflect the residential unit count and commercial square footage of the primary building located at 4 S. Clinton. Please note that this site plan application pertains only to the proposed change of use within the existing rear accessory building ("carriage house"). Information for the primary building is provided for reference purposes only.

2. *Required fees submitted:*

City Comment: No. All parking to be calculated (all uses on site to be enumerated).

Response:

Per conversation with Judith Knauss on November 2, 2023, the provided application fee is in fact correct. If the Planning Board decides that a parking requirement greater than zero should be assigned to the proposed art studios within the rear accessory structure, payment of the appropriate fee will be listed as a condition of approval and paid prior to the final signature of plans.

3. *Date (including any revision dates), north arrow, written and graphic scale.*

City Comment: No. "Graphic scale" circled.

Response:

As the accessory structure is existing and no modifications are proposed, we request that the provided survey be permitted to serve as a de facto site plan in this unique case (i.e. approval of a change of use as opposed to new construction or addition). The provided survey is dated 6/15/17 and contains a north arrow and graphic scale. The provided floor plans dated 5/30/2023 also contain a north arrow and a scale of $\frac{1}{4}''=1'-0''$. As noted above the revisions box on the provided floor plans, drawings were printed at 24'x36' so that the provided numerical scale is accurate.

4. *Vicinity Map (i.e., Sanborn, tax map).*

City Comment: No

Response:

Please see Sheet SPN-1, which provides a vicinity map utilizing the city's adopted zoning map. This vicinity map was also provided within the cover letter dated August 25, 2023.

5. *Existing zoning with accurate lot and bulk tables.*

City Response: No.

Response:

Please see Sheet SPN-1, which provides a lot and bulk table with regulations from the C-2 zoning district that are applicable to a request for change of use within an existing structure. This table was also provided within the cover letter dated August 25, 2023.

6. *Names of owners of adjoining properties, as shown on the latest tax records.*

City Comment: To be updated on plan.

Response:

Please see Sheet SPN-2 which provides updated names of owners of adjoining properties.

7. *Are any waivers requested? If so, list here (and provide a written request for waiver, outlining the item to be waived and the rational for waiver.*

City Comment: Waivers requested: Not Recommended.

Response:

Please see following responses and waiver request form submitted on August 25, 2023

II) Existing Site Conditions:

8. *Location, width and purpose of all existing easements, rights-of-way, setbacks, reservations and rears dedicated to public use, outline of existing covenants and deed restrictions within, adjacent or otherwise pertaining to the property.*

City Comment: No. If none, so note.

Response:

There are no easements, rights-of-way, reservations, covenants or deed restrictions of record existing in relation to the site.

9. *Plan view must show all existing features extending a minimum of twenty-five (25) feet past property boundary lines. Such features include all of the following:*

City Comment: No.

Response:

See responses below.

- a) *Existing contours with intervals of five (5) feet or less, referred to a datum satisfactory to the Planning Board. If parcel is relatively flat, contours MAY be waived if spot elevations are shown which indicate the general slope of the terrain;*

City Comment: Note that site is level, 100% developed.

Response:

The following note has been added to sheet SPN-1: “site is level, 100% developed”

- b) *Paved areas, walkways, parking areas, vehicular access to public streets, internal drives (detailed as required by material type: i.e., asphalt, concrete, stone, etc.); existing landscaping, screening and fences (detailed);*

City Comment: No. Note how access to rear building is obtained.

Response:

Paved areas, driveway, wood decks, fences, etc. are all detailed on the submitted survey dated 6/15/17. As the area is urban and the existing structures occupy a majority of the parcel, there is no existing landscaping on site. Open space is provided via an internal courtyard (see photographs). For abundant clarity, the following note has been added to sheet SPN-1: “Access to the accessory rear building is obtained through the driveway, which is detailed on the survey dated 6/15/17.” Although referenced on the survey as a “driveway,” the corridor is more appropriately referred to as an accessway as all tenants of the primary and accessory structure are prohibited from using the accessway for vehicle access. All tenants have a key to the accessway gate and are permitted to utilize the accessway for pedestrian access to the internal courtyard. Tenants of the rear accessory building utilize the accessway as their main point of access, but may also access the courtyard and rear building through the primary building when the ground floor art gallery is open. Additional photos of the accessway have been provided in our resubmission materials.

- c) *Outlines and descriptions of all structures.*

City Comment: No. “descriptions” circled.

Response:

Height and material description of all structures on and adjoining site have been provided on Sheet SPN-2.

- d) *All watercourses, marshes, wooded areas, rock outcrops, isolated trees with a diameter of eight (8) inches or more measured three (3) feet above the base of the trunk, curbs, sidewalks, adjacent streets with names and widths, and any other significant features.*

City Comment: No. Isolated tree is noted without details.

Response:

As indicated on the provided survey, the isolated tree exists on the adjoining parcel and is therefore not part of the subject site plan.

10. *Show locations of ALL existing utilities, underground and aboveground, within and adjacent to property and in the street, including, but not limited to: sanitary storm, water, gas and electric, and any utility poles, including light poles. Show all existing connections.*

City Comment: No. Must be shown.

Response:

All utilities to the building are existing and adequate for the proposed change of use. Please see letter from LRC Engineering Group dated November 7, 2023 with attached maps of city water, sewer, and stormwater services to the building.

11. *Approximate boundaries of any areas subject to storm water overflows (reference FEMA 100 year floodplain panel or study).*

City Comment: No. If none, so note.

Response:

The site is not located within the FEMA 100 year floodplain, as was shown in the EAF Supplemental Narrative provided on August 25, 2023.

III) Proposed Site Improvements:

12. *Detailed elevations of all principal and accessory structures.*

City Comment: No. Only selected views provided without details. Are any improvements proposed?

Response:

We request that this item be waived as all structures exist and no improvements are proposed. Photographs of the existing buildings have been provided.

13. *The location and design of all uses not requiring structures, such as off-street parking and loading areas, walkways, other paved areas and any open space for tenant recreational purposes for residential development. Profiles indication grading and cross section showing location and width of roadways and walkways. Any proposed direct pedestrian connection to public parking lots or structures will also be shown.*

City Comment: No. How is access to rear building obtained?

Response:

Access to the accessory rear building is obtained through the driveway, which is detailed on the survey dated 6/15/17 and documented in supplemental photographs. We request that the requirement for profiles indicating grading and cross sections be waived as all structures and improvements currently exist and no modifications are proposed.

14. *Proposed screening and landscaping, including a planting plan, in accordance with Section 19-4.11 of the Zoning Ordinance.*

City Comment: No. North property line? General courtyard?

Response:

No additional landscaping or screening is currently proposed. We request that this requirement be waived as all structures and improvements currently exist and no modifications are proposed.

15. *Location, direction, power and time of uses of any proposed outdoor lighting or public address systems. Provide an Isometric Footcandle Distribution Diagram, maximum illuminance levels shown in footcandle measurements on a grid of the site showing readings in every ten-foot square, overlaid on the site plan, as a separate sheet or view. One-half (1/2) foot candle is recommended. All lights to be shielded to prevent glare on adjoining properties, including public property.*

City Comment: No. Is any courtyard lighting proposed?

Response:

No new lighting is proposed. Four wall pack fixtures currently exist on site to illuminate the internal courtyard, accessway, and rear access door to the primary building (see photographs).

16. *An outline of any proposed easements, deed restrictions, covenants or rights-of-way and a notation of any areas to be dedicated to a public agency, and any public improvements contemplated on or adjoining the property.*

City Comment: No. if none, so note.

Response:

No easements, deed restrictions, covenants, rights-of-way, or public improvements are proposed.

17. *Location of any fire alarm boxes and/or connections to the city fire alarm system.*

City Comment: If none, so note.

Response: The following note has been added to sheet SPN-1: “No fire alarm boxes and/or connections to the city fire alarm system exist within the rear accessory structure”

18. *A determination of storm water quality requirements based on NYSDEC Phase II requirements. Further, ANY improvements within the Fallkill Creek watershed, which has been declared a 303(D) impaired watershed, require full stormwater treatment as per NYSDEC.*

City Comment: To be determined by Engineering.

Response:

All structures and improvements currently exist and no modifications are proposed. The project site is not located in the Fallkill Creek watershed. No new discharge points will be created, and stormwater runoff will not exceed historic levels.

IV) Other Required Plan Notes:

19. *Show plans for all proposed stormwater improvements. Applicant shall mitigate storm historic runoff rates for the 10-year storm.*

City Comment: To be determined by Engineering.

Response:

We request that this requirement be waived as all structures and improvements currently exist and no modifications or stormwater improvements are proposed.

Thank you,

Natalie Quinn

Natalie Quinn

CITY OF POUGHKEEPSIE, NY
BUILDING, PLANNING & ENGINEERING DEPARTMENTS

SITE PLAN CHECK SHEET

ADDRESS: 4 S. CLINTON STREET

The following is a list of deficiencies noted to exist in the submission relative to the requirements for site plan review.

D) APPLICATION REQUIREMENTS AND GENERAL SITE PLAN INFORMATION

<u>Compliance Certified</u>	<u>Item Description</u>
☐ YES ☒ NO	Properly completed application form, including accurate contact information and applicant signature. <u>ALL USES IN BOTH BUILDINGS TO BE DETAILED.</u>
☐ YES ☒ NO <u>ALL PARKING TO BE CALCULATED (ALL USES ON SITE TO BE</u>	Required fees submitted: ☐ Site Plan: \$500, plus \$150 for 1000 square feet of floor area; ☐ Special Permit: \$500 ☐ Full EAF: \$500; ☐ \$35 x ____ (# of Required Parking Spaces) = ____ <u>ENUMERATED)</u>
☒ YES ☐ NO	Short Environmental Assessment Form submitted. <u>SUBMITTED BUT NOT REQUIRED</u>
☐ YES ☐ NO	Full Environmental Assessment Form submitted.
☒ YES ☐ NO	Name and address of owner of record appears on plan.
☒ YES ☐ NO	Name and address of person, firm or organization preparing the plan.
☐ YES ☒ NO	Date (including any revision dates), north arrow, written and <u>graphic scale.</u>
☐ YES ☒ NO	Vicinity map (i.e., Sanborn, tax map).
☐ YES ☒ NO	Existing zoning with accurate lot and bulk tables.
☒ YES ☐ NO	Professional signature and seal on plan, as required by law.
☐ YES ☐ NO	Names of owners of adjoining properties, as shown on the latest tax records. <u>TO BE UPDATED ON PLAN.</u>
☐ YES ☐ NO	A statement indicating the financial capability of the applicant to complete the proposed development.
☐ YES ☐ NO *	Are any waivers requested? If so, list here (and provide a written request for waiver, outlining the item to be waived and the rational for waiver: <u>WAIVERS REQUESTED: NOT RECOMMENDED.</u>

ADDRESS: 4 S CLINTON STREET

II) EXISTING SITE CONDITIONS

Compliance Certified

☒ YES ☐ NO

☐ YES ☒ NO

IF NONE, SO NOTE

☐ YES ☒ NO

☐ YES ☐ NO

NOTE THAT SITE IS
LEVEL, 100% DEVELOPED.

☐ YES ☒ NO NOTE HOW
ACCESS TO REAR
BUILDING IS OBTAINED

☐ YES ☒ NO

☐ YES ☐ NO
ISOLATED TREE IS NOTED
W/OUT DETAILS

☐ YES ☒ NO

MUST BE SHOWN.

☐ YES ☒ NO

IF NONE, SO NOTE

Item Description

Site plan is prepared under the direction of a licensed surveyor or licensed professional engineer or references a properly prepared survey with surveyor name, license number and survey date indicated. If a city digital map is utilized, field verification of all features and elevations must be certified by a licensee surveyor or licensed professional engineer.

Location, width and purpose of all existing easements, rights-of-way, setbacks, reservations and rears dedicated to public use, outline of existing covenants and deed restrictions within, adjacent or otherwise pertaining to the property.

Plan view must show all existing features extending a minimum of twenty-five (25) feet past property boundary lines. Such features include all of the following:

Existing contours with intervals of five (5) feet or less, referred to a datum satisfactory to the Planning Board. If parcel is relatively flat, contours MAY be waived if spot elevations are shown which indicate the general slope of the terrain;

Paved areas, walkways, parking areas, vehicular access to public streets, internal drives (detailed as required by material type: i.e., asphalt, concrete, stone, etc); existing landscaping, screening and fences (detailed);

Outlines and descriptions of all structures;

All watercourses, marshes, wooded areas, rock outcrops, isolated trees with a diameter of eight (8) inches or more measured three (3) feet above the base of the trunk, curbs, sidewalks, adjacent streets with names and widths, and any other significant features.

Show locations of ALL existing utilities, underground and aboveground, within and adjacent to property and in the street, including, but not limited to: sanitary, storm, water, gas and electric, and any utility poles, including light poles. Show all existing connections.

Approximate boundaries of any areas subject to storm water overflows (reference FEMA 100 year floodplain panel or study).

ADDRESS: 4 S. CLINTON STREET

III) PROPOSED SITE IMPROVEMENTS

Compliance Certified

☐ YES ☐ NO

☐ YES ☒ NO

☐ YES ☒ NO

☐ YES ☒ NO

HOW IS ACCESS TO REAR BUILDING OBTAINED?

☐ YES ☒ NO
NORTH PROPERTY LINE?
GENERAL COURTYARD?

☐ YES ☒ NO
IS ANY COURTYARD LIGHTING PROPOSED?

☐ YES ☐ NO

☐ YES ☒ NO
IF NONE, SO NOTE

☐ YES ☐ NO
IF NONE, SO NOTE.

☐ YES ☐ NO
TO BE DETERMINED BY ENGINEERING.

Item Description

The location of all proposed buildings or structural improvements.

Detailed elevations of all proposed principal and accessory structures. ONLY SELECTED VIEWS PROVIDED WITHOUT DETAILS. ARE ANY IMPROVEMENTS PROPOSED?
Show locations, size and material of all proposed utilities and service connections.

The location and design of all uses not requiring structures, such as off-street parking and loading areas, walkways, other paved areas and any open space for tenant recreational purposes for residential development. Profiles indication grading and cross sections showing location and width of roadways and walkways. Any proposed direct pedestrian connection to public parking lots or structures will also be shown.

Proposed screening and landscaping, including a planting plan, in accordance with Section 19-4.11 of the Zoning Ordinance.

Location, direction, power and time of use of any proposed outdoor lighting or public address systems. Provide an Isometric Footcandle Distribution Diagram, maximum illuminance levels shown in footcandle measurements on a grid of the site showing readings in every ten-foot square, overlaid on the site plan, as a separate sheet or view. One-half (1/2) foot candle is recommended. All lights to be shielded to prevent glare on adjoining properties, including public property.

Any proposed new grades, indicating clearly how such grades will meet existing grades of adjacent properties or the street.

An outline of any proposed easements, deed restrictions, covenants or rights-of-way and a notation of any areas to be dedicated to a public agency, and any public improvements contemplated on or adjoining the property.

Location of any fire alarm boxes and/or connections to the city fire alarm system.

A determination of storm water quality requirements based on NYDES Phase II requirements. Further, ANY improvements within the Fallkill Creek watershed, which has been declared a 303(D) impaired watershed, require full stormwater treatment as per NYSDEC.

ADDRESS: 4 S CLYNTON ST.

IV) OTHER REQUIRED PLAN NOTES

Compliance Certified

☐ YES ☐ NO

TO BE DETERMINED
BY ENGINEERING.

☐ YES ☐ NO

N/A

Item Description

Show plans for all proposed stormwater improvements.
Applicant shall mitigate storm to historic run-off rates
for the 10-year storm.

The following note concerning the protection of trees must be
shown on all site plans where existing trees are proposed to be
retained: "Prior to the start of construction or excavation, trees to
be retained shall be protected by silt fence, snow fence,
construction fence, or equal, which will be placed at the drip line
or several feet beyond. However, the fencing shall not cross
public sidewalks. No storage or stockpiling of construction
materials, debris or equipment shall occur within the fenced
area."

In addition to the note on the plan, the location of the protective
fencing must be shown on the site plan or landscape plan.
Where a trees close proximity to a proposed building, driveway,
parking area, or construction vehicle access way prevents the
installation of fencing at the drip line, the modification of the
fence position shall be shown on the site plan and shall be
subject to the approval of the Planning Board. Field
modification of the fence position may be granted by the
Building Inspector/Zoning Administrator or Deputy
Building Inspector.

V) REQUIRED REFERRALS TO OTHER AGENCIES OR BOARDS

☒ YES

☐ NO

Dutchess County Planning Department

☐ YES

☐ NO

Zoning Board of Appeals

☐ YES

☒ NO

Historic District and Landmark Preservation Commission

☐ YES

☒ NO

Waterfront Advisory Committee

☐ YES

☒ NO

Shade Tree Commission

ADDITIONAL COMMENTS

* PREFERENCE IS FOR A PLAN LARGE ENOUGH TO
BEAR THE STAMP OF APPROVAL

* PLEASE NOTE THERE IS NO RECORD OF ANY USE
OTHER THAN STORAGE FOR THE REAR BUILDING.

Martine Quinn, Senior Planner

Judith Knauss, Deputy Zoning Administrator

JOSEPH O'BRIEN
(J. DONAT)

9/25/23
Date



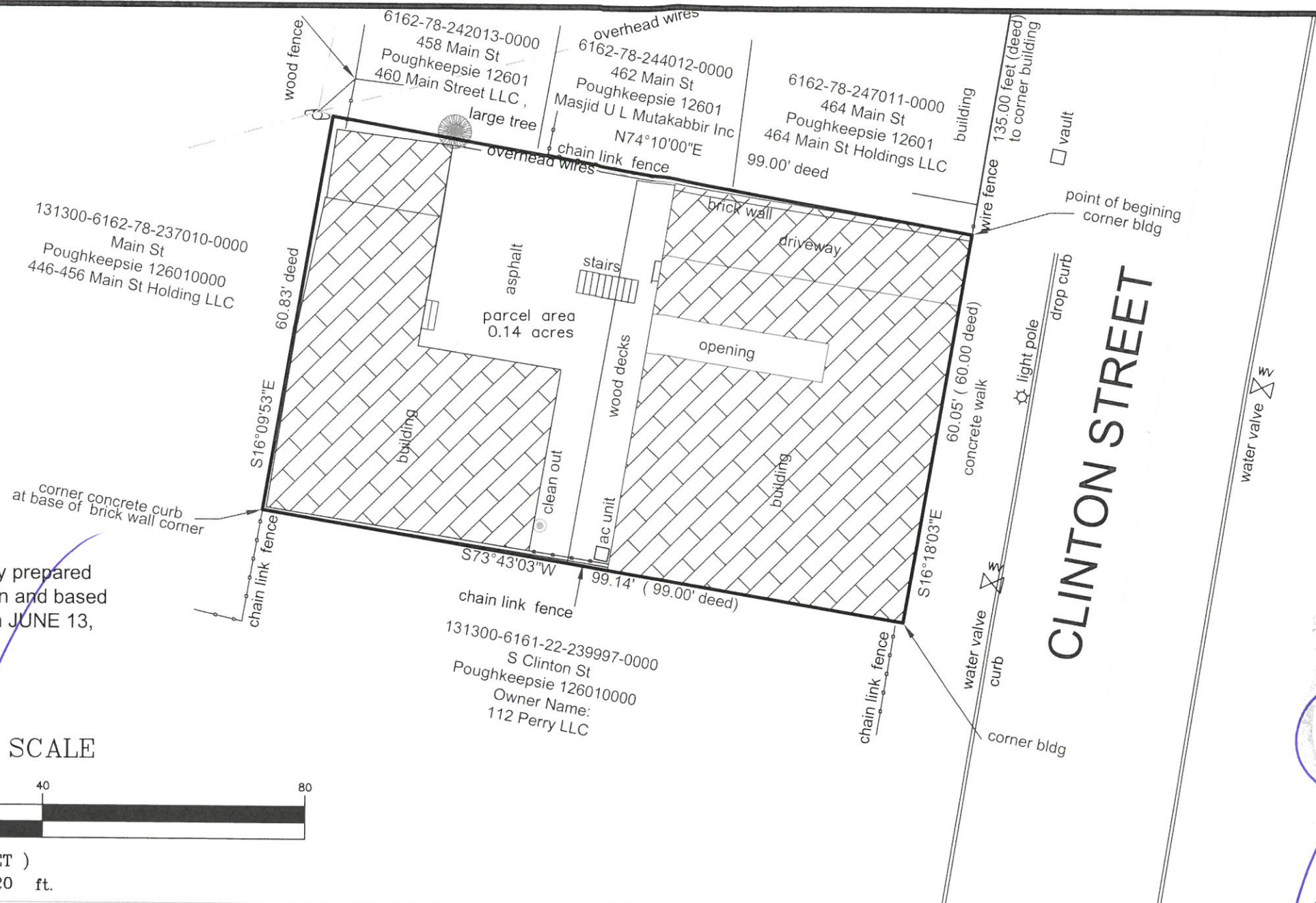
Perimeter Boundary survey prepared under my direct supervision and based on field work completed on JUNE 13, 2017 and deeds of record.

Joseph P Berger PE LS

GRAPHIC SCALE



(IN FEET)
1 inch = 20 ft.



CERTIFIED TO:
4 S Clinton LLC
Michael P. Rossi
First Hudson Title Agency LLC
Fidelity National Title Insurance Company
its successors and or assigns
Lakeland Bank, its successors and or assigns

SITE DATA
TAX NO 131300-6162-78-243002
4 CLINTON ST POUGHKEEPSIE NY 12601
DEED DOC 22014 PAGE 6591
DEED LIBER 1183 PAGE 19 (0.14 acre)

SUBSURFACE STRUCTURES NOT VISIBLE OR READILY APPARENT ARE NOT SHOWN AND THEIR LOCATION AND EXTENT ARE NOT CERTIFIED.

SUBJECT TO THE RIGHTS OF THE UTILITIES TO MAINTAIN THEIR POLES, GUYS, AND WIRES.

SUBJECT TO ALL EASEMENTS OF RECORD.

CERTIFICATIONS INDICATED HEREON SIGNIFY THAT THIS SURVEY WAS PREPARED IN ACCORDANCE WITH THE EXISTING CODE OF PRACTICE FOR PROFESSIONAL LAND SURVEYORS AS ADOPTED BY THE NEW YORK STATE ASSOCIATION OF PROFESSIONAL LAND SURVEYORS. SAID CERTIFICATIONS SHALL RUN ONLY TO THE PERSON SO NOTED. CERTIFICATIONS ARE NOT TRANSFERABLE TO ADDITIONAL INSTITUTIONS, THEIR SUCCESSORS OR ASSIGNS, OR SUBSEQUENT OWNERS UNLESS EXPRESSLY STATED HEREON.

ONLY COPIES FROM AN ORIGINAL OF THIS MAP MARKED WITH EITHER AN EMBOSSED OR INK SEAL SHALL BE CONSIDERED A TRUE AND VALID COPY.

THE LOCATION OF UNDERGROUND IMPROVEMENTS OR ENCROACHMENTS ARE NOT ALWAYS KNOWN AND OFTEN MUST BE ESTIMATED. IF ANY UNDERGROUND IMPROVEMENTS OR ENCROACHMENTS EXIST OR ARE SHOWN, THE IMPROVEMENTS OR ENCROACHMENTS ARE NOT COVERED BY THIS CERTIFICATE.

UNAUTHORIZED ALTERATION OR ADDITION TO A SURVEY MAP BEARING A LICENSED LAND SURVEYORS SEAL IS A VIOLATION OF SECTION 7209, SUBDIVISION 2 OF THE NEW YORK STATE EDUCATION LAW.

NORTH BASED ON DEED OF RECORD
© Berger Engineering and Surveying 2017, all rights reserved

06/15/17

Sheet 1 of 1

SURVEY

4 CLINTON STREET

POUGHKEEPSIE NY

Scale: 1" = 20'

PREPARED FOR

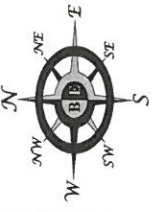
4 CLINTON LLC

278 MILL STREET SUITE 100

Poughkeepsie, NY 12601

BERGER ENGINEERING AND SURVEYING

100 FULTON AVE.
POUGHKEEPSIE N.Y. 12603
(845) 471-7383



**4 South Clinton Street
Site Plan Notes**

**Adjoining Property Owners & Structure Descriptions as Listed on
Dutchess County Parcel Access November 06, 2023**



4 S. CLINTON STREET - PHOTOGRAPHS



Front Façade of Primary Structure, including accessway shown in bottom righthand corner

4 S. CLINTON STREET - PHOTOGRAPHS



Close up of accessway from sidewalk, leads to internal courtyard and rear accessory building.

4 S. CLINTON STREET - PHOTOGRAPHS



Accessway looking from internal courtyard out towards sidewalk

4 S. CLINTON STREET - PHOTOGRAPHS



Rear of primary structure from internal courtyard / rear accessory building

4 S. CLINTON STREET - PHOTOGRAPHS



View of internal courtyard and rear accessory structure from accessway

4 S. CLINTON STREET - PHOTOGRAPHS



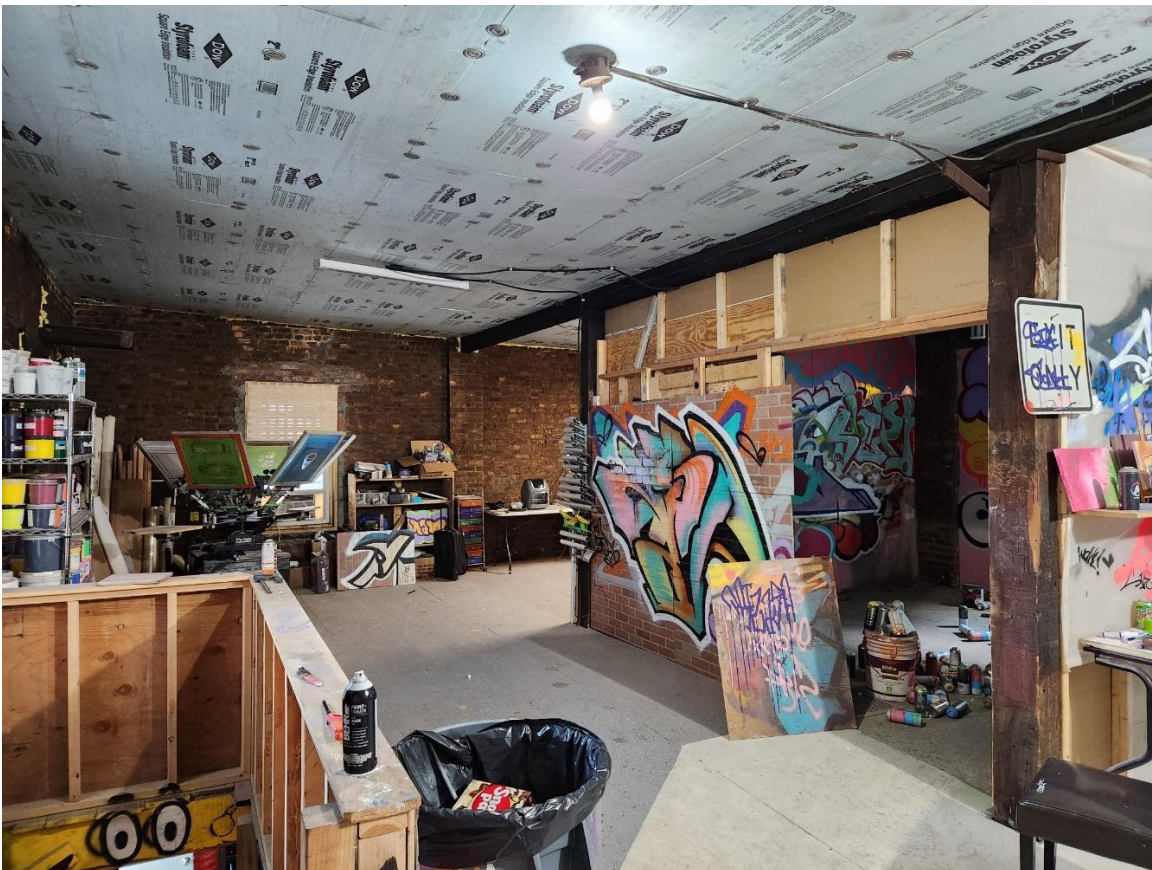
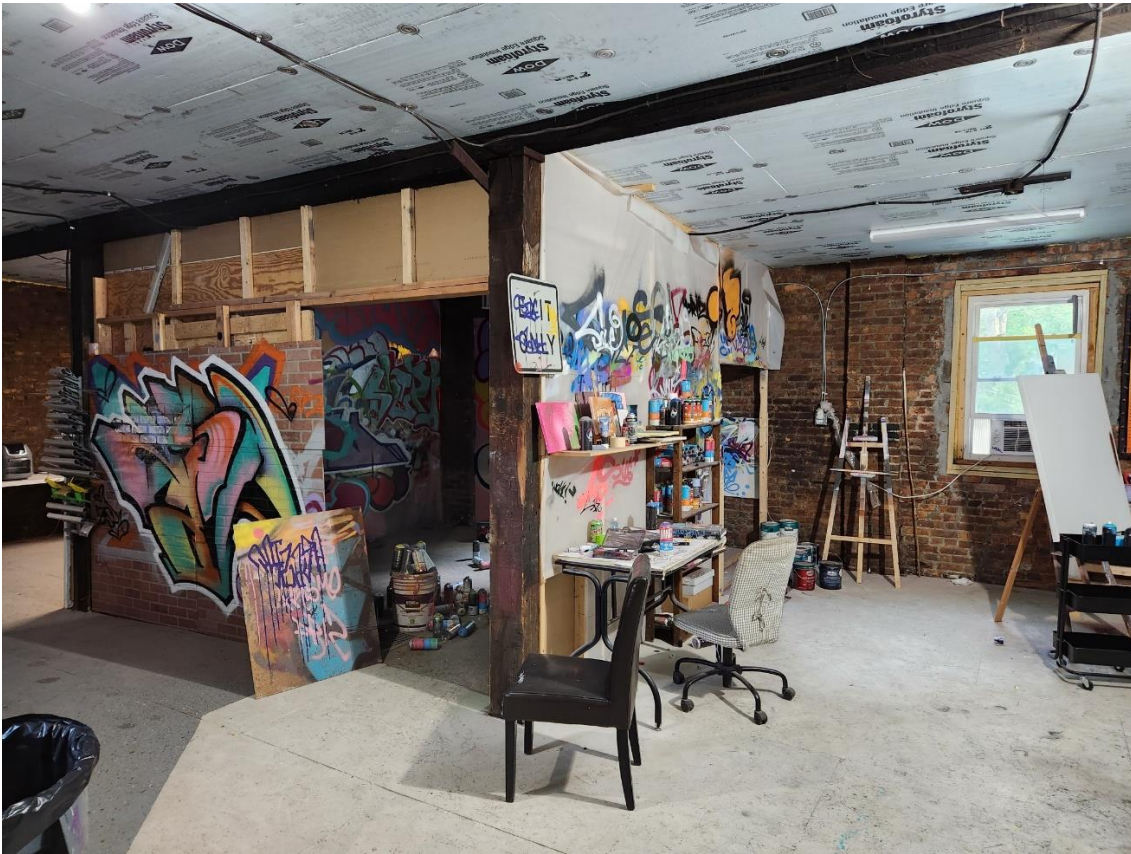
View of rear accessory structure ("carriage house") from internal courtyard

4 S. CLINTON STREET - PHOTOGRAPHS



View of rear accessory structure (“carriage house”) from internal courtyard. Third level no longer accessible.

4 S. CLINTON STREET - PHOTOGRAPHS



Views of large artist studio

4 S. CLINTON STREET - PHOTOGRAPHS



The above pictures show existing lighting used to illuminate the courtyard, accessway, and rear entrance to primary building. As existing lighting has proven to be sufficient for all tenants, no new lighting is proposed.



November 7, 2023

Joseph Kane
Assistant Civil Engineer
City of Poughkeepsie Department of Public Works & Engineering
62 Civic Center Plaza
Poughkeepsie, NY 12601

RE: 4-6 South Clinton Street, Poughkeepsie, NY

Dear Mr. Kane,

In review of the existing development at 4-6 South Clinton Street it was identified that the single parcel is comprised of two buildings, a rear carriage house and a front building, which lists the street addresses as 4 South Clinton and 6 South Clinton and functions essentially as two separate parcels for water and sewer connections.

4 South Clinton Street consists of an existing art Gallery and artist studio on the ground floor with three residential units above (two residential bedrooms and 1 residential studio) for a total of 3 bedrooms. The address is served by its own standalone 1" water line and a 6" cast iron sewer line that both connect directly to the municipal services within the street. The projected water/sewer demand for the address would be ~405 GPD for the address. Per the New York State Plumbing Code Table 710.1(1) the 6" will have more than enough capacity when sloped at 1/8" per foot min.

6 South Clinton Street consists of two existing art Galleries and two artist studios on the ground floor with four residential units above the studios for a total of 4 residential bedrooms along with two artist studios within the Carriage house as the second building on the property. The carriage house and 6 South Clinton are served by one standalone 1" water line and a 6" cast iron sewer line that both connect directly to the municipal services within the street. The projected water/sewer demand for the address would be ~590 GPD for the address. Per the New York State Plumbing Code Table 710.1(1) the 6" will have more than enough capacity when sloped at 1/8" per foot min.

If any further information is needed at this time, please let us know.

Sincerely,
The LRC Group

A handwritten signature in blue ink, reading "Kenneth Casamento", is written over a horizontal line.

Kenneth Casamento

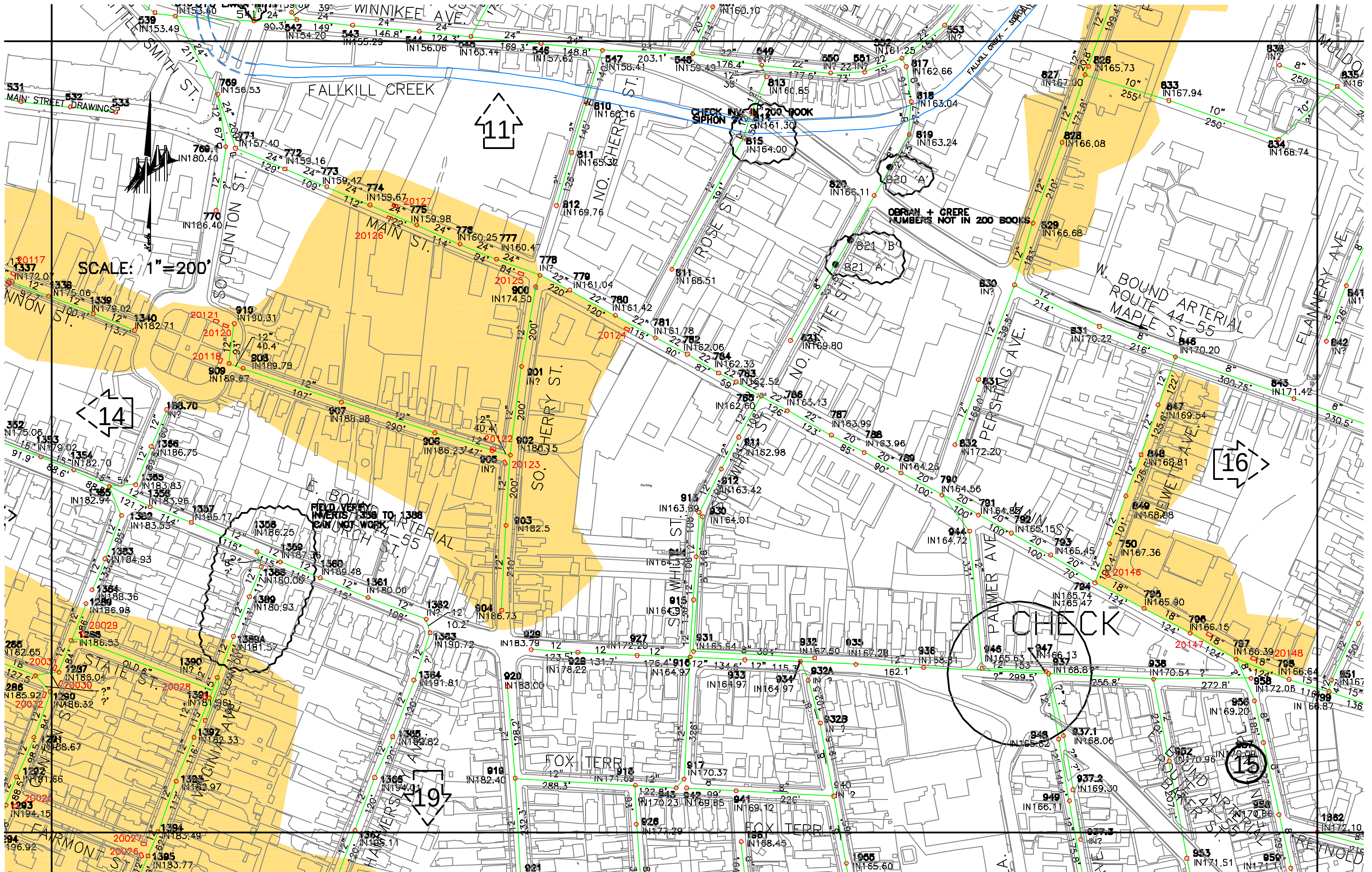
LRC Engineering & Surveying, DPC
LRC Engineering and Surveying, LLC
LRC Environmental Services, Inc.
Land Resource Consultants, Inc.

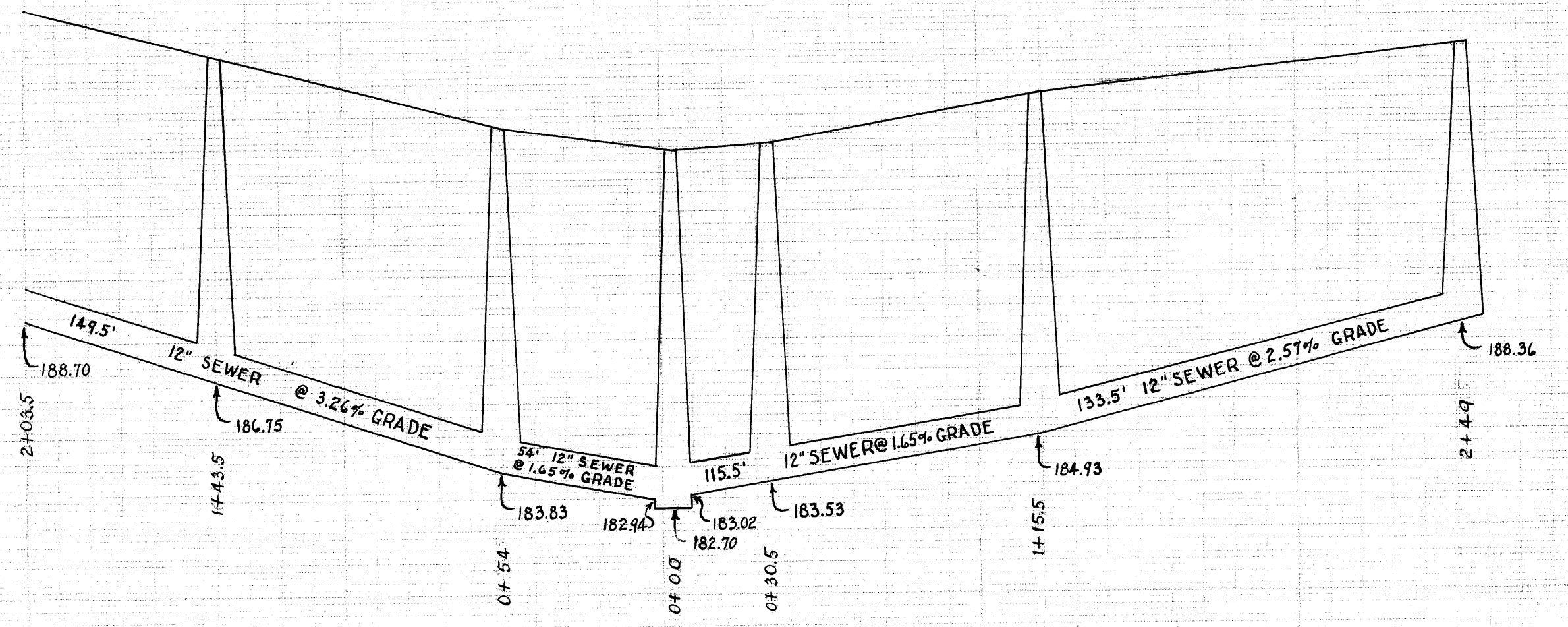
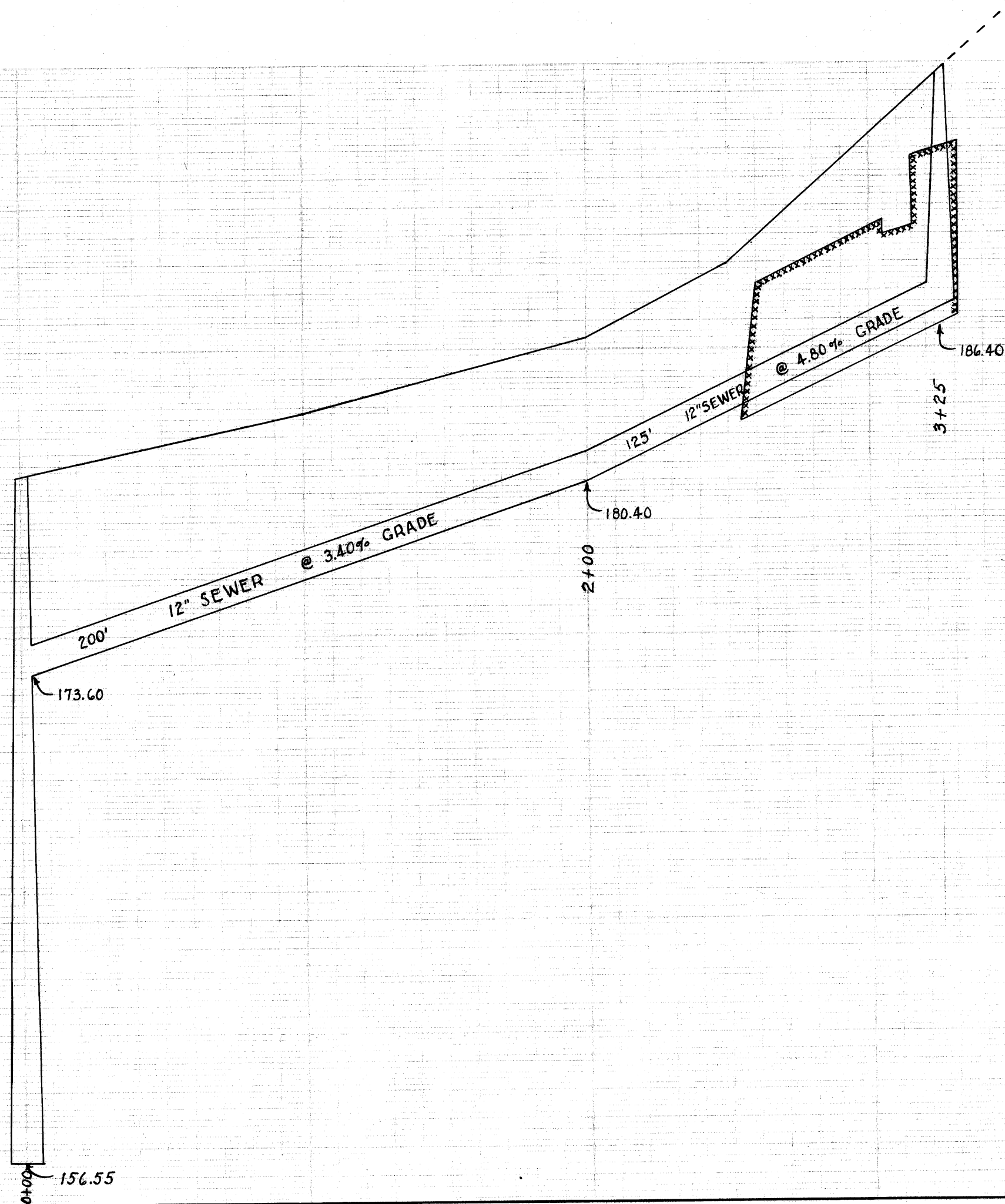
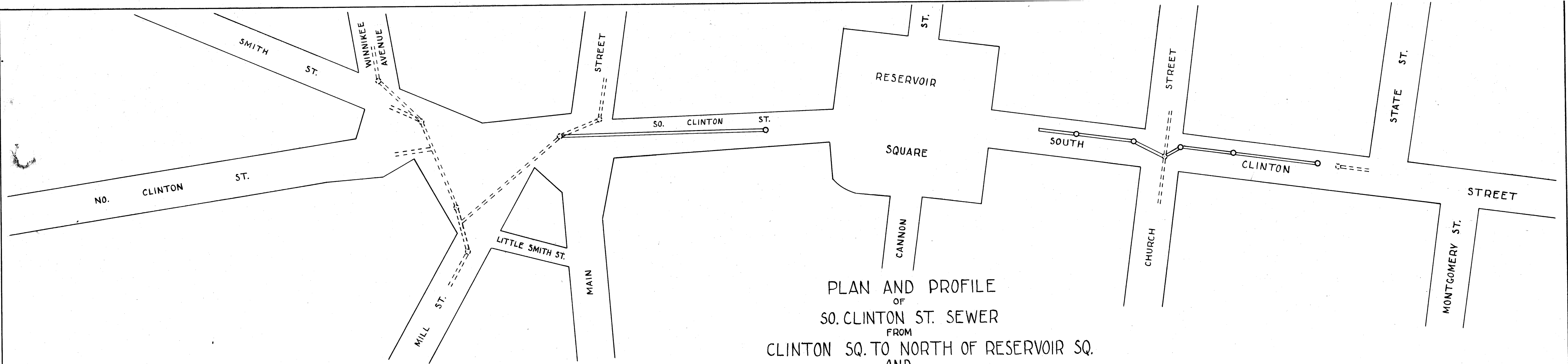
85 Civic Center Plaza, Suite 204
Poughkeepsie, New York 12601
Tel: (845) 243-2880
Fax: (845) 265-8175

Offices in New York, Connecticut and New Jersey

www.lrcconsult.com

Land Planning ♦ Civil Engineering ♦ Environmental Services ♦ Land Surveying ♦ Landscape Architecture



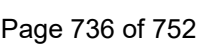


SOUTH CLINTON STREET
MAIN TO STATE STREET

sheet 6 of 7

BOARD OF PUBLIC WORKS
DOUGHERTIE, N.Y.
C.O. CONGER SUPERINTENDENT
JAN. 19, 1938
WHM.

-50.0 12





**THE CITY OF POUGHKEEPSIE
NEW YORK**

DEVELOPMENT DEPARTMENT

62 CIVIC CENTER PLAZA, 2ND FLOOR

POUGHKEEPSIE, NY 12601

Phone: (845) 451-4263 Fax: (845) 451-4006

MEMORANDUM

To: 4 S. Clinton, LLC

From: Tyler Maegerle, Junior Planner

Date: 8/31/2023

Re: 4 South Clinton Street

Upon review of project materials provided by the applicant, including Short EAF Part 1 and site plan application, it is my understanding that the proposed action to be undertaken includes site plan approval for the use of an existing accessory building for commercial use as artist studios.

Based on the above description it is the determination of the planning division that the proposed action is classified as Type II Action with regards to SEQRA pursuant to 6 NYCRR Section 617.5 (c)(18) "reuse of a residential or commercial structure, or of a structure containing mixed residential and commercial uses, where the residential or commercial use is a permitted use under the applicable zoning law or ordinance, including permitted by special use permit, and the action does not meet or exceeds any of the thresholds in section 617.4 of this Part;." The action is therefore not subject to further environmental review.

APPLICATION TO THE PLANNING BOARD
CITY OF POUGHKEEPSIE, NEW YORK

Application for Site Plan, Special Permit & Facade Approval

I. **PROPERTY ADDRESS:** 50 Delafield Street, Poughkeepsie, NY 12601

II. **PROPERTY OWNER:** Astor Services
c/o Yvette Bairan, Chief Executive Officer
ADDRESS: 6339 Mill Street

Rhinebeck
NY 12572 (state and zip code)

PHONE NUMBER: (845) 871-1000 (include area code)

III. **NAME OF APPLICANT:** Jay Diesing, RA AIA - Mauri Architects PC

ADDRESS: 73 Mansion Street
Poughkeepsie
NY 12601 (state and zip code)

PHONE NUMBER: (845) 452-1030 (include area code)

EMAIL ADDRESS: Jay@Mauri-Architects.com

(If applicant is NOT the owner, proof of owner's consent to the application MUST be provided.)

IV. **CONSULTANT:** Jay Diesing, RA AIA - Mauri Architects PC

ADDRESS: 73 Mansion Street
Poughkeepsie
NY 12601 (state and zip code)

PHONE NUMBER: (845) 452-1030 (include area code)

V. **PROPOSED USE or FAÇADE CHANGE** (Summarize proposed use or uses): Existing (No Change)

Proposed outdoor HVAC units and miscellaneous as-built site conditions as summarized in 8/14/23 cover letter.

VI. **ZONING DISTRICT:** R-4

OFFICE USE ONLY

CODE: PLSF

ID # _____

FEE: _____

Total, from:
\$500, plus
\$ _____
(\$150 per 1000
square feet of floor
area), plus
\$ _____
(\$35 per number of
required parking
spaces), plus
\$ _____
(\$500 for special
permit, if applicable)

(Proceed to XVII for special permit & façade approval only)

PROJECT INFORMATION

New construction: _____ (yes) X (no)

Change of use: _____ (yes) X (no)

Expansion/addition: _____ (yes) X (no)

Alteration: X (yes) _____ (no)

Exterior HVAC and Miscellaneous as-built site conditions as summarized in August 14, 2023 cover letter.

Cost of Construction: \$ 883,000

LOT AND BULK INFORMATION:

Lot area: 1.18 (Existing) (acres) 51,408 (square feet)

Building Footprint: N/A (proposed square feet for new construction)

N/A (existing square feet for change of use/expansion)

N/A (additional proposed square feet for expansion)

10,730 (Existing) (total building footprint, square feet)

Lot Coverage: 20.9% (Existing) (percentage, building coverage only)

Gross Floor Area: N/A (proposed square feet for new construction)

N/A (existing square feet for change of use/expansion)

N/A (additional proposed square feet for expansion)

Existing (total gross floor area, square feet)

Building Height: Existing (stories) _____ (feet)

Floor Area Ratio: Existing (ratio of gross floor area divided by lot area)

VIII. PROJECT PARKING INFORMATION

A) Number of Off-Street Parking Spaces required: Existing (No Change)

B) Number of Off-Street Parking Spaces provided: 52 existing spaces vs. 46 spaces on previously approved site plan

N/A C) Waiver Requested: _____ (Yes) X (No)

Is municipal parking available within 600 feet? _____ (Yes) _____ (No)

If yes, name all such municipal facilities: N/A

(If a waiver is requested based on proximity to municipal facilities, a written request for such waiver, addressed to the City Planning Staff and the Chairman of the Planning Board, must be submitted with the application.)

If a private parking facility is to be utilized, is such facility within 600 feet?

_____ (Yes) _____ (No)

If yes, provide the address of the facility, name of the property owner, number of spaces in the lot, number of spaces available for lease, and length of lease. Attach copy of lease. Additional information may be required by the Planning Board):

N/A

IX. RESIDENTIAL PROPOSALS

A) Type of Development: _____
(Townhouse, condominium, multiple residence rental, etc.)

B) Unit Breakdown:

Number of efficiency or "studio" units: _____

Number of one (1) bedroom units: _____

Number of two (2) bedroom unit's _____

Number with three or more bedrooms: _____

Total number of units: _____

C) Is funding public? ____ Yes ____ No

If yes, describe funding source: _____

X. OFFICE PROPOSALS (____ Medical/Dental ____ Professional ____ Business)

A) Number of Employees: _____

B) Number of doctors/dentists/medical practitioners: _____

C) Days/Hours of Operation: _____

XI. SERVICE BUSINESS PROPOSALS

- A) Specify Business: _____
(e.g., laundromat, drycleaner, beauty parlor, travel agency, banks)
- B) Number of Employees: _____
- C) Number of Washing Machines (for laundromat): _____
- D) Days/Hours of Operation: _____

XII. MERCANTILE PROPOSALS (Retail _____ Wholesale _____)

- A) Type of Mercantile (specify): _____
- B) Number of Employees: _____
- C) Days/Hours of Operation: _____

XIII. STANDARD RESTAURANT/FAST FOOD RESTAURANT/COFFEE SHOP/DONUT SHOP/NIGHTCLUB/DISCOTHEQUE/BAR PROPOSALS

- A) Specify Use: _____
- B) Number of seats (excluding bar stools and outdoor seating): _____
- C) Meals served (i.e., breakfast, lunch, dinner): _____
- D) Type of Menu: _____
- E) Days/Hours of Operation: _____
- F) Entertainment (live and/or recorded): _____
- G) Will the restaurant contain a bar/lounge?: _____ Yes _____ No
- H) If yes, how many seats at the bar? _____
- I) Is a drive-through window proposed? _____
- J) Is a walk-up window proposed? _____
- K) Is an outdoor seating area proposed? _____
- L) If yes, how many seats are proposed? _____
- M) If yes, is outdoor cooking proposed? _____

XIV. INDUSTRIAL/COMMERCIAL PROPOSALS (Automobile repair, taxi, motor vehicle sales, warehouse, manufacturing, animal hospital, research facilities, etc.)

- A) Specify Use: _____
- B) Number of shifts (if any): _____
- C) Number of employees: _____
(If shifts are proposed, list number of employees per shift)
- E) Number of work bays (if automobile repair): _____
- F) Days/Hours of operation: _____

XV. EDUCATIONAL/INSTITUTIONAL/RECREATIONAL PROPOSALS (e.g., Museums, Theatres, Conference Centers, Hotels, Motels, Hospitals, Nursing Homes, Assisted Living Facilities, Nursery Schools, Private Schools, Day Care Centers, Places of Worship, Marinas, Membership Clubs, etc.)

A) Specify: Astor Services - Head Start Pre-School

B) Number of shifts (if any): Existing (No Change)

C) Number of employees: Existing (No Change)
(If shifts are proposed, list number of employees per shift)

D) Number of beds (hospitals, nursing homes, etc): N/A

E) Number of seats in largest assembly space: Existing (No Change)

F) Number of classrooms (schools): Existing (No Change)

G) Number of rental units (hotels, motels, etc.) N/A

H) Number of boat slips/courts (e.g., tennis, handball, etc.), alleys: N/A

I) Days/Hours of Operation: Existing (No Change)

XVI. PROPOSALS NOT MENTIONED ABOVE (Please be specific):

N/A

XVII. APPLICANT CERTIFICATION

Jay Diesing, RA AIA -
I, Mauri Architects PC, certify that the this application has addressed the objectives outlined in the Zoning Ordinance with consideration given to the public health, safety and welfare; the comfort and convenience of the public in general or the residents or users of the proposed development and of the immediate neighborhood.

The applicant certifies that all information contained herein is accurate and complete as of the date of this application.



Signature of Applicant

25 July 2023
14 August 2023 - Revised

Date



MARTIN J DIESING AIA
RICHARD K TOMPKINS AIA

November 3, 2023

Ms. Anne Saylor, Chair
City of Poughkeepsie Planning Board
62 Civic Center Plaza
Poughkeepsie, NY 12601

**RE: Astor Head Start
50 Delafield Street
Poughkeepsie, NY 12601**

Dear Chair Anne Saylor and Planning Board Members:

Enclosed for your review is a revised Site Plan regarding the above referenced project. The project has recently appeared before the Zoning Board of Appeals and is expecting approval of two area variances at the November 14th meeting. The variances will allow installation of the proposed outdoor heat pump units in a front yard and also provide relief from the off-street parking space requirements in the code.

Our office is in receipt of a project review letter prepared by Joseph O'Brien, Deputy Zoning Administrator, dated August 16, 2023. Following is in response to that letter:

- 1. Section 19-4.13(1) requires that accessory uses be located either within the principal building or in any side or rear yard and such uses shall not be located within six feet of any adjoining property line. The new HVAC units are proposed to be located within the front yard of the building along Hoffman Street.**

The project has recently appeared before the Zoning Board of Appeals and is expecting approval of an area variance at the November 14th meeting to allow installation of the proposed outdoor heat pump units in a front yard.

- 2. Section 19-4.3(1) requires that whenever a parking area containing three or more spaces faces a street or a property line, a planting area of a minimum width of three feet with plantings at least three feet high planted three feet on center shall be provided between the parking area and the street line or property line; a landscape strip for the northern property line between the parking area and Hoffman Street was previously approved by the Planning Board and was a condition of variances granted on 11/5/97 which has now been removed and not proposed on the provided plans.**

A new row of plantings has been indicated on the Plan along Hoffman Street – See “Proposed Site Plan”, 1/PB-1.

3. **Section 19-4.3(1) requires that not more than 12 parking spaces shall be permitted in a continuous row, and not more than 24 spaces shall be permitted in a single parking area without being interrupted by landscaping approved by the Planning Board; a variance was granted on 11/5/97 for this section with a condition that 2 trees shall be planted and maintained on the southern side of the parking area within triangular shaped hatched areas, this was also approved by the Planning Board. These trees have been removed and are not proposed along with the parking area being reconfigured.**

Two new trees along with triangular planting areas have been indicated on the plan at the south-east corner of the parking area - See “Proposed Site Plan”, 1/PB-1.

4. **Section 19-4.3(2) requires that where a parking area for three or more vehicles abuts a residential property, it shall be screened from view by landscaping, opaque fence or wall or other means approved by the Planning Board; green slats for the existing chain link fencing on the western and southern property lines was previously approved by the Planning Board which have been removed and not proposed on the provided plans.**

An existing landscape hedge is located along the western property line and is now indicated on the Site Plan - See “Proposed Site Plan”, 1/PB-1.

5. **Section 19-4.3(S)(b) requires that each parking space shall be not less than 20 feet long; all parking spaces are less than 20' long with varying dimensions other than the 2 parallel spaces which comply.**

The parking lot is proposed to be re-stripped with the parking spaces at the required size - See “Proposed Site Plan”, 1/PB-1.

6. **Section 19-4.3(S)(b) requires that each parking space shall be not less than 9 feet wide; the proposed angle parking spaces excluding the accessible space along with the westernmost accessible space are less than 9' wide with varying dimensions and the proposed western access aisle is 6.25' where 8' is required per NYS Building Code Section 1106.1.1.**

The parking lot is proposed to be re-stripped with the parking spaces at the required size. This portion of the parking area will be reconfigured in compliance with the code - See “Proposed Site Plan”, 1/PB-1.

7. **Section 19-4.3(S)(b) requires that parking spaces shall be served by an aisle between rows of parking spaces of not less than 22 feet wide; the southernmost 6 parking spaces do not meet this requirement with varying dimensions.**

The parking lot is proposed to be re-stripped. This portion of the parking area will be reconfigured in compliance with the code - See "Proposed Site Plan", 1/PB-1.

- 8. Section 19-4.3(11)(a) requires that for Pre-schools, 1 parking space shall be provided per staff member plus 1 space for each classroom; as stated above, your Site Plan application was not completed and more information is required to confirm compliance with this section.**

An updated parking analysis chart has been added to the Site Plan. As mentioned above, the project has recently appeared before the Zoning Board of Appeals and is expecting approval of an area variance to provide relief from the off-street parking space requirements in the code.

- 9. A 4' black rod fence was to be installed per previous Site Plan approval on the Delafield Street property line along with 40' of the northeastern property line; a chain link fence exists with no proposal to install 4' black rod fence.**

The previously proposed black rod fence included horizontal members that would allow climbing. In addition, the top of the fence included spires that could be a danger if climbed. This fence was not an appropriate design for an area occupied by children. The existing chain-link fence is proposed to remain.

- 10. Refuse enclosure was moved and apron extended from what was previously approved without Site Plan or a building permit.**

Comment acknowledged – The existing refuse enclosure is proposed to remain.

- 11. Existing blacktop on northern portion of building was demolished and grass planted without a building permit.**

Comment acknowledged – The existing lawn is proposed to remain.

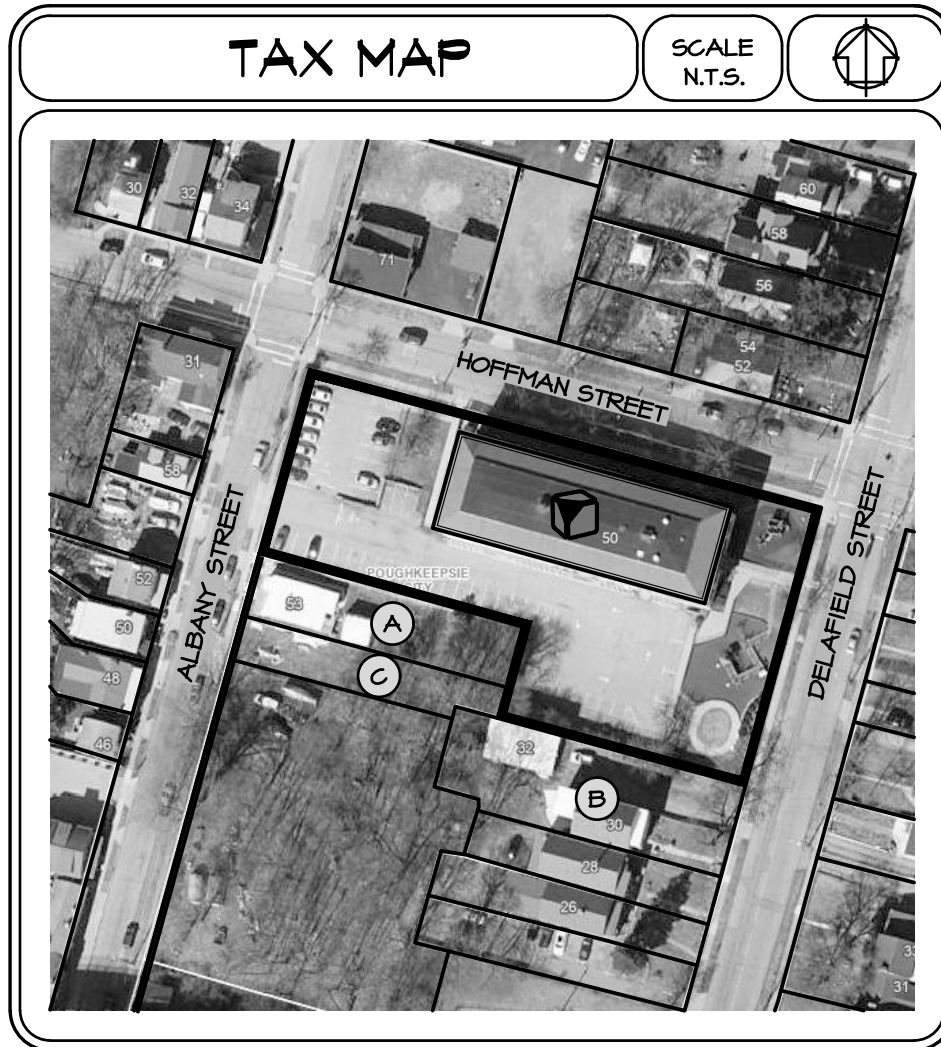
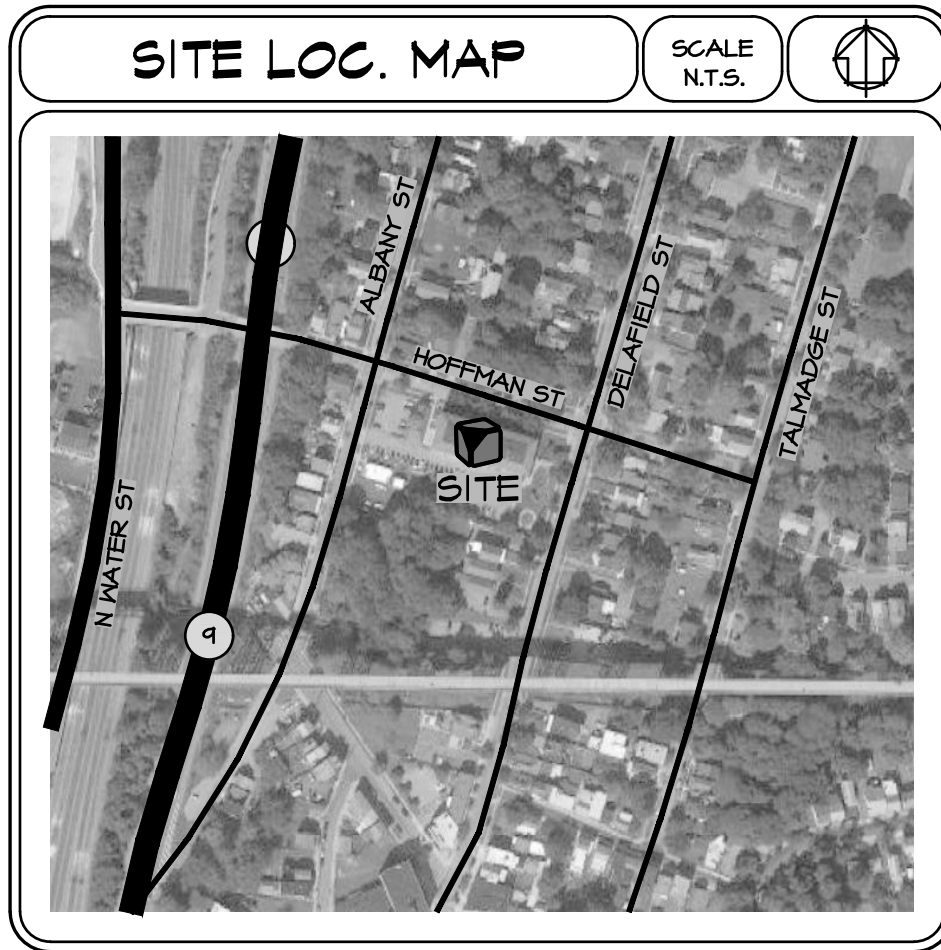
I look forward to meeting with you and the Board at the next available Planning Board meeting to discuss this project further. If you have any questions prior to the meeting, please do not hesitate to contact my office.

Sincerely,



Jay Diesing RA AIA

ZONING DATA CHART				
CITY OF Poughkeepsie PARCEL # 131300-6062-54-051380-0000				
ZONE	R-4	REQUIRED	EXISTING	PROPOSED
LOT AREA	MIN	N/A	1.18 ± ACRES (51,408 ± SF)	NO CHANGE
LOT WIDTH	MIN	50'-0"	336'-0" ±	NO CHANGE
LOT FRONTAGE	MIN	30'-0"	336'-0" ±	NO CHANGE
BUILDING HEIGHT	MAX	3 STORIES	3 STORIES	NO CHANGE
LOT COVERAGE	MIN	35%	20.4%	NO CHANGE
OPEN SPACE	MIN	N/A	N/A	NO CHANGE
FRONT YARD SETBACK	MAX	10'-0"	44'-2" ±	NO CHANGE
SIDE YARD SETBACK	MAX	6'-0"	L= 44'-8" ± R= 14'-1" ±	NO CHANGE
REAR YARD SETBACK	MIN	15'-0"	112'-4" ±	NO CHANGE
REQUIRED PARKING SPACES	MIN	(1) PER CLASSROOM = (13) (1) PER STAFF ± (82) 101 TOTAL SPACES	51 SPACES	51 SPACES



TAX MAP INFORMATION

SYMBOL	GRID NUMBER	OWNER OF RECORD
■	6062-54-051380	ASTOR HOME FOR CHILDREN PO BOX 5005 RHINEBECK, NY 12512
(A)	6062-54-045316	FEDOR IRKLIENSKI 53 ALBANY ST POUGHKEEPSIE, NY 12601
(B)	6062-67-059363	JACQUELINE J. BRACCO 30 DELAFIELD ST POUGHKEEPSIE, NY 12601
(C)	6062-67-044373	FEDOR IRKLIENSKI 53 ALBANY ST POUGHKEEPSIE, NY 12601



PLANNING BOARD APPROVAL

CITY PLANNING BOARD
CITY OF Poughkeepsie, NEW YORK

SITE PLAN APPROVAL:

FILE NO.:
PROJECT NAME: ASTOR HEAD START
PROJECT LOCATION: 50 DELAFIELD STREET,
POUGHKEEPSIE NY 12601
APPLICANT: JAY DIESING, RA - MAURI
ARCHITECTS PC

OFFICIALLY APPROVED AT THE PLANNING BOARD MEETING
HELD THE DAY OF 2

PLANNING CHAIR:
DESIGNEE:
DATE SIGNED:
CONDITION (S):

OWNER / APPLICANT SIGNATURE

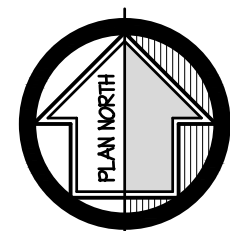
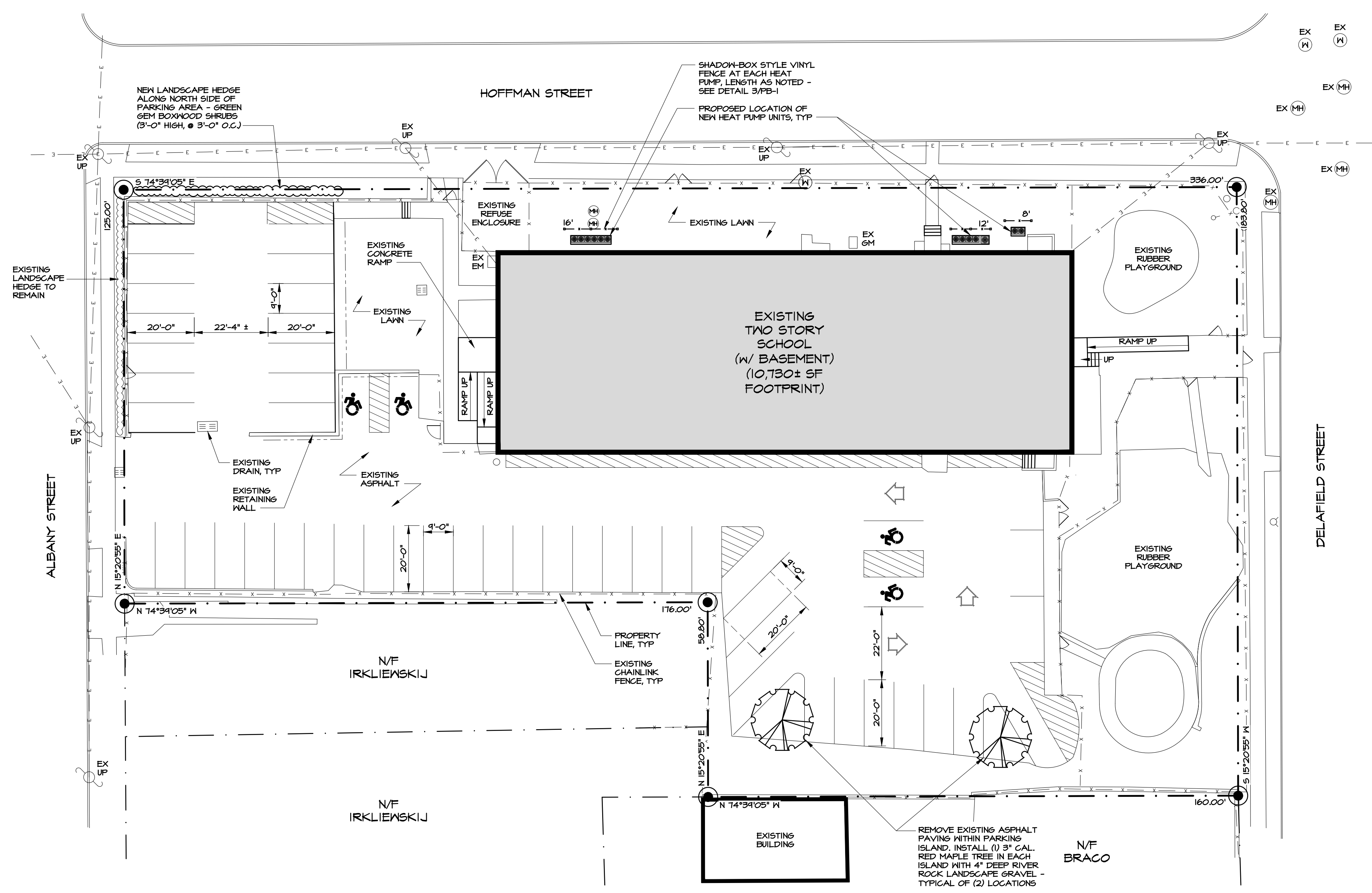
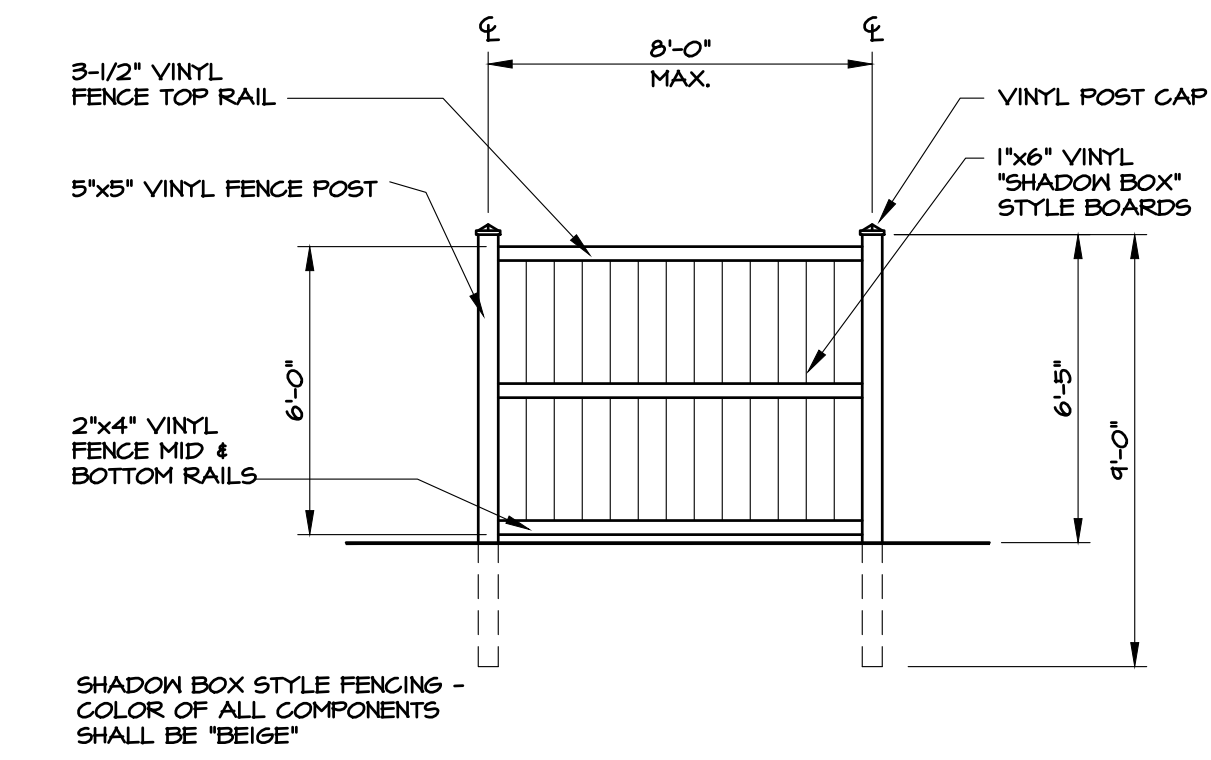
THE UNDERSIGNED APPLICANT(S) FOR THE PROPERTY AND THE UNDERSIGNED OWNER(S) OF THE PROPERTY SHOWN HEREIN CERTIFY THAT THEY ARE FAMILIAR WITH THIS MAP AND ITS NOTES AND ITS CONTENTS AS STATED HEREON INCLUDING ALL CONDITION OF APPROVAL.

THE APPLICANT AND OWNER UNDERSTAND THEIR OBLIGATION TO THE TOWN TO KEEP THIS PREMISES AS PER PLAN. APPROVAL BY THE PLANNING BOARD UNTIL A NEW OR REVISED PLAN FOR DEVELOPMENT OR USE OF THE SITE IS APPROVED BY THE PLANNING BOARD. THE APPLICANT AND OWNER UNDERSTAND THEIR OBLIGATION TO THE TOWN NOT TO OCCUPY THE PREMISES BEFORE A CERTIFICATE OF OCCUPANCY IS ISSUED BY THE TOWN FOR THE OCCUPANCY AS APPROVED HEREON.

OWNER DATE
APPLICANT DATE

DEPARTMENTAL SIGNATURES

FIRE CHIEF DATE
POLICE CHIEF DATE
BUILDING INSPECTOR DATE
CITY ENGINEER DATE
COMMISSIONER OF PUBLIC WORKS DATE



project no.	22-04
date	25 JUL 2023
drawn by	DC
revision	
description	FENCING PARKING CITY COMMENTS
date	11 OCT 23

PROPOSED RENOVATIONS FOR:

50 DELAFIELD

50 DELAFIELD STREET Poughkeepsie, NY

astor
SERVICES

PROPOSED SITE PLAN

MAURI ARCHITECTS PC

73 MANSON STREET Poughkeepsie NY 12601 845.452.0050 mauri-architects.com

REGISTERED ARCHITECT

MAURIN T. DIESING

039240

STATE OF NEW YORK



**THE CITY OF POUGHKEEPSIE
NEW YORK**

DEVELOPMENT DEPARTMENT
62 CIVIC CENTER PLAZA, 2ND FLOOR
POUGHKEEPSIE, NY 12601
Phone: (845) 451-4263 Fax: (845) 451-4006

MEMORANDUM

To: Jay Diesing, RA

From: Tyler Maegerle, Junior Planner

Date: 8/14/2023

Re: 50 Delafield St

Upon review of project materials provided by the applicant, including Short EAF Part 1 and site plan application, it is my understanding that the proposed action to be undertaken includes installation of air-conditioning and heat-pump HVAC systems.

Based on the above description it is the determination of the planning division that the proposed action is classified as Type II Action with regards to SEQRA pursuant to 6 NYCRR Section 617.5 (c)(1) "maintenance or repair involving no substantial changes in an existing structure or facility." The action is therefore not subject to further environmental review.

Short Environmental Assessment Form

Part 1 - Project Information

Instructions for Completing

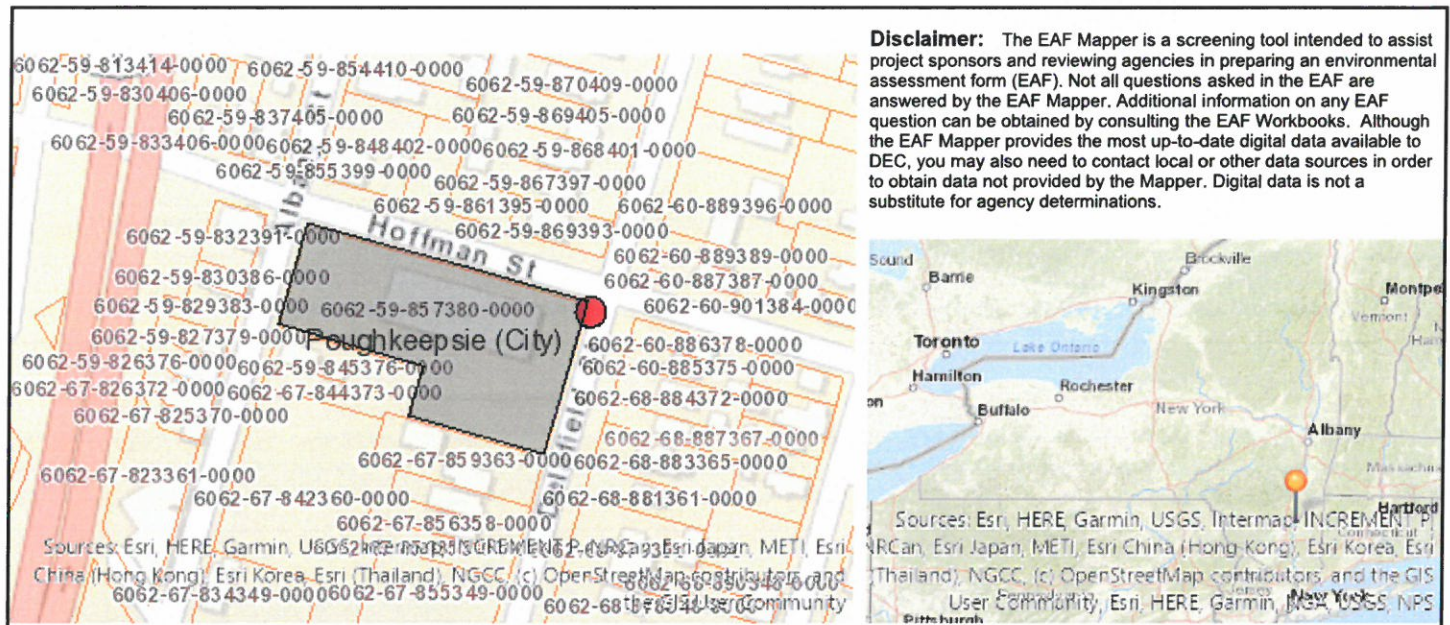
Part 1 – Project Information. The applicant or project sponsor is responsible for the completion of Part 1. Responses become part of the application for approval or funding, are subject to public review, and may be subject to further verification. Complete Part 1 based on information currently available. If additional research or investigation would be needed to fully respond to any item, please answer as thoroughly as possible based on current information.

Complete all items in Part 1. You may also provide any additional information which you believe will be needed by or useful to the lead agency; attach additional pages as necessary to supplement any item.

Part 1 – Project and Sponsor Information			
Name of Action or Project: Astor Head Start			
Project Location (describe, and attach a location map): 50 Delafield Street, Poughkeepsie, NY 12601			
Brief Description of Proposed Action: Installation of ductless-split heat-pump and air-conditioning units throughout the existing building including all necessary electrical and plumbing work.			
Name of Applicant or Sponsor: Jay Diesing, RA AIA - Mauri Architects PC		Telephone: (845) 452-1030 E-Mail: Jay@Mauri-Architects.com	
Address: 73 Mansion St			
City/PO: Poughkeepsie		State: NY	Zip Code: 12601
1. Does the proposed action only involve the legislative adoption of a plan, local law, ordinance, administrative rule, or regulation? If Yes, attach a narrative description of the intent of the proposed action and the environmental resources that may be affected in the municipality and proceed to Part 2. If no, continue to question 2.		NO ☒	YES ☐
2. Does the proposed action require a permit, approval or funding from any other government Agency? If Yes, list agency(s) name and permit or approval: City of Poughkeepsie Building Department - Building Permit		NO ☐	YES ☒
3. a. Total acreage of the site of the proposed action? b. Total acreage to be physically disturbed? c. Total acreage (project site and any contiguous properties) owned or controlled by the applicant or project sponsor?		1.18 acres 0.002 (100 sf) acres 1.18 acres	
4. Check all land uses that occur on, are adjoining or near the proposed action:			
5. ☒ Urban ☐ Rural (non-agriculture) ☐ Industrial ☒ Commercial ☒ Residential (suburban) ☐ Forest ☐ Agriculture ☐ Aquatic ☐ Other(Specify): ☐ Parkland			

5. Is the proposed action,	NO	YES	N/A
a. A permitted use under the zoning regulations?	☐	☒	☐
b. Consistent with the adopted comprehensive plan?	☐	☒	☐
6. Is the proposed action consistent with the predominant character of the existing built or natural landscape?	NO	YES	
	☐	☒	
7. Is the site of the proposed action located in, or does it adjoin, a state listed Critical Environmental Area?	NO	YES	
If Yes, identify: _____	☒	☐	
8. a. Will the proposed action result in a substantial increase in traffic above present levels?	NO	YES	
	☒	☐	
b. Are public transportation services available at or near the site of the proposed action?	☐	☒	
c. Are any pedestrian accommodations or bicycle routes available on or near the site of the proposed action?	☐	☒	
9. Does the proposed action meet or exceed the state energy code requirements?	NO	YES	
If the proposed action will exceed requirements, describe design features and technologies: _____ _____	☐	☒	
10. Will the proposed action connect to an existing public/private water supply?	NO	YES	
If No, describe method for providing potable water: _____ Existing	☐	☒	
11. Will the proposed action connect to existing wastewater utilities?	NO	YES	
If No, describe method for providing wastewater treatment: _____ Existing	☐	☒	
12. a. Does the project site contain, or is it substantially contiguous to, a building, archaeological site, or district which is listed on the National or State Register of Historic Places, or that has been determined by the Commissioner of the NYS Office of Parks, Recreation and Historic Preservation to be eligible for listing on the State Register of Historic Places?	NO	YES	
	☐	☒	
b. Is the project site, or any portion of it, located in or adjacent to an area designated as sensitive for archaeological sites on the NY State Historic Preservation Office (SHPO) archaeological site inventory?	☐	☒	
13. a. Does any portion of the site of the proposed action, or lands adjoining the proposed action, contain wetlands or other waterbodies regulated by a federal, state or local agency?	NO	YES	
	☒	☐	
b. Would the proposed action physically alter, or encroach into, any existing wetland or waterbody?	☒	☐	
If Yes, identify the wetland or waterbody and extent of alterations in square feet or acres: _____ _____ _____			

14. Identify the typical habitat types that occur on, or are likely to be found on the project site. Check all that apply: ☐ Shoreline ☐ Forest ☐ Agricultural/grasslands ☐ Early mid-successional ☐ Wetland ☒ Urban ☐ Suburban		
15. Does the site of the proposed action contain any species of animal, or associated habitats, listed by the State or Federal government as threatened or endangered? Atlantic Sturgeon, Bald Eag...	NO	YES
	☐	☒
16. Is the project site located in the 100-year flood plan?	NO	YES
	☒	☐
17. Will the proposed action create storm water discharge, either from point or non-point sources? If Yes,	NO	YES
a. Will storm water discharges flow to adjacent properties?	☒	☐
b. Will storm water discharges be directed to established conveyance systems (runoff and storm drains)?	☒	☐
If Yes, briefly describe: _____ _____		
18. Does the proposed action include construction or other activities that would result in the impoundment of water or other liquids (e.g., retention pond, waste lagoon, dam)? If Yes, explain the purpose and size of the impoundment: _____	NO	YES
	☒	☐
19. Has the site of the proposed action or an adjoining property been the location of an active or closed solid waste management facility? If Yes, describe: _____	NO	YES
	☒	☐
20. Has the site of the proposed action or an adjoining property been the subject of remediation (ongoing or completed) for hazardous waste? If Yes, describe: _____	NO	YES
	☐	☒
I CERTIFY THAT THE INFORMATION PROVIDED ABOVE IS TRUE AND ACCURATE TO THE BEST OF MY KNOWLEDGE Applicant/sponsor name: Jay Diesing, RA AIA - Mauri Architects PC Date: 25 July 23 Signature: <u>Jay Diesing</u> Title: <u>ARCHITECT</u>		



Part 1 / Question 7 [Critical Environmental Area]	No
Part 1 / Question 12a [National or State Register of Historic Places or State Eligible Sites]	Yes
Part 1 / Question 12b [Archeological Sites]	Yes
Part 1 / Question 13a [Wetlands or Other Regulated Waterbodies]	No
Part 1 / Question 15 [Threatened or Endangered Animal]	Yes
Part 1 / Question 15 [Threatened or Endangered Animal - Name]	Atlantic Sturgeon, Bald Eagle, Shortnose Sturgeon
Part 1 / Question 16 [100 Year Flood Plain]	No
Part 1 / Question 20 [Remediation Site]	Yes



July 17, 2023

Ms. Anne Saylor, Chair
City of Poughkeepsie Planning Board
62 Civic Center Plaza, 2nd Floor
Poughkeepsie, NY 12601

**RE: Astor Services
50 Delafield Street
Poughkeepsie, NY 12601**

Dear Ms. Anne Saylor and Planning Board Members,

Please be advised that Jay Diesing, RA AIA of Mauri Architects PC is hereby authorized to submit an Application for Site Plan Review to the City of Poughkeepsie and represent Astor Services as related to this project.

Should you have any questions, please do not hesitate to contact me.

Sincerely,

Yvette Bairan, *Chief Executive Officer*
Email: ybairan@astorservices.org

Cc: Sean McLaughlin, *Assistant Executive Director, Agency Operations*
Email: smclaughlin@astorservices.org

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